



A G E N D A
CITY OF WAUPUN SPECIAL PLAN COMMISSION
MEETING
Waupun City Hall – 201 E. Main Street, Waupun WI
Monday, August 31, 2020 at 4:45 PM

VIDEO CONFERENCE AND TELECONFERENCE

The Waupun Plan Commission will meet virtually at 4:45 pm on August 31, 2020, via Zoom.
The public may access the conference meeting online or by phone. Instructions to join the meeting are provided below:

To Join the Zoom Meeting:

<https://us02web.zoom.us/j/89385440703?pwd=YUYvaUp6NzBXZzR2Y25UY1dEdnpiQT09>

Meeting ID: 893 8544 0703

Passcode: 754763

Phone: 312 626 6799 US (Chicago)

CALL TO ORDER

ROLL CALL

PERSONS WISHING TO ADDRESS THE PLAN COMMISSION--State name, address, and subject of comments. (2 Minutes)

No Public Participation after this point.

FUTURE MEETINGS AND GATHERING INVOLVING THE PLAN COMMISSION

CONSIDERATION - ACTION

1. Approve Minutes of the August 19, 2020 meeting.
2. Public Hearing - Conditional Use Permit - 310 E. Main St- to provide outside service.
3. Site Plan Review - Municipal Well & Pump - 1212 Storbeck Dr.

ADJOURNMENT

Upon reasonable notice, efforts will be made to accommodate disabled individuals through appropriate aids and services. For additional information, contact the City Clerk at 920-324-7915.



(DRAFT)
CITY OF WAUPUN PLAN COMMISSION MEETING
Video Conference and Teleconference
Wednesday, August 19, 2020 at 4:45 pm

VIDEO CONFERENCE AND TELECONFERENCE

The Waupun Plan Commission met virtually at 4:45 pm on August 19, 2020, via Zoom.

CALL TO ORDER

The Waupun Plan Commission met virtually at 4:45 pm via Zoom.

ROLL CALL

Members Present: Julie Nickel, Fred Lueck, Elton TerBeest, Jerry Medema, Jeff Daane, and Jill Vanderkin

Member Excused: Mike Matoushek

Staff Present: Sarah VanBuren and Sue Leahy

PERSONS WISHING TO ADDRESS THE PLAN COMMISSION-

Chairman Nickel asked if there were any persons wishing to address the committee on any items not included on today's agenda. Hearing nothing, Chairman Nickel proceeded to address the items listed for consideration and action on today's committee agenda.

FUTURE MEETINGS AND GATHERING INVOLVING THE PLAN COMMISSION

Chairman Nickel indicated the next meeting of the Plan Commission will be August 31, 2020, at 4:45 pm

CONSIDERATION - ACTION

1. **APPROVE MINUTES** of the July 15, 2020 Plan Commission Meeting.
Motion by Medema, seconded by Vanderkin to approve the minutes of the July 15, 2020 Plan Commission meeting as presented. Motion carried, unanimously.
2. **PUBLIC HEARING – CONDITIONAL USE PERMIT** - Wind & Unwind Coffee & Wine House, 310 E Main St. Waupun.
Application to provide outside service under Section 11.12(4)(a) of the City of Waupun ordinance in a B-2 Central Business District subject to a Conditional Use Permit. Chairman Nickel noted this matter has been withdrawn as the Public Hearing Notice was not published. This matter is now rescheduled for Monday, August 31, 2020 at 4:45 pm in the Council Chambers, City Hall, Waupun.
3. **SITE PLAN REVIEW** – Judson's – 412 E. Main St
Glen Marwitz, agent for Judson's submitted a site plan to construct an 18' x 22' covered patio on the west dimension of their present building. They would provide a green space for the patio. Sue Leahy said the structure will meet all yard and setback requirements of the zoning ordinance.

Glen asked if he could bring in fill on Saturday. Sue said he can go ahead.

Chairman Nickel asked for a motion to act on this Site Plan.

Motion by Medema, seconded by Nickel to approve the site plan for the patio addition at Judson's at 412 E. Main St.

Vote: Vanderkin, Daane, TerBeest, Medema, Lueck, and Nickel – "AYE". Motion carried unanimously 6/0.

4. **SITE PLAN REVIEW** – 412 E Main St. An application for a permit has been submitted by Gysbers Jewelry to construct a 16' x 24' single story goldsmith shop on the rear or south dimension of their present jewelry store business. The addition may create 1-3 new jobs. There is an asphalt area there from a former garage. Sue said the addition meets all yard and setback requirement. Lueck asked if the City has a setback requirement from a platted alley. Sue indicated there is no minimum setback from an alley. Lueck feels there should be a minimum setback otherwise someone could build right up to the platted alley or maybe even on it and obstruct traffic.

Chairman Nickel called for a motion to act on the Gysbers Jewelry Store Addition

Motion by Vanderkin, seconded by Daane to approve the site plan for a 16' x 24' single story goldsmith addition on the south dimension of the jewelry store.

Vote: Vanderkin, Daane, TerBeest, Medema, Lueck, and Nickel – “AYE” Motion carried, unanimously 6/0.

5. **EXTRATERRITORIAL ZONING REVIEW** – N10565 Cottonwood Rd. Dodge County has submitted a letter of intent notification of a proposed minor subdivision that lies within the City of Waupun's extraterritorial zoning requirements. The City's subdivision ordinance has jurisdiction within 1 ½ miles of the City limits. Charles Schranz proposes to separate an approximate 3.8 acre parcel from an existing 40 acre parcel. The proposed new lot contains an existing set of buildings at N10656 Cottonwood Rd., Section 24, Town of Trenton, Dodge County, Wisconsin. The intended use would be single family residential.

Lueck questioned whether the proposed development was in accordance with the City's 2006 Comprehensive Plan for extraterritorial area. Sue Leahy said there is no problem with the City's plan.

Chairman Nickel called for a motion to act on the minor land division letter of intent from Dodge County.

Motion by Vanderkin, seconded by Medema to send an approval recommendation to Dodge County on the proposed land division of Charles and Roberta Schranz, Section 24, Town of Trenton, N10565 Cottonwood Rd., Waupun. Motion carried, unanimously.

6. **ADJOURNMENT** Motion by Vanderkin, seconded by Nickel to adjourn the meeting. Motion carried, unanimously. Meeting adjourned at 5:00 pm

Fred Lueck
Secretary



Application for Outdoor Service, Sidewalk Café and Beer Garden License

Restrictions and Guidelines are in accordance with Chapter 11.12 of the City of Waupun Ordinance

- A. Type of License ☐ Initial
☐ **Renewal** (Plan Commission review is not necessary unless the Police Chief, Building Inspector or City Clerk request such review of the Conditional Use Permit. The Plan Commission may review & revoke/revise the permit conditionals as it considers appropriate)

B. Licensing Period: Date of Council Approval to June 30, _____

C. Current Valid Liquor License Holder Information:

☒ "Class B" Sales of intoxicating liquor to consumers for on premise consumption
☐ Class "B" Sales of beer to consumers for on premise or off-premise consumption
☐ "Class C" Restaurants who sell wine by the glass or in an opened original container for consumption on the premises where sold
Name of Business: Wind to Wine
Address of Premise: 310 East Main St. Waupun WI 53963
Contact Number of Premise: 920-345-1169
Agent's Name: Jeff Collins
Agent's Contact Number: 920-948-7724

D. Application Type (Select your application type)

☒ **Sidewalk Café**

RESTRICTIONS & REQUIREMENTS

Sidewalk cafés may be permitted only from May 1 through October 15 in any year, no later than 10:00pm on properties located in the following districts who derive more than fifty percent (50%) of gross business revenue from the sale of food on the premises. **Mark your District:**

☐ B-1 Business/Professional Office District ☒ B-2 Central Business District

Guidelines and required measurements for the barrier surrounding the sidewalk café are provided in Chapter 11.12(5)(a). (A building permit may be required. Contact the City Building Inspector)

Type of Barrier Used: Stanchions & Chains
Height: 3' Width: 4' on Ends

MAP REQUIRED: (Mapping area – see Section E)

The application shall include a detailed map describing the outdoor area sought to be included within the description of the licensed premises. For a sidewalk café, the map shall also identify the number and location of tables, chairs and other furniture, fixtures and equipment to be installed or used, and their location and dimensions in relation to the pedestrian walkway.

Number of Tables: 8 Number of Chairs: 16
Other: _____ Number of other: _____
Other: _____ Number of other: _____

CERTIFICATE OF LIABILITY

Liability insurance naming the City as an unrestricted additional insured on the sidewalk café owner's insurance policy for the licensed sidewalk cafe site is required, including insurance to cover liquor liability.

Outdoor Service/Beer Garden

RESTRICTIONS & REQUIREMENTS

Outdoor premises may be permitted only on properties located in the following districts. **Mark your District:**

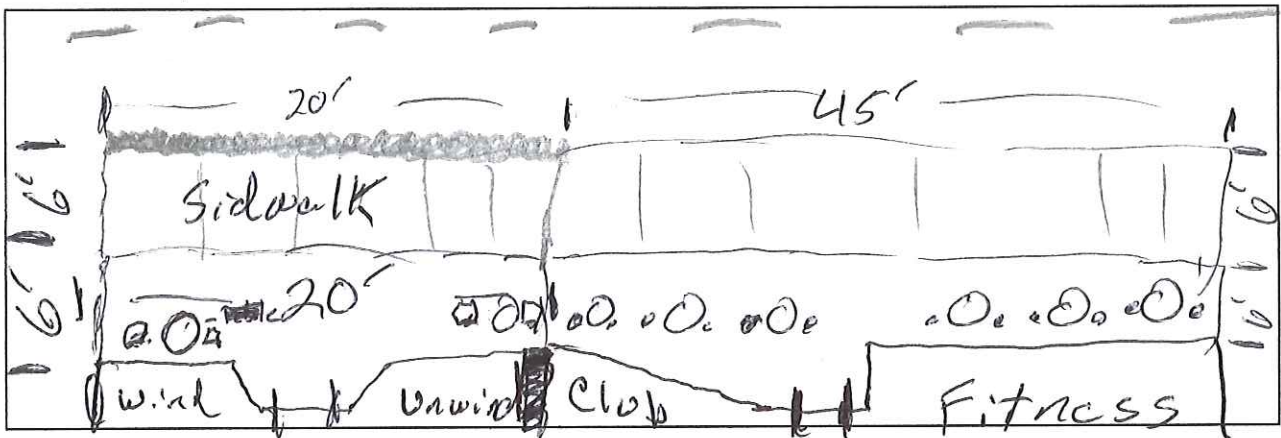
☒ B-1 Business/Professional Office District ☐ B-4 Interchange Business District
☒ B-2 Central Business District ☐ Planned Development District ☐ B-3 Shopping Center Business District

Guidelines and required measurements for an outdoor premise are provided in Chapter 11.12(4)(b)(c). (A building permit may be required. Contact the City Building Inspector)

MAP REQUIRED: (Mapping area – see Section E)

The application shall include a detailed map describing the outdoor area sought to be included within the description of the licensed premises.

E. Detailed Map



F. Conditional Use Permit Required

The City of Waupun Plan Commission is to review/approve a Conditional Use Permit, for initial licenses, which you may obtain from the City's Building Inspector. (Not a requirement for renewal licenses unless the Police Chief, Building Inspector or City Clerk request a Plan Commission review for reasons stated in the request, in which case the Plan Commission may review and revoke the permit or revise the permit conditions as it considers appropriate)

G. Signature Required

Business Agent/Owner Signature _____
Property Owner/Landlord Signature (if applicable) _____

Date: 7-31-2020
Date: _____

Official Staff Use:

Building Permit Required:

☐ No ☐ Yes- Date Permit Acquired/Paid _____ Initials: _____

Plan Commission Approval Required

☐ No ☐ Yes- Date of Meeting _____ (Circle: Approved / Denied) Initials: _____

Common Council Approval Required

Date of Meeting _____ (Circle: Approved / Denied)
License # Issued (if applicable) _____ Initials: _____

Fee: \$150.00 Paid: Date:



CITY OF WAUPUN
201 E. Main Street
WAUPUN, WISCONSIN 53963

Conditional Use Permit Application

Applicant Name: Jeff Collier Phone # 920-345-1169
Address: 310 East Main E-mail: jcollier@charter.net
City, State, Zip Waupun WI 53963

Property Description and address:

Wind & Unwind Coffee & Wine House

Conditional Use Requested:

Zoning Ordinance Section Involved:

Date Presented to Plan Commission:

CONDITIONAL USE: ☐ Granted ☐ Denied

Comments:

Signature of Applicant (s)

NOTICE OF PUBLIC HEARING

NOTICE IS HEREBY GIVEN, that at a special meeting of the Plan Commission of the City of Waupun, to be held on Monday, the 31st day of August at 4:45 O'clock in the P.M., there will be considered the application for a Conditional Use Permit of:

1. Wind & Unwind at 310 E Main St. to provide Outside Service. Section 11.12(4)(a) states that Outdoor premises may be permitted in the B-1, B-2, B-3, B-4, and PD district and shall be subject to a Conditional Use Permit.

The meeting will be conducted via zoom.

Topic: Special Plan Commission August 31, 2020

Join Zoom Meeting

<https://us02web.zoom.us/j/89385440703?pwd=YUYvaUp6NzBXZzR2Y25UY1dEdnpiQT09>

Meeting ID: 893 8544 0703

Passcode: 754763

Phone: 312 626 6799 US (Chicago)

PLEASE TAKE FURTHER NOTICE that all persons desiring to be heard on the proposed Conditional Use in support thereof or in opposition thereto, must contact the Zoning Administrator prior to said meeting of the Plan Commission of the City of Waupun.

August 19, 2020

Susan Leahy
Zoning Administrator
City of Waupun

(PUBLISH August 22, 2020)



July 10, 2020

CUST ID No. 905255

ATTN: Buildings & Structures Building Inspector

AARON J HALBERG
HALBERG ENGINEERING LLC
10335N DUFFY RD
HAYWARD WI 54843

BUILDING INSPECTION
CITY OF WAUPUN
201 E MAIN ST
WAUPUN WI 53963

CONDITIONAL APPROVAL

(Please forward a copy of this letter to the fire department conducting inspections of this project.)

PLAN APPROVAL EXPIRES: 07/10/2022

SITE:

Municipal Well and Pump
1212 Storbeck Dr
City of Waupun, 53963
Dodge County

FOR:

Facility: 798545 MUNICIPAL WELL AND PUMP
1212 STORBECK DRIVE
WAUPUN 53963

Identification Numbers
Transaction ID No. 3307204 Site ID No. 637919
Please refer to both identification numbers, above, in all correspondence with the agency.

Object Type: Building ICC Regulated Object ID No.: 1849409 Code Applies Date: 04/29/20
Major Occupancy: Storage; Type VB Combustible Unprotected class of construction; New plan; 8,640 project sq ft;
Unsprinklered; Occupancy: S-1 Storage Moderate-Hazard; Truss, Roof

Object Type: Truss, Roof Regulated Object ID No.: 1849410 Code Applies Date: 04/29/20

SITE REQUIREMENTS

- Contact both the State Inspector and the local municipality PRIOR to the start of construction.
- A full size copy of the approved plans, specifications and this letter shall be on-site during construction and open to inspection by authorized representatives of the Department, which may include local inspectors. If plan index sheets were submitted in lieu of additional full plan sets, a copy of this approval letter and index sheet shall be attached to plans that correspond with the copy on file with the Department. If these plans were submitted in an electronic form, the designer is responsible to download, print, and bind the full size set of plans along with our approval letter. A department electronic stamp and signature shall be on the plans which are used at the job site for construction.

The following conditions shall be met during construction or installation and prior to occupancy or use:

KEY ITEM(S)

- **IBC 1604.4** – Load effects on structural members and their connections shall be determined by methods of structural analysis that take into account equilibrium, general stability, geometric compatibility and both short- and long-term material properties. The vertical load from the columns creates an eccentric load on the grade beam foundation which this reviewer requested the design professional address. The building shall be adequately designed and constructed to provide stability including resistance to overturning effects on structural elements created by eccentric loads. This has been done partially by some revisions to the foundations as reflected on the revised plans particularly with the isolated square footings at the jamb columns of the overhead doors. The sidewall grade beam for the remainder of the building has been re-designed as an isolated footing but with a factor of safety against overturning of 0.5. When questioned about the inadequate factor of safety to overturning effects created by the eccentric load the design professional Aaron Halberg provided the following explanation: “The same connection of the interior slab to the top of the thickened edge “beam” and the 45° taper between them is also what prevents overturning... The safety

factor is set so low on the isolated footing because it's a false assumption. It is very reasonable to ignore the overturning calculations on this structural analog when a portion of a united whole is analyzed as a long skinny footing, as if it is a deep beam on edge that can roll over if you put an eccentric load on top of it. In reality, the overturning prevented by the connection to the interior slab and the unity it has with the slab reinforcement. Requiring any overturning check on an analysis of a separate beam that is not really separate seems unnecessary."

- **SPS 361.31(2)** - For new buildings, building additions, and building alterations in an existing building, the Department is NOT requiring lighting plans to be submitted for review and conditional approval. However, the owner is reminded that proper plans and calculations, demonstrating compliance with the general lighting requirements as contained in the IECC; and emergency illumination requirements as addressed in the IBC, are to be on-site and made available to a Dept. representative upon request.
- SUBMIT – The following systems require submittal for review and approval prior to construction.

- **SPS 361.30(3)** - This approval does not include heating, ventilating or air conditioning. The owner should be reminded that HVAC plans, calculations, and appropriate fees are required to be submitted for review and approval prior to installation in the field. The HVAC plans shall be directed to the office where building plans were originally submitted and conditionally approved (as appropriate). Building Designer should coordinate with HVAC design to avoid problems with clearance to combustibles, dampers etc. The submitted HVAC plans and calculations shall match the approved building plans. Building Designer is requested to provide a complete set of plans, Energy Calculations and the Building plan review Transaction I.D. number to the HVAC Designer to help coordinate review. **Note as per SPS 302.10 installation of HVAC without approved plans could result in double plan review fees.**

REMINDERS

- **IBC 311.2** - This building has been submitted, reviewed and conditionally approved as an S-1 moderate hazard storage building. Motorized vehicles may not be driven into the building for loading or unloading, or stored unless appropriate ventilation requirements for an enclosed parking garage are met. Additionally, hazardous materials such as fuel, gun power, fireworks, fertilizers, etc. may not be stored in the building unless the limitations in the amounts addressed in IBC 307 are adhered to. The owner shall be responsible to address these storage materials with the ensuing HVAC design professional so that appropriate exhausts and make up air are facilitated. Failure to do so may cause delays in HVAC plan approvals, or if a change of use is recognized after plan approval, may require revised plans addressing the change in use with additional fees to be submitted.
- **IBC 2902.1** - Since this building lacks toilet facilities, it is approved as unoccupied storage only.
- **SPS 361.36(1)(a) & (b)** - The building shell shall be closed within two years of the initial approval date of this project. Also, this approval will expire three years after the date of initial approval of this project if the work covered by this approval is not completed and the building ready for occupancy within those three years.
- This plan has not been reviewed for conformance to any fire dept. access (lane) and water supply requirements. The designer is reminded that the requirements for fire lanes and water supply requirements are now contained in Chapter SPS 314, Fire Prevention Code. Consult with the local fire authority having jurisdiction for applicable codes to be used in the design of fire lanes. The designer shall refer to the NFPA 1 Chapter 18 for information regarding fire dept. access (lane) and water supply requirements and design.
- The department does not have an email address for the building owner Municipal Well and Pump. Due to logistical constraints created by the current work environment in response to the coronavirus pandemic a hard copy of this correspondence will not be mailed to the owner. Please provide a copy of this correspondence to the owner Municipal Well and Pump.

The submittal described above has been reviewed for conformance with applicable Wisconsin Administrative Codes and Wisconsin Statutes. The submittal has been **CONDITIONALLY APPROVED**. The owner, as defined in chapter 101.01(10), Wisconsin Statutes, is responsible for compliance with all code requirements. Only those object types listed above have been approved; other submittals such as plumbing and those listed above under **REQUIRED SUBMITTAL(S)**, may also be required.

All permits required by the state or the local municipality shall be obtained prior to commencement of construction/installation/operation. You are responsible for complying with state and federal laws concerning construction near or on wetlands, lakes, and streams.

This plan has not been reviewed for compliance with fire code requirements, including those for fire lanes and fire protection water supply, so contact the local fire department for further information.

In granting this approval, the Division of Industry Services reserves the right to require changes or additions, should conditions arise making them necessary for code compliance. As per state stats 101.12(2), nothing in this review shall relieve the designer of the responsibility for designing a safe building, structure, or component. The Division does not take responsibility for the design or construction of the reviewed items.

Per s. SPS 361.40(4), projects for buildings of over 50,000 cubic feet total volume shall have supervising professionals who file compliance statements with this agency and the local code officials prior to occupancy of the project. The compliance statement is available on our website. <http://verification.dsps.wi.gov/IndustryServices/Commercial-Buildings-Compliance/DSPSMainForm.aspx>

Inquiries concerning this correspondence may be made to me at the telephone number listed below, or at the address on this letterhead.

Sincerely,

Fee Required \$ 600.00

This Amount Will Be Invoiced. When You
Receive That Invoice, Please Include a Copy
With Your Payment Submittal.

Steven P Dobratz, P.E.
Building Plan Reviewer, Division of Industry Services
(920)492-2214, M-F 7:45 a.m. - 4:30 p.m.
steve.dobratz@wisconsin.gov

cc: Municipal Well & Pump
David Pedersen, Building Inspector, (608) 669-0372, 7:45 A.M. - 4:30 P.M.

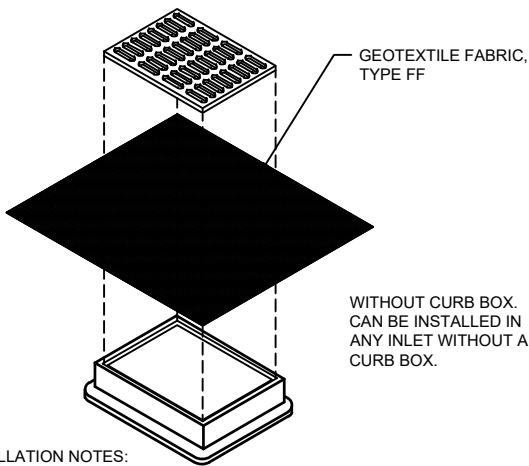
CONSTRUCTION SITE
EROSION CONTROL REQUIREMENTS

- SECTION NR216.46 OF WISCONSIN STATE ADMINISTRATIVE CODE IDENTIFIES REQUIREMENTS FOR CONSTRUCTION SITE AND POST-CONSTRUCTION EROSION CONTROL. IT IS THE INTENT OF THESE PLANS TO SATISFY THESE REQUIREMENTS. THE METHODS AND STRUCTURES USED TO CONTROL EROSION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. CONTRACTOR SHALL IMPLEMENT AN APPROPRIATE MEANS OF CONTROLLING EROSION DURING SITE OPERATION AND UNTIL THE VEGETATION IS RE-ESTABLISHED. ADJUSTMENTS TO THE CONTROL SYSTEM SHALL BE MADE AS REQUIRED.
- ALL WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE WISCONSIN DNR'S CONSERVATION PRACTICE STANDARDS. THESE STANDARDS ARE PERIODICALLY UPDATED AND IT IS THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN AND REFERENCE THE MOST RECENTLY RELEASED STANDARD.
- THIS INFORMATION IS ONLY ONE PART OF THE OVERALL EROSION CONTROL REQUIREMENTS. ADDITIONAL REQUIREMENTS MAY ALSO BE SHOWN ON THE CONTRACT DRAWINGS AND IN THE ACCOMPANYING SPECIFICATIONS.
- ADDITIONAL EROSION CONTROL MEASURES, AS REQUESTED IN WRITING BY THE STATE OR LOCAL INSPECTORS, OR THE OWNER'S ENGINEER, SHALL BE INSTALLED WITHIN 24 HOURS.
- THE AREA OF EROSION LAND EXPOSED TO THE ELEMENTS BY GRUBBING, EXCAVATION, TRENCHING, BORROW AND FILL OPERATIONS AT ANY ONE TIME SHALL BE MINIMIZED TO THE MAXIMUM EXTENT PRACTICABLE. FOR ANY DISTURBED AREA THAT REMAINS INACTIVE FOR GREATER THAN 7 WORKING DAYS, OR WHERE GRADING WORK EXTENDS BEYOND THE PERMANENT SEEDING DEADLINES, THE SITE MUST BE TREATED WITH TEMPORARY STABILIZATION MEASURES SUCH AS SOIL TREATMENT, TEMPORARY SEEDING AND/OR MULCHING. ALL DISTURBED AREAS SHALL BE TREATED WITH PERMANENT STABILIZATION MEASURES WITHIN 3 WORKING DAYS OF FINAL GRADING.
- ALL EROSION CONTROL MEASURES AND STRUCTURES SERVING THE SITE MUST BE INSPECTED AT LEAST WEEKLY OR WITHIN 24 HOURS OF THE TIME 0.5 INCHES OF RAIN HAS OCCURRED. ALL NECESSARY REPAIR AND MAINTENANCE WILL BE DONE AT THIS INSPECTION TIME.
- ALL EROSION CONTROL DEVICES AND/OR STRUCTURES SHALL BE PROPERLY INSTALLED PRIOR TO CLEARING AND GRUBBING OPERATIONS WITHIN THEIR RESPECTIVE DRAINAGE AREAS. THESE SHALL BE PROPERLY MAINTAINED FOR MAXIMUM EFFECTIVENESS UNTIL VEGETATION IS RE-ESTABLISHED.
- ALL EROSION CONTROL DEVICES SHALL BE PROPERLY INSTALLED PRIOR TO ANY SOIL DISTURBANCE.
- ANY SLOPES STEEPER THAN 3H:1V SHALL BE STAKED WITH EROSION CONTROL FABRIC UNLESS INDICATED ON THE PLAN.
- ALL WASTE AND UNUSED BUILDING MATERIALS (INCLUDING GARBAGE, DEBRIS, CLEANING WASTES, WASTEWATER, TOXIC MATERIALS, OR HAZARDOUS MATERIALS) SHALL BE PROPERLY DISPOSED OF AND NOT ALLOWED TO BE CARRIED OFF-SITE BY RUNOFF OR WIND.
- WIND EROSION SHALL BE KEPT TO A MINIMUM DURING CONSTRUCTION. WATERING, MULCH, OR A TACKING AGENT MAY BE REQUIRED TO PROTECT NEARBY RESIDENCES AND WATER RESOURCES.
- CHANNELIZED RUNOFF ENTERING THE PROJECT SITE FROM ADJOINING LANDS SHALL BE DIVERTED THROUGH NATURALLY OR ARTIFICIALLY EROSION-RESISTANT CONVEYANCES. IF CHANNELIZED RUNOFF CANNOT BE DIVERTED, SITE BEST MANAGEMENT PRACTICES MUST ACCOUNT FOR THE ADDITIONAL FLOW RATES AND EROSION POTENTIAL THAT SUCH RUNOFF PRESENTS.
- THE CONTRACTOR SHALL TAKE ALL POSSIBLE PRECAUTIONS TO PREVENT SOILS FROM BEING TRACKED ONTO PUBLIC OR PRIVATE ROADWAYS. PAVED SURFACES ADJACENT TO CONSTRUCTION SITE VEHICLE ACCESS SHALL BE SWEEPED AND/OR SCRAPED (NOT FLUSHED) PERIODICALLY TO REMOVE SOIL, DIRT, AND/OR DUST.
- EROSION CONTROLS SHALL BE INSTALLED ON THE DOWNSTREAM SIDE OF TEMPORARY STOCKPILES. ANY SOIL STOCKPILE THAT REMAINS FOR MORE THAN 30 DAYS SHALL BE COVERED OR TREATED WITH STABILIZATION PRACTICES SUCH AS TEMPORARY OR PERMANENT SEEDING AND MULCHING. ALL STOCK PILES SHALL BE PLACED AT LEAST 75 FEET FROM STREAMS OR WETLANDS.
- ADDITIONAL EROSION CONTROL FOR UTILITY CONSTRUCTION (STORM SEWER, SANITARY SEWER, WATER MAIN, ETC.) SHALL INCLUDE THE FOLLOWING:
 - PLACE EXCAVATED TRENCH MATERIAL ON THE HIGH SIDE OF THE TRENCH.
 - BACKFILL, COMPACT, AND STABILIZE THE TRENCH IMMEDIATELY AFTER PIPE CONSTRUCTION.
 - DISCHARGE OF TRENCH WATER OR DEWATERING EFFLUENT MUST BE PROPERLY TREATED TO REMOVE SEDIMENT IN ACCORDANCE WITH THE WDNR CONSERVATION PRACTICE STANDARD 1061 - DEWATERING OR A SUBSEQUENT WDNR DEWATERING STANDARD PRIOR TO DISCHARGE INTO A STORM SEWER, DITCH, DRAINAGEWAY, OR WETLAND OR LAKE.
- ALL DRAINAGE CULVERTS, STORM DRAIN INLETS, MANHOLES, OR ANY OTHER EXISTING STRUCTURES THAT COULD BE DAMAGED BY SEDIMENTATION SHALL BE PROTECTED ACCORDING TO THE VARIOUS METHODS PROVIDED IN THE PRINTED CONSERVATION PRACTICE STANDARDS.
- ANY SOIL EROSION THAT OCCURS AFTER FINAL GRADING AND/OR STABILIZATION MUST BE REPAIRED AND THE STABILIZATION WORK REDONE.
- THE FIRST SIX WEEKS AFTER INITIAL STABILIZATION, ALL NEWLY SEEDDED AND MULCHED AREAS SHALL WATERED WHENEVER 7 DAYS ELAPSE WITHOUT A RAIN EVENT.
- WHEN THE DISTURBED AREA HAS BEEN STABILIZED BY PERMANENT VEGETATION OR OTHER MEANS, TEMPORARY BMP'S SUCH AS SILT FENCES, STRAW BALES, AND SEDIMENT TRAPS SHALL BE REMOVED AND THESE AREAS STABILIZED.
- ALL TEMPORARY BEST MANAGEMENT PRACTICES SHALL BE MAINTAINED UNTIL THE SITE IS STABILIZED.
- ALL DISTURBED AREAS SHALL BE PERMANENTLY STABILIZED WITH SEED AND MULCH UNLESS OTHERWISE SPECIFIED. A MINIMUM OF FOUR INCHES OF TOPSOIL SHALL BE APPLIED TO ALL AREAS TO BE SEEDDED OR SODDED.

GENERAL NOTES - INLET PROTECTION

INLET PROTECTION DEVICES SHALL BE MAINTAINED OR REPLACED AT THE DIRECTION OF THE ENGINEER. MANUFACTURED ALTERNATIVES APPROVED AND LISTED ON THE DEPARTMENT'S EROSION CONTROL PRODUCT ACCEPTABILITY LIST MAY BE SUBSTITUTED. WHEN REMOVING OR MAINTAINING INLET PROTECTION, CARE SHALL BE TAKEN SO THAT THE SEDIMENT TRAPPED ON THE GEOTEXTILE FABRIC DOES NOT FALL INTO THE INLET. ANY MATERIAL FALLING INTO THE INLET SHALL BE REMOVED IMMEDIATELY.

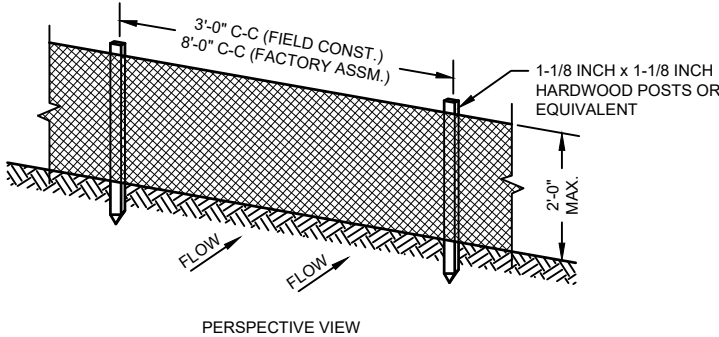
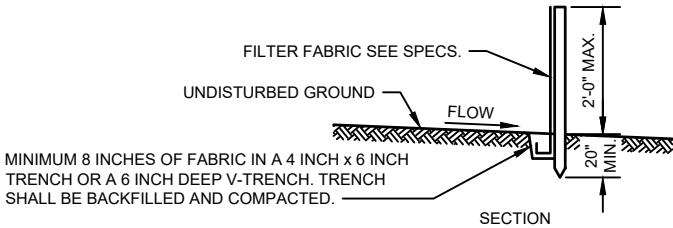
- FINISHED SIZE, INCLUDING FLAP POCKETS WHERE REQUIRED, SHALL EXTEND A MINIMUM OF 10" AROUND THE PERIMETER TO FACILITATE MAINTENANCE OR REMOVAL.
- FOR INLET PROTECTION, TYPE C (WITH CURB BOX), AN ADDITIONAL 18" OF FABRIC IS WRAPPED AROUND THE WOOD AND SECURED WITH STAPLES. THE WOOD SHALL NOT BLOCK THE ENTIRE HEIGHT OF THE CURB BOX OPENING.
- FLAP POCKETS SHALL BE LARGE ENOUGH TO ACCEPT WOOD 2X4.



INSTALLATION NOTES:

TRIM EXCESS FABRIC IN THE FLOW LINE TO WITHIN 3" OF THE GRATE. THE CONTRACTOR SHALL DEMONSTRATE A METHOD OF MAINTENANCE, USING A SEWN FLAP, HAND HOLDS OR OTHER METHOD TO PREVENT ACCUMULATED SEDIMENT FROM ENTERING THE INLET.

INLET PROTECTION, TYPE B
NO SCALE



GENERAL NOTES:

- ENDS OF FENCE SHALL BE TURNED UPSLOPE 1 TO 2 FEET IN ELEVATION TO PREVENT FLANKING.
- STAPLE FABRIC WITH 1/2 INCH (MINIMUM) STAPLES TO THE UPSLOPE SIDE OF THE POSTS.
- WHEN TWO SECTIONS OF FILTER FABRIC ADJOIN EACH OTHER THEY SHALL BE OVERLAPPED BY SIX INCHES AND FOLDED.

TYPICAL SILT FENCE INSTALLATION AT SITE PERIMETER DETAIL
NO SCALE

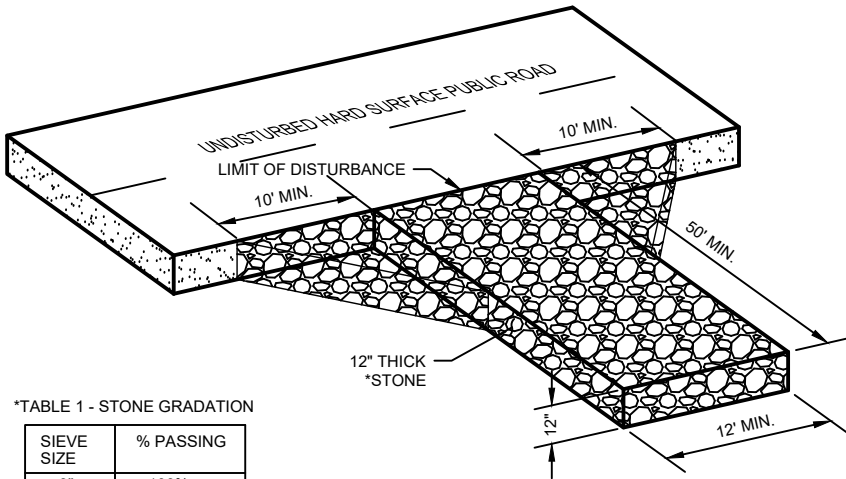


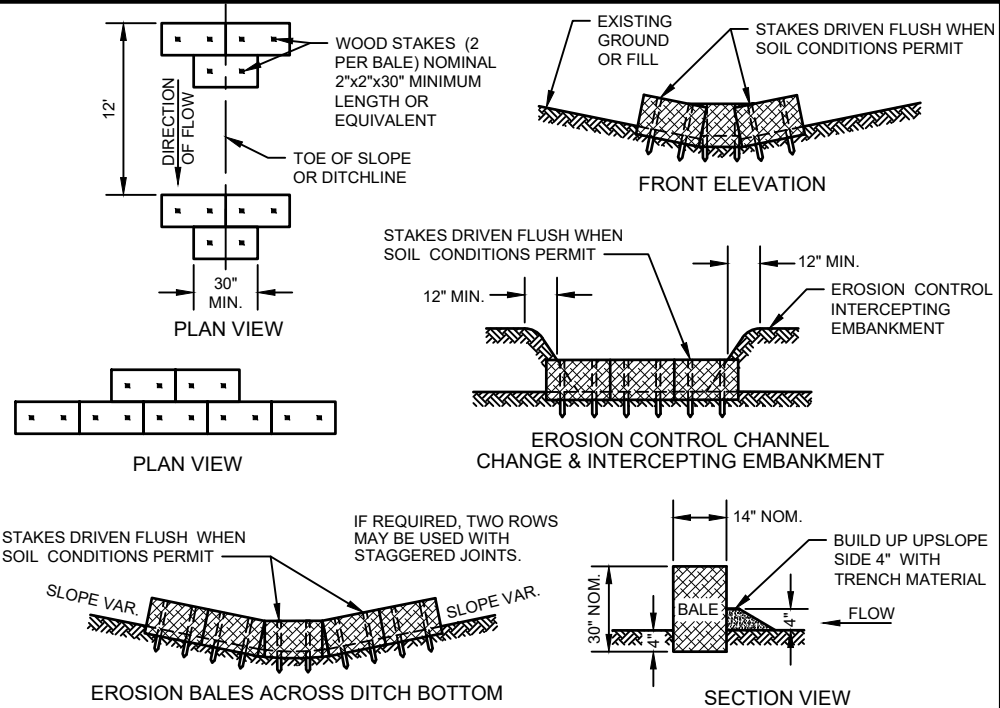
TABLE 1 - STONE GRADATION

SIEVE SIZE	% PASSING
3"	100%
2 1/2"	90 - 100%
1 1/2"	25 - 60%
3/4"	0 - 20%
3/8"	0 - 5%

NOTES:

- TRACKING PAD WIDTH SHALL BE AT LEAST THE FULL WIDTH OF HTE EGRESS POINT OR 12' WIDE MINIMUM.
- TRACKING PAD LENGTH SHALL BE 50' FOR CONSTRUCTION SITES, 30' FOR SINGLE FAMILY RESIDENTIAL, OR AS SPECIFIED IN THE CONTRACT DOCUMENTS. LENGTH OF TRACKING PAD MAY NEED TO BE INCREASE OR ADDITIONAL SEDIMENT CONTROL PRACTICES SHALL BE INSTALLED BY THE CONTRACTOR IS SEDIMENT TRACK-OUT OCCURS.
- GEOTEXTILE FABRIC TYPE R SHALL BE INSTALLED BETWEEN THE STONE AND SUBGRADE ON SITES WHERE HIGH GROUND WATER IS OBSERVED.
- CONTRACTOR SHALL CLEAN STREET/ROADWAY ADJACENT TO ALL CONSTRUCTION ACCESS POINTS AT THE END OF EACH WORKDAY OR MORE FREQUENTLY IF REQUESTED.

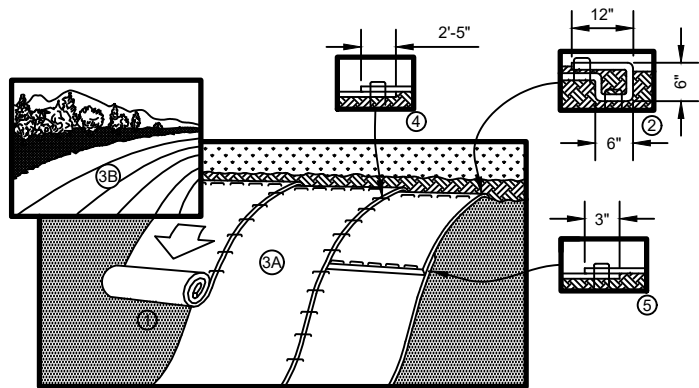
VEHICLE TRACKING PAD
NO SCALE



GENERAL NOTES:

- DETAILS OF CONSTRUCTION, MATERIALS AND WORKMANSHIP NOT SHOWN ON THIS DRAWING SHALL CONFORM TO THE PERTINENT REQUIREMENTS OF THE SPECIFICATIONS.
- BALES SHALL BE PLACED END TO END OR OVERLAPPING AT RIGHT ANGLES TO THE DIRECTION OF FLOW. BALES SHALL EXTEND FAR ENOUGH UP THE SLOPES TO PREVENT ERODING AROUND ENDS.
- BALES SHALL BE PLACED WITH TWINE OR TIE WIRES PARALLEL TO THE GROUND.
- STAKES TO BE BATTERED IN OPPOSITE DIRECTIONS.
- BALES SHALL BE EMBEDDED 4 INCHES MINIMUM.

EROSION BALES IN DRAINAGE WAY DETAIL
NO SCALE



- PREPARE SOIL BEFORE INSTALLING BLANKETS, INCLUDING ANY NECESSARY APPLICATION OF LIME, FERTILIZER, AND SEED.
- BEGIN AT THE TOP OF THE SLOPE BY ANCHORING THE BLANKET IN A 6" (15 CM) DEEP X 6" (15 CM) WIDE TRENCH WITH APPROXIMATELY 12" (30cm) OF BLANKET EXTENDED BEYOND THE UP-SLOPE PORTION OF THE TRENCH. ANCHOR THE BLANKET WITH A ROW OF STAPLES/STAKES APPROXIMATELY 12" (30 CM) APART IN THE BOTTOM OF THE TRENCH. BACKFILL AND COMPACT THE TRENCH AFTER STAPLING. APPLY SEED TO COMPACTED SOIL AND FOLD REMAINING 12" (30 CM) PORTION OF BLANKET BACK OVER SEED AND COMPACTED SOIL. SECURE BLANKET OVER COMPACTED SOIL WITH A ROW OF STAPLES/STAKES SPACED APPROXIMATELY 12" (30 CM) APART ACROSS THE WIDTH OF THE BLANKET.
- ROLL THE BLANKETS (A) DOWN OR (B) HORIZONTALLY ACROSS THE SLOPE. BLANKETS WILL UNROLL WITH APPROPRIATE SIDE AGAINST THE SOIL SURFACE. ALL BLANKETS MUST BE SECURELY FASTENED TO SOIL SURFACE BY PLACING STAPLES/STAKES IN APPROPRIATE LOCATIONS AS SHOWN IN THE STAPLE PATTERN GUIDE.
- THE EDGES OF PARALLEL BLANKETS MUST BE STAPLED WITH APPROXIMATELY 2'-5" (5 CM-12.5 CM) OVERLAP DEPENDING ON BLANKET TYPE.
- CONSECUTIVE BLANKETS SPICED DOWN THE SLOPE MUST BE PLACED END OVER END (SHINGLE STYLE) WITH AN APPROXIMATE 3" (7.5 CM) OVERLAP. STAPLE THROUGH OVERLAPPED AREA, APPROXIMATELY 12" (30 CM) APART ACROSS ENTIRE BLANKET WIDTH.

NOTE:

*IN LOOSE SOIL CONDITIONS, THE USE OF STAPLE OR STAKE LENGTHS GREATER THAN 6" (15 CM) MAY BE NECESSARY TO PROPERLY SECURE THE BLANKETS.

EROSION CONTROL BLANKET DETAIL
NO SCALE

PROJECT DATE:	DRAWN BY:	NO.	DATE	REVISION	BY:
	NAR
	DESIGNED BY:	NAR	.	.	.
	CHECKED BY:	JML	.	.	.
PLOT DATE: 8/21/2020 9:52 AM, G:\2121046\21046000\CADD\C3D\21046000 Design.dwg					

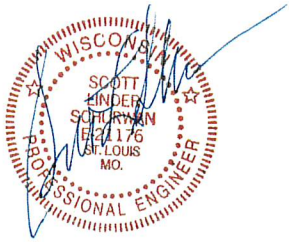


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201 Corporate Drive, Beaver Dam WI 53916
(920) 887-4242 www.msa-ps.com
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MUNICIPAL WELL & PUMP BUILDING ADDITION
M.P.B. BUILDERS, INC.
CITY OF WAUPUN, DODGE COUNTY, WISCONSIN

CONSTRUCTION DETAILS

PROJECT NO:
21046000
SHEET
D1



Alpine
13723 Riverport Dr
Suite 200
Maryland Heights, MO 63043
Phone: (800)326-4102 (314)344-9121
Fax: (314)344-3152
alpineitw.com

Site Information:	Page 1:
Customer: Truss's Plus, Inc.	Job Number: Q20-361
Job Description: Job: Q20-361 - Municipal Well & Pump / MPB	
Address:	

Job Engineering Criteria:	
Design Code: IBC 2015	IntelliVIEW Version: 18.02.01A JRef #: 1WVZ71760001
Wind Standard: ASCE 7-10 Wind Speed (mph): 115	Roof Load (psf): 30.00- 4.00- 0.00- 5.00
Building Type: Closed	Floor Load (psf): None

This package contains general notes pages, 1 truss drawing(s) and 0 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	161.20.1125.01390	72' Common 30-4-5@8			

The seal affixed hereto indicates acceptance of professional engineering responsibility for the documents listed on this sheet. Documents which may be attached with this sheet that do not bear my seal have neither been prepared by me or under my direct supervision.
ITWBCG ICC-ES Plate Evaluation Report ESR-1118.

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

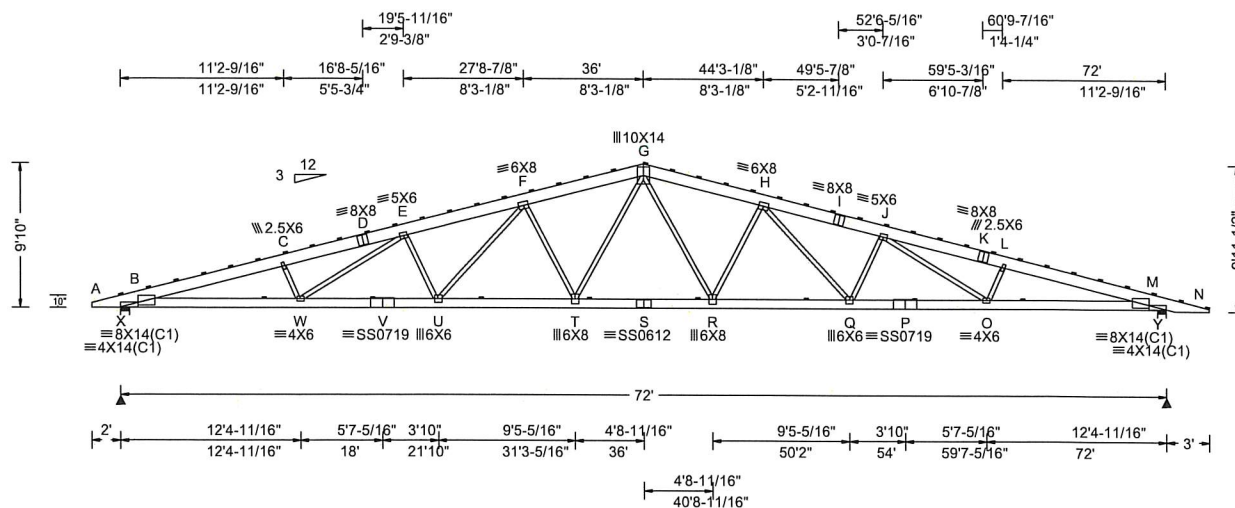
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 13723 Riverport Drive, Suite 200, Maryland Heights, MO 63043; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00 TCDL: 4.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 39.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 96.0 "	Wind Std: ASCE 7-10 Speed: 115 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.96 ft TCDL: 2.4 psf BCDL: 2.4 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 7.20 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: 30.0 Ct: 1.1 CAT: II Pf: 23.1 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15	PP Deflection in loc L/defl L/# VERT(LL): 1.692 T 508 240 VERT(CL): 2.188 T 393 240 HORZ(LL): 0.450 O - - HORZ(TL): 0.582 O - - Creep Factor: 2.0 Max TC CSI: 0.846 Max BC CSI: 0.996 Max Web CSI: 0.751	Gravity Loc R+ / R- / Rh Non-Gravity / Rw / U / RL X 11740 /- /- /2771 /3007 /315 Y 11884 /- /- /2779 /3047 /- Wind reactions based on MWFRS X Brg Width = 7.3 Min Req = 4.9 Y Brg Width = 7.3 Min Req = 4.9 Bearings X & Y are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
		Code / Misc Criteria Bldg Code: IBC 2015 TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/2(0) Plate Type(s): WAVE, 18SS	VIEW Ver: 18.02.01A.0205.19	B - C 8603 - 18058 C - D 8517 - 17626 D - E 8524 - 17442 E - F 7695 - 15668 F - G 6556 - 12835 G - H 6573 - 12833 H - I 7708 - 15554 I - J 7695 - 15658 J - K 8398 - 17542 K - L 8381 - 17590 L - M 8470 - 18020

Lumber

Top chord: 2x10 SP 2400f-2.0E;
Bot chord: 2x8 SP 2400f-2.0E;
Webs: 2x4 SPF 2100f-1.8E;

Nailnote

Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @ 7.50" o.c.
Bot Chord: 1 Row @ 12.00" o.c.
Webs: 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Purlins

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
TC	24	-1.74	36.00
TC	24	36.00	74.24
BC	101	0.15	71.85

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

Loading

Truss designed for unbalanced snow loads.

Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

Refer to General Notes for additional information
WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and untrained people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - W	17334 - 8046	S - R	11069 - 4979
W - V	15872 - 7314	R - Q	13493 - 6261
V - U	15872 - 7314	Q - P	15858 - 7328
U - T	13497 - 6241	P - O	15858 - 7328
T - S	11069 - 4979	O - M	17294 - 7990

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - W	598 - 815	G - R	2873 - 1346
W - E	1394 - 682	R - H	1449 - 2665
E - U	1009 - 1834	H - Q	2450 - 1072
U - F	2459 - 1094	Q - J	987 - 1824
F - T	1451 - 2689	J - O	1368 - 600
T - G	2898 - 1349	O - L	558 - 805



06/09/2020

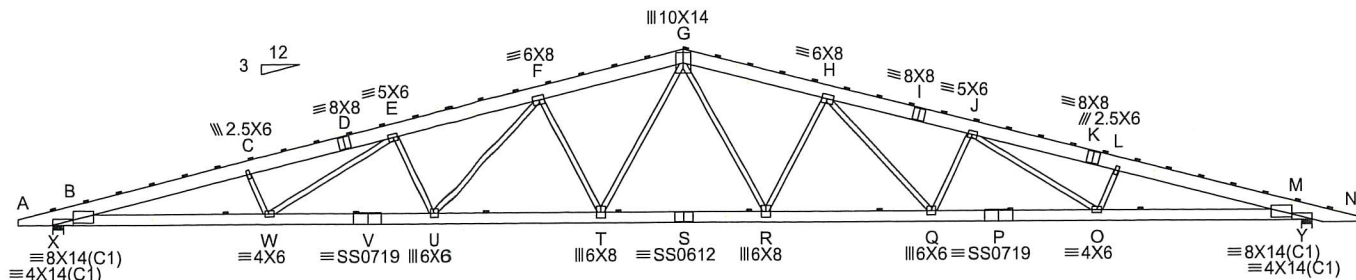
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org



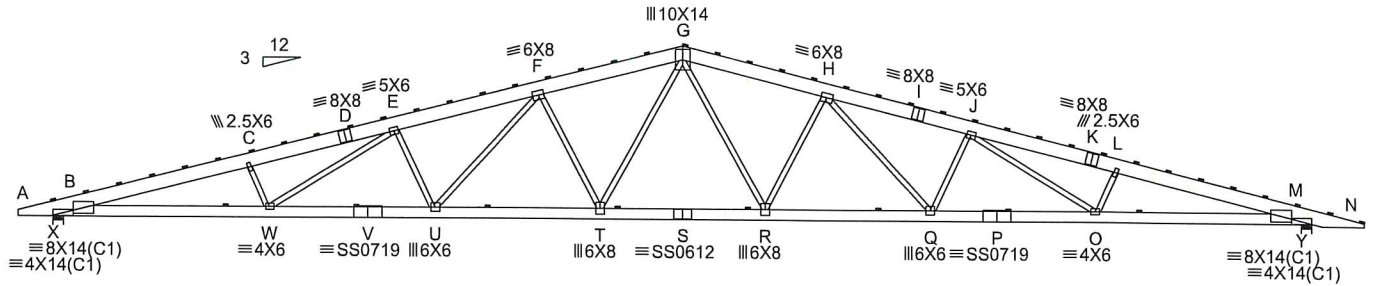
Member	MaxTen	MaxComp	Axial	Bend	CSI	GLC	Size	Material	Length	Brace
A - B	57.74	-0.00	0.002	0.042	0.044	1	2x10	SP 2400f-2.0E	21.57"	Purl 24
B - C	8603.49	-18058.21	0.508	0.337	0.846	1	2x10	SP 2400f-2.0E	110.71"	Purl 24
C - D	8517.26	-17626.27	0.489	0.301	0.790	1	2x10	SP 2400f-2.0E	102.21"	Purl 24
D - E	8523.71	-17441.80	0.484	0.192	0.676	1	2x10	SP 2400f-2.0E	102.21"	Purl 24
E - F	7695.37	-15667.74	0.384	0.133	0.516	1	2x10	SP 2400f-2.0E	102.21"	Purl 24
F - G	6555.66	-12835.20	0.251	0.153	0.404	1	2x10	SP 2400f-2.0E	103.36"	Purl 24
G - H	6572.81	-12833.13	0.251	0.153	0.404	1	2x10	SP 2400f-2.0E	103.36"	Purl 24
H - I	7707.53	-15553.91	0.383	0.132	0.515	1	2x10	SP 2400f-2.0E	102.21"	Purl 24
I - J	7695.21	-15657.54	0.390	0.111	0.501	1	2x10	SP 2400f-2.0E	102.21"	Purl 24
J - K	8397.83	-17541.79	0.487	0.302	0.789	1	2x10	SP 2400f-2.0E	102.21"	Purl 24
K - L	8381.47	-17589.78	0.490	0.271	0.760	1	2x10	SP 2400f-2.0E	102.21"	Purl 24
L - M	8469.52	-18020.04	0.506	0.334	0.839	1	2x10	SP 2400f-2.0E	110.71"	Purl 24
M - N	74.29	-0.00	0.002	0.070	0.072	1	2x10	SP 2400f-2.0E	27.76"	Purl 24
B - W	17333.58	-8045.54	0.940	0.056	0.996	16	2x8	SP 2400f-2.0E	121.56"	Purl 101
W - V	15871.86	-7313.90	0.659	0.193	0.852	1	2x8	SP 2400f-2.0E	113.32"	Purl 101
V - U	15871.86	-7313.90	0.659	0.135	0.794	1	2x8	SP 2400f-2.0E	113.32"	Purl 101
U - T	13497.11	-6241.28	0.561	0.048	0.609	1	2x8	SP 2400f-2.0E	113.32"	Purl 101
T - S	11069.38	-4978.81	0.460	0.043	0.503	1	2x8	SP 2400f-2.0E	113.32"	Purl 101
S - R	11069.38	-4978.81	0.460	0.043	0.503	1	2x8	SP 2400f-2.0E	113.32"	Purl 101
R - Q	13493.06	-6261.21	0.560	0.048	0.608	1	2x8	SP 2400f-2.0E	113.32"	Purl 101
Q - P	15857.88	-7327.78	0.659	0.136	0.794	1	2x8	SP 2400f-2.0E	113.32"	Purl 101
P - O	15857.88	-7327.78	0.659	0.196	0.855	1	2x8	SP 2400f-2.0E	113.32"	Purl 101
O - M	17294.35	-7989.72	0.934	0.056	0.990	16	2x8	SP 2400f-2.0E	121.56"	Purl 101
C - W	598.47	-815.08	0.092	---	0.092	1	2x4	SPF 2100f-1.8E	29.98"	---
W - E	1393.76	-682.34	0.223	---	0.223	21	2x4	SPF 2100f-1.8E	99.45"	---
E - U	1008.78	-1833.75	0.337	---	0.337	1	2x4	SPF 2100f-1.8E	58.10"	---
U - F	2458.56	-1093.70	0.389	---	0.389	21	2x4	SPF 2100f-1.8E	104.20"	---
F - T	1451.47	-2689.17	0.751	---	0.751	5	2x4	SPF 2100f-1.8E	87.00"	---
T - G	2897.69	-1349.26	0.583	---	0.583	22	2x4	SPF 2100f-1.8E	115.99"	---
G - R	2873.48	-1346.13	0.582	---	0.582	21	2x4	SPF 2100f-1.8E	116.00"	---
R - H	1448.73	-2665.26	0.744	---	0.744	4	2x4	SPF 2100f-1.8E	87.00"	---
H - Q	2449.63	-1072.41	0.381	---	0.381	22	2x4	SPF 2100f-1.8E	104.20"	---
Q - J	987.26	-1824.13	0.335	---	0.335	1	2x4	SPF 2100f-1.8E	58.10"	---
J - O	1367.70	-599.93	0.196	---	0.196	22	2x4	SPF 2100f-1.8E	99.45"	---
O - L	557.99	-805.04	0.091	---	0.091	1	2x4	SPF 2100f-1.8E	29.98"	---
Joint	x	y	Plate	Cq	JSI	Method	deflY(L)	deflY(T)	deflX(L)	deflX(T)
A	-1.74'	16.00'					-0.20" L/103	-0.32" L/65	-0.05"	0.07"
B	2.17'	16.98'					0.25" L/999	0.40" L/999	0.06"	0.08"
B	2.26'	16.30'					0.26" L/999	0.41" L/999	0.00"	0.01"
C	11.12'	19.22'	2.5X6	0.80	0.74	N	1.23" L/696	1.96" L/439	0.25"	0.32"
D	16.69'	20.61'	8X8	0.80	1.00	N				
E	19.38'	21.28'	5X6	0.80	0.90	N	1.56" L/551	2.48" L/347	0.28"	0.36"

*****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. **A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.**

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org



F	27.64'	23.35'	6X8	0.80	0.85	N	1.68"	L/511	2.67"	L/322	0.27"	0.34"
G	36.00'	25.44'	10X14	0.80	0.93	N	1.67"	L/514	2.65"	L/324	0.23"	0.29"
H	44.36'	23.35'	6X8	0.80	0.84	N	1.68"	L/511	2.67"	L/322	0.19"	0.24"
I	49.49'	22.06'	8X8	0.80	1.00	N						
J	52.62'	21.28'	5X6	0.80	0.89	N	1.56"	L/551	2.47"	L/347	0.17"	0.22"
K	59.43'	19.58'	8X8	0.80	1.00	N						
L	60.88'	19.22'	2.5X6	0.80	0.73	N	1.23"	L/697	1.96"	L/439	0.20"	0.26"
M	69.83'	16.98'					0.25"	L/999	0.40"	L/999	0.39"	0.50"
M	69.74'	16.30'					0.25"	L/999	0.40"	L/999	0.44"	0.57"
N	74.24'	15.88'					-0.25"	L/106	-0.40"	L/67	0.06"	0.08"
O	59.61'	16.30'	4X6	0.80	0.82	N	1.30"	L/659	2.07"	L/415	0.37"	0.47"
P	54.00'	16.30'	SS0719	0.80	0.97	N						
Q	50.16'	16.30'	6X6	0.80	0.96	N	1.61"	L/533	2.56"	L/336	0.30"	0.39"
R	40.72'	16.30'	6X8	0.80	0.91	N	1.69"	L/508	2.68"	L/320	0.25"	0.32"
S	36.00'	16.30'	SS0612	0.80	0.69	N	1.69"	L/508	2.68"	L/320	0.25"	0.32"
T	31.28'	16.30'	6X8	0.80	0.92	N	1.69"	L/508	2.68"	L/320	0.20"	0.26"
U	21.84'	16.30'	6X6	0.80	0.96	N	1.61"	L/533	2.56"	L/335	0.15"	0.19"
V	18.00'	16.30'	SS0719	0.80	0.97	N						
W	12.39'	16.30'	4X6	0.80	0.83	N	1.30"	L/659	2.07"	L/415	0.08"	0.11"
X	0.00'	16.44'										
X	0.15'	16.35'	4X14(C1)/8X14(C0)	0.80	0.96		0.00"	L/999	0.00"	L/999	0.00"	0.00"
X	0.15'	16.40'					0.00"	L/999	0.00"	L/999	0.00"	0.00"
Y	71.85'	16.40'					0.00"	L/999	0.00"	L/999	0.45"	0.58"
Y	71.85'	16.35'	4X14(C1)/8X14(C0)	0.80	0.96		0.00"	L/999	0.00"	L/999	0.45"	0.58"
Y	72.00'	16.44'										

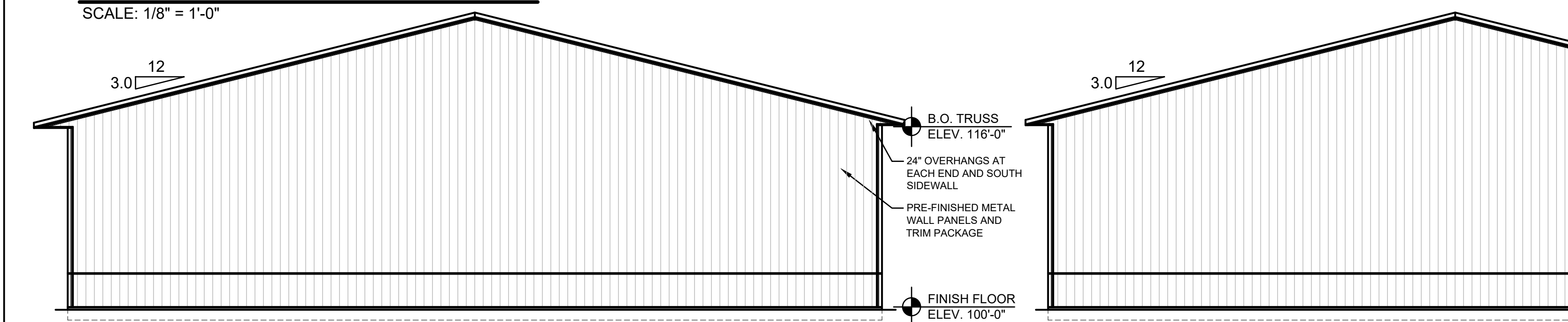
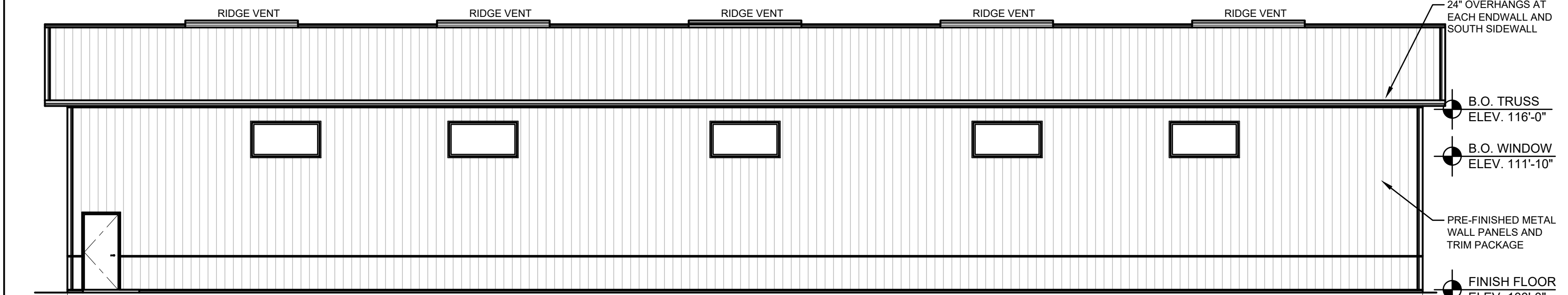
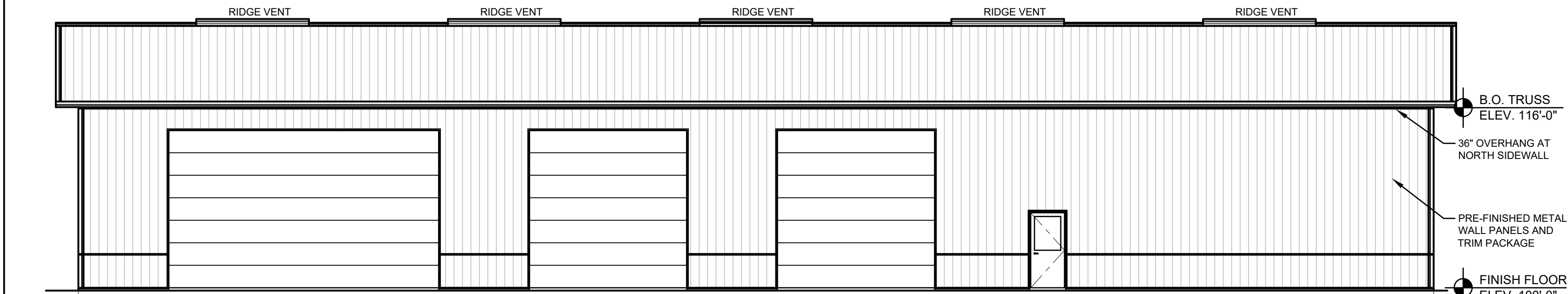
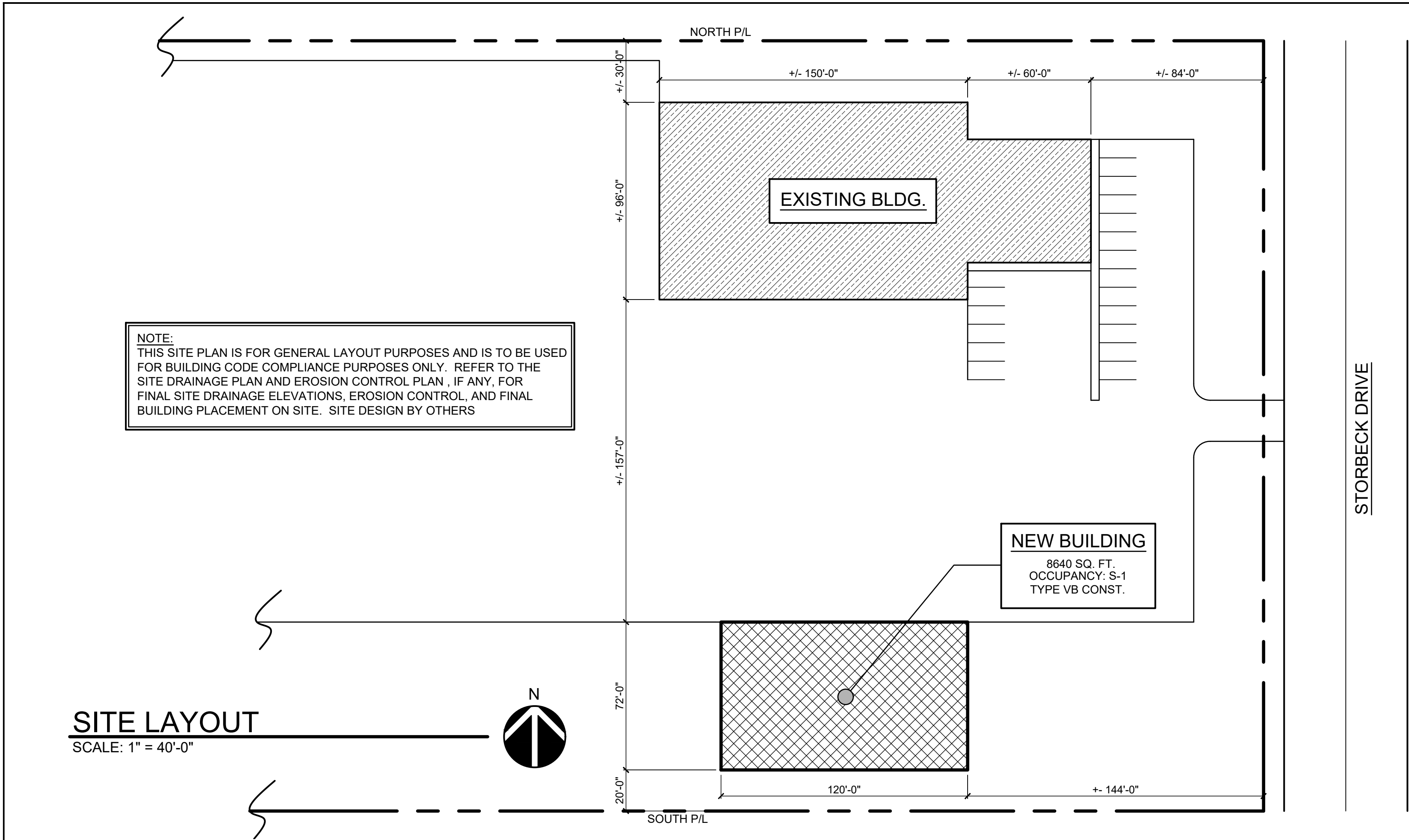
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

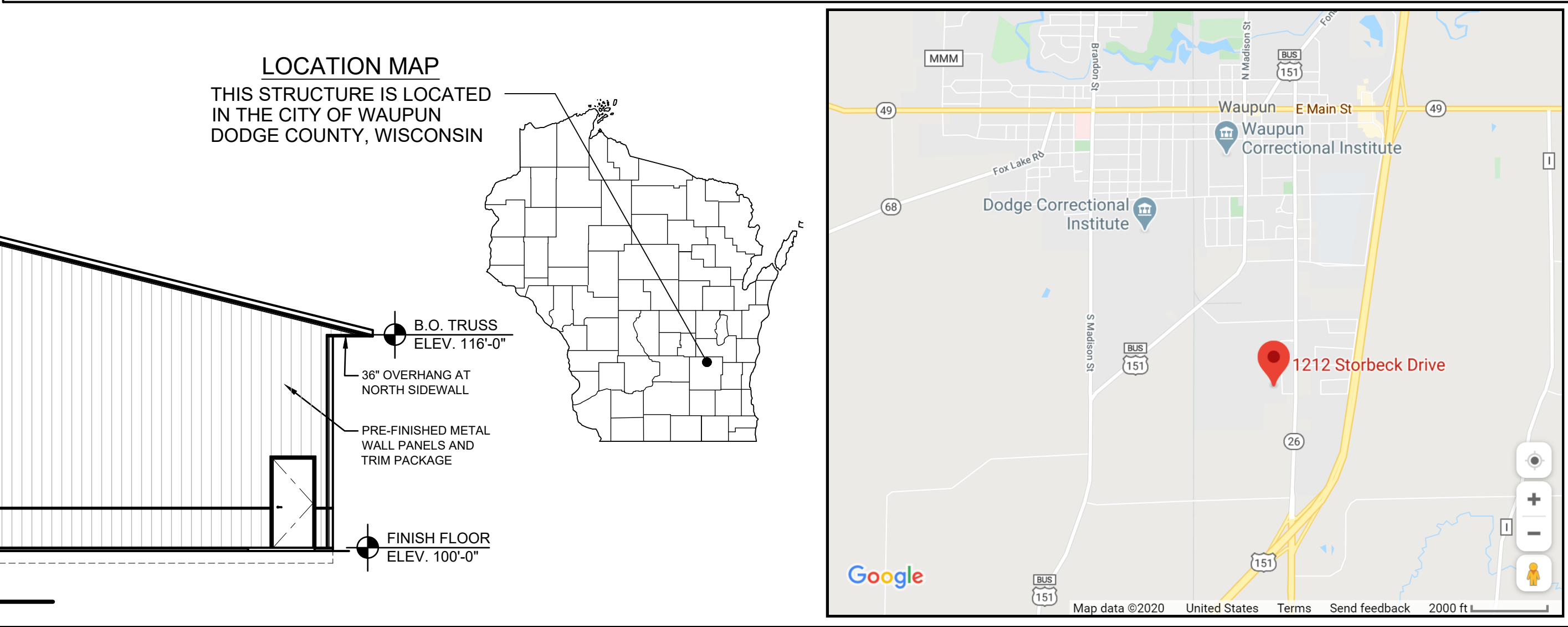
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

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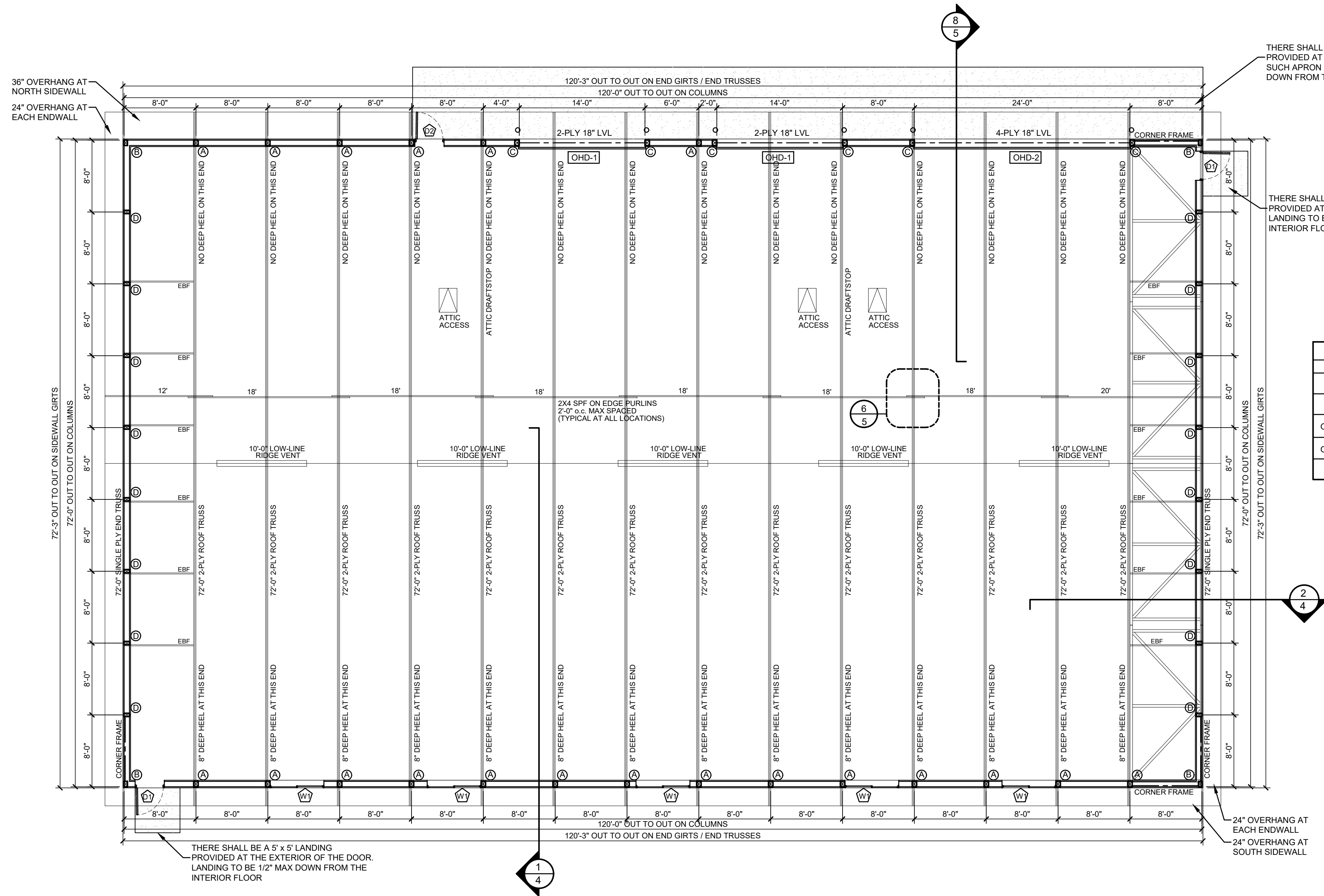
DESIGN LOADS		
Design Focus	Project-specific comments	Project Values
General Information: Risk Category = Normal Occupied Building Concrete Compressive Strength (f'_c) = 28 day minimum strength		
II		
3500 psi		
\$1603.1.2 Roof Live Load (L_L): $TC_{Live} = L_R$ Non-occupiable pitched roof		
20.0 psf		
Roof Dead Loads: TC_{Dead} Top Chord Dead Load BC_{Dead} Bottom Chord Dead Load		
4.0 psf		
5.0 psf		
\$1603.1.3 Roof Snow Load Data: Ground Snow Load (P_g) = Meets or exceeds ASCE 7 value Snow Exposure Factor (C_e) = Snow Load Importance Factor (I_s) = Based on Risk Category above Thermal Factor (C_t) = Flat Roof Snow Load (P_f) = $0.7 \times C_e \times I_s \times C_t \times P_g$ Sloped Roof Factor (C_d) = Slippery assumption may be invalid Balanced Roof Snow Load (P_s) = $S_g = C_d \times P_f$ (or greater) Unbalanced Roof Snow Load = (Windward / Leeward)		
30.0 psf		
1.0		
1.0		
1.1		
30.0 psf		
1.00		
30.0 psf		
Not Required		
See Detailed Snow Load Sheet for more details		
\$1603.1.4 Wind Design Data: Basic Wind Speed = ASCE 7-10 differs from earlier versions Wind Exposure = Presumed to apply in All Directions Building Enclosure Type = Internal Pressure Coefficient = Design Pressure for Components & Cladding = See chart for various Zones (for portions of structure not specified by Design Professional)		
115 mph		
C		
Enclosed		
± 0.18		
$+31.1, -33.7$ psf		
\$1603.1.5 Earthquake Design Data: Seismic Importance Factor I_e = Based on Risk Category above Mapped spectral response parameters: S_{ds} = S_{d1} = Site Class = Presumed in absence of soils test Design Spectral Coefficients: $S_{ds} = (2/3) \times F_a \times S_s$ $S_{d1} = (2/3) \times F_a \times S_1$ Seismic Design Category = Basic Seismic Force-Resisting System = Seismic Response Coefficient = Response Modification Coefficient = (6.5 might be OK for wood frame) Effective Seismic Weight = Dead Load x Area Design Base Shear = $V = C_d \times W$ Analysis Procedure Used =		
1.0		
10.4%		
4.1%		
D		
11.1%		
6.6%		
A		
A15 - Light-frame walls with shear panels (other)		
0.055		
2		
77,760 lbs		
4,313 lbs		
Equivalent Lateral Force Procedure		
\$1603.1.6 Geotechnical Information: Class of Soil Materials = Presumed in absence of soils test Allowable Soil Pressure = As Defined in EP486.1 Shallow Post Foundation Design (ANS/ASAE EP486.1 OCT00), adopted in IBC 2015 §2306.1		
4		
2000 psf		
\$1603.1.7 Flood Hazard Information: Not Considered		
\$1603.1.8 Special Loads: Not Applicable		
\$1603.1.9 Special Seismic Inspections: None Required		

GENERAL REQUIREMENTS	
NOTES & DETAILS ON THE DRAWINGS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES. ALL MATERIALS AND WORK PERFORMED SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE WISCONSIN ADMINISTRATIVE CODE INCLUDING LOCAL ORDINANCES AND AMENDMENTS. ALL MATERIAL SHALL BE FURNISHED AS SHOWN HEREIN UNLESS THE OWNER OR ENGINEER APPROVES EQUAL ALTERNATIVES. NO CHANGES ARE TO BE MADE TO THESE PLANS WITHOUT THE KNOWLEDGE AND WRITTEN CONSENT OF THE ENGINEER. THE CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO BRACING AND SHORING. OBSERVATION VISITS TO THE SITE BY THE ENGINEER AND/OR THE ENGINEER'S REPRESENTATIVE(S) SHALL NOT INCLUDE INSPECTION OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. EVERY EFFORT HAS BEEN MADE IN PREPARING THESE PLANS AND CHECKING THEM FOR ACCURACY. THE CONTRACTOR MUST CHECK ALL STRUCTURAL DETAILS AND DIMENSIONS, AND BE RESPONSIBLE FOR THE SAME. THIS DRAWING AND ITS COPIES ARE THE COPYRIGHT OF THE DESIGN PROFESSIONAL, AND MAY NOT BE USED FOR PROJECTS OTHER THAN THE DESIGNATED PROJECT WITHOUT THE SPECIFIC WRITTEN CONSENT OF HALBERG ENGINEERING, LLC.	
PROVIDE FIRE EXTINGUISHERS PER IFC 906. FIRE EXTINGUISHERS SHALL BE SELECTED, INSTALLED AND MAINTAINED IN ACCORDANCE WITH IFC 906 AND NFPA 10. HANDLES, PULLS, LATCHES, LOCKS, AND OTHER OPERABLE PARTS ON ACCESSIBLE DOORS SHALL HAVE A SHAPE THAT IS EASY TO GRASP WITH ONE HAND AND DOES NOT REQUIRE TIGHT GRASPING, PINCHING OR TWISTING OF THE WRIST. THE HARDWARE SHALL BE PLACED AT LEAST 34 INCHES, BUT NOT MORE THAN 48 INCHES ABOVE THE FLOOR SURFACE. DOORS THERE SHALL BE A FLOOR OR LANDING PROVIDED ON EACH SIDE OF DOOR SUCH FLOOR OR LANDING TO BE AT THE SAME ELEVATION ON EACH SIDE OF DOOR GENERAL TRUSS INSTALLATION NOTES: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING, AND BRACING. REFER TO, AND FOLLOW, THE LATEST EDITION OF BCSI (BUILDING COMPONENT SAFETY INFORMATION, BY "TPI" AND "SBCA") FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. INSTALLERS SHALL PROVIDE TEMPORARY BRACING PER BCSI, UNLESS NOTED OTHERWISE. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL SHEATHING AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. LOCATIONS SHOWN FOR PERMANENT LATERAL RESTRAINT OF WEBS SHALL HAVE BRACING INSTALLED PER BCSI SECTIONS B3, B7, OR B10, AS APPLICABLE	
SEE CORRESPONDENCE	
Trans 3307204 New Building July 10, 2020	



NEW BUILDING FOR: MUNICIPAL WELL & PUMP WAUPUN, WISCONSIN	
DESIGN PROFESSIONAL:	
HALBERG ENGINEERING LLC 10335 N DUFFY ROAD HAYWARD, WI 54843 EMAIL: aaron@halbergengineering.com	
Engineering Scope: Building Design for IBC 2015 / SPS 361/362 Compliance AARON J. HALBERG 34982 HAYWARD, WIS. PROFESSIONAL ENGINEER July 07, 2020 ENGINEER'S SEAL	
GENERAL BUILDING SPECIFICATIONS ALL WORK GOVERNED BY STATE AND LOCAL CODES, ORDINANCES, AND REGULATIONS WHEREVER THEY MAY APPLY. DESIGNERS LIABILITY TO THE PREPARATION OF THE DRAWINGS, WITH THE PARAMETERS CONTRACTED, AND ASCERTAINING CODE COMPLIANCE. BUILDING ADDRESS: 1212 STORBECK DRIVE CITY OF WAUPUN DODGE COUNTY, WISCONSIN "S-1" MODERATE HAZARD STORAGE, HEATED, NO PLUMBING, BUT ALSO UNATTENDED STORAGE	
SHEET INDEX	
SHEET NO.	DATE
SHEET 1 OF 5	6/1/20
SHEET 2 OF 5	6/1/20
SHEET 3 OF 5	7/7/20
SHEET 4 OF 5	7/7/20
SHEET 5 OF 5	7/7/20
CONTENTS	
- GENERAL SPECS, SITE LAYOUT, ELEVATIONS	
- FLOOR PLAN, DETAILS	
- CONCRETE PLAN	
- BUILDING SECTIONS, DETAILS	
- BUILDING SECTIONS, DETAILS	
Conditionally APPROVED DEPT. OF SAFETY AND PROFESSIONAL SERVICES DIVISION OF INDUSTRY SERVICES	

NOTE: ALTHOUGH EVERY EFFORT HAS BEEN MADE TO PREPARE THESE PLANS WITH THE MOST ACCURACY, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE VERIFICATION OF ALL DIMENSIONS AND REQUIREMENTS FOR CONSTRUCTION TO ALL APPLICABLE STATE AND LOCAL AND OTHER REQUIREMENTS.	
M.P.B. BUILDERS, INC. 654 EAST OSHKOSH STREET RIPON, WISCONSIN 54971 (920) 748 - 2601 (800) 762 - 9632 SINCE 1961	
PROJECT: NEW 2' X 120' HEATED STORAGE	
OWNER: MUNICIPAL WELL AND PUMP WAUPUN, WI	
APPROVED PLANS	BY: BUILDING OWNER DATE: PROJECT MANAGER DATE: CEO/OWNER DATE: PRESIDENT DATE:
REVISIONS	DATE DESCRIPTION BY DATE
6/1/20 PRELIMINARY DRAWINGS BVL	
6/1/20 ISSUED FOR STATE PLAN APPROVAL A/H	
7/7/20 REVISED GRADE BEAMS A/H	
DRAWN BY: BVL	
DATE DRAWN: JUNE 1, 2020	
PROJECT MANAGER: DOYLE POKORNY	
JOB NUMBER: 20A37	
1 SHEET OF 5	



FLOOR PLAN
SCALE: 1/8" = 1'-0"



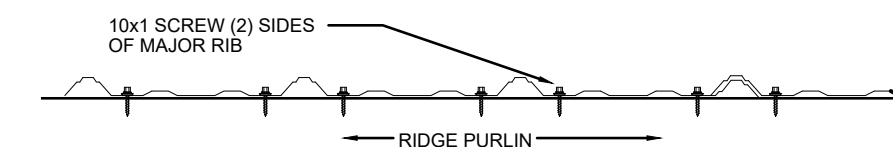
THERE SHALL BE A 8' x 8' APRON
PROVIDED AT THE FRONT OF THE BLDG.
SUCH APRON TO BE 1/2" (MAX)
DOWN FROM THE INTERIOR FLOOR

THERE SHALL BE A 5' x 5' LANDING
PROVIDED AT THE EXTERIOR OF THE DOOR.
LANDING TO BE 1/2" MAX DOWN FROM THE
INTERIOR FLOOR

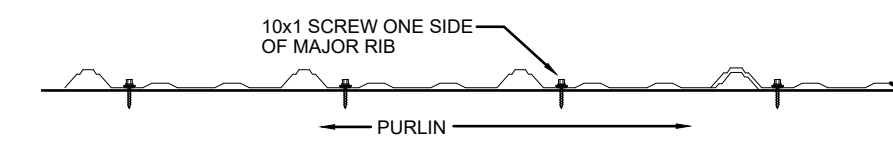
WINDOW AND DOOR SCHEDULE						
I.D.	QTY	SERIES	"U" VALUE	SIZE	ROUGH OPENING	ADD'L NOTES
D-1	2	AJ 7100	0.24	3'-0" x 6'-8"	40-5/16" x 81-1/8"	-
D-2	1	AJ7100	0.24	3'-0" x 6'-8"	40-5/16" x 81-1/8"	- 22" x 36" GLASS LITE
OHD-1	2	CHI 3216	0.19	14'-2" x 14'-0"	14'-0" x 14'-0"	R-17.54, DOUBLE SIDED STEEL
OHD-2	1	CHI 3216	0.19	24'-2" x 14'-0"	24'-0" x 14'-0"	R-17.54, DOUBLE SIDED STEEL
W-1	5	AJ HARMONY	0.29	6'-0" x 3'-0"	6'-0" x 3'-0"	VINYL FIXED PANE

COLUMN SCHEDULE		
ID	COLUMN	SEE DETAIL
A	4-PLY 2X8 SYP#2 NAIL LAMINATED COLUMN SET IN PRE-MANUFACTURED EMBEDDED STEEL H-BRACKET	1/5
B	3-PLY 2X8 SYP#2 NAIL LAMINATED COLUMN SET WITH (1) STURDIWALL SW80 UNIVERSAL BRACKET AT THE SIDEWALL AND (1) SW6C UNIVERSAL BRACKET AT THE ENDWALL	4/5
C	3-PLY 2X8 SYP#2 NAIL LAMINATED COLUMN WITH (1) FIELD APPLIED 2X8 SYP TO COVER THE "TUCK UNDER" SIDE OF THE BRACKET. BRACKETS: USE (2) STURDIWALL SW80 UNIVERSAL BRACKETS (ONE TUCKED UNDER COLUMN)	3/5
D	3-PLY 2X8 SYP#2 NAIL LAMINATED COLUMN SET IN PRE-MANUFACTURED EMBEDDED STEEL H-BRACKET	2/5

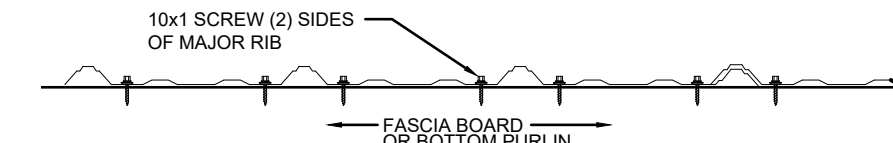
EBF - PRE-MANUFACTURED PLATED END BRACE FRAME



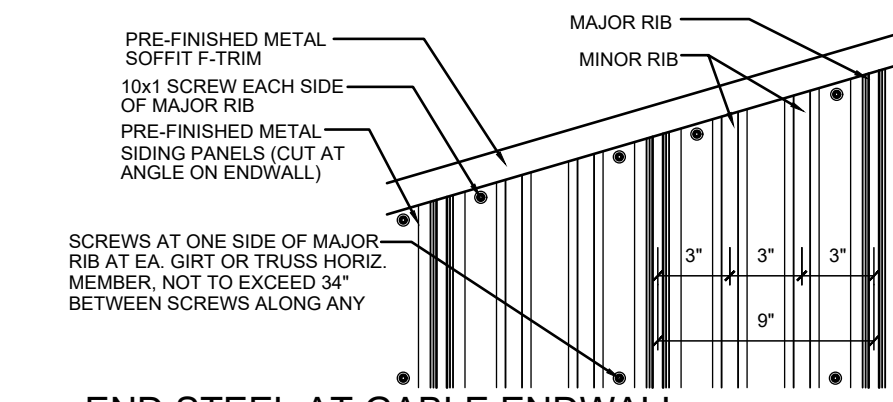
ROOF STEEL AT TOP OF EACH PANEL



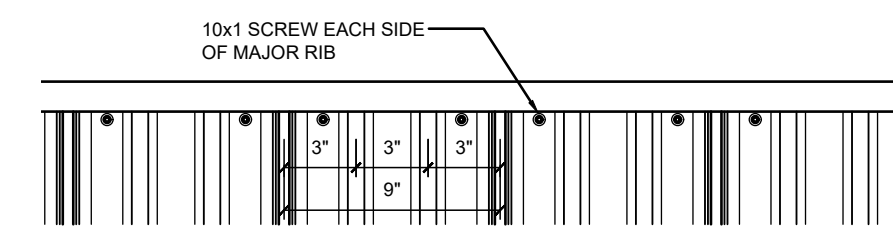
ROOF STEEL TO EACH PURLIN



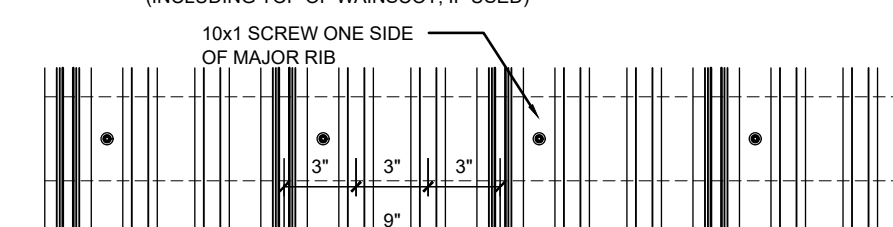
ROOF STEEL AT BOTTOM OF EA. PANEL



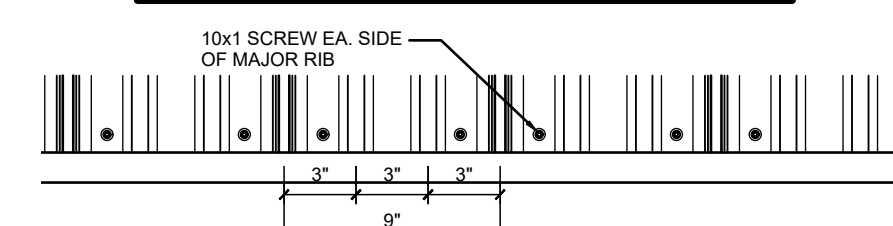
END STEEL AT GABLE ENDWALL



WALL STEEL AT TOP OF PANEL
(INCLUDING TOP OF WAINSCOT, IF USED)

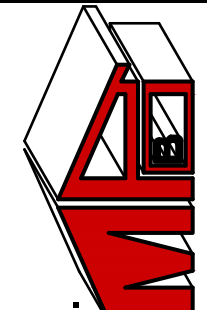


WALL STEEL INTERMEDIATE GIRTS



WALL STEEL AT BOTTOM OF PANEL
(INCLUDING BOTTOM OF WAINSCOT TRANSITION, IF USED)

NOTE:
ALTHOUGH EVERY EFFORT HAS BEEN MADE
TO PREPARE THIS DRAWING IN ACCORDANCE
WITH THE REQUIREMENTS OF THE CONTRACT,
THE CONTRACTOR SHALL BE RESPONSIBLE FOR
VERIFYING THE ACCURACY OF ALL INFORMATION
AND BE RESPONSIBLE FOR THE
CONSTRUCTION TO ALL APPLICABLE STATE AND
LOCAL AND OTHER CODE REQUIREMENTS.



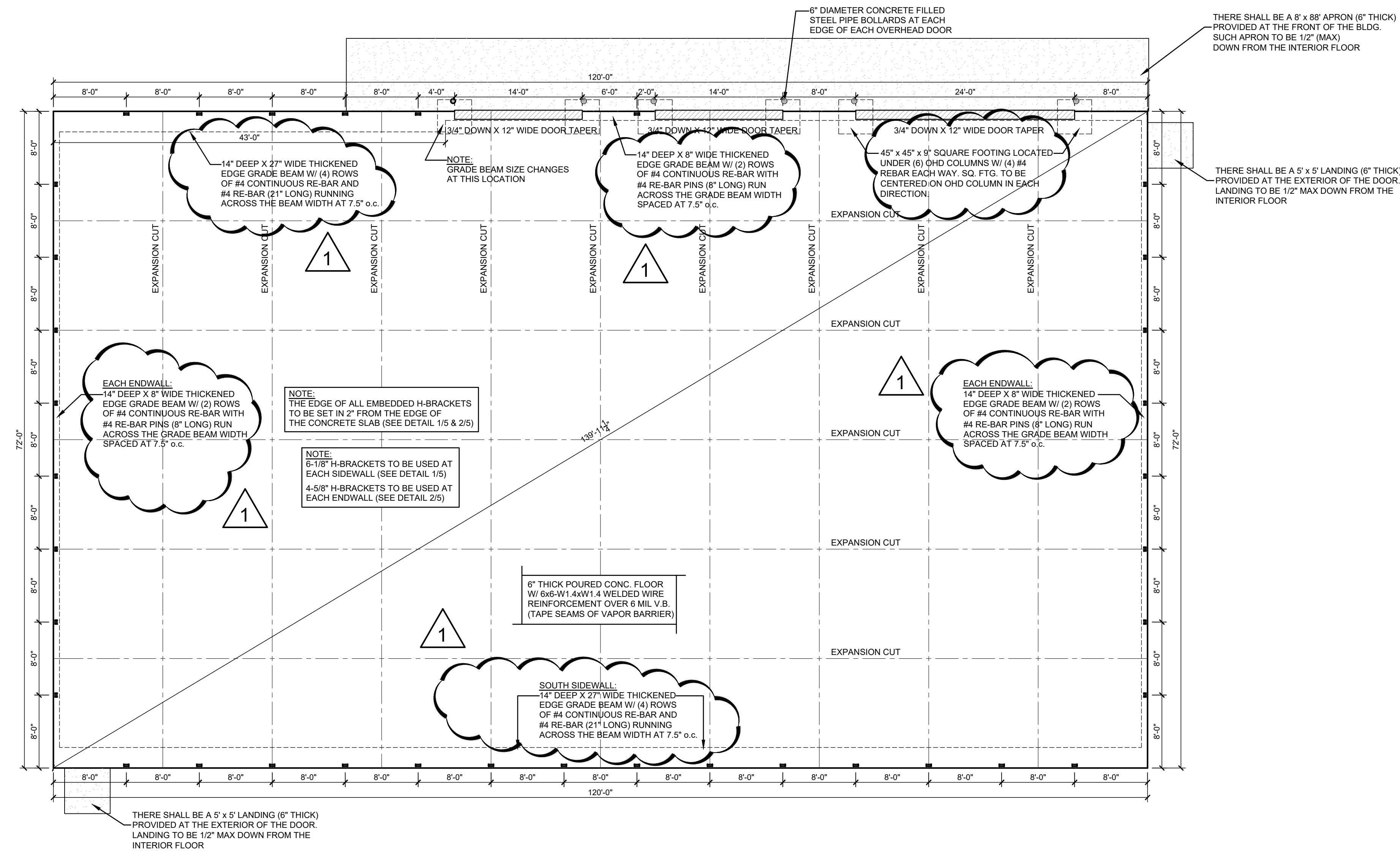
M.P.B. BUILDERS, INC.
654 EAST OSHKOSH STREET
RIPON, WISCONSIN 54971
(920) 748 - 2601 (800) 782 - 9632
SINCE 1961

PROJECT: NEW 72' X 120'
HEATED STORAGE
OWNER: MUNICIPAL WELL AND PUMP
WAUPUN, WI

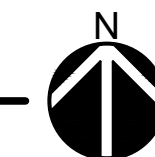
APPROVED PLANS		REVISIONS	
BY	BUILDING OWNER	DATE	DATE
BVL		6/1/20	
DATE	DATE	DATE	DATE
6/1/20	PROJECT MANAGER	6/1/20	PROJECT MANAGER
7/7/20	LEO SCUDER, PRESIDENT	7/7/20	LEO SCUDER, PRESIDENT

DRAWN BY: BVL
DATE DRAWN: JUNE 1, 2020
PROJECT MANAGER: DOYLE FOKORNY
JOB NUMBER: MPB H.E.
... 20A37

3
SHEET OF 5



CONCRETE PLAN
SCALE: 1/8" = 1'-0"



ALTHOUGH EVERY EFFORT HAS BEEN MADE IN PREPARING THESE PLANS AND CHECKING THEM FOR ACCURACY, THE CONTRACTOR OR SUBCONTRACTOR MUST CHECK ALL DIMENSIONS AND DETAILS FOR ACCURACY AND BE RESPONSIBLE FOR THE SAME. ALL SUBCONTRACTORS SHALL COMPLETE CONSTRUCTION TO ALL APPLICABLE STATE AND FEDERAL AND OTHER CODE REQUIREMENTS.



654 EAST OSHKOSH STREET
RIPON, WISCONSIN 54971

PROJECT.

APPROVED PLANS

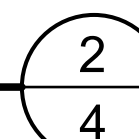
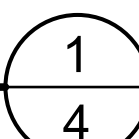
REVISIONS

DATE DRAWN:
JUNE 1, 2020

DOYLE POKORNY
JOB NUMBER:

...	20A37
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4.



ALTHOUGH EVERY EFFORT HAS BEEN MADE IN PREPARING THESE PLANS AND CHECKING THEM FOR ACCURACY, THE CONTRACTOR AND SUBCONTRACTOR MUST CHECK ALL DIMENSIONS AND DETAILS FOR ACCURACY AND BE RESPONSIBLE FOR THE SAME. ALL SUBCONTRACTORS SHALL COMPLETE CONSTRUCTION TO ALL APPLICABLE STATE AND LOCAL AND OTHER CODE REQUIREMENTS.



SEATED STORAGE APAL WELL AND PUMP WALUPIN WI

APPROVED PLANS

DRAWN BY: BVL	
DATE DRAWN: JUNE 1, 2020	
PROJECT MANAGER: DOYLE POKORN	
JOB NUMBER:	
MPB	H.E.
...	20A3

5
EET OF 5

