PLANNING AND ZONING BOARD MEETING CITY OF LAKE CITY

June 11, 2024 at 5:30 PM Venue: City Hall

AGENDA

The meeting will be held in the City Council Chambers on the second floor of City Hall located at 205 North Marion Avenue, Lake City, FL 32055. Members of the public may also view the meeting on our YouTube channel. YouTube channel information is located at the end of this agenda.

INVOCATION

ROLL CALL

MINUTES

- i. Meeting Minutes 04-09-2024
- ii. Meeting Minutes 05-14-2024

OLD BUSINESS

- **SPR24-05**, Petition submitted by Randall Olney, P.E.. (agent) for Concept Companies (owner), for a Site Plan Review for Dollar General, in the Commercial Intensive Zoning District, and located on parcel 08127-005, which is regulated by the Land Development Regulations section 4.13.
 - ***Note, meeting on 05-14-2024 was cancelled due to lack of a quorum.***
- iv. ***POSTPONED***SPR23-10, Petition submitted by Carol Chadwick (agent) for Affiliated Property Management (owner), for a Site Plan Review for Aspire Dental Addition, in the Commercial Intensive Zoning District, and located on Parcel 07604-102, which is regulated by the Land Development Regulations section 4.13.

NEW BUSINESS

<u>v.</u> SPR24-06, Petition submitted by Christopher A Gmuer, P.E.. (agent) for ERA Investments (owner), for a Site Plan Review for Lake City Hotels Phase 2, in

the Commercial Intensive Zoning District, and located on parcel 02582-002, which is regulated by the Land Development Regulations section 4.13.

WORKSHOP- None

ADJOURNMENT

YouTube Channel Information

Members of the public may also view the meeting on our YouTube channel at: https://youtube.com/c/CityofLakeCity

Pursuant to 286.0105, Florida Statutes, the City hereby advises the public if a person decides to appeal any decision made by the City Council with respect to any matter considered at its meeting or hearings, he or she will need a record of the proceedings, and that, for such purpose, he or she may need to ensure that a verbatim record of the proceedings is made, which record includes the testimony and evidence upon which the appeal is to be based.

Pursuant to 286.26, Florida Statutes, persons needing special accommodations to participate in this meeting should contact the City Manager's Office at (386) 719-5768.

File Attachments for Item:

i. Meeting Minutes 04-09-2024

MEETING MINUTES

DATE: 04/09/2024

ROLL CALL:

Mrs. McKellum- Present Mr. McMahon- Present Mr. Nelson- Present

Mr. Lydick- Present City Attorney- Clay Martin- Present

MINUTES: March 5, 2024 Planning and Zoning Meeting.

Comments or Revisions: None

Motion to approve 03/05/2024 Meeting Minutes by Mr. Nelson and seconded by Mrs. McKellum.

Ex Parte Communications

Mr. Martin polled the Board if they had any ex parte communications for petitions SPR 24-04, CPA 24-01, and Z 24-01

Mrs. McKellum- No, Mr. McMahon- No, Mr. Nelson- No, and Mr. Lydick- Only the regular exercise of his duties on briefing of the agenda. Mr. Martin asked if it would those conversations affect your ability to render a fair decision.

OLD BUSINESS: None

Petition # LDR 24-03 Presented By: Dave Young, CBO

As owner or agent and gives address of:

Petitioner is Sworn in by: Clay Martin, City Attorney

Motion to un-table petition LDR 24-03 by; Mr. McMahon and seconded by Mr. Nelson. Approved by hand vote unanimously.

Discussion:

Mr. Young introduced text amendment. He stated that this text amendment is bringing up to date the parking requirements to other communities our size. Mr. Lydick asked if there were any major changes other than, adding section 4.2.15.17. Mr. Young stated no.

Mr. Martin asked about the strike thru's and the addition of where is states see section 4.2.15 is different then the ordinance that is prepared for council. Mr. Martin asked which one do we want to be recommended by the board to go to Council? Mr. Lydick and Mr. Martin discussed briefly. Robert stated that we could go with how the ordinance is prepared. Mr. Martin stated that the would change the verbiage to subsection instead of paragraph and leave the numbering as is.

Public Comment:

Carol Chadwick stated that per the Boards request, she believes that the City did a great job. Mr. Lydick asked if she seen anything that look like it may be a problem. She stated that until you start applying it you will not know.

Motion to close public comment by: Mr. Nelson Seconded by: Mr. McKellum

MEETING MINUTES

Board Discussion:

Mr. Lydick asked if any of the other departments weighed in on the amendment. Robert stated that the other departments were aware of them but they did not weigh in. Robert stated that they City did send the text amendment out to over 700 businesses in the City. He stated that we only go a handful of comments back, all in support of it. Mr. McMahon asked if this would go on to the City Council once approved.

Motion to approve petition LDR 24-03, with the amendments suggested by council by: Mr. McMahon Motion Seconded By: Mr. Nelson

Mrs. McKellum: Aye Mr. Nelson: Aye Mr. McMahon: Aye

Mr. Lydick: Aye

NEW BUSINESS:

Petition # SPR24-03 Presented By: Brandon Stubbs

As owner or agent and gives address of: 1450 SW SR 47, Lake City, FL

Petitioner is Sworn in by: Mr. Lydick Staff is Sworn in by: Mr. Martin

Discussion:

Mr. Young introduced petition SPR 24-04. He stated that proposed use of the land is for multifamily and is conducive for use per the Land Development Regulations 4.9.2.3. He stated that after review of the site plan that it is consistent with the Land Development Regulations.

Mr. Kurtz the land is currently vacant. He stated that they plan to put one to three bed room town homes. He stated SRWMD has reviewed the project along with the City and they have no concerns. He stated that FDOT said they need a drainage permit. He stated that they will work on that with FDOT.

Mr. McMahon asked about how many units. Mr. Kurtz stated that they want to put in 192 units. Mr. Lydick asked about the size of the large retention pond. Mr. Kurtz stated that due to the slope of the land they had to do two ponds stair stepped to accommodate for the amount of water.

Mr. Martin asked Mr. Young if he was going to move the staff records into evidence. Mr. Young stated yes.

Exhibits introduced: None

Public Comment:

Loretta Nicholas asked about how this is going to affect them as far as traffic, sewer, and water. Mr. Kurtz stated that as far as water and sewer, this will not impact the citizens. He stated as far as traffic they will use Hall of Fame and will not enter Aster Way.

Motion to close public comment by: Mr. Nelson Seconded by: Mrs. McKellum

MEETING MINUTES

Board Discussion:

No comments.

Motion to approve SPR24-04 as submitted by: Mr. McMahon Motion Seconded By: Mr. Nelson

Mrs. McKellum: Aye Mr. Nelson: Aye Mr. McMahon: Aye

Mr. Lydick: Aye

Petition # CPA 24-01 Presented By: Carol Chadwick

As owner or agent and gives address of: 1208 SW Fairfax Glen

Petitioner is Sworn in by: Mr. Lydick **Staff is Sworn in by:** Mr. Martin

Discussion:

Mr. Young introduced petition CPA 24-01. He stated that the City staff has determined the petition is consistent with the Land Development Regulations. He stated that he is introducing the staff records into the record.

Carol stated that they are planning to change the Future Land Use and Zoning to allow for a second phase of Sugarmill Apartments. She stated that the site will be accessed from the existing site. She stated they are planning on 46 dwelling units. Mr. Lydick asked if the property ever had a City zoning. She stated that it has not. Mr. Martin asked if she was going to introduce her application into the record.

Public Comment:

David Kraft stated that he owns the property next to it. He stated that there is water all in his yard and would like them to address this in the future review. Mr. Lydick asked Robert if this was going to be in front of the board. Robert stated yes.

Exhibits introduced: None

Motion to close public comment by: Mrs. McKellum Seconded by: Mr. Nelson

Board Discussion: None

Motion to approve CPA24-01 as submitted by: Mr. McMahon Motion Seconded By: Mr. Nelson

Mrs. McKellum: Aye Mr. Nelson: Aye Mr. McMahon: Aye

Mr. Lydick: Aye

MEETING MINUTES

Petition # Z 24-01	Presented By	v: Carol Chadwick
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As owner or agent and gives address of: 1208 SW Fairfax Glen

Petitioner is Sworn in by: Mr. Lydick **Staff is Sworn in by:** Mr. Martin

Discussion:

Mr. Young introduced petition Z 24-01. He stated that the City staff has determined the petition is consistent with the Land Development Regulations. He stated that he is introducing the staff records into the record.

Carol stated that project is the rezoning for the previous project. She stated that she is introducing her application into the record.

Public Comment: None

Exhibits introduced: None

Motion to close public comment by: Mr. Nelson Seconded by: Mr. Nelson

Board Discussion: None

Motion to approve Z24-01 as submitted by: Mr. Nelson Motion Seconded By: Mr. McMahon

Mrs. McKellum: Aye Mr. Nelson: Aye Mr. McMahon: Aye

Mr. Lydick: Aye

WORKSHOP: None

ADJOURNMENT

Mr. Lydick closed the meeting.

Motion to Adjourn by: Mr. McMahon

Time: 6:11 pm

Motion Seconded By: Mr. Nelson

Mr. Lydick, Board Chairperson	Date Approved
Robert Angelo, Secretary	 Date Approved

PLANNING AND ZONING MEETING MINUTES

File Attachments for Item:

ii. Meeting Minutes 05-14-2024

MEETING MINUTES

DATE: 05/14/2024

ROLL CALL:

Mrs. McKellum- Present Mr. McMahon- Present Mr. Nelson- Not Present

Mr. Lydick- Present City Attorney- Clay Martin- Present

MINUTES:

Comments or Revisions: No Quorum.

Ex Parte Communications

No Quorum.

OLD BUSINESS: None

NEW BUSINESS:

Petition # SPR24-05 Presented By: As owner or agent and gives address of: Petitioner is Sworn in by:

Staff is Sworn in by:

Discussion:

No Quorum.

Petition # SPR24-05 Presented By: As owner or agent and gives address of: Petitioner is Sworn in by: Staff is Sworn in by:

WORKSHOP: None

ADJOURNMENT

Mr. Lydick closed the meeting.

Motion to Adjourn by: Mrs. McKellum

Time: 5:36 pm

Motion Seconded By: Mr. McMahon

MEETING MINUTES

Mr. Lydick, Board Chairperson	Date Approved
Robert Angelo, Secretary	 Date Approved

File Attachments for Item:

iii. SPR24-05, Petition submitted by Randall Olney, P.E.. (agent) for Concept Companies (owner), for a Site Plan Review for Dollar General, in the Commercial Intensive Zoning District, and located on parcel 08127-005, which is regulated by the Land Development Regulations section 4.13.

Note, meeting on 05-14-2024 was cancelled due to lack of a quorum.



GROWTH MANAGEMENT

205 North Marion Ave. Lake City, FL 32055 Telephone: (386)719-5750

E-Mail:

growthmanagement@lcfla.com

FOR PLANNING USE ONLY	
Application # <u> </u>	
Application Fee: \$200.00	
Receipt No. 2024-000421	47
Filing Date $3/2i/24$	
Completeness Date	

Site Plan Application

A.	PRO	JECT INFORMATION
	1.	Project Name: Commercial Retail Store - Marvin Burnett
	2.	Address of Subject Property: Northwest of the intersection of SR 47 and SW Marvin Burnett Road, Lake City, Florida 32025
	3.	Parcel ID Number(s): 07-48-17-08127-005
	4.	Future Land Use Map Designation: Commences
	5.	Zoning Designation: Commercial, Intensive
	6.	Acreage: 2.70
	7.	Existing Use of Property: Vacant
	8.	Proposed use of Property: Commercial Retail Store
	9.	Type of Development (Check All That Apply):
		Increase of floor area to an existing structure: Total increase of square footage
		New construction: Total square footage 10,640
		Relocation of an existing structure: Total square footage
B.	APP	LICANT INFORMATION
	1.	Applicant Status □ Owner (title holder) ■ Agent
	2.	Name of Applicant(s): Randall Olney, P.E. Title: Director of Engineering
		Company name (if applicable): CHW
		Mailing Address: 11801 Research Drive
		City, Alachua State: Florida Zip: 32015
		Telephone:(352) 331-1976 Fax:()Email: ranoyo@cnw-inc.com
		PLEASE NOTE: Florida has a very broad public records law. Most written communications to
		or from government officials regarding government business is subject to public records
		requests. Your e-mail address and communications may be subject to public disclosure.
	3.	If the applicant is agent for the property owner*.
		Property Owner Name (title holder): Concept Companies
		Mailing Address: 1449 SW 74th Dr. Suite 200
		City: Gainesville State: Florida Zip: 32607 Telephone: (352) 333-3233 Fax: Email:
		Telephone: (352) 333-3233 Fax: Email: Email:
		PLEASE NOTE: Florida has a very broad public records law. Most written communications to
		or from government officials regarding government business is subject to public records
		requests. Your e-mail address and communications may be subject to public disclosure.
		*Must provide an executed Property Owner Affidavit Form authorizing the agent to act on
		behalf of the property owner.

C. ADDITIONAL INFORMATION

1.	Is there any additional contract for the sale of, or options to purchase, the subject property?
	If yes, list the names of all parties involved: St. Johns, LLC, Concept Development, Inc.
	If yes, is the contract/option contingent or absolute: ■ Contingent □Absolute
2.	Has a previous application been made on all or part of the subject property? □Yes ■No_
	Future Land Use Map Amendment: Yes No
	Future Land Use Map Amendment Application No.
	Site Specific Amendment to the Official Zoning Atlas (Rezoning): □YesNo
	Site Specific Amendment to the Official Zoning Atlas (Rezoning) Application No.
	Variance: Tes
	Variance Application No
	Special Exception:
	Special Exception Application No.

D. ATTACHMENT/SUBMITTAL REQUIREMENTS

- 1. Vicinity Map Indicating general location of the site, abutting streets, existing utilities, complete legal description of the property in question, and adjacent land use.
- 2. Site Plan Including, but not limited to the following:
 - a. Name, location, owner, and designer of the proposed development.
 - b. Present zoning for subject site.
 - c. Location of the site in relation to surrounding properties, including the means of ingress and egress to such properties and any screening or buffers on such properties.
 - d. Date, north arrow, and graphic scale not less than one inch equal to 50 feet.
 - e. Area and dimensions of site (Survey).
 - f. Location of all property lines, existing right-of-way approaches, sidewalks, curbs, and gutters.
 - g. Access to utilities and points of utility hook-up.
 - h. Location and dimensions of all existing and proposed parking areas and loading areas.
 - i. Location, size, and design of proposed landscaped areas (including existing trees and required landscaped buffer areas).
 - j. Location and size of any lakes, ponds, canals, or other waters and waterways.
 - k. Structures and major features fully dimensioned including setbacks, distances between structures, floor area, width of driveways, parking spaces, property or lot lines, and percent of property covered by structures.
 - l. Location of trash receptacles.
 - m. For multiple-family, hotel, motel, and mobile home park site plans:
 - i. Tabulation of gross acreage.
 - ii. Tabulation of density.
 - iii. Number of dwelling units proposed.
 - iv. Location and percent of total open space and recreation areas.
 - v. Percent of lot covered by buildings.

City of Lake City - Growth Management Department 205 North Marion Ave, Lake City, FL 32055 ◆ (386) 719-5750

- vi. Floor area of dwelling units.
- vii. Number of proposed parking spaces.
- viii. Street layout.
- ix. Layout of mobile home stands (for mobile home parks only).
- 3. Stormwater Management Plan—Including the following:
 - a. Existing contours at one foot intervals based on U.S. Coast and Geodetic Datum.
 - b. Proposed finished elevation of each building site and first floor level.
 - c. Existing and proposed stormwater management facilities with size and grades.
 - d. Proposed orderly disposal of surface water runoff.
 - e. Centerline elevations along adjacent streets.
 - f. Water management district surface water management permit.
- 4. Fire Department Access and Water Supply Plan: The Fire Department Access and Water Supply Plan must demonstrate compliance with Chapter 18 of the Florida Fire Prevention Code, be located on a separate signed and sealed plan sheet, and must be prepared by a professional fire engineer licensed in the State of Florida. The Fire Department Access and Water Supply Plan must contain fire flow calculations in accordance with the Guide for Determination of Required Fire Flow, latest edition, as published by the Insurance Service Office ("ISO") and/or Chapter 18, Section 18.4 of the Florida Fire Prevention Code, whichever is greater.
- Concurrency Impact Analysis: Concurrency Impact Analysis of impacts to public facilities. For commercial and industrial developments, an analysis of the impacts to Transportation, Potable Water, Sanitary Sewer, and Solid Waste impacts are required.
- Comprehensive Plan Consistency Analysis: An analysis of the application's consistency with
 the Comprehensive Plan (analysis must identify specific Goals, Objectives, and Policies of the
 Comprehensive Plan and detail how the application complies with said Goals, Objectives, and
 Policies).
- 7. Legal Description with Tax Parcel Number (In Word Format).
- 8. Proof of Ownership (i.e. deed).
- 9. Agent Authorization Form (signed and notarized).
- 10. Proof of Payment of Taxes (can be obtained online via the Columbia County Tax Collector's Office).
- 11. Fee. The application fee for a Site and Development Plan Application is \$200.00. No application shall be accepted or processed until the required application fee has been paid.

NOTICE TO APPLICANT

All eleven (11) attachments are required for a complete application. Once an application is submitted and paid for, a completeness review will be done to ensure all the requirements for a complete application have been met. If there are any deficiencies, the applicant will be notified in writing. If an application is deemed to be incomplete, it may cause a delay in the scheduling of the application before the Planning & Zoning Board.

A total of ten (10) copies of proposed site plan application and all support materials must be submitted along with a PDF copy on a CD. See City of Lake City submittal guidelines for additional submittal requirements.

THE APPLICANT ACKNOWLEDGES THAT THE APPLICANT OR AGENT MUST BE PRESENT AT THE PUBLIC HEARING BEFORETHE PLANNING AND ZONING BOARD. AS ADOPTED IN THE BOARD RULES AND PROCEDURES. OTHERWISE THE REQUEST MAY BE CONTINUED TO A FUTURE HEARING DATE.

I hereby certify that all of the above statements and statements contained in any documents or plans submitted herewith are true and accurate to the best of my knowledge and belief.

Randall Olney, P.E.	
Applicant/Agent Name (Type or Print)	2/21/20
10/1/	3/21/24
Applicant/Agent Signature	Date
Applicant/Agent Name (Type or Print)	
Applicant/Agent Signature	Date
STATE OF FLORIDA COUNTY OF <u>Alachua</u>	
The foregoing instrument was acknowledged before me this	day of 3, 2024, by (name of person acknowledging).
TRINA LEMNAH Notary Public - State of Florida Notary Assn.	Signature of Notary Printed Name of Notary
Personally Known OR Produced Identification Type of Identification Produced	

CRS Lake City Marvin Burnett - Cover Letter

JOB NO. 22-0653



March 21, 2024

Robert Angelo Lake City Growth Management

RE: CRS Lake City Marvin Burnett

Dear Robert:

Please find attached the following items for review:

- Check #0018976 in the amount of \$200.00
- Site Plan Application
- Property Appraiser Information
- · Agent Authorization Form
- Deed
- Legal Description
- Property Owner Affidavit
- Proof of Tax Payment
- Traffic Study
- · Geotechnical Study
- Comprehensive Plan Analysis
- Concurrency Analysis
- · Meter Calculations
- Fire Flow Memo
- Lift Station Report
- Stormwater Report
- Signed and Sealed Plans

The ± 2.72 acre site is located on SR 47 and SW Marvin Burnett Road in Lake City, Florida on a portion of tax parcel number 07-4S-17-08127-005. The site is currently undeveloped and heavily wooded. The development intent is to construct a $\pm 10,640$ s.f. commercial retail store on the parcel with the associated parking, stormwater management, and utility connections. Utility connections consist of a gravity sewer lateral to a private onsite lift station and connection to an existing forcemain within SR 47 ROW. Water and fire protection will be provided by extending a 600' water main along the western ROW of SR 47 and crossing via directional drill under SR 47 to wet tap an existing City water main. Offsite roadway improvements consist of a sidewalk along the project frontage, driveway connection and an eastbound left turn lane to Marvin Burnett Road.

We trust you will find this submittal to be complete for review and approval. If you have any questions, or need additional information, please contact me at (352) 331-1976 or via email at randyo@chw-inc.com.

Sincerely, CHW

Randall Olney, PE

Director of Engineering, Land Development

Beyond Engineering

Florida Region

www.chw-inc.com

CRS Lake City Marvin Burnett - Cover Letter

JOB NO. 22-0653



Columbia County Property Appraiser

Jeff Hampton

Parcel: @ 07-4S-17-08127-005 (29833) 📀

 Owner & Property Info
 Result: 1 of 0

 ST JOHNS LLC
 13820 W NEWBERRY RD STE 100

 NEWBERRY, FL 32669
 NEWBERRY, FL 32669

 Site
 S1/2 OF NE1/4 OF NE1/4 W OF SR-47. 482-143, LE 1318-91, DC 1327-1297, DC 1327-1298, WD 1330-1324, 372-81, 804-766, 894-679, 912-1064, 1037-1953

 Area
 9.694 AC
 S/T/R
 07-4S-17

 Use Code**
 VACANT (0000)
 Tax District
 1

"The <u>Description</u> above is not to be used as the Legal Description for this percel in any legal bransaction.
"The <u>Description</u> above is not to be used as the Legal Description for this percel in any legal bransaction.
"The <u>Use Code</u> is a FL Dept. of Revenue (DOR) code and is not maintained by the Property Appraiser's office. Places contact your city or county Planning 8 Zoning office for specific zoning information.

Property & Assessment Values

Flobelty of Ma	Sessifierit values		
20	23 Certified Values	20	24 Working Values
Mkt Land	\$266,978	Mkt Land	\$266,978
Ag Land	\$0	Ag Land	\$0
Building	\$0	Building	\$0
XFOB	\$0	XFOB	\$0
Just	\$266,978	Just	\$266,978
Class	\$0	Class	\$0
Appraised	\$266,978	Appraised	\$266,978
SOH Cap [?]	\$0	SOH Cap [?]	\$0
Assessed	\$266,978	Assessed	\$266,978
Exempt	\$0	Exempt	\$0
Total Taxable	county:\$266,978 city:\$266,978 other:\$0 school:\$266,978		county:\$266,978 city:\$266,978 other:\$0 school:\$266,978



▼ Sales History

Sale Date	Sale Price	Book/Page	Deed	V/I	Qualification (Codes)	RCode
2/6/2017	\$70,000	1330/1324	WD	1	Q	01
7/7/2016	\$100	1318/0991	LE		U	14
4/7/1995	\$727,500	0804/0766	WD	V	U	35
7/31/1990	\$127	1037/1953	WD	V	U	03
7/21/1990	\$127	1036/1953	WD	V	U	03

▼ Building Characteristics

- II. 12. II.					
Bldg Sketch	Description*	Year Blt	Base SF	Actual SF	Bldg Value
5.1.5					

NONE

Codo	Door	Year Blt	Value	Units	Dim
Code	Desc	Year Bit	value	Units	

T L	and Bre	akdown
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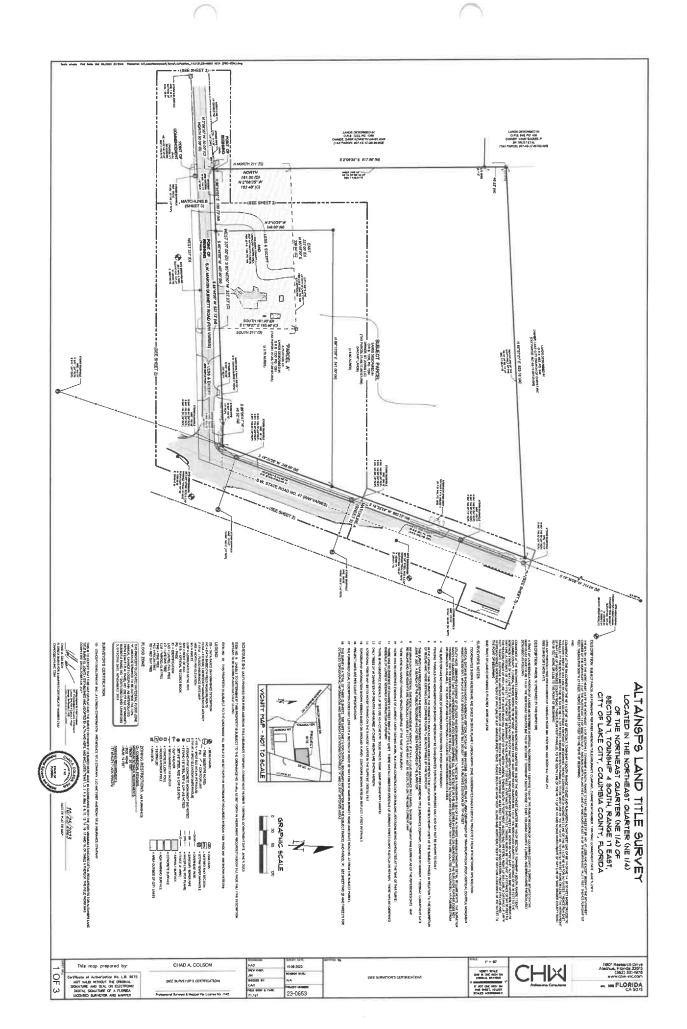
1	Code	Desc	Units	Adjustments	Eff Rate	Land Value
	0000	VAC RES (MKT)	1.000 AC	1,0000/1.0000 1.0000/ /	\$35,500 /AC	\$35,500
	0000	VAC RES (MKT)	8.694 AC	1.0000/1.0000 1.0000/.7500000 /	\$26,625 /AC	\$231,478

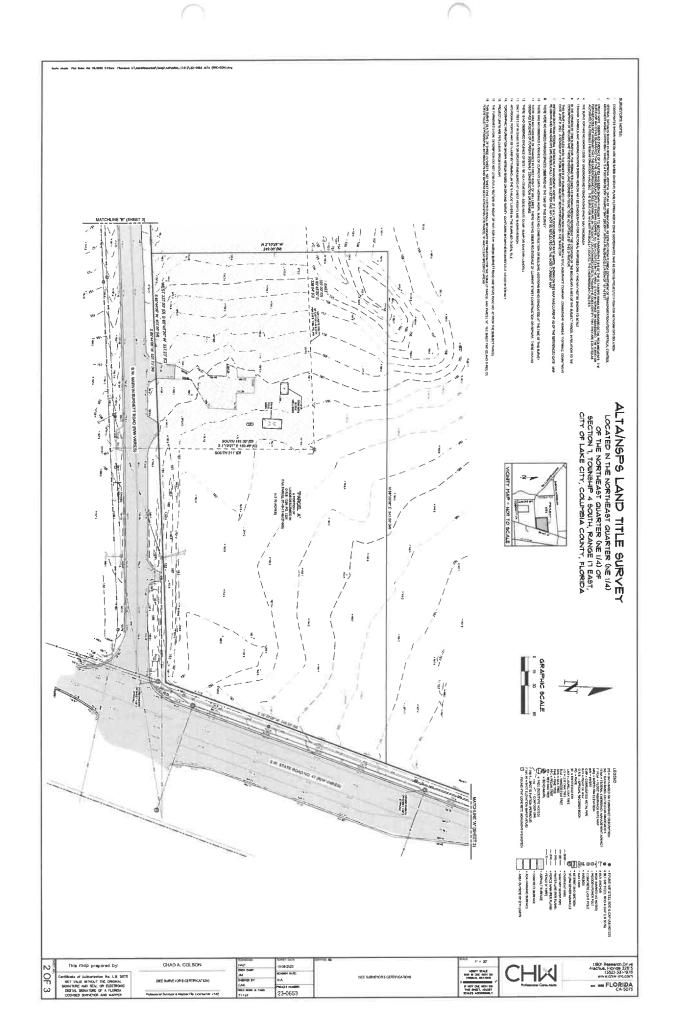
Search Result: 1 of 0

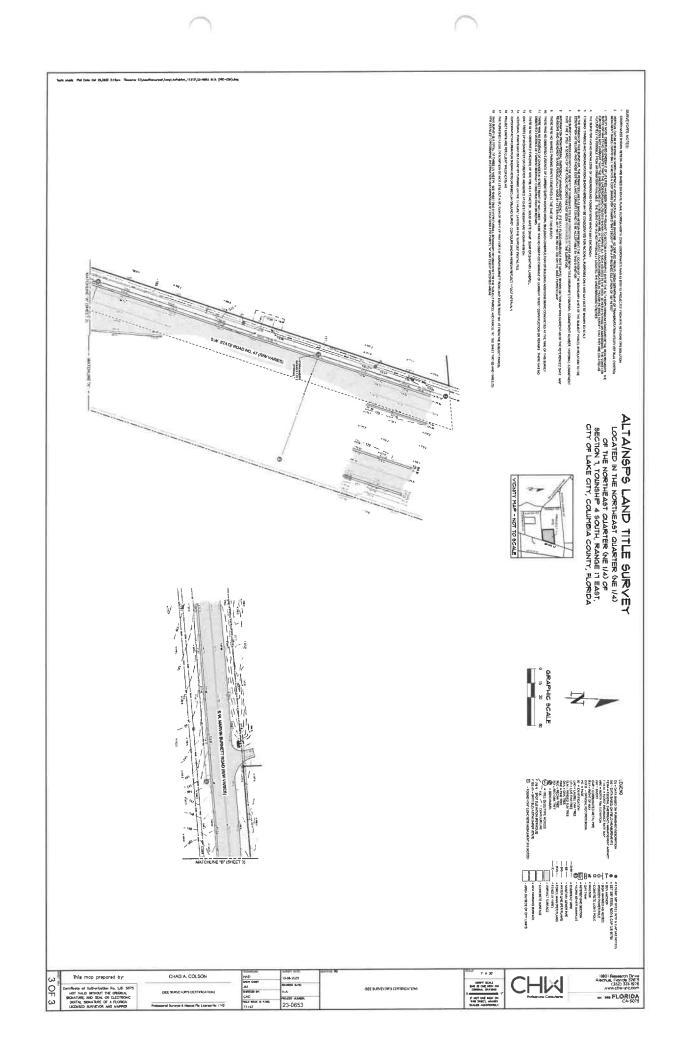
by:GrizzlyLogic.com

2024 Working Values updated: 11/23/2023

© Columbia County Property Appraiser | Jeff Hampton | Lake City, Florida | 386-758-1083







CONSTRUCTION PLANS

FOR:

OWNER I, JOHNS LIMITED LINGUITY COMPANY 3820 W. NEWBERRY ROAD, SUITE 100 NEWBERRY, FL 12669 MARVIN BURNETT RETAIL STORE

LAKE CITY, FLORIDA

SECTION 7, TOWNSHIP 4 SOUTH, RANGE 17 EAST

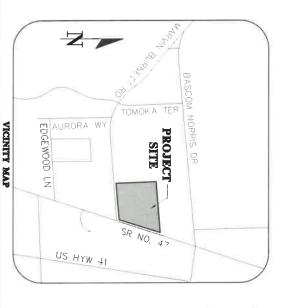
SUDVEXOR OF BRANCED CHAR A COLSON CHAR THEOLOGY CHAR ALCHAR R. 32615 (322) 331-1976

NGHN FAR OF TROOTS RAPOLL S. OLHEY P.E. CHW 11901 RESEARCH DRING ALACHIM, FL 32815 (352) 331-1978

SUBMITTED TO:

COLUMBIA COUNTY CITY OF LAKE CITY

FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION SUWANNEE RIVER WATER MANAGEMENT DISTRICT FLORIDA DEPARTMENT OF TRANSPORTATION





HINTERS 3/28/24 CITY OF LAKE CITY, COLUMBIA COUNTY, FOOT



PROJECT SITE

LOCATION MAP

MAKKINAL RETAIL STORE MAKKIN BURNETT

TAX 85. #07-45-17-08127-005
TYPE OF CONSTRUCTION PER FR6 2020, TYPE = 8

UPERMOUS AREA:

CHM

11801 Research Drive Alachus, Floride 32616 (362) 331-1976 www.chw-inc.com - m FLORIDA

- HE LOCATION OF ALL DETRING CHAIRTS SOOMS ON THE ALMS (MES EAR) FOR EXCHANGED AND THE MESSAGE AND THE ALMS CHAIRTS ON THE ALMS
- THE CONTACTION SHALL CREDICE CONTROL CALFORNIA MASSA OF MANDE FUTURES AND SHALL RECORD AT LEVER 44 MORNING TO THE AMERICANE THE THE CONTROL OF CONTROL AND ANALONS OF THE CONTROL OF THE CONTROL OF THE CONTROL OF THE CONTROL OF CONTROL OF THE CONTR
- THE CONTRACTOR SHALL READ VERSITY THE HEARDON'S LOCATION AND RETRIEVAL LOCATIONS OF ALL INSTRUMENTS SHYRIN THE LIMITS OF THE HOUSET ENVELOR'S ADMINISTRATED WHICH THE COMMINISTRATED CONTRACTORS ALL CLIALLY LITTIC COMMINISTS WHICH THE ACTION THE OF THE ARRIBLATE AND IT DECORPRISES CONSTRUCTION OF CONSTRUCTION, ANY DISCREPANCIES SHALL BE BROADT TO THE ATTENTIONS OF THE ARRIBLATE AND IT DECORPRISES CONSTRUCTION OF CONSTRUCTION, ANY DISCREPANCIES SHALL BE BROADT TO THE
- THE CONTRACTOR IS RESPONSIBLE FOR REPAIRING ANY DAMAGE TO EXISTING FACILITIES, ABOVE ON BELOW GROUND THAT MAY OCCUR AS A RESULT OF THE WORK PERFORMED BY THE CONTRACTOR.
- ALL REVIATE AND PRIACE PROPERTY ASSECTED IT THEN YOURS SHALL BE RESTORED FOR A CHARTICHE RIVAL TO OR RETTER THALL EXCEPTION COMPITIONS REPORT COMPRESSIONS CONSTRUCTION WHOM AND REST SECRETAL EXTENTED BY THE PLANE. ADDITIONAL CONTRACTION AND NO LITTRA COMPONENTION WILL BE ALLOWED.
- ALL WORR PEPORNED SHALL COMPLY WITH THE REGULATIONS AND ORDMANCES OF THE VARIOUS GOVERNMENTAL ACCICIES MAYING JURISDICTION OVER THE WORR INCLUDING LUMPSCAPING.
- IT STHE CONTRACTOR'S RESPONSIBILITY TO RECORD FAMILIAN WITH THE FERMIT AND INFRECTION BEQUIREMENTS OF THE EVALUATE CONTRIMENTIAL ACROSS, THE CONTRACTOR SHALL OF HAM ALL NECESSARY PRIMITS FROM TO CONSTRUCTION AND SCHEDULL INSTRECTIONS ACCORDING TO ACRICY AND/ON INVINCIPALITY INSTRUCTIONS.
- IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO COMPLY WITH AND ENFORCE ALL APPLICABLE SAFETY REGULATIONS. THE CONTRACTOR SOLL HE REPORTING FOR PROTETTING EXCENTIONS ADJUST COLLARS AND SOLL PRODUCE MACHE, SHEETING OF SOURCES ARCESSENT, TEXTORES SOLL REST OR Y WHAT LIFES ARE RIFE FUNCED, DEMORTING SOLLIE MESTE ARE RESIDED, AND TEXTOTITE THROUGH COCIL CONSTINUENTAL ACTIONS AND WATER MAMAGEMENT DISTRICT HE CONTRACT ON.
- CONTRACTOR TO REVIEW CEOTECHNICAL REPORT AND BOXINGS PRIOR TO BIDDING THE PROJECT AND POLLOW DUTLINED CONSTRUCTION TECHNIQUES.
- THE COMPACTION OF EXPONENT FOR COCKINGHING, APPLICABLE (1910) WHIT IS SERVED OF AN APPROXIDED THING, LOPENDATION FOR COMPACTION OF THE COM NETAL SET PACK PION TO SETE CHANTON ON MY SETE CONTENTION, NOTALL SET PACE RES ECONAL PROMINETS ENCOMA AND SCHMENTATION CONTEN AMPRICADES AND ALLA ON PROPRET SET PERM. ITS CONTENTION SHALL HANDAM THE SET FIRST HANDAM PROMINES CONTENTION OF THE CONTENTION FOR AN AND ANY CONSECTION MACKINES SHALL RE COMPACTED WITHIN AS MODELS.
- ALL THEE BARBICADES AND SILT FENCING SHALL RE INSTALLED AND INSPECTED BY THE CITY OF LAKE CITY PRIOR TO COMMEMCENENT OF ANY DEMOCITION OR CONSTRUCTION ACTIVITIES.
- S. THE CONTRACTOR IS TO PREPARE THE SITE PRIOR TO REGIMBING ACTUAL CONSTRUCTION IN ACCORDANCE WITH THE GEOTECHINCAL REPORT. ALL DELITINGS MATRIAL IAS MACK, MATE, MANSO DEBUIS IS DIE DICLIANTIO IN ACCIDENCE WITH THESE TAMS ON AS DIECTIO IT THE OMNIES SCHOOLES ON OWNERS SON, ITEMPOS COMMANS, DOLTERIOUS MANDELA, ES TO BE STOCKIMED MAD LIBRODIO FROM THE SITE. EICHVATED ARDAS AND TO BE EACHFLLED WITH APPROVED MATRIALS AND COMPACTED AS SHOWN ON THESE RELIS.
- WORK EITS CREASONED WASH THIS CONTRACT PARLEY REPEASE, SMOOTHE! WITH OTHER WORK EITHG. REPEASED ON THE SITE AF OTHER CONTRACTIONS AND/ON ITUITY COMMITMENT, IT WILL AR KICESSAAF FOR THE CONTRACTOR TO COORDINATE AND SCHEDULE HIS ACTIVITIES, WHERE MECESSAAY, WITH OTHER CONTRACTORS AND UTILITY COMPANIES. CONTRACTIN SIALL CLAM AND GRIU GAN' THOSE RORTIONS OF THE STE NECESCARY FOR CONSTRUCTION, DETURBED AREAS SIALL RE SOCIECE, SEEDE, HALFRED, OR FLAUTE WITH OTHER APPROVED LANDSCAPE MATERIAL, AS DIRECTED RT THESE PLANS, IMMEDIATELY FOLLOWING CONSTRUCTION FRE LOCAL INSPECTION.
- ALL PAVEMENT DIMENSIONS SHOWN ARE TO EDGE OF PAVEMENT UMEESS OTHERWISE NOTED.
- TI. ALL NEW TRAFFIG CONTROL REVICES GIGHT AND PAYEMENT WARRINGS SHALL COMFORM TO THE MANUAL ON INMOVEN TRAFFIC CONTROL DEVICES SMUTCO) AND FDOT \$TANDARDS.
- ALL STREPMG WITHIN THE COLUMBA COUNTY RIGHT OF WAY SPALL BE PLACED FIRST AS TEMPORARY STREPMG FOLLOWED BY APPLICATION OF THERMOPLASTIC STREPMG 30 DAYS LATER.
- CONTRACTOR IS RESPONSIBLE FOR MAINTANING PROPER BENCHMARKS ON-SITE. EUSTING BENCH MARKS SCHEDULED FOR REMOVAL SHALL BE RELOCATED AT CONTRACTORS EXPENSE AND REESTABLISHED BY A LICENSED SURVIYOR.
- ALL HANDICAP RAMPS SHALL COMPLY WITH THE PLORIDA ACCESSMENTY CODE AND AMERICANS WITH DISABILITIES ACT.
- 35. A PERCONTRIBUTION CONTREME SHALL BE REQUIRED. THE CONTRACTION, DACHERS OF RECORD, AND THE OWNER SHALL MEET WITH THE CITY OF LAKE CITY MUNIC WORKS DEPARTMENT PRIOR TO INSTITUTION OF SITE CONSTRUCTION.
- 26. AMY CHANGE ORDER REQUESTS, SITE REVISIONS, AND PAY REQUESTS MUST BE SUBMITTED TO AND APPROVED BY THE ENCINEER OF RECORD
- CONTINCTOR IS RESPONSIBLE FOR ALL DEWATERING AS MEDED THROUGHOUT ALL CONSTRUCTION ACTIVITIES COVERED BY THESE MAINS. DEWATERING SHALL BE DONE IN ACCORDANCE WITH FOOT STANDARD SPECIFICATIONS, FY 2023-24 EDITION, SECTION 128.
- ТИК СОМТИСТОВ В КАЗООВЫЕ ТОЯ ТОЯ ТОЯТИКЬ, ПОТИТЬ В ТОВОВЕТОВ В ТОВОВЕТОВ В ТОВОВЕТОВ В ТОВОВЕТОВ В ТОВОВЕТОВ В МОВЕ В ПОМИЛЬ ОВ ВЕЗАСИЛНО ГО БЕЗІПИЕ МОБИТОВИТЬ, ПОТОВЕТОВ ТОЯТИКИ ТОВОВЕТОВ В ТОВОВ В ТОВОВЕТОВ В ТОВОВ В ТОВОВ В ТОВОВ В ТОВОВЕТОВ В ТОВОВЕТОВ В ТОВОВЕТОВ В ТОВОВЕТОВ
- AN ASSUUT SURVET HAY 18 REQUERTO BY RECULATORY ACENCIES. CONTRACTOR TO COORDINATE WITH PROJECT OWNER FOR COMPLETION OF ASSUULT SURVEYS PHIOR TO PROJECT / PERMIT CLOSE-OUT.

MAINTENANCE OF TRAFFIC (MOT) NOTES

- THE CONTRUCTOR AS REPORTED FOR CHARMAS A MARKEMANC OF TRAINE SOOT THAT OR CONTRECTION ACTION THAT COMMAND THE THE MARKE ACTION OF THE CHARMAS AND THAT THE CHARMAS ACTION AND ACTION THAT ACTION AS THE CHARMAS ALL ACCIONATION AND ACTION ACTION AND ACTION A
- THE CONTRACTOR SHALL SUBMIT THE MOT TO THE APPROPHIATE REGULATORY AUTHORITY PRIOR TO WORK REQUIRING THE MOT FOR APPROVAL. NO WORK IN THE ROW SHALL OCCUM UNTIL THE MOT IS APPROVED.

DEMOLITION GENERAL NOTES

- THE CONTRACTOR SHALL BE RESPONSEDE TO DISPOSE OF ALL DENDLITION HATERIALS IN A SAFE AND LABVOR, HAMMER. THE CONTRACTOR SHALL SALVAGE TO THE DINNER ANY ITEM AS DETERMINED BY THE OWNER. OWICE DENOLISHED, MATERIAL SHALL BE DISPOSED OF PROPERLY AND HAMEDIATELY.
- ebione all improvements defined on the demolition flam. Salvage items to owner as defined by the onners refrescitative And construction pocument steencations.
- EOSTRIG PAYEMENT AND SIDENIALK EDGES THAT BONDER MEN CONSTRUCTION ON DEMOLITION AND TO BE SANK-CUT TO PROVIDE A SMOOTH TRANSTRON.
- ALL EXSTING TREES ARE TO REMAIN UNLESS OTHERWISE NOTED
- ROOTS LANCER THAN I INCH MY DIAMETER ON THEIS TO BE PRESERVED THAT ARE ENCOUNTERED DITHING CONSTRUCTION MIST BE CUT CLEAMS. AND COVERED OVER WITH SOIL BY THE EMO OF THE INDIKING DAY.
- ALL ASPALL NO LIMERCO NIAL EL COMPLITEIX IRRONDO PROM-ALES INN'I PILL, RE L'ADRICATE, DI MAINTACHE, ALLES MERE ASPAUL MEL DE REMONDE DIETT DE L'ET RECONDE PRINCE, L'HIBFOCA, DIA COMMANDIA, DEL REMONDE, DEL TRACCHET COS, CHILLE RESONOTE TELLO PRISE S. K., THE DETFIGE MECHANICITE DES PRINCE DE TALINGE MOST EL RÉ L'ELS I FELT DE ACCIONATION DE TELLO DES PRINCE DE TRACCHET COS CHILLES DOOT CORDITA, NO LIBERCOL, LUCIEL TIMESE, ON DEREC CASTITACTION DEBETS CAS RESONANT MASSET, DE L'ALDRICATED.

PAVING, GRADING, AND DRAINAGE GENERAL NOTES RECONSTRUCTOR SERVICES (CARESCONSERVATATION) CONTROL MACROS CONTROLLED AND TO MODIFICATIONS CONSTRUCTORS AND TO MODIFICATIONS CONSTRUCTORS AND TO MODIFICATIONS AND TO MODIFICA

- A CRESSIAN DE SUMMETTION CONTRO, MALL SE THE SERVICIENT OF THE CONTRACTOR, ALLS OF FETTI DECIMALE DAMA CONTROLLED AND CONTROLL
- KOTE: EROSION/SEDIMENTATION CONTROL SHOLL BE PLACED PRIOR TO SITE EXCAVATION AND SHALL REMAIN IN PLACE UNTIL SITE VECETATION AND LANDSCLYING IS COMPLETE.
- I. ALLI MAET RINGCTMES AND MYE SAULL BE FROTECTED FIGH SELTATION BY CONSTITUCTING IMPET PROTECTION AS DEFINED BY THESE TAMES OR MY THE FOOT TAMANAULUS. I HE ALTHEON OCCURE, THE CONTINACTOR IS RESPONSIBLE TO REMOVE SELTATION AS PART OF THE BASE CONTRACT AT HIS ADDITIONAL COST TO THE OWNER.
- CONVINCE DEMANTAL MACINES SALL SE CONTROLTE AS MATE OF THE MATEL, CONTROLTED, THE ACCUSED SALL SE CONTROLTED SALL SE CONTROLTED
- D. PERMAHENT VEGETATIVE STABLEATION SHALL BE APPLIED ON FINE GAADED SITES AS 500M AS PRACTICAL. TEMPORANY SEEDING SHOULD BE EMPLOYED TO PREVENT EXPOSINEE OF BARREN SONS UNTIL PERMAHENT VEGETATION CAN BE APPLIED.
- E ALL SLOPES 1:3 OR STEEPER REQUIRE LAPPED OR PECCED SOO.
- F, EROSOM, SERMENT AND THERROTT CONTROL ARE THE RESPONSIBLETT OF THE CONTRACTOR. THESE DELIMENTED MEASURES ARE THE MANAMEN RESPONSED, HITH ADDITIONAL CONTROLS TO BE UTILIZED AS WEIGHD, DEPONDERT HOW ACTUAL SITE CONDITIONS AND CONSTRUCTION OFMALINDS.
- THE CONTRACTOR SHALL MAINTAIN MIRS POSSESSION A COPY OF THE WATER MANAGEMENT DISTRICT CONSTRUCTION FEMIT. HE SHALL BE RESPONSIBLE FOR ADMERENCE TO ALL CONDITIONS CONTAINED IN THE PERMIT. H. G. ALL SYNTHETIC BALES, SILT FUNCE, AND OTHER ENOSIGN CONTROL MEASURES SYMIL BE REMOVED AT THE COMPLETION OF THE PROJECT.
- PROPOSED SPOT ELEVATIONS REPRESENT FINISHED PAVEMENT ON GROUND SUNFACE GRADE UNLESS OTHERWISE NOTED ON DRAWINGS.
- CONTACTOR SHALL SHART FOR MEXISTE TO THE ORBITA AND ORBITS INCREMENT SHOP DRAWINGS OF ALL RECEIVES AND AMBINISTICS OF THE PROPERTY AND AMBINISTICS OF THE SECONDARY OF THE ALL ORDINAL AND EXECUTION AND AND ALL ORBITS AND ALL OR A SHOP DRAWING DOES NOT SELFINE THE CONTRACTOR'S RESPONSIBILITY FOR THE PROPERTY CONTRACTOR'S RESPONSIBILITY FOR THE PROPERTY OF THE PROPERTY OF THE ALL ORBITS AND ALL OR A SHOP DRAWING DOES NOT SELFINE THE CONTRACTOR'S RESPONSIBILITY FOR THE
- THE COST OF ALL TESTING OF COMPACTION AND OTHER REQUIRED TESTS SHALL BE PAID BY THE CONTRACTOR AND MADE AVAILABLE TO THE ENGINEER OF RECORD DURING SITE HISTECTIONS.
- GCMEAL CONTRACTOR TO CONTACT ENGINEER OF RECORD AND THE OWNER REPRESENTATIVE 48 HOURS IN ADVANCE PROFETO BACKHILING TRENCHES FOR FREID INSPECTION AND PRIOR TO LAYING ASPIALT FOR FIREID INSPECTION.
- CONTRACTION IS TO CHAIRT FORTY APPROVED ASTAILT DESCRIPANTS TO THE CONTREST REPRESENTATION AND REACHES OF RECORD RESORD AND AND TO CHAIRDESS OF RECORD RESORD AND CONTRACTION OF THE TREAT OF AN UTHE ACAD SHALL HAVE CONTRESTED 315 RECEITED 315 RECEITED THE CONTRACTION FOR ALL MOTHET THE OWNER'S REPRESENTATIVE AND PROVIDES TEMPERATURE READINGS PROPRIET LATING ASPINALT.
- REPORT LETE PATTORNE FRONT OF ALL COSTS DOOS. THE LOVER WINDER ON POINTED OF A DOOR SHALL AS IT HE MAI MEMPARY, WIS LOOK SHOULD ON A LOOK ON AND ASSES OF THE COSTS ON THAIL EXTENS AND HE DOOK OF THE COSTS ONTITION A DESTRUCT COUL TO THE DOOR WITH AND MALL COMMY WITH SECTION 4.134 MANDYERING CLUMBACTS AT DOORS OF THE FURBINA ACCESSMENT FOOD FOR SHAME CONTRICTIONS. PLL MATERIAL RECTION O RECTION IT HE CITY OF LAKE CITY FRILL WORKS DEPARTMENT AN REMOKEST OF RECHOSED, THE CONTRACTION MALL IMPORTLY ALL IMPORTABLE AND LALL AL MAKEES ROUTH THE ROTTOM OF ANY PROPOSED LINESDOCK BASE, AND SHALL BACKEL METH PLL MATERIAL HELTING FROT STANDARD STECHECATIONS FOR ROAD AND RINGEE CONSTRUCTION, SAL PROT MIDE: 120-401 AND 120-402.
- RAMPS SHALL HAVE LIVEL LANDRICS AT THE BOTTOM AND TOP OF EACH RAMP RIM. CURE RAMPS ARE NOT REQUIRED TO HAVE LANDRICS: LANDRICS SHALL HAVE THE POLLDWING FEATURES:
- THE LANDING SHALL SE AT LEAST AS HIDE AS THE RAMP RUN LEADING TO IT.
- ALL LANDINGS ON BAMPS, SMILL BE NOT LESS THAN 68" CLEAR, AND THE ROTTOM OF EACH BAMP SHALL HAVE NOT LESS THAN 32" OF STRAIGHT AND LEVEL CLEARANCE. F SAMPS CLUMAT DREETING AT LANDINGS, THE MINIMAL LANDING SET SCHALL BE OFFIGET IF A EARP REMIRES A RESTORATED THAN FO OR A HORIZONTAL REGISTROM CRASTER THAN 37 THRENT SCHALL WAVE HANDINASS ON BOTH SIDES, HANDINASS AND HOT REQUIRED ON CLUM RAMPS, HANDINASS SHALL BE SHOWN ON THE STEE PLAN.
- THE CONTRACTOR SHALL STOCKPILE TOPSOR AND CONSTRUCTION MATERIALS AN AREAS DESIGNATED BY THE OWNER
- THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING RECORD DRAWINGS AS MOTED IN MOTE 429 UNDER SITE GENERAL HOTES.

13. ALL CONCRETE USED SHALL BE 2,500 PSI NINHBUN.

- ALL WELLS, CLEANOUTS, MANHOLE TOPS, PULL BOY CONDIS AND OTHER UTBLITY APPARTENANCES IN THE AREA OF REDEVELOPHENT SHALL BE PROTECTED AND TOPS ADJUSTED TO MATCH PROPOSED GRADES,
- SOD SHALL REPLACED AROUND ALL STRUCTURES AS DIRECTED BY THE FOOT MODES 524-001 AND FOOT MODES 425. AND 430, SERKS AS APPROPRIATE. ALL DTHER DISTURDED AREAS SHALL RESIDEDD AND HULCHED. CONTRACTOR SHALL SAW CUT, TACK, AND MATCH EXISTING PAVENERT AT LOCATIONS WHERE NEW PAVENERT MEETS ANY EXISTING PAVENERT.
- ALL STONM SEWER CUNE AND DITCH BOTTOM MILETS SHALL COMPORM TO THE APPLICABLE FROT INDEX. ALL DRAWAGE STRUCTURES WITH GRATES THAT ARE LOCATED IN GRASSED AREAS SHALL HAVE THE GRATE CHAINED TO THE STRUCTURE USING AN EYE BOLT AND CHAIN.
- ALL CONCRETE STRUCTURES SHALL HAVE ALL DIPOSED EDGES CHAMPERED 3/4" AND CLASS I SURFACE FINISH
- 19. ALL HOME FITTINGS AND CONNECTORS SHALL BE WATER TIGHT. SEE SPECIFICATIONS FOR MORE INFORMATION.

- 20. COMPACTION OF ALL MATERIALS SHALL BE LIMITED TO STATIC MODE ONLY, UNLESS DIRECTED OTHERWISE BY THE ENGINEER OF RECORD

WATER AND WASTEWATER GENERAL NOTES

I, MATEMAL MO CONSTRUTION METHODS FOR MATE AND RESTRICTES STATE SEAL BE ALECONOMICE THE THE COCAL REGULATION AGENET COME, AMAL, AND RESERVENCIANS OF CONSTRUCTION, LATER RESPONS HEREOF AND SPAT-MARTINE, SECURALISMOS METETO, AMPROVAL AND CONSTRUCTION OF ALL CITATY ESTENSIONS AND COMMETTONS MUST BE COMMINATED THROUGH THE REGULATION AGENCY DEMANDERS FOR MUSIC CHILDTES.

2. IT IS THE CONTRACTORY RESPONSIBILITY TO MOTEY ALL UTILITY COMPANIES TO DISCONNECT ON REMOVE THEIR FACILITIES PAIOR TO REMOVE OR DEMOLISHING ANY EXISTING STRUCTURES FROM THE SITE.

I, THE CONTRACTOR IS RESPONSIBLE FOR ANY MECESSARY UTILITY FIELD LOCATION AND RELOCATION AS REQUIRED.

E, THE COMPACTION HALL MEADER AN INTELLIPANCIATE THAT IS TO IC CAN'TY SENSE IN COCREANICE WITH THE REGULATORY INTELLIPANCE AND TEXT ALT OF ECONTRICOR'S REPONSIBILITY.
AND NOTIFICATION OF PARTIES BY THE CONTRICTOR'S REPONSIBILITY. a, the cost of all testing of compaction and other required tests shall be faid by the contractor and hade available to the Encineer of record during site inspections.

6. ALL PIRE MANS BULL AS SANKET TO A MYDROTATE PRESIDE TEST IN ACCORDANCE WITH THE RECOLATORY ACRICY IMMONE JIRISDECTION SAID TESTE ARE TO A CONTROOR BY THE REMARKA AND SAIR TIEST TO TO THE RECOLATORY ACRICY FOR APPROVAL COGRIMATION AND MOTHER/ATION OF PARTIES IS THE CONTRACTOR'S RESPONSIBILITY.

, Contractor simil share for einem to the ower and dreak's eighers sood bahmisco hult beeckt and amaratines time To eurso on the site, affeit to differ approm, eigher estamat man eigher herdial and eigher at contractors to Ediske. Dhaneets appromia of a sign damme doos hot eilete the contractors eisponseelit for the personement of the time.

A A MONDOVILL SPANION OF THE PRET MEDERAL REPORTS LIST THAN IS EET. SALL IS ANAMADE REPORTS POTALL WITH MOSS AN COUTED SCIENCES AND ANAMADEL ASSAULTION OF THE PRESEND REPORTS THAN THE FIRST HE WAS A CONTROLOGIST BY THE POTALL BUT REPORTS AND ANAMADE POLICE MANS, AND ECCANED IN FIRST MAN THE FIRST HE WAS A CONTROLOGIST BUT FOR THE ANAMADE POLICE MAN THE WAS A CONTROLOGIST BUT OF THE ANAMADE POLICE MAN THE WAS A CONTROLOGIST BUT OF THE ANAMADE POLICE MAN THE MAN THE ANAMADE POLICE MAN THE WAS A CONTROLOGIST BUT OF THE POLICE WAS A CONTROLOGIST BUT OF THE ANAMADE POLICE MAN THE WAS A CONTROLOGIST BUT OF THE POLICE WAS A CONTROLOGIST BUT OF THE WAS A CONTROLOGIS

S HINNEY PLAN MATE MANG CHOS GYRSE FIFES, THE THE MASS SHALL MAY SHAVE A SHAWMAN OF OSCIOLET THAN THE CORRECT WHITH OF LIAR AND HER MANG CHOS GYRSE FIFES, THE MEMBAN MATE OF THE LIAR AND SHAW SHAW HER PROPERTY HAVE AND THE MASS OF THE MEMBAN SHAW HER WITH MATE HAVE AND THE MASS OF THE MEMBAN SHAW HER WITH MATE HAVE AND THE MASS OF THE MEMBAN SHAW HER WITH MATE HAVE AND THE MASS OF THE MEMBAN HER HAVE A SHAW HER WITH MATE HAVE AND THE MASS OF THE MEMBAN HER HAVE A SHAW HER WITH MATE HAVE AND THE MASS OF THE MASS OF THE THE MEMBAN HER HAVE AND THE MASS OF THE MASS OF THE MEMBAN HER HAVE AND THE MASS OF THE MASS OF THE MEMBAN HER HAVE AND THE MASS OF THE MASS O

TO, ALL WATER MAINS SHALL HAVE A MINIMUM OF 36 INCINES OF COVER.

12. ALL PYC WATER SERVICE LINES SHILL BE SCH 48 PYC. I I. RESTRAINED JOINTS SHALL BE PROVIDED AT ALL FITTINGS AND HYDRANTS IN ACCORDANCE WITH AWWA STANDARDS

13. THE SITE WORK CONTRACTOR SHALL ENGAGE THE SURVICES OF A LICENSED UNDERGROUND UTILITY AND EXCAVATION CONTRACTOR TO MISTALI THE NEW MATER SERVICE LINE AND FORCE HALM COMMETTIONS. IS, PAULK UTLITY DASCHENTS WILL RE PHOVINDO AS REQUIRDO FON ALL UTLITIES SHOWN HEREON BY METES AND BOUND DESCRIPTION AND IN ACCONDANCE WITH THE REGULATORY ACENCY DEPAITMENT FOR PURILS UTLITIES. A, ALL SAMTHAY SOME STRACE LATEALE SHALL BE 4"PM, ESA 15 AN FUM SOM 15 MYTH A CLAHAUT LOCATED AST THE FLAKS, MHHHHH SLOMF ON FLATEALL SHALL BE LOK AND A MAMMAN CLAHAUT SAKONE OF 75 RET ON-CEPTER AND HHHHHHH SLOMF FOR 8" LATEALS SHAL BE DEK AND A MANNAN CLAHAUT SAKONE OF THE TET ON-CEPTEL.

ELECTRIC SERVICE GENERAL NOTES

LALECTRIC MENTAL INTERES ON REPORTED SERVICE SERVICES ON LOCATION AND COMMUNITION PROPERTY REPORTS TO SERVICE SERVICE SERVICES.

FDOT GENERAL NOTES

- A SYMMAND SPECIFICATIONS FOR MAND AND REIDE CONSTRUCTION FY 2023-24). A FIONT SYMMAND FAMILY FY 2023 FANDA CONSTRUCTION C. FROT ESPECY MANUAL FIZE 0.

SHOULD A CONTLET ARES RETWESH THE DETAILS SHOWN IN THE PLANS AND THE DEPAITTIBLE OF TRANSPORTATION STANDAUDS THE ENGHERS. PREMITTE SHALL AMERICATED, COMERN WITH THE ORDERING PROMETER IN ORDER TO RESIZE THE DESCRISANCE, IN TO CASE THE JUSTIMENT BEAUTITES FOR THE DEPAIT AMERICAN FROM THE BEAUTITES FOR THE DEPAIT AMERICAN FROM THE BEAUTITES FOR THE DEPAIT AMERICAN FROM THE BEAUTITES.

2. ALL TRAFFIC STRIPING AND MARKINGS ARE TO BE LEAD FREE. MONSOLVENT BASED THERMOPLASTIC

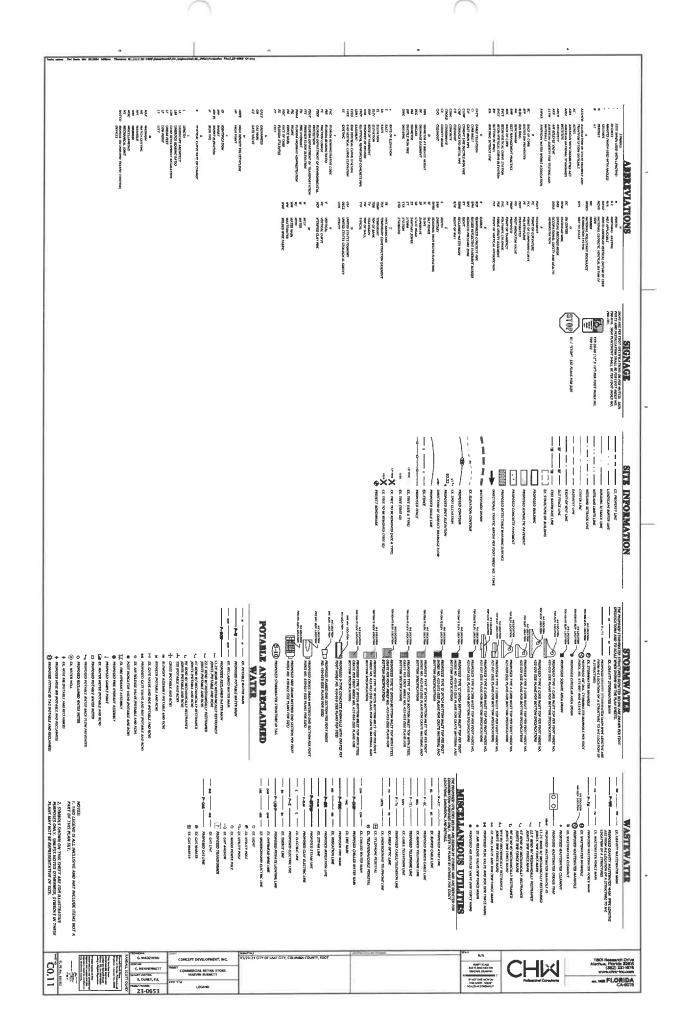
TTHAI 20/24 CITY OF LAKE CITY, COLUMBIA COUNTY, FDOT

I. REMOVAL OF EXISTING STRIMMG SHALL BE ACCOMPLISHED USING THE "HYDRO-BLAST" HETHOD. 4. ALL CURB AND GUTTER AND SIDEWALK WILL BE REMOVED AND REPLACED JOHN TO JOHN.

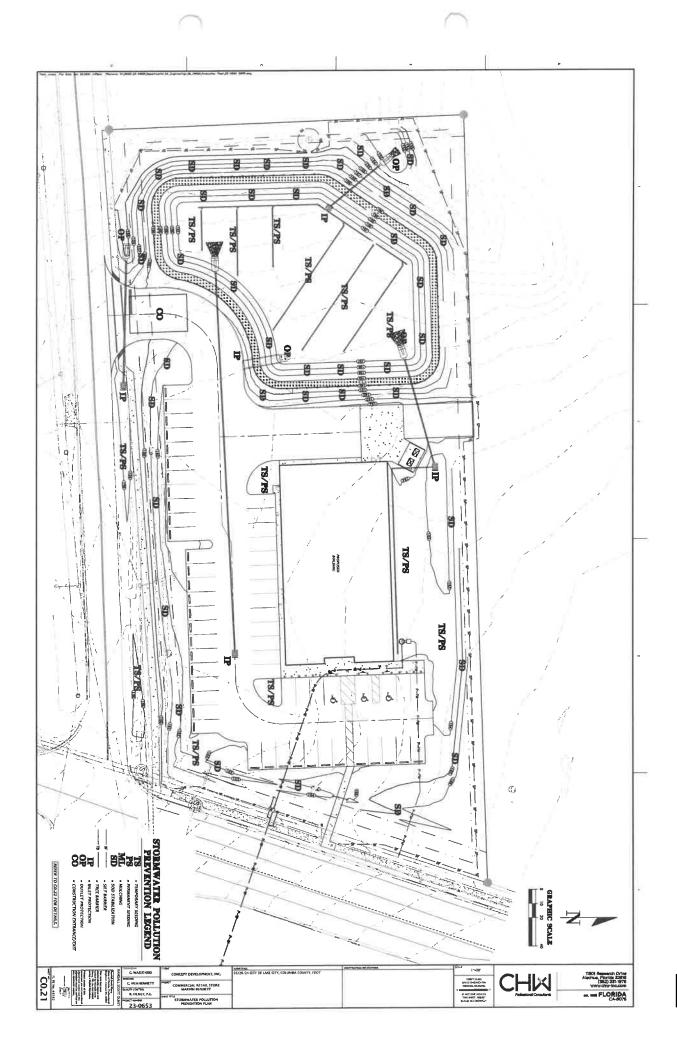
S, ALL DISTURBED AREA WITH THE DEPARTMENT OF TRANSPORTATION RIGHT OF WAY WILL RESTORED TO GRADING AND SODONIG THE AREA DISTURBED (BERANDA IN RURAL, CENTIPEDE IN UTILITY STRIPS).

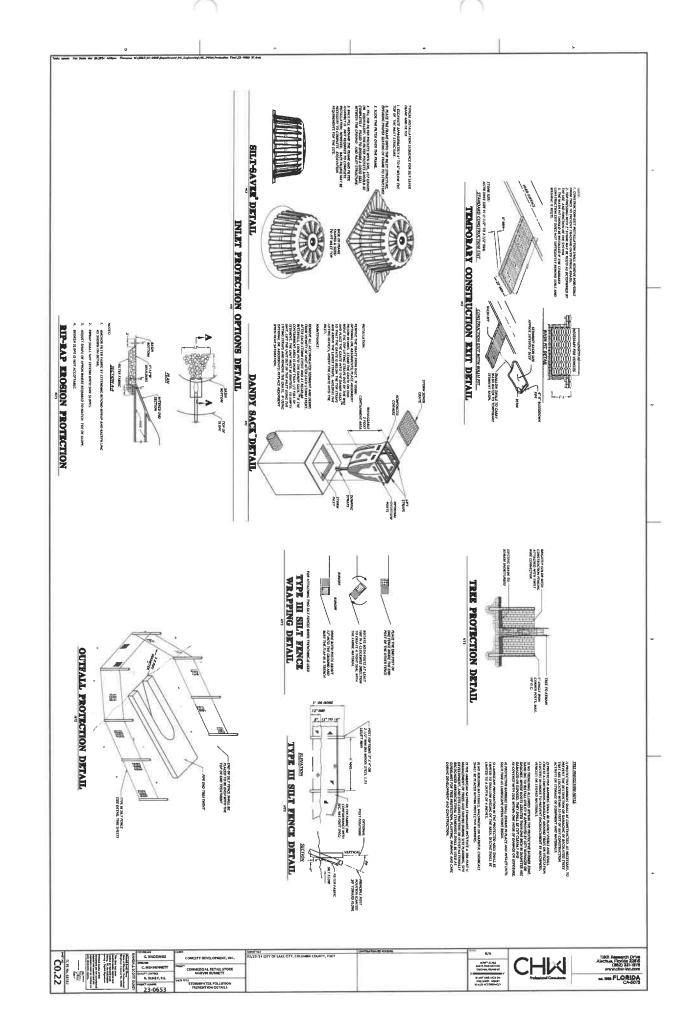
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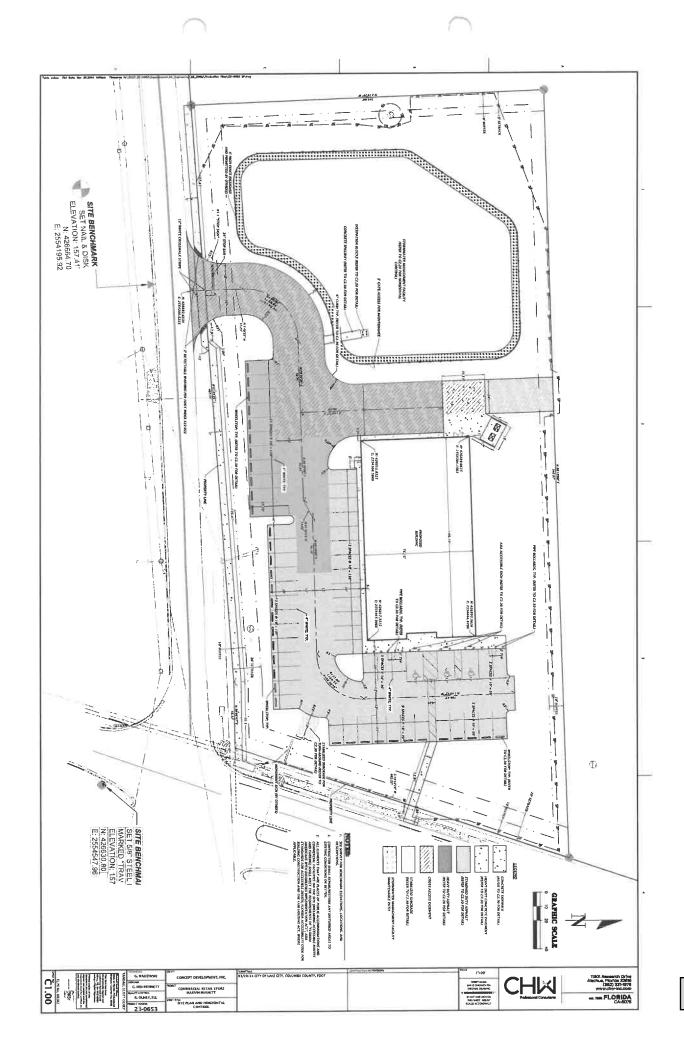


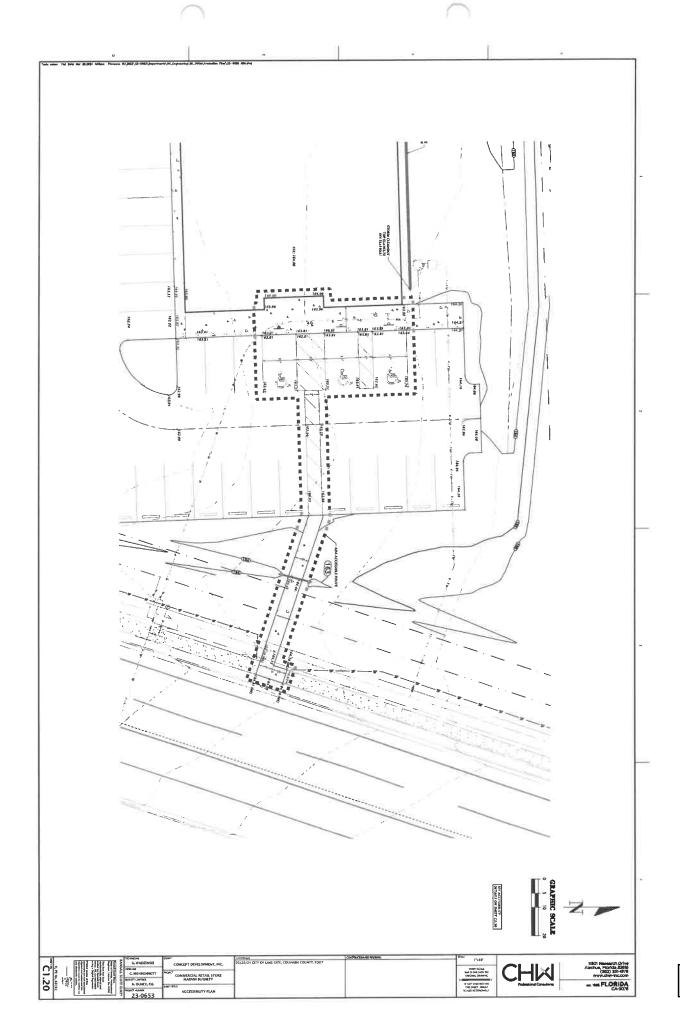
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	Contractor / Subcontractor / S			AD THE PRINCE PROGRAMMENT OF THE STEEL TO SENSOR IT CONDUCTIONS THE PRINCE. LIGHT STEEL THE PRINCE	TYL MAN THE MA
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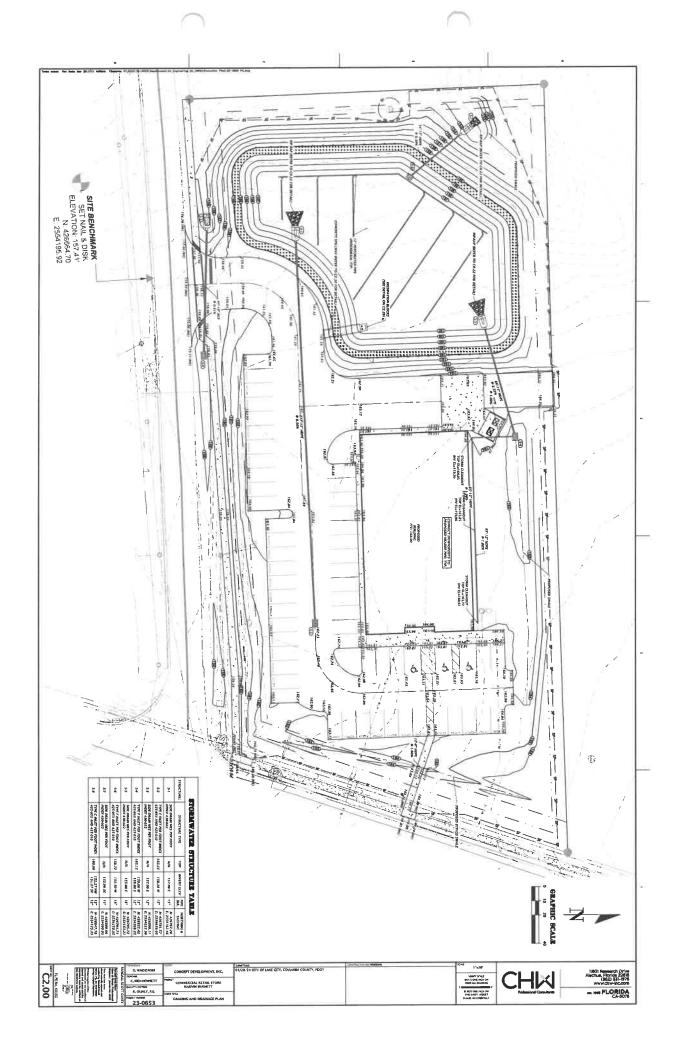


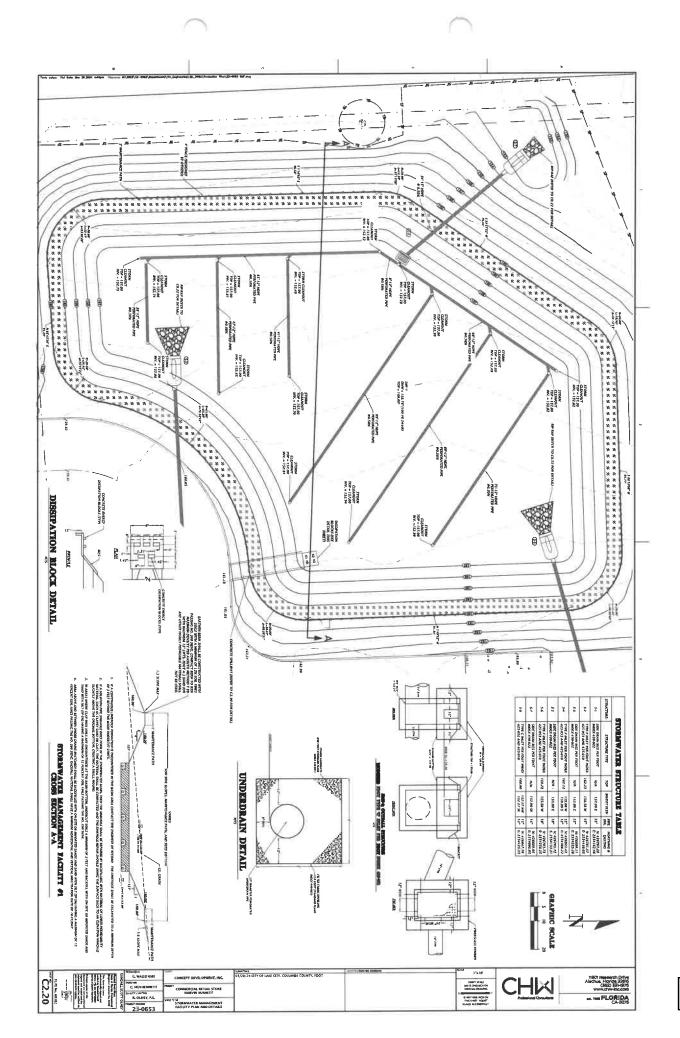


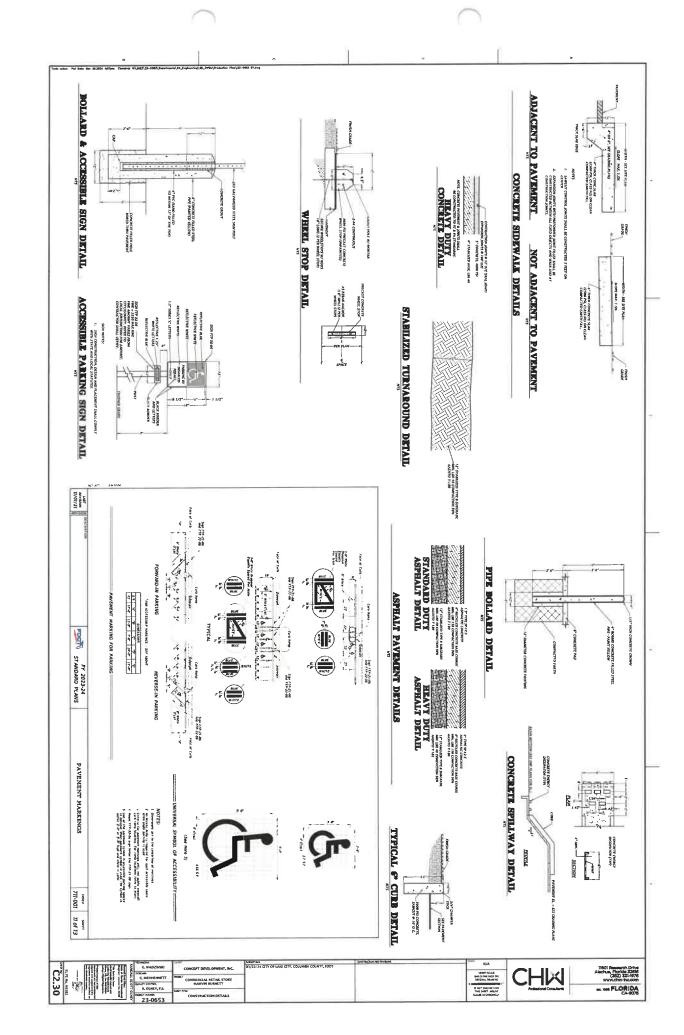


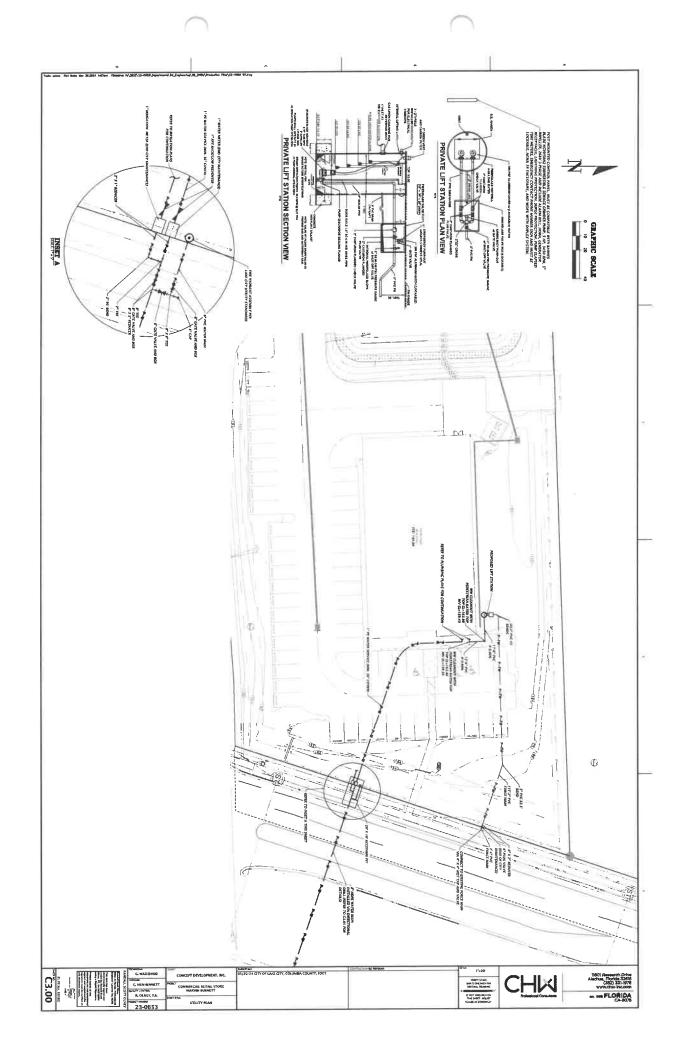


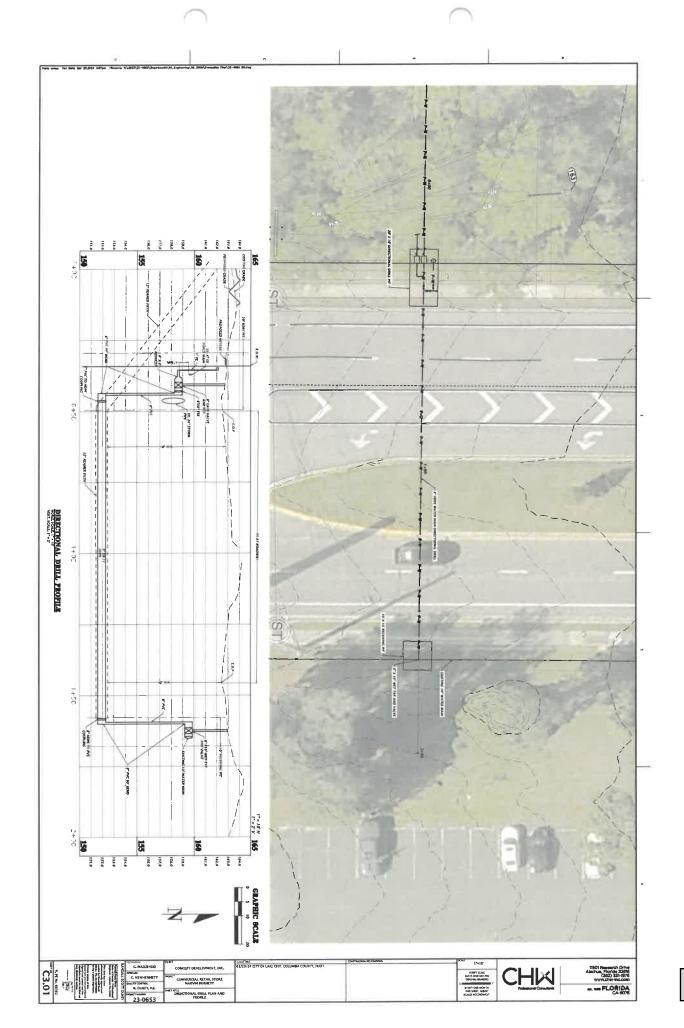


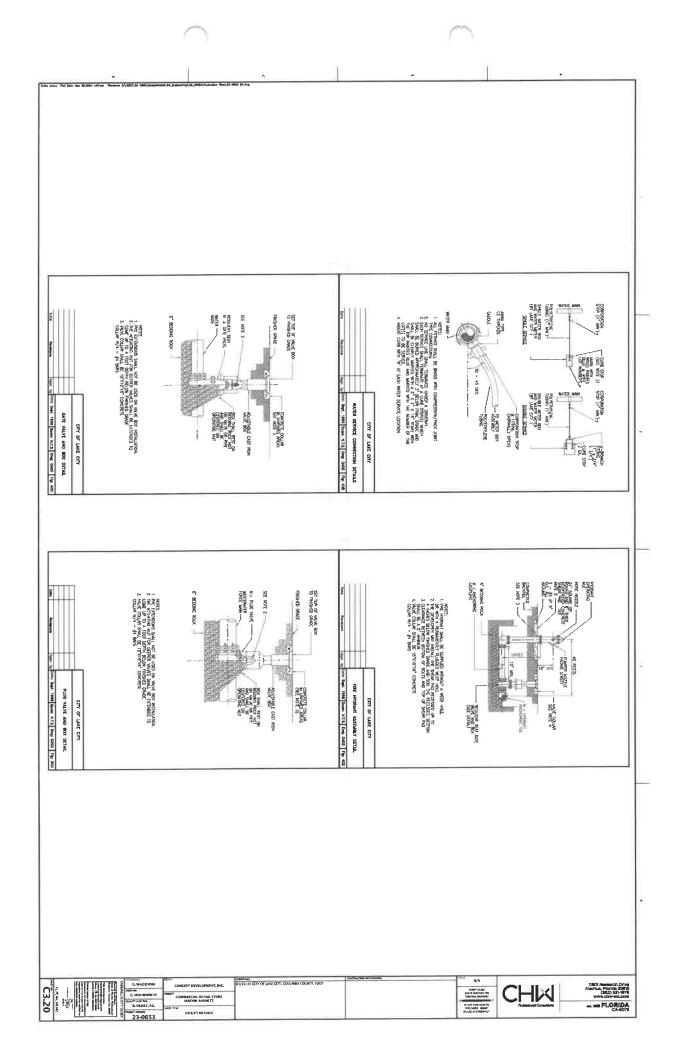


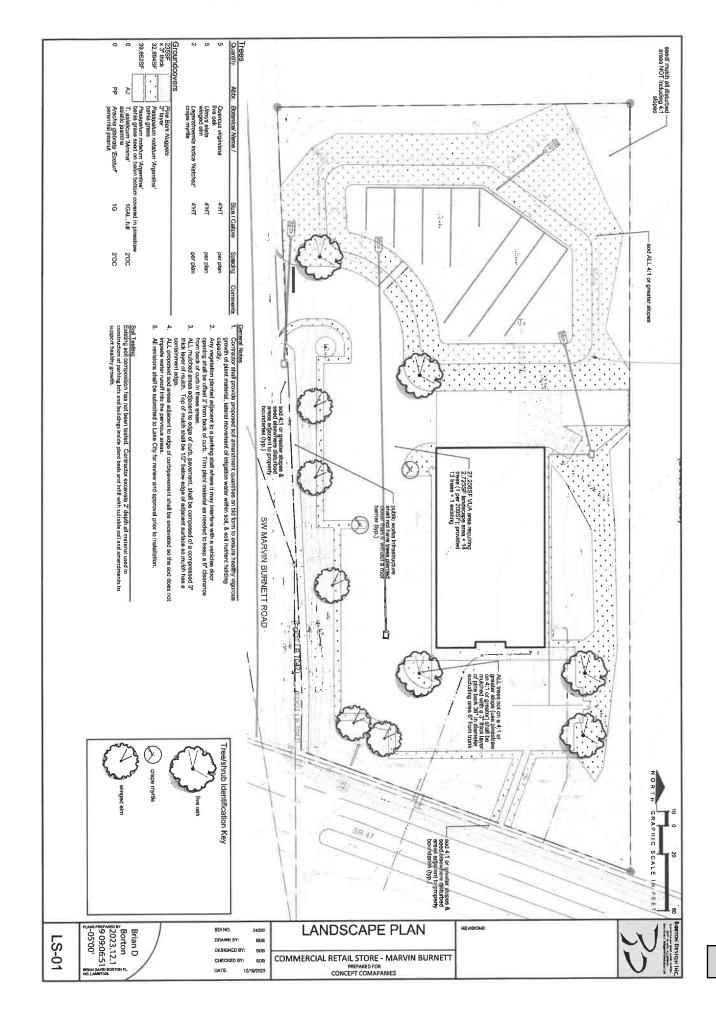


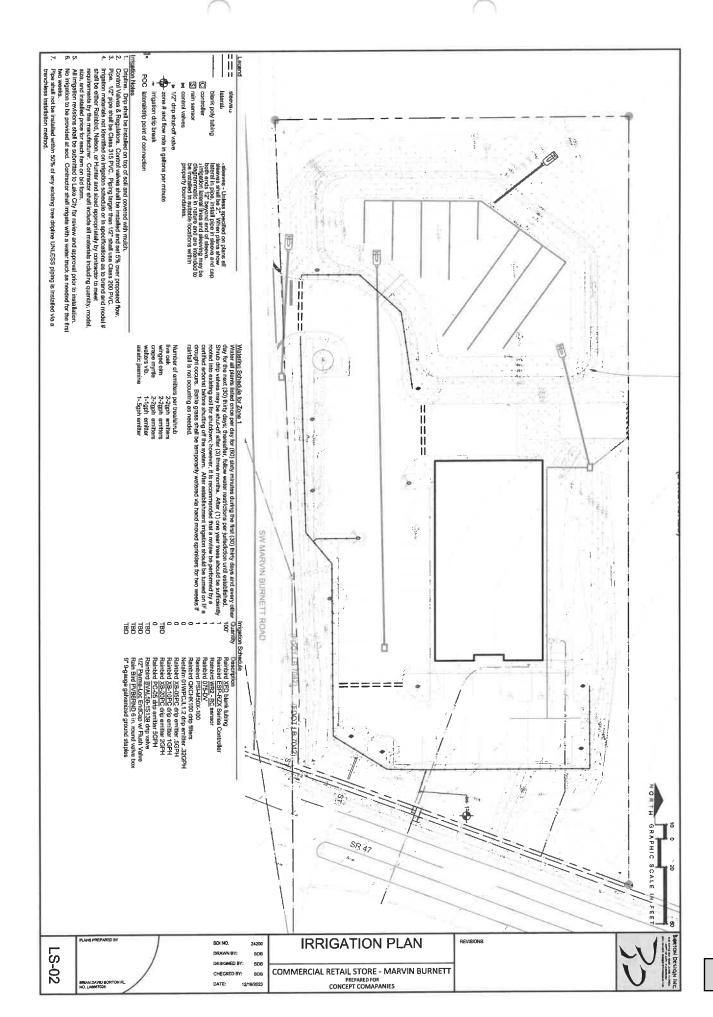












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actor shall be responsible for adding past, humus, fedilizer, manus, pt-commercially accepted acil additive to ensure normal, healthy plant growth

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LAMBAGUE BART E PLATITIOS PROCEDINGS

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General. First to commonwell of the year, he is undersized cabela, could other.

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SCAPS PART 2 SUBARTIALS of contractor shall be exponence for teating acts in planted eating for them Material. The Contractor shall be exponence for teating acts in 10 confirm that cold is adubble for healthy plant growth and shall studie a report to the 15 Representative prior to construction showing last data and any conflicts determined by

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If project/19 is April 7 tribling, digital first, ib. (See u.) project/19 is April 19/25/ETA/ASC)

In the control of control suplements.

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Install Rainbird BIVALSD-15139 or equal shut-off valve at all frost. Install Rainbird or equal XFO blank tubing around each thee with appropriate amount of equally spaced drip entitiess. Quantity of Rainbird XB-WFC or equal entitlers por tree are specified on the irrigation sheet. Drip shall entirate the earlier of the art of th

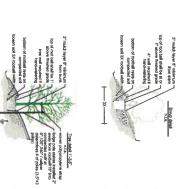
Detail for individual tree drip

shrub

Rainbird or equal XFD blank tubing with Rainbird XS-BPC or equal amilter at same pracipitation rate installed overtop of robbill or attach 1.4" tubing to emitter with bug pap to reach plant rootball of of 1.2" tubing when eppirable. Use ground staples to secure emitter locations. Start all drip runs with a Rainbird BI/ALSU-15129 articled rives. Terminate all drip runs with a compression hose and flush caps. Secure all emitter locations with

Detail for shrub emitters not to Scale ground staples.

Use ground staples to secure ALL emitter locations. Pressure regulators shall be used in appropriate locations so all emitters receive pressures between 20-50 psi.



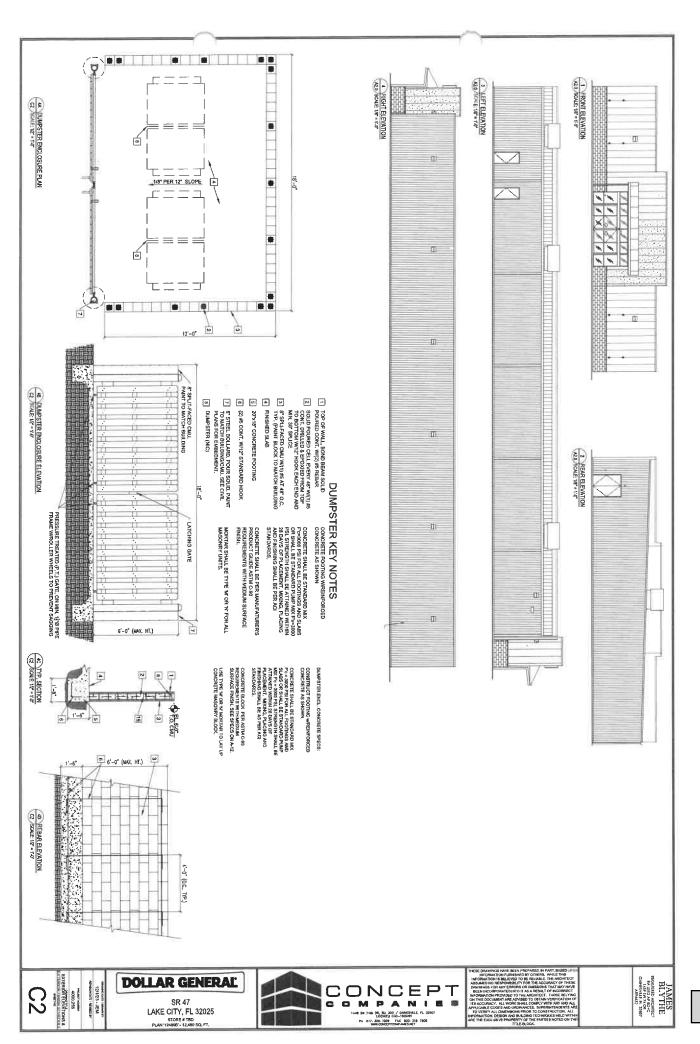
LS-03

BDI NO DRAWN BY BOB DESIGNED BY: BDØ CHECKED BY: BDB DATE: 12/19/2023

LANDSCAPE PLAN

Specifications/Details COMMERCIAL RETAIL STORE - MARVIN BURNETT PREPARED FOR CONCEPT COMAPANIES

REVISIONS.



MEMORANDUM

CRS Marvin Burnett





To: The City of Lake City From: Randall Olney, P.E. March 20, 2024 Date:

RE: CRS Marvin Burnett - Meter Sizing Calculations

The following is a calculation for meter sizing for the proposed project based on the City of Lake City Utility Standards.

CRS Marvin Burnett Building data is based on the information available from the project architect at the time of this memo. Any changes to the building data will void the provided meter sizing calculation and requires a revised analysis to verify calculations are compliant with the City of Lake City Utility Standards criteria.

PROJECT NAME:

CRS Marvin Burnett

PROJECT No.:

23-0653

FILE PATH:

N:\2023\23-0653\Departments\04_Engineering\01_Regulatory Permitting\Utilities\Meter Sizing

ADF and ADF METER SIZING CALCULATIONS

Proposed Average Daily Flow - Stores per Bathroom = 200 gpd per bathroom per FAC 62E-6

Proposed Average Water Demand	2 bathrooms	X	200 gpd		400	gpd
				Total=	400	gpd
Proposed Average Water Demand ERC (Eqv. Re Peak Water Demand ERC (PF=4) Peak Flow for Meter Sizing based on ADF (PF=		=350)			1.14 4.6 1.67	ERC ERC gpm
Peak Flow meter size per Lake City Utility Stand	dards 2010				5/8	inch

Use 5/8" Meter with 1" RPZ Backflow Preventer

Randall Scott Olney

Randall Olney, FL P.E. No. 68382

03/21/2024

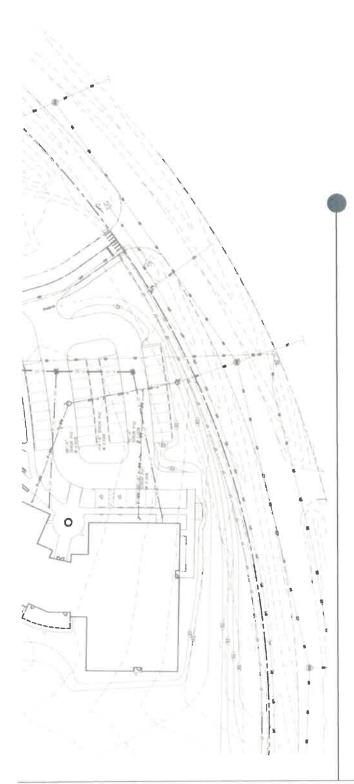
Date

Randall Scott Olney, State of Florida, Professional Engineer, License No. 68382

This item has been electronically signed and sealed by Randall Scott Olney, PE. On 03/21/2024 using a Digital Signature.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.





CRS Marvin Burnett

Private Lift Station Report 3/20/2024

Prepared for:

Florida Department of Environmental Protection City of Lake City Utility Department

Prepared on behalf of:

Concept Development, Inc. 1449 SW 74th Drive. Suite 200 Gainesville, FL 32607

Prepared by:

Randall S. Olney CHW

Deptily egned by Rendell Scott Othey

Rendell Scott Olney

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Randall Scott Olney, State of Florida, Professional Engineer, License No. 68382

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23-0653

N:\2023\23-0653\Departments\04_Engineering\01_Regulatory Permitting\Utilities\Lift Station



Flow Generation

Notes: 1) Estimated sewage flow per FAC 62-6.008, Table I

2 Water closets

200 gpd

400 gpd

Total average daily flow = 400 GPD

Average Daily In-Flow (Based on 16-hour Operation period) = 0.42 gpm

Peak Hour Factor = 4.0 Design In-Flow = 1.7 gpm

Wetwell Design

PROPOSED

Force Main Hydraulics

Inside Wet Well/Valve Box Pipe Diameter = 2.0 in
Onsite Forcemain Pipe Diameter = 2.0 in

Onsite Forcemain Pipe Diameter = 2.0 in
Offsite Forcemain Pipe Diameter = 4.0 in
Hazen-Williams C = 120

Normal Operating Tie-in Pressure = 35.0 psi = 80.9 ft

Static Elevation Head = 8.7 ft

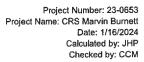
 $\label{eq:min_Volume} \begin{aligned} \mathit{Min_Volume} &= \frac{T}{\frac{1}{\mathit{DF} - \mathit{ADF}} + \frac{1}{\mathit{ADF}}} \end{aligned}$

Project Number: 23-0653 Project Name: CRS Marvin Burnett

Date: 1/16/2024 Calculated by: JHP Checked by: CCM

Calculation for friction head loss:

 $H_L = L \frac{10.5}{D^{4.87}} \left(\frac{Q}{C}\right)^{1.852}$



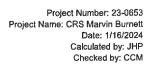


Equivalent Length of Straight Pipe for Fittings

Fitting Type			Equivalent Length (ft)	x	Quantity	=	Subtotal (ft) Eq. Length of same diam. PVC	Subtotal (ft) Eq. Length of 2 in. PVC Pipe	$L_2 = L_1 \left(\frac{D_2}{D_1} \right)^4$
Inside Pump Station and Valve	2,00	in							
Vault		•••					40.0	40.0	
Straight Pipe			12	x	1	=	12.0	12.0	
90° Bend			3.1	Х	2	=	6.2	6.2	
Plug Valve			2.6	x	1	=	2.6	2.6	
Tee Branch Flow			6.6	×	1	=	6.6	6.6	
Check Valve			17.0	x	1	=	17.0	17.0	
Gate Valve			1,5	x	1	=	1.5	1.5	
Onsite Forcemain	2.00	in							
Straight Pipe			155	x	1	=	155.0	155.0	
22.5° Bend			1.7	x	1	=	1.7	1.7	
45° Bend			1.7	x	4	=	6.8	6.8	
90° Bend			3.1	x	0	=	0.0	0.0	
Check Valve			17.0	x	0	=	0.0	0.0	
Plug Valve			2.6	x	1	=	2,6	2.6	
Offsite Forcemain (Proposed)	4.00	in							
Straight Pipe			2	x	1	=	2.0	0.1	
11.25° Bend			3.5	x	0	=	0.0	0.0	
22.5° Bend			3.5	x	0	=	0.0	0.0	
45° Bend			3.5	x	Ö	=	0.0	0.0	
Tee Branch Flow			12.0	x	1	=	12.0	0.4	
Check Valve			38.0	×	0	=	0.0	0.0	
Gate Valve			2.5	x	1	=	2.5	0.1	
NORMAL CONDITION:			Proposed Force Ma				(Includes leng	gth within lift sta	ation)
		Total Proposed	Effective Force Ma	in Length =	213 ft				

Out-Flow	Doctor	

Design Out-Flow Velocity = 2.5 ft/s (No less than 2.0 ft/s, No more than 8 ft/s)





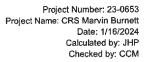
System Performance Curve

Normal Operation (with tie-in pressure)

Flow (gpm) Head (ft) 0 89.6 10 90.3 20 92.3 30 95.4 40 99.5	
0 89.6 10 90.3 20 92.3 30 95.4 40 99.5	
20 92.3 30 95.4 40 99.5	
30 95.4 40 99.5	
40 99.5	
50 104.6	
60 110.7	
70 117.7	
80 125,6	
90 134.3	
100 144.0	
110 154.5	
120 165.9	
130 178.1	
140 191.1	
150 204.9	
160 219.6	
170 235.0	
180 251.3	
190 268.3	
200 286.1	
210 304.7	
220 324.1	
230 344.2	
240 365.1	
250 386.7	
260 409.1	
270 432.2	
280 456.1	
290 480,7	
300 506.1	
310 532.2	
320 559.0	
330 586.5	
340 614.7	
350 643.7	
360 673.4	
370 703,8	

Pump Curve

One Pump - Simplex Flow (gpm)	Head (ft)
0	102.2
10	100.9
20	99.7
30	97.3
40	95,1
50	91.7
60	86,9
70	81.7
80	75.2
90	68,1
100	59.7





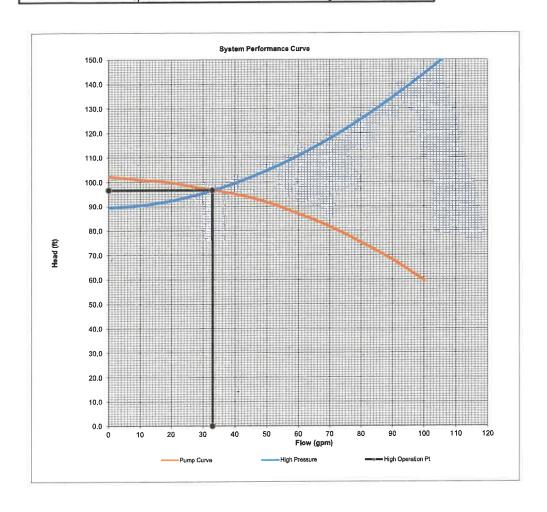
 Pump Specifications:
 Imp Dia: 5

 Pump:
 Barnes
 Imp Dia: 5

 Model:
 NGVH50N2
 Power: 5
 hp

 Discharge Flange Dia:
 2.5"
 Electrial Req: 3 Phase

 Speed:
 3450
 Voltage: 208
 V





Project Number: 23-0653 Project Name: CRS Marvin Burnett

Date: 1/16/2024 Calculated by: JHP Checked by: CCM

Design Operation Point (1 Pump - Normal Operation)

Flow (gpm)	Velocity (fps) in 2" FM		Head (ft)		
33.0	3.4		96.6		
System Performance at Norma Forcemain Velocity =	al Operation Design Point		3.37	ft/s	$Pump \ Run \ Time = \frac{Storage \ Volume}{Design \ Operation \ Flow}$
1 Ordernam velocity =			-		
Pump Run Time =			0.37	min	
Cycle Time =			29.70 0.49	min hrs	$\textit{Cycle Time} = \frac{\textit{Storage Volume}}{\textit{ADF}} + \textit{Pump Run Time}$
Fiberglass Wetwell					
Displaced Volume =		101.8	cf		
Unit Weight of Water =		62.4	pcf		
Weight of Displaced Water =		6,352	lb		
Bouyant Weight of Concrete =		77.60	pcf		
Bouyant Weight of Soil above C	oncrete Ring =	47.60	pcf		
Width of Bouyancy Concrete I	Ring =	1	ft		
Required Height of Bouyancy Co	oncrete Ring =	1.97	ft		



18 Frame

Submersible Grinder Pumps

Specifications:

DISCHARGE:

NGV Vertical 2" NPT

VOLUTE Cast Iron ASTM A-48, Class 30

MOTOR HOUSING Cast Iron ASTM A-48, Class 30

On Back Side. Dynamically Balanced,

ISO G6.3

Material Cast Iron ASTM A-48, Class 30

SHREDDING RING Hardened 440C Stainless Steel, Rockwell® C-55

CUTTER Hardened 440C Stainless Steel,

Rockwell® C-55
SHAFT416 Stainless Steel

SQUARE RINGS Buna-N

Material Rotating Faces - Carbon

Stationary Faces - Ceramic

Elastomer - Buna-N Hardware -300 Series Stainless

Cable 2000V - Ordered Separately

UPPER BEARING:

Design Single Row, Ball, Oil Lubrication

Load Radial

LOWER BEARING:

Design Double Row, Ball, Oil Lubrication

Load Radial & Thrust

MOTOR: Design NEMA B

Three Phase Torque Curve Oil-Filled, Squirrel Cage Induction,

Inverter Duty rated per NEMA MG1

included in control panel. Requires start

components to be included in panel.
Provided with pump

included in control panel

MOISTURE SENSORS......Normally Open (N/O), Requires relay
in control panel

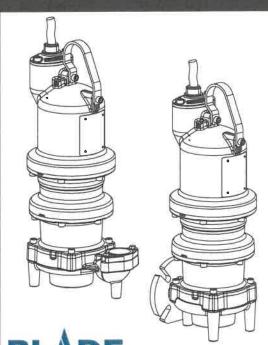
TEMPERATURE SENSOR Normally Closed (N/C)

To be wired in series with control circuit

OPTIONAL EQUIPMENT..... Seal Material, Impeller Trims,
Cord Length, Leg Kit, 3" Spool Kit







BLADE

Series: NGV 3, 5 , 7.5 & 10HP, 3450RPM, 60Hz

Sample Specifications: Section 3 Page 12.

DESCRIPTION:

THE GRINDER PUMP IS DESIGNED TO REDUCE DOMESTIC, COMMERCIAL, INSTITUTIONAL AND LIGHT INDUSTRIAL SEWAGE TO A FINELY GROUND SLURRY.



WARNING:

CANCER AND REPRODUCTIVE HARM - WWW.P65WARNINGS.CA.GOV



PUMPS & SYSTEMS

PAGE DATE

SECTION 3B

33

2/20

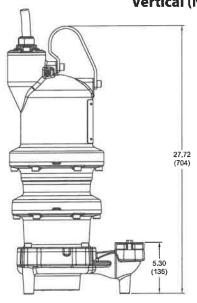


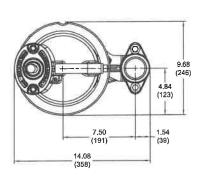


Submersible Grinder Pumps

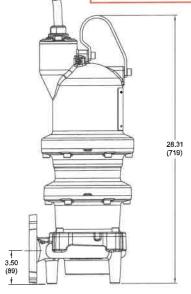
Vertical (NGV - 18 Frame)

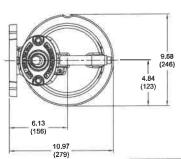


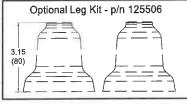




Horizontal (NGVH - 18 Frame)







IMPORTANT!

- 1.) MOISTURE AND TEMPERATURE SENSORS MUST BE CONNECTED TO VALIDATE THE WARRANTY.
- 2.) A SPECIAL MOISTURE SENSOR RELAY IS REQUIRED IN THE CONTROL PANEL FOR PROPER OPERATION OF THE MOISTURE SENSORS. CONTACT BARNES PUMPS FOR INFORMATION CONCERNING MOISTURE SENSING RELAYS FOR CUSTOMER SUPPLIED CONTROL PANELS.
- 3.) THESE PUMPS ARE CSA LISTED FOR PUMPING WATER AND WASTEWATER. DO NOT USE TO PUMP FLAMMABLE LIQUIDS.
- 4.) INSTALLATIONS SUCH AS DECORATIVE FOUNTAINS OR WATER FEATURES PROVIDED FOR VISUAL ENJOYMENT MUST BE INSTALLED IN ACCORDANCE WITH THE NATIONAL ELECTRIC CODE ANSI/NFPA 70 AND/OR THE AUTHORITY HAVING JURISDICTION. THIS PUMP IS NOT INTENDED FOR USE IN SWIMMING POOLS, RECREATIONAL WATER PARKS, OR INSTALLATIONS IN WHICH HUMAN CONTACT WITH PUMPED MEDIA IS A COMMON OCCURRENCE.
- 5.) THIS PUMP IS NOT APPROPRIATE FOR THOSE APPLICATIONS SPECIFIED AS CLASS 1 DIVISION 1 HAZARDOUS LOCATIONS.



PUMPS & SYSTEMS



Horizontal



Submersible Grinder Pumps

Subii									_			_		_	_	_	
CORD O.D.	86 ± 02	20. ± 00.	86 + 02	20. = 00.	.86 ± .02	.86 ± .02		1 12 + 02	4 I	86 + 02		.86 ± .02		.86 ± .02	CO + 30	20. ± 00.	ZO. I DO.
CORD	12/4 18//	t 50	12/4 - 18/4		12/4 - 18/4	12/4 - 18/4		8/4 - 18/4		12/4 - 18/4	4004	12/4 - 18/4		12/4 - 18/4	191 VICT	12/4 - 18/4	#/OI - #/7
CORD P/N A	125.10E	00107	125496	201-021	125497	125497		125498	00107	125496	407707	125497		125496	105407	125/07	10 P.
DRIVER	8	2		18		18		~	2			18			<u>∞</u>	ă	2
LOCKED ROTOR AMPS	113.2	131.2	85.4	95.2	47.6	38.1		113.2	131.2	85.4	95.2	38.1		173.9	201.0	200.2	**************************************
SERVICE FACTOR	7	<u>.</u>		1.0		1.0		10	2		1.0	1.0			1.0	,	2
FULL LOAD AMPS	25.0	23.2	12.1	12.6	6.3	5.0		34.1	29.9	17.8	16.0	6.4		28.0	28.2	4 4	d d m m
NEMA START CODE	ſ	Σ		₫		۵		ш	თ		7	7			Σ	2	3 BAF.
RPM (Nom)	3450	2		3450		3450		3450	200		3450	3450			3450	3450	for use of a NGVH with a 3x3 BAF, integral to power cord. (J - 75 Feet, or XL - 100 Feet.) Ints to be included in panel. Provide
ž	2	3		99		09	1	9	3		09	8			09	9	a NG range in the state of the
Ŧ	7	-		က		3		+	-		က	ы			က	۳	use o odor.
VOLT	208	230	208	230	460	575		208	230	208	230	575		208	230	775	nended for hended for alltage at m Feet, XJ - omponents
Д Н	0,00	5		3.0		3.0		5.0	3		2.0	5.0			7.5	7.5	sor lee
PART	141350NI	1000		141351N		141353N		141354N	1		141355N	141357N			141358N	141380NI	nool Kit is rature sen ration at ± ′30 Feet, X ttely.
MODEL	NGVH3072	100		NGVH30N2		NGVH3052		NGVH5072	2100110011		NGVH50N2	NGVH5052			NGVH75N2	NGVH7552	MOTE: A 3" Pipe Spool Kit is recommended for use of a NGVH with a 3x3 BAF. Moisture and Temperature sensor leads are integral to power cord. Pump rated for operation at ± 10% voltage at motor. A cord Sulfix: XC - 30 Feet, XF - 50 Feet, XJ - 75 Feet, or XL - 100 Feet. A cord sold separately. Single Phase pumpes require start components to be included in panel. Provided with pump.
			_	e)ı	102	zino M	H -	dι	un	d H	EΛΙ	E N	dA	18		-	Moisis M

SECTION 3B PAGE 38 DATE 2/20 CRANE

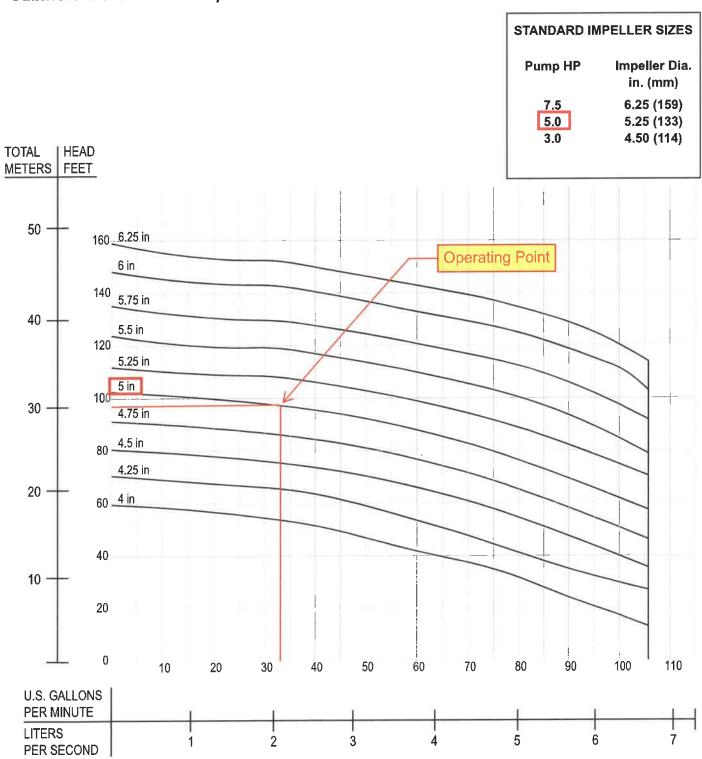
A Crane Co. Company

BLADE Series NGV / NGVH

Performance Curve 3, 5 & 7.5HP, 3450RPM, 60Hz



Submersible Grinder Pumps



Testing is performed with water, specific gravity 1.0 @ 68° F @ (20°C), other fluids may vary performance

SECTION 3B PAGE 40 DATE 12/18



MEMORANDUM

CRS Marvin Burnett 23-0653



To: City of Lake City

From: Randall S. Olney, PE

Date: March 20, 2024

RE: CRS Marvin Burnett- Required Fire Flow

The following is a calculation for the required fire flow for the proposed project based on the NFPA 1: Fire Code.

Building data is based on the information available from the project architect at the time of this memo. Any changes to the building data will void the provided fire flow calculation and requires a revised analysis to verify the building complies with the applicable fire protection criteria. The building will not be protected by an approved automatic fire sprinkler system.

NFPA Required Flow Calculations:

Building: Commercial Retail Store

Construction Type: II (000)
Fire Flow Area: ±10,640 SF

Required Fire Flow per NFPA Table 18.4.5.1.2: 2,250 gpm

Available Fire Flow:

Based on the hydrant flow data supplied by the City of Lake City, the total available fire flow at 20 PSI is as follows:

Total Available: 2,345 gpm

Minimum Required Fire Flow to be provided: 2,250 gpm

Conclusions:

The total available flow (2,345 GPM) is higher than the minimum required (2,250 GPM).

As part of this development, a new hydrant will be installed onsite.

Oney
DN: Evandya@chw-inc.com
Randall Scott Oiney
O-Randal Scott Oiney
L=Alachas, S=Picrise, C=US
Date: 2024,03.21

Date: 03/21/2024

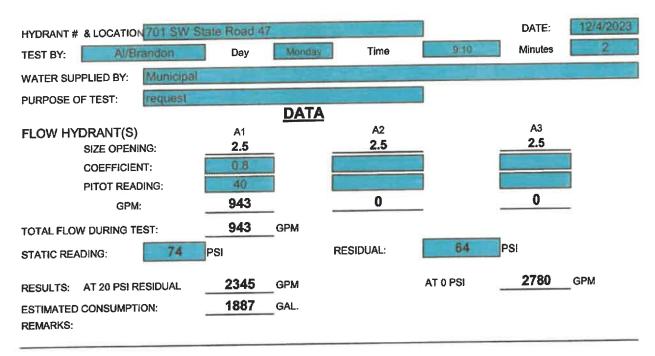
Randall S. Olney, P.E. 68382

Randall Scott Olney, State of Florida, Professional Engineer, License No. 68382

This item has been electronically signed and sealed by Randall Scott Olney, PE. On 03/21/2024 using a Digital Signature.

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City of Lake City Water flow report



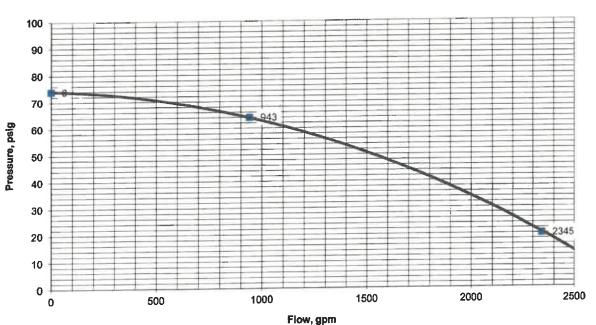


Table 18.4.5.1.2 Minimum Required Fire Flow and Flow Duration for Buildings

	Fire Flow Arc	a ft² (× 0.0929 for r	n ²)		Fire Flow gpm	
I(443), I(332),	71/011)	IV(2HH), V(111)*	11(000), 111(200)*	V(000)*	(x 3.785 for L/min)	Flow Duratio (hours)
11(222)*	II(111), III(211)*	0-8200	0-5900	0-3600	1500	
0-22,700	0-12,700		5901-7900	3601-4800	1750	
22,701-30,200	12,701-17,000	8201-10,900	7901-9800	4801-6200	2000	
30,201-38,700	17,001-21,800	10,901-12,900		6201-7700	2250	2
38,701-48,300	21,801-24,200	12,901-17,400	9801-12,600	7701-9400	2500	1
48,301-59,000	24,201-33,200	17,401-21,300	12,601-15,400	9401-11,300	2750	1
59,001-70,900	33,201-39,700	21,301-25,500	15,401-18,400		3000	
70,901-83,700	39,701-47,100	25,501-30,100	18,401-21,800	11,301-13,400	3250	1
83,701–97,700	47,101-54,900	30,101-35,200	21,801-25,900	13,401-15,600		3
97,701-112,700	54,901-63,400	35,201-40,600	25,901-29,300	15,601-18,000	3500	-∤
112,701-128,700	63,401-72,400	40,601-46,400	29,301-33,500	18,001-20,600	3750	-
	72,401-82,100	46,401-52,500	33,501-37,900	20,601-23,300	4000	-
128,701-145,900	82,101-92,400	52,501-59,100	37,901-42,700	23,301-26,300	4250	-1
145,901-164,200	92,401–103,100	59,101-66,000	42,701-47,700	26,301-29,300	4500	4
164,201-183,400		66,001-73,300	47,701-53,000	29,301-32,600	4750	_
183,401-203,700	103,101-114,600	73,301-81,100	53,001-58,600	32,601-36,000	5000	
203,701-225,200	114,601-126,700	81,101-89,200	58,601-65,400	36,001-39,600	5250	
225,201-247,700	126,701-139,400		65,401-70,600	39,601-43,400	5500	
247,701-271,200	139,401-152,600	89,201-97,700	70,601–77,000	43,401-47,400	5750	
271,201-295,900	152,601-166,500	97,701-106,500		47,401-51,500	6000] 4
Greater than 295,900	Greater than 166,500	106,501-115,800	77,001-83,700	51,501-55,700	6250	7
	1	115,801-125,500	83,701-90,600	55,701-60,200	6500	
		125,501-135,500			6750	
		135,501-145,800		60,201-64,800	7000	-
		145,801-156,700		64,801-69,600		-
	1	156,701-167,900		69,601-74,600	7250	-
		167,901-179,400	121,301-129,600	74,601-79,800	7500	-
		179,401-191,400	129,601-138,300	79,801-85,100	7750	
		Greater than 191,400	Greater than 138,300	Greater than 85,100	8000	

^{*}Types of construction are based on NFPA 220.

*Measured at 20 psi (139.9 kPa).

COMPREHENSIVE PLAN CONSISTENCY ANALYSIS

CRS Marvin Burnett



To: Robert Angelo, City of Lake City Growth Management

From: Braxton Linton III, Project Planner

Date: December 7th, 2023

RE: CRS Marvin Burnett - Comprehensive Plan Consistency Analysis

This Concurrency Analysis is submitted for CRS Marvin Burnett Lake City. The proposed use is a ±10,640-square-foot Commercial Retail Store and associated parking and stormwater. The site is on a portion of tax parcel is 07-4S-17-08127-005 in Columbia County, FL. A lot split is being completed on the site to create a 2.72-acre parcel adjacent to the intersection. One driveway connection will be to Marvin Burnett Road which is a county road. The future land use category is Commercial, and the zoning district is Commercial, Intensive (CI).

The following analysis estimates potential impacts on Lake City public facilities that may result from the proposed development. The following tables include data obtained within the City Comprehensive Plan and Florida Administrative Code (F.A.C.).

Future Land Use Element

Policy I.1.1:

The location of higher density residential, high intensity commercial and heavy industrial uses shall be directed to areas adjacent to arterial or collector roads, identified on the Future Traffic Circulation Map, where public facilities are available to support such higher density or intensity.

COMMERCIAL

Lands classified as commercial use consist of areas used for the sale, rental, and Distribution of products or performance of services, as well as public, charter and private elementary, middle and high schools. In addition, off-site signs, churches and other house of worship, private clubs and lodges, residential dwelling units, which existed within this category on the date of adoption of this objective, and other similar uses compatible with commercial uses may be approved as special exceptions and be subject to an intensity of less than or equal to 0.25 floor area ratio except withing the (CG) Commercial, General, (CI) Commercial, Intensive, (C-CBD) Commercial-Central Business District and (CHI) Commercial, Highway Interchange districts being subject to intensity of less than or equal to 1.0 floor area ratio.

The proposed use, a Commercial Retail Store, is considered a retail use, which is consistent with the Commercial FLU category. Development will be consistent with standards set forth by the Commercial FLU category and CI Zoning District.

Objective I.2

The City shall adopt performance standards which regulate the location of land development consistent with topography and soil conditions and the availability of facilities and services.

The site is composed of three soils:

1. Blanton Fine Sand, 0 to 5 percent slopes (hydro group: A)
N:2023/23-0653\Departments\02_Planning\Reports\RPT 2301204 Comprehensive Plan Consistency - CRS Marvin Burnett.docx

- 2. Pelham Fine Sand, 0 to 2 percent slopes (hydro group: B/D)
- 3. Ichetucknee Fine Sand, 5 to 8 percent slopes (hydro group: D)

According to the NRCS soil database, these soil types are conducive to the proposed development, which is also demonstrated on adjacent sites with similar uses and soil types.

Currently, there are no buildings located on the project site, there are developments to the north and east of the site.

Objective I.3

The City shall require that all proposed development be approved only where the public facilities meet or exceed the adopted level of service standard

Currently, there is no development on the project site. The proposed commercial retail store does not result in a degradation of Level of Service (LOS) standards, as is demonstrated in the Concurrency Impact Analysis memorandum submitted as part of this application.

Policy I.6.2

The City shall continue to include provisions for drainage, stormwater management, open space and safe and convenient on-site traffic flow including the provisions of needed vehicle parking for all development.

The proposed development will have onsite stormwater management facilities and adhere to all open space requirements set forth in Lake City's LDR. Safe and convenient on-site traffic flow will include one ingress and egress point on Marvin Burnett Road which is a county road. Parking will adhere to standards set in Lake City's LDR.

Objective I.6.5

The City shall continue to require that where a commercial or industrial use is erected or expanded on land abutting a residential district, then the proposed use shall provide a landscaped buffer. A masonry or wood opaque structure may be substituted for the planted buffer.

The subject property, specifically tax parcel 07-4S-17-08127-005, abuts a residential district to on the southwestern boundary. A landscape buffer is provided following guidelines from Lake City's LDR as shown in the landscape plan.

Transportation Element

Objective II.1: Level of Service

The City shall establish a safe, convenient, and efficient level of service standard which shall be maintained for all roadways.

The proposed Commercial Retail Store (Institute of Transportation Engineers (ITE) Land Use Code 814) will not result in a degradation of transportation Level of Service (LOS) standards. Demonstrated in the Concurrency Impact Analysis memorandum submitted as part of this application, there will be 447 net total projected daily trips.

Policy II.1.2

The City shall control the number and frequency of connection and access points of driveways and roads to arterials and collectors by requiring access points for state roads to be in conformance with Chapter 14-96 and 14-97, Florida Administrative Code, and the following requirements for non-state roads:

1. Permitting 1 access point for ingress and egress purposes to a single property or development.

The subject property includes one ingress and egress point one ingress and egress point on Marvin Burnett Road which is a county road.

Policy II.1.3 The City shall continue to require development to provide safe and convenient on-site traffic flow, which includes the provisions for vehicle parking.

Safe and convenient on-site traffic flow will be achieved by having one ingress and egress point one ingress and egress point on Marvin Burnett Road which is a county road. The parking lot will adhere to parking requirements set forth in Lake City's LDR.

Conservation Element

Policy V.5.2: Soils. The City shall protect soil resources through erosion and sedimentation control, by requiring proper design criteria on specific soils.

In an effort to conserve potable water, that at least 50 percent of the following required landscaped areas be comprised of vegetation native or indigenous to the north Florida area:

1. 10 percent of offstreet parking areas;

Parking area landscaped islands have been provided as shown in the landscape plan.

2. 10 foot buffer between residential and commercial uses;

This buffer has been provided following guidelines from Lake City's LDR as shown in the landscape plan.

3. 15 foot buffer between single family uses and multi-family uses or mobile home parks; and

Not Applicable.

4. 25 foot buffer between residential and industrial uses.

Not Applicable.

Sanitary Sewer, Solid Waste, Drainage, Potable Water and Natural Groundwater aquifer Recharge Element

Objective IV.3: The City shall coordinate the extension of, or increase in the capacity of facilities by scheduling the completion of public sanitary sewer improvements concurrent with projected demand.

Policy IV.3.1: The City hereby establishes the following Level of Service standards for sanitary sewer facilities:

FACILITY TYPE
City of Lake City
Community Sanitary
Sewer System

LEVEL OF SERVICE STANDARD
135 gallons per capita per day

The proposed development will allow a use of $\pm 10,640$ sq. ft. of nonresidential use. As is demonstrated in the Concurrency Impact Analysis submitted as part of this application, this facility is estimated to use 1,064 Gallons Per day by the proposed use.

- Objective IV.4: The City shall continue to coordinate the extension of, or increase in the capacity of solid waste facilities by scheduling the completion of public facility improvements and requiring that they are concurrent with projected demand.
- Policy IV.4.1: The City hereby establishes the following level of service standards for solid waste disposal facilities:

FACILITY TYPE
Solid Waste Landfill
Solid Waste Landfill
LEVEL OF SERVICE STANDARD
.85 tons per capita per year
Residual capacity of landfill

The proposed development will allow a use of ±10,640sq. ft. of nonresidential use. As is demonstrated in the Concurrency Impact Analysis submitted as part of this application, this facility is estimated to use 233.02 lbs./day and 42.53 tons/year.

- Objective IV.6: The City shall continue to coordinate the extension of, or increase in the capacity of potable water facilities by scheduling the completion of public facility improvements and requiring that they are concurrent with projected demand.
- Policy IV.4.1: The City hereby establishes the following level of service standards for potable water.

FACILITY TYPE
City of Lake City

LEVEL OF SERVICE STANDARD
150 gallons per capita per day
Residual capacity of landfill

The proposed development will allow a use of ±10,640 sq. ft. of nonresidential use. As is demonstrated in the Concurrency Impact Analysis submitted as part of this application, this facility is estimated to use 1,064 Gallons Per day by the proposed use.

CONCURRENCY ANALYSIS

CRS Marvin Burnett

23-0653



To: Robert Angelo, City of Lake City Growth Management

From: Braxton Linton III, Project Planner

Date: December 7th, 2023

Re: CRS Marvin Burnett - Concurrency Impact Analysis

This Concurrency Analysis is submitted for CRS Marvin Burnett Lake City. The proposed use is a ±10,640-square-foot Commercial Retail Store and associated parking and stormwater. The site is on a portion of tax parcel is 07-4S-17-08127-005 in Columbia County, FL. A lot split is being completed on the site to create a 2.72 acre parcel adjacent to the intersection. One driveway connection will be to Marvin Burnett Road which is a county road. The future land use category is Commercial, and the zoning district is Commercial, Intensive (CI).

The following analysis estimates potential impacts on Lake City public facilities that may result from the proposed development. The following tables include data obtained within the City Comprehensive Plan and Florida Administrative Code (F.A.C.).

Roadways / Transportation

Trip generation figures are based on the Institute of Transportation Engineers (ITE) Trip Generation Manual, 11th Edition.

Table 1: Projected Trip Generation

Land Use ¹	V : II KCE	Daily	A	M Pea	k	Р	M Peal	k
(ITE)	Variable KSF	Total	Total	In	Out	Total	In	Out
Variety Store (ITE 814)	10.640	677	32	18	14	71	36	35
Pass-by Rate:	= 34%*	230	11	6	5	24	12	12
Net Total Proje	ect Trips	447	21	12	9	47	24	23

Source: ITE Trip Generation 11th Edition

Conclusion: Approval of this application may generate **447** daily vehicle trips. This is not anticipated to negatively impact the adopted LOS for adjacent and nearby roadways.

^{*} The IT Trip Generation Manual, 11th Edition provides a pass-by rate of 34% during the PM peak but does not provide a pass-by rate for the AM and daily conditions, therefore, a pass-by rate of 34% is applied to the AM and daily scenarios.

Potable Water / Sanitary Sewer / Solid Waste

Table 2: Projected Potable Water Impacts

Land Use	Maximum Units	Gallons Per Day ¹	Estimated Demand (GPD)
Shopping center without food or laundry	10,640	.01 gallons / sq. ft. / day	1,064

^{1.} Source: Ch. 62E-6.008, Table 1, Florida Administrative Code

Conclusion: The project site will be served by the existing Lake City potable water infrastructure. The subject property is served by Lake City's potable water, and it's anticipated to generate 1,064 Gallons per day.

Table 3: Projected Sanitary Sewer Impacts

Land Use	Maximum Units	Gallons Per Day ¹	Estimated Demand (GPD)
Shopping center without food or laundry	10,640	.01 gallons / sq. ft. / day	1,064

^{1.} Source: Ch. 62E-6.008, Table 1, Florida Administrative Code

Conclusion: The project site will be served by the existing Lake City wastewater infrastructure. The subject property is served by Lake City's sanitary sewer, and it's anticipated to generate 1,064 Gallons per day.

Table 4: Projected Solid Waste Impacts

Land Use	Units	Solid Waste Generated (lbs/day) ¹	Solid Waste Generated (tons/year) ²
Nonresidential	10,640 sq. ft.	233.02	42.53

Formulas per Sincero and Sincero; Environmental Engineering: A Design Approach. Prentice Hall, New Jersey, 1996.
 a. Formula used, nonresidential: (((12 lbs. / 1,000 sq. ft./day * [10,640 sq. ft.]) * 365)/2,000)

Conclusion: Solid waste facility capacity exists to adequately serve the intended office development for the subject property. The subject property is served by Lake City's solid waste, and it's anticipated to generate 233.02 pounds per day and 42.53 tons per year.

Formula used, pounds per day to tons per year: ([lbs/day] * 0.005) * 365



SUMMARY REPORT OF A GEOTECHNICAL SITE EXPLORATION – REVISION 1

DOLLAR GENERAL – LAKE CITY SW MARVIN BURNETT LAKE CITY, COLUMBIA COUNTY, FLORIDA

GSE PROJECT NO. 16251

Prepared For:

CONCEPT DEVELOPMENT, INC.

DECEMBER 2023



December 7, 2023

Andrea Barnett Concept Development, Inc. 1449 SW 74th Drive, Suite 200 Gainesville, Florida 32607

Summary Report of a Geotechnical Site Exploration - Revision 1 Subject:

Dollar General - Lake City SW Marvin Burnett

Lake City, Columbia County, Florida

GSE Project No. 16251

GSE Engineering & Consulting, Inc. (GSE) is pleased to submit this geotechnical site exploration report for the above referenced project.

Presented herein are the findings and conclusions of our exploration, including the geotechnical parameters and recommendations to assist with building foundation, pavement, and stormwater management designs. This revision includes recommended soil parameters for stormwater management design with underdrains.

GSE appreciates this opportunity to have assisted you on this project. If you have any questions or comments concerning this report, please contact us.

Sincerely,

GSE Engineering & Consulting, Inc.

Jason E

Digitally signed by Jason E Gowland Date: 2023.12.07 15:39:40 -05'00'

on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

This item has been digitally signed and sealed by

Jason E. Gowland, P.E. Principal Engineer

Florida Registration No. 66467

Angelina X. Liu, E.I.

Staff Engineer

AXL / JEG: tlf Q:\Projects\16251 Dollar General - Lake City SW Marvin Burnett\16251 Rev.1.docx

Distribution: Addressee (1 - Electronic)

File (1)

GSE Engineering & Consulting, Inc. 5590 SW 64th Street, Suite B Gainesville, Florida 32608 (352) 377-3233 Phone • (352) 377-0335 Fax www.gseengineering.com

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- 1. Project Site Location Map
- 2. Site Plan Showing Approximate Locations of Field Tests

1.0 INTRODUCTION

1.1 General

GSE Engineering & Consulting, Inc. (GSE) has completed this geotechnical exploration for the proposed commercial retail store located on SW Marvin Burnett Road in Lake City, Columbia County, Florida. This exploration was performed in accordance with GSE Proposal No. 2023-589 dated September 12, 2023. Ms. Andrea Barnett authorized our services on September 15, 2023.

1.2 Project Description

We understand that you are coordinating due diligence related work related to the development of this site into a commercial retail store. The site is located on the northwest corner of the State Road 47 and SW Marvin Burnett Road intersection in Lake City, Columbia County, Florida. The site is approximately +/-2.72 acres.

You provided GSE with information about the project. We understand the project will consist of an approximate 10,640 square foot building, a parking lot, and a stormwater management facility.

The structure is expected to be a single-story, high wall concrete masonry unit (CMU) and steel frame construction. Structural loads have not been provided but are expected to be on the order of 1 to 2 kips per foot for non-load bearing CMU walls, and less than 50 kips for columns. The finished floor of the structure is anticipated to be constructed within 1 to 2 feet of the existing site grades.

The building will be located in the northern portion of the site. The parking lot will be located west, south, and east of the structure. The stormwater management facility will be located on the western portion of the site.

A recent aerial photograph of the site was obtained and reviewed. The site plan and aerial photograph were used in preparation of this exploration and report.

1.3 Purpose

The purpose of this geotechnical exploration was to determine the general subsurface conditions, evaluate these conditions with respect to the proposed construction, and prepare geotechnical parameters and recommendations to assist with building foundation, stormwater management, and pavement designs.

2.0 FIELD AND LABORATORY TESTS

2.1 General Description

The procedures used for field sampling and testing are in general accordance with industry standards of care and established geotechnical engineering practices for this geographic region. This exploration consisted of performing five (5) Standard Penetration Test (SPT) borings to a depth of 20 feet below land surface (bls) within the proposed building area, five (5) auger borings to a depth of 5 feet bls in the area of the parking lot and driveways, and five (5) auger borings to depths of 15 feet bls in the area of the stormwater management facility.

The soil borings were performed at the approximate locations as shown on Figure 2. The borings were located at the site using the provided site plan, Global Positioning System (GPS) coordinates, and obvious site features as reference. The boring locations should be considered approximate. The soil borings were performed on September 20, 2023.

2.2 Auger Borings

The auger borings were performed in accordance with ASTM D1452. The borings were performed with flight auger equipment that was rotated into the ground in a manner that reduces soil disturbance. After penetrating to the required depth, the auger was retracted and the soils collected on the auger flights were field classified and placed in sealed containers. Representative samples of each stratum were retained from the auger boring. Results from the auger borings are provided in Section 5.1.

2.3 Standard Penetration Test Borings

The soil borings were performed with a drill rig employing mud rotary drilling techniques and Standard Penetration Testing (SPT) in accordance with ASTM D1586. The SPTs were performed continuously to 10 feet and at 5-foot intervals thereafter. Soil samples were obtained at the depths where the SPTs were performed. The soil samples were classified in the field, placed in sealed containers, and returned to our laboratory for further evaluation.

After drilling to the sampling depth, the standard two-inch O.D. split-barrel sampler was seated by driving it 6 inches into the undisturbed soil. The sampler was then driven an additional 12 inches by blows of a 140-pound hammer falling 30 inches. The number of blows required to produce the next 12 inches of penetration were recorded as the penetration resistance (N-value). These values and the complete SPT boring logs are provided in Section 5.2.

Upon completion of the sampling, the boreholes were abandoned in accordance with Water Management District guidelines.

2.4 Soil Laboratory Tests

The soil samples recovered from the soil borings were returned to our laboratory, and examined to confirm the field descriptions. Representative samples were then selected for laboratory testing. The laboratory tests consisted of nine (9) percent soil fines passing the No. 200 sieve, nine (9) natural moisture content determinations, two (2) Atterberg Limits tests, and three (3) constant head hydraulic conductivity tests. These tests were performed in order to aid in classifying the soils and to further evaluate their engineering properties. The laboratory tests are provided in Section 5.3.

3.0 FINDINGS

3.1 Surface Conditions

Karen Roylos with GSE visited the site on September 18, 2023 to observe the site conditions and mark the boring locations. Mr. Jason Kite with Jason Kite, LLC was retained by GSE to clear lanes to allow access to the boring locations for drilling equipment.

The majority of the site is densely vegetated with trees, scattered saw palmettos, shrubs, vines and weedy groundcover. Portions of the site were densely vegetated and more difficult to traverse. To the south of the site is SW Marvin Burnett Road. State Road 47 is located east of the site. Undeveloped wooded land borders the site to the north and west.

The topography at the site is moderately sloping from northeast towards southwest. Regional topography can be characterized as gently to moderately sloping. The Lake City West USGS Topographic Map indicates the ground surface elevations at the site are near 155 to 165 feet NAVD 88.

3.2 Subsurface Conditions

The locations of the auger and SPT borings are provided on Figure 2. Complete logs for the borings are provided in Sections 5.1 and 5.2. Descriptions for the soils encountered are accompanied by the Unified Soil Classification System symbol (SM, SP-SM, etc.) and are based on visual examination of the recovered soil samples and the laboratory tests performed. Stratification boundaries between the soil types should be considered approximate, as the actual transition between soil types may be gradual.

The auger borings located within the proposed parking lot and driveways encountered relatively similar soil conditions. Auger borings A-1 to A-3 encountered poorly graded sand, and sand with silt (SP, SP-SM) to the explored depths of 5 feet bls. Auger borings A-4 and A-5 initially encountered sand with silt (SP-SM) to depths of 1.5 to 3.5 feet bls. This was underlain by clayey to very clayey sand (SC, SC/CL) to the explored depths of 5 feet bls.

The auger borings located within the stormwater management facility encountered relatively consistent soil conditions. Auger boring P-1 encountered 6 feet of silty sand, and poorly graded sand (SM, SP) overlying clayey to very clayey sand, and clay with sand (SC, SC/CL, CL/CH) to the explored depth of 15 feet bls. Auger borings P-2 to P-4 initially encountered poorly graded sand, sand with silt, and silty sand (SP, SP-SM, SM) to depths of 2 to 5 feet bls, overlying silty clayey sand, and clayey to very clayey sand (SM-SC, SC, SC/CL) to depths of 7 to 10.5 feet bls. This was underlain by sand with silt (SP-SM) to depths of 12 to 13.5 feet bls, followed by clayrich soils (CL/CH) to the explored depth of 15 feet bls. Auger boring P-5 initially encountered 5.5 feet of clayey sand (SC) and 5 feet of sand with silt (SP-SM) overlying clay with sand (CL/CH) to a depth of 12.5 feet bls. This was underlain by sand with silt (SP-SM) to the explored depth of 15 feet bls.

.

¹ United States Geological Survey, Lake City West Quadrangle, 2021.

The SPT borings located within the proposed building footprint indicate the soils across these areas are relatively consistent. SPT boring B-1 initially encountered 3 feet of sand with silt (SP-SM), and 4.5 feet of sandy clay (CL) overlying sand with clay, and poorly graded sand (SP-SC, SP) to a depth of 12 feet bls. This was underlain by clay (CL/CH) to the explored depth of 20 feet bls. SPT borings B-2 to B-5 encountered poorly graded sand, sand with silt, sand with clay, silty sand, and silty clayey sand (SP, SP-SM, SP-SC, SM-SC) with some interbedded layers of clayey to very clayey sand (SC, SC/CL) to depths of 13.5 to 17.5 feet bls. This was underlain by clay-rich (CL, CL/CH) soils to the explored depths of 20 feet bls.

The sandy soils (SP, SP-SM, SP-SC) encountered are generally in a very loose to dense condition with N-values ranging from 2 to 45 blows per foot. The silty sand, silty clayey sand, and clayey to very clayey sands (SM, SM-SC, SC, SC/CL) encountered are generally in a very loose to dense condition with N-values ranging from 4 to 38 blows per foot. The sandy clay, clay with sand, and clay (CL/CH, CL) encountered are generally in a very soft to hard condition with N-values ranging from 3 to 33 blows per foot.

Weight-of-rod strength material was encountered in SPT boring B-2 at depth range from 13.5 to 14.5 feet bls. This isolated occurrence is likely related to depositional characteristics of the soil materials and transitions between material types.

The groundwater table was encountered in the auger and SPT borings at depths of 6.1 to 8.8 feet bls at the time of our investigation.

3.3 Review of Published Data

The majority of the site is mapped as three soil series by the Soil Conservation Service (SCS) Soil Survey for Columbia County². The following soil descriptions are from the Soil Survey.

Blanton fine sand, 0 to 5 percent slopes

Map Unit Setting

- National map unit symbol: 2w0q2
- Elevation: 30 to 200 feet
- Mean annual precipitation: 51 to 59 inches
- Mean annual air temperature: 64 to 72 degrees F
- Frost-free period: 258 to 310 days
- Farmland classification: Not prime farmland

Map Unit Composition

- Blanton and similar soils: 85 percent
- Minor components: 15 percent
- Estimates are based on observations, descriptions, and transects of the map unit.

² Soil Survey of Hamilton County, Florida. Soil Conservation Service, U.S. Department of Agriculture.

Description of Blanton

Setting

- Landform: Knolls on marine terraces, ridges on marine terraces
- Landform position (two-dimensional): Backslope
- Landform position (three-dimensional): Side slope, interfluve, riser
- Down-slope shape: Convex
- Across-slope shape: Linear
- Parent material: Sandy and loamy marine deposits

Typical profile

- A 0 to 7 inches: fine sand
- E-7 to 52 inches: fine sand
- Bt 52 to 80 inches: fine sandy loam

Properties and qualities

- Slope: 0 to 5 percent
- Depth to restrictive feature: More than 80 inches
- Drainage class: Moderately well drained
- Runoff class: Negligible
- Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.20 to 6.00 in/hr)
- Depth to water table: About 42 to 72 inches
- Frequency of flooding: None
- Frequency of ponding: None
- Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
- Sodium adsorption ratio, maximum: 4.0
- Available water supply, 0 to 60 inches: Low (about 3.6 inches)

Interpretive groups

- Land capability classification (irrigated): None specified
- Land capability classification (nonirrigated): 3s
- Hydrologic Soil Group: A
- Forage suitability group: Sandy soils on rises, knolls, and ridges of mesic uplands (G138XA121FL)
- Other vegetative classification: Sandy soils on rises, knolls, and ridges of mesic uplands (G138XA121FL)
- Hydric soil rating: No

Minor Components

Albany

- Percent of map unit: 6 percent
- Landform: Ridges on marine terraces
- Landform position (two-dimensional): Shoulder
- Landform position (three-dimensional): Interfluve, talf
- Down-slope shape: Convex
- Across-slope shape: Linear
- Other vegetative classification: Sandy soils on rises and knolls of mesic uplands (G138XA131FL), North Florida Flatwoods (R138XY004FL)
- Hydric soil rating: No

Troup

- Percent of map unit: 4 percent
- Landform: Ridges, knolls
- Landform position (two-dimensional): Summit
- Landform position (three-dimensional): Interfluve
- Down-slope shape: Convex
- Across-slope shape: Linear
- Other vegetative classification: Sandy soils on ridges and dunes of xeric uplands (G133AA111FL), Longleaf Pine-Turkey Oak Hills (R133AY002FL)
- Hydric soil rating: No

Chipley

- Percent of map unit: 3 percent
- Landform: Knolls on marine terraces, rises on marine terraces, flats on marine terraces
- Landform position (two-dimensional): Shoulder, footslope
- Landform position (three-dimensional): Interfluve
- Down-slope shape: Convex
- Across-slope shape: Linear
- Other vegetative classification: Sandy soils on rises and knolls of mesic uplands (G138XA131FL)
- Hydric soil rating: No

Alpin

- Percent of map unit: 2 percent
- Landform: Flatwoods on marine terraces, knolls on marine terraces, ridges on marine terraces
- Landform position (two-dimensional): Shoulder, backslope
- Landform position (three-dimensional): Interfluve
- Down-slope shape: Convex
- Across-slope shape: Linear
- Other vegetative classification: Sandy soils on ridges and dunes of xeric uplands (G138XA111FL), Sand Pine Scrub (R153AY001FL)
- *Hydric soil rating:* No

Ichetucknee fine sand, 5 to 8 percent slopes

Map Unit Setting

- National map unit symbol: vrt4
- Elevation: 330 to 660 feet
- Mean annual precipitation: 50 to 58 inches
- Mean annual air temperature: 64 to 72 degrees F
- Frost-free period: 258 to 288 days
- Farmland classification: Not prime farmland

Map Unit Composition

- Ichetucknee and similar soils: 80 percent
- Minor components: 20 percent
- Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Ichetucknee

Setting

- Landform: Hills on marine terraces, ridges on marine terraces
- Landform position (three-dimensional): Interfluve, side slope
- Down-slope shape: Convex
- Across-slope shape: Linear
- Parent material: Sandy and clayey marine deposits over limestone

Typical profile

- A 0 to 4 inches: fine sand
- E-4 to 7 inches: fine sand
- Bg 7 to 75 inches: clay
- 2R 75 to 79 inches: weathered bedrock

Properties and qualities

- Slope: 5 to 8 percent
- Depth to restrictive feature: 50 to 75 inches to lithic bedrock
- Drainage class: Somewhat poorly drained
- Runoff class: Negligible
- Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
- Depth to water table: About 18 to 36 inches
- Frequency of flooding: None
- Frequency of ponding: None
- Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
- Sodium adsorption ratio, maximum: 4.0
- Available water supply, 0 to 60 inches: Moderate (about 8.4 inches)

Interpretive groups

- Land capability classification (irrigated): None specified
- Land capability classification (nonirrigated): 6e
- Hydrologic Soil Group: D
- Forage suitability group: Loamy and clayey soils on rises, knolls, and ridges of mesic uplands (G138XA322FL)
- Other vegetative classification: Loamy and clayey soils on rises, knolls, and ridges of mesic uplands (G138XA322FL)
- Hydric soil rating: No

Minor Components

Goldsboro

- Percent of map unit: 10 percent
- Landform: Knolls on marine terraces, ridges on marine terraces
- Landform position (three-dimensional): Interfluve
- Down-slope shape: Convex
- Across-slope shape: Linear
- Other vegetative classification: Loamy and clayey soils on flats and rises of mesic lowlands (G138XA331FL)
- Hydric soil rating: No

Ocilla

- Percent of map unit: 10 percent
- Landform: Rises on marine terraces
- Landform position (three-dimensional): Interfluve
- Down-slope shape: Convex
- Across-slope shape: Linear
- Other vegetative classification: Sandy over loamy soils on rises and knolls of mesic uplands (G138XA231FL)
- Hydric soil rating: No

Pelham fine sand, 0 to 2 percent slopes

Map Unit Setting

- National map unit symbol: 2tg56
- Elevation: 0 to 190 feet
- Mean annual precipitation: 48 to 63 inches
- Mean annual air temperature: 57 to 79 degrees F
- Frost-free period: 251 to 293 days
- Farmland classification: Not prime farmland

Map Unit Composition

- Pelham and similar soils: 75 percent
- Minor components: 25 percent
- Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Pelham

Setting

- Landform: Flatwoods
- Landform position (three-dimensional): Talf
- Down-slope shape: Linear
- Across-slope shape: Linear
- Parent material: Sandy and loamy marine deposits

Typical profile

- A 0 to 6 inches: fine sand
- Eg 6 to 26 inches: fine sand
- Btg1 26 to 42 inches: sandy clay loam
- Btg2 42 to 83 inches: sandy clay loam

Properties and qualities

- Slope: 0 to 2 percent
- Depth to restrictive feature: More than 80 inches
- Drainage class: Poorly drained
- Runoff class: High
- Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.20 to 5.95 in/hr)
- Depth to water table: About 6 to 12 inches
- Frequency of flooding: None
- Frequency of ponding: None
- Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
- Sodium adsorption ratio, maximum: 4.0
- Available water supply, 0 to 60 inches: Moderate (about 7.0 inches)

Interpretive groups

- Land capability classification (irrigated): None specified
- Land capability classification (nonirrigated): 3w
- Hydrologic Soil Group: B/D
- Ecological site: F153AY060NC Wet Loamy Flats and Depressions
- Forage suitability group: Sandy over loamy soils on flats of hydric or mesic lowlands (G153AA241FL)
- Other vegetative classification: Sandy over loamy soils on flats of hydric or mesic lowlands (G153AA241FL)
- Hvdric soil rating: No

Minor Components

Unnamed

- Percent of map unit: 13 percent
- Landform: Flatwoods
- Landform position (three-dimensional): Talf
- Down-slope shape: Linear
- Across-slope shape: Linear
- Other vegetative classification: Sandy over loamy soils on flats of hydric or mesic lowlands (G153AA241FL)
- Hydric soil rating: Yes

Albany

- Percent of map unit: 6 percent
- Landform: Flatwoods
- Landform position (three-dimensional): Talf
- Microfeatures of landform position: Rises
- Down-slope shape: Convex
- Across-slope shape: Convex
- Ecological site: F153AY040NC Moist Loamy Rises and Flats
- Other vegetative classification: Sandy soils on rises and knolls of mesic uplands (G153AA131FL)
- Hydric soil rating: No

Meggett

- Percent of map unit: 3 percent
- Landform: Flatwoods
- Landform position (three-dimensional): Talf
- Down-slope shape: Linear
- Across-slope shape: Linear
- Ecological site: F153AY090NC Flooded Mineral Soil Floodplains and Terraces
- Other vegetative classification: Loamy and clayey soils on flats of hydric or mesic lowlands (G153AA341FL)
- Hydric soil rating: Yes

Surrency

- Percent of map unit: 3 percent
- Landform: Drainageways, depressions
- Landform position (three-dimensional): Dip
- Down-slope shape: Linear, concave
- Across-slope shape: Convex, concave
- Ecological site: F153AY060NC Wet Loamy Flats and Depressions
- Other vegetative classification: Sandy over loamy soils on stream terraces, flood plains, or in depressions (G153AA245FL)
- Hydric soil rating: Yes

3.4 Laboratory Soil Analysis

Selected soil samples recovered from the soil borings were analyzed for the percent soil fines passing the No. 200 sieve, natural moisture content, Atterberg Limits, and hydraulic conductivity. Samples selected for laboratory testing were collected at depths ranging from near-surface to 15 feet bls. These tests were performed to confirm visual soil classification and evaluate their engineering properties. The complete laboratory report is provided in Section 5.3.

The laboratory tests indicate the tested soils consist sand with silt, silty sand, silty sand with clay, sand with clay, clayey sand, very clayey sand, and sandy clay. The tested sand with silt (SP-SM) contains approximately 11 percent soil fines passing the No. 200 sieve with a natural moisture content of about 8.7 percent. The tested silty sand, and silty sand with clay (SM, SM-SC) contains approximately 14 to 27 percent soil fines passing the No. 200 sieve with natural moisture contents of about 7.8 to 18 percent. The tested sand with clay (SP-SC) contains approximately 11 percent soil fines passing the No. 200 sieve with a natural moisture content of about 17 percent. The tested clayey sand (SC) contains approximately 30 percent soil fines passing the No. 200 sieve with a natural moisture content of about 13 percent. The tested very clayey sand (SC/CL) contains approximately 34 percent soil fines passing the No. 200 sieve with a natural moisture content of about 18 percent. The tested sandy clay (CL) contains approximately 56 to 62 percent soil fines passing the No. 200 sieve with natural moisture contents of about 17 to 23 percent.

Atterberg Limits tests indicate the tested sandy clay (CL) has Liquid Limit (LL) values of 35 and 41, Plastic Limit (PL) values of 15 to 18, and Plasticity Index (PI) values of 17 and 26. These values correspond to materials with low potential (LL < 50) to marginal potential (PI \leq 35) for expansive behavior³.

The constant head hydraulic conductivity test results indicate the near-surface silty sand (SM) has hydraulic conductivity values of 0.8 to 1.1 feet per day. The tested clayey sand (SC) has no flow. Tests were not conducted on the deeper very clayey sand due to the limitations of the test method on soils having moderate to high fines content, but these soils are expected to have permeability values at least one order of magnitude lower than the sandy soils.

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³ U.S. Department of the Army USA, 1983, Foundations in Expansive Soils, TM 5-818-7, p. 4-1.

4.0 EVALUATION AND RECOMMENDATIONS

4.1 General

The following recommendations are made based upon our understanding of the proposed construction, a review of the attached soil borings and laboratory test data, and experience with similar projects and subsurface conditions. If plans or the location of proposed construction changes from those discussed previously, GSE requests the opportunity to review and possibly amend our recommendations with respect to those changes.

The final design of a foundation system is dependent upon adequate integration of geotechnical and structural engineering considerations. Consequently, GSE must review the final foundation design in order to evaluate the effectiveness and applicability of our initial analyses, and to determine if additional recommendations may be warranted. Without such a review, the recommendations presented herein could be misinterpreted or misapplied resulting in potentially unacceptable performance of the foundation system.

The performance of site improvements may be sensitive to their post-construction relationship to site groundwater levels, seepage zones, or soil/rock characteristics exposed at final site grades. GSE recommends that use of boring information for final design of all site improvements be predicated on proper horizontal and vertical control of borings.

In this section of the report, we present our geotechnical parameters and recommendations to assist with building foundation, stormwater management, and pavement designs as well as our general site preparation guidelines.

4.2 Groundwater

The groundwater table was encountered in the borings at depths of 6.1 to 8.8 feet bls at the time of our exploration. The Soil Survey indicates the groundwater table is typically at a depth of near-surface to 6 feet bls. We anticipate the seasonal high groundwater table will be near depths of 1 to 3.5 feet bls. Estimates for the seasonal high groundwater table are shown on the individual boring logs.

4.3 Building Foundations

The SPT borings located within the proposed building footprint indicate the soils across these areas are relatively consistent. SPT boring B-1 initially encountered 3 feet of sand with silt (SP-SM), and 4.5 feet of sandy clay (CL) overlying sand with clay, and poorly graded sand (SP-SC, SP) to a depth of 12 feet bls. This was underlain by clay (CL/CH) to the explored depth of 20 feet bls. SPT borings B-2 to B-5 encountered poorly graded sand, sand with silt, sand with clay, silty sand, and silty clayey sand (SP, SP-SM, SP-SC, SM-SC) with some interbedded layers of clayey to very clayey sand (SC, SC/CL) to depths of 13.5 to 17.5 feet bls. This was underlain by clay-rich (CL, CL/CH) soils to the explored depths of 20 feet bls.

Based upon the soil conditions encountered and our limited understanding of the structural loads and site grading, we recommend the building be supported by conventional, shallow strip and/or spread foundations. We recommend the shallow foundations be designed for a maximum allowable gross bearing pressure of 2,500 psf. The gross bearing pressure is defined as the soil contact pressure that can be imposed from the maximum structural loads, weight of the concrete foundations, and weight of the soil above the foundations. The foundations should be designed based upon the maximum load that could be imposed by all loading conditions.

The foundations should be embedded a minimum of 18 inches below the lowest adjacent grade. Interior foundations or thickened sections should be embedded a minimum of 12 inches. The foundations should have minimum widths of 18 inches for strip footings, and 24 inches for columns, even though the maximum soil bearing pressure may not be fully developed.

Due to the mostly sandy nature of the majority of the near-surface soils, we expect settlement to be mostly elastic in nature. The majority of the settlement will occur on application of the loads, during and immediately following construction. Using the recommended maximum bearing pressure, the assumed maximum structural loads, and the field and laboratory test data which we have correlated into the strength and compressibility characteristics of the subsurface soils, we estimate the total settlements of the structure to be 1 inch or less, with approximately half of it occurring upon load application (during construction).

Differential settlement results from differences in applied bearing pressures and the variations in the compressibility characteristics of the subsurface soils. For the building pad prepared as recommended, we anticipate differential settlement of less than 1/2 inch.

Post-construction settlement of the structures will be influenced by several interrelated factors, such as (1) subsurface stratification and strength/compressibility characteristics of the bearing soils; (2) footing size, bearing level, applied loads, and resulting bearing pressures beneath the foundation; (3) site preparation and earthwork construction techniques used by the contractor, and (4) external factors, including but not limited to vibration from off-site sources and groundwater fluctuations beyond those normally anticipated for the naturally-occurring site and soil conditions which are present.

Our settlement estimates for the structure are based upon our limited understanding of the structural loads and site grading and the use of successful adherence to the site preparation recommendations presented later in this report. Any deviation from our project understanding and/or our site preparation recommendations could result in an increase in the estimated post-construction settlement of the structure.

4.4 Flexible Pavement

Overall soil conditions encountered by our borings at this site are suitable for supporting conventional limerock base and asphalt wearing surface pavements. We have not been provided the anticipated traffic loading conditions; therefore, the following pavement component recommendations should be used only as guidelines. The below recommendations are intended to be minimums. Increasing base course and asphalt thicknesses would increase the design life of the pavement.

The seasonal high groundwater table is estimated to be approximately 12 inches to about 3.5 feet beneath existing grade across the site. We recommend a minimum of either 12 to 24 inches of separation (depending upon the pavement section design) be present between the bottom of the base course and the estimated seasonal high groundwater table. If this separation cannot be achieved by site grading, GSE recommends underdrains be used beneath the base course.

4.4.1 Stabilized Subgrade

If a crushed limerock or recycled concrete base is used, we recommend a stabilized subgrade be located beneath the base. The stabilized subgrade should have a minimum Limerock Bearing Ratio (LBR) of 40, with minimum thicknesses of 6 inches for automobile parking areas and 12 inches for driveways.

The stabilized subgrade can be imported material or a mixture of imported and on-site material. If a mix is proposed, a mix design should be performed to determine the optimum mix proportions. The stabilized subgrade should be compacted to a minimum of 98 percent of the Modified Proctor maximum dry density (ASTM D1557) for soils with less than 15 percent fines content. Soils with 15 percent or greater fines content should be compacted to 100 percent of the Standard Proctor maximum dry density (ASTM D698).

4.4.2 Base Course

The base course can consist of either crushed limerock, soil cement, or recycled concrete. If you should use a soil cement base course, a stabilized subgrade is not required.

Limerock should have an LBR of at least 100, be obtained from a FDOT approved source and meet FDOT gradation requirements. The base course thickness should be a minimum of 6 inches in automobile parking areas, and 8 inches in driveway areas. The base course should be compacted to at least 98 percent of the Modified Proctor maximum dry density (ASTM D1557). We recommend a minimum 24 inches separation between the bottom of the limerock base course and the estimated seasonal high-water table. If site grading does not allow for this separation, we recommend underdrains be considered.

Soil cement can consist of an imported material or a blend of the on-site soils and cement. A mix design should be performed to determine the optimum cement content. We recommend the soil cement have a minimum 28-day compressive strength of 500 psi. Soil cement can be blended off-site (in a pug mill) or on site. Soil cement pills should be cast from each day's production to verify the recommended compressive strength has been achieved at 28 days. We recommend the soil cement base course be a minimum of 8 inches thick throughout the project. We recommend a minimum 18 inches separation between the bottom of the soil cement base course and the estimated seasonal high-water table. If site grading does not allow for this separation, we recommend underdrains be considered.

Recycled concrete should have an LBR of at least 150, be obtained from a FDOT approved source and meet FDOT gradation requirements. The base course thickness should be a minimum of 8 inches. The base course should be compacted to at least 98 percent of the Modified Proctor maximum dry density (ASTM D1557). We recommend a minimum 12 inches separation between the bottom of the recycled concrete base course and the estimated seasonal high-water table. If site grading does not allow for this separation, we recommend underdrains be considered.

4.4.3 Wearing Surface

The asphalt-wearing surface should consist of an FDOT Type SP Hot Mix Asphalt mixture. For automobile parking areas, the thickness should be a minimum of 1.5 inches. For driveway areas, the thickness should be a minimum of 2 inches. The asphalt-wearing surface should consist of an SP-12.5 mix. The asphalt should be compacted to at least 95 percent of the mix design density.

The constructability of differing asphalt thicknesses may be difficult, and having a uniform 2-inch thick asphalt wearing surface may be more practical.

4.5 Rigid Pavement

Concrete pavement is a rigid pavement that results in smaller load transfers to the subgrade soils than flexible pavement. For concrete pavement subgrade, we recommend using the existing surficial sands or recommended clean sand (SP) fill, compacted to at least 98 percent of the Modified Proctor maximum dry density without additional stabilization with the following stipulations:

- 1. Subgrade soils must be compacted to at least 98 percent of Modified Proctor maximum dry density to a depth of at least 2 feet prior to placement of concrete.
- 2. The surface of the subgrade soils must be smooth and any disturbances or wheel rutting corrected prior to placement of the concrete.
- 3. The subgrade soils must be moistened prior to placement of concrete.
- 4. Concrete pavement thickness should be uniform throughout, with the exception of thickened edges (curb or footing).
- 5. The bottom of the pavement should be separated from the estimated seasonal high groundwater level by at least 18 inches.
- 6. Limerock or any other impermeable base is not suitable unless it meets the minimum recommended permeability of 10 ft/day.
- 7. The upper 12 inches of subgrade underlying the base course must also be "free-draining" and water that enters the base and subgrade must be allowed to seep out by gravity or if this is not possible, underdrains must be incorporated into the subgrade. A "bathtub" condition within the base/subgrade must be avoided.

Our recommendations for slab thickness for both light-duty and heavy-duty concrete pavements is based on a.) subgrade soils are compacted to 98 percent of the Modified Proctor maximum dry density, b.) modulus of subgrade reaction (k) of 200 pounds per cubic inch, c.) a 20-year design life, and d.) previously stated design parameters. For an anticipated light-duty traffic group, a minimum pavement thickness of 5.5 inches is recommended, using Table 2.4 from the ACI 330 Guide for Design and Construction of Concrete Parking Lots, ACI 330R-01. For an anticipated heavy-duty traffic group, a minimum pavement thickness of 8 inches is recommended, using Table 3.4 from the FDOT *Rigid Pavement Design Manual*, January 2019.

We recommend using concrete with a minimum 28-day compressive strength of 4,000 pounds per square inch and a minimum 28-day flexural strength (modulus of rupture) of at least 600 pounds per square inch based on the third point loading of concrete beam test samples. Maximum control joint spacing of 12.5 by 12.5 feet is suggested for light-duty concrete pavements. Maximum control joint spacing of 15 by 15 feet is suggested for heavy-duty concrete pavements. Layout of sawcut control joints should form square panels, and the depth of sawcut joint should be at least 1/4 of the concrete slab thickness. The joints should be sawed within six hours of concrete placement or as soon as the concrete has developed sufficient strength to support workers and equipment.

For further details on concrete pavement construction, refer to "Guide to Jointing Non-reinforced Concrete Pavements" published by the Florida Concrete and Products Associates, Inc. and "Building Quality Concrete Parking Areas", published by the Portland Cement Association.

4.6 Site Preparation

The soils at this site should be suitable for supporting the proposed construction using normal, good practice site preparation procedures. The following recommendations are our general guidelines for site preparation.

4.6.1 Stripping

Strip the construction limits and 10 feet beyond the perimeter of all grass, roots, topsoil, and other deleterious materials. You should expect to strip to depths of 12 or more inches. Deeper stripping will likely be necessary due to major root systems present at the site.

4.6.2 Dewatering

Temporary dewatering may be necessary for this project. If needed, we anticipate dewatering can be accomplished with sumps placed near the construction area, or with underdrains connected to a vacuum pump.

In any case, the site should always be graded to promote runoff and limit the amount of ponding. Localized ponding of stormwater is expected without proper grading during construction, and could render previously acceptable surfaces unacceptable.

4.6.3 Proof-Rolling

Proof-roll the subgrade with heavy rubber-tired equipment, such as a loaded front-end loader or dump truck, to identify any loose or soft zones not found by the soil borings. The proof-rolling should be monitored by a geotechnical engineer or qualified technician. Undercut or otherwise treat these zones as recommended by the geotechnical engineer in this report.

4.6.4 Proof Compaction

Compact the subgrade to a density of at least 95 percent of the Modified Proctor maximum dry density (ASTM D1557). The specified compaction should be obtained to a depth of 1 foot below the foundation bottoms and the existing grade prior to placing fill. Vibratory roller equipment should not be used within approximately 100 feet of existing structures. Lighter "walk-behind" compaction equipment may be used to achieve the degree of compaction.

Should clayey sand be encountered at the bearing surface, this material should be probed and visually confirmed to be unyielding in the upper 12 inches in lieu of density testing. If the foundation excavations penetrate the clayey sand, the excavation should be performed in a manner that reduces soil disturbance. Clayey sand soils (with fines content in excess of 15 percent) that are removed and replaced or appreciably disturbed need to be re-compacted to 98 percent of the Standard Proctor maximum dry density (ASTM D698).

4.6.5 Fill Placement

Imported fill placed to raise the site grades should consist of clean sand having less than 10 percent passing the No. 200 sieve. On-site soils meeting the requirements of Section 4.9 may also be used as structural fill. The fill should be placed in maximum 12-inch loose lifts that are compacted to at least 95 percent of the Modified Proctor maximum dry density (ASTM D1557). If lighter "walk-behind" compaction equipment is used, this may require lifts of 4 inches or less to achieve the required degree of compaction.

4.7 Quality Control and Construction Materials Testing

It should be noted that the geotechnical engineering design does not end with the advertisement of the construction documents. As the geotechnical engineer of record, GSE is the most qualified to perform the construction materials testing that will be required for this project. The benefits of having the geotechnical engineer of record also perform the construction materials testing are numerous. If GSE continues to be involved with the project through construction, we will be able to constantly re-evaluate and possibly alter our geotechnical recommendations in a timely and cost effective manner once final design and construction techniques are developed. This often results in cost savings for the project.

We recommend performing compaction testing beneath the concrete floor slab and the building foundations. We recommend one test be performed every 50 linear feet of continuous footing and every other column footing, per foot depth of fill or native material. We recommend a compaction test be performed for each 2,500 square feet of floor area or 10,000 square feet of pavement area per foot of fill or native material, or a minimum of three tests each, whichever is greater. Test all footing excavations to a depth of 12 inches at the frequencies stated above.

4.8 Stormwater Management

The auger borings located within the stormwater management facility encountered relatively consistent soil conditions. Auger boring P-1 encountered 6 feet of silty sand, and poorly graded sand (SM, SP) overlying clayey to very clayey sand, and clay with sand (SC, SC/CL, CL/CH) to the explored depth of 15 feet bls. Auger borings P-2 to P-4 initially encountered poorly graded sand, sand with silt, and silty sand (SP, SP-SM, SM) to depths of 2 to 5 feet bls, overlying silty clayey sand, and clayey to very clayey sand (SM-SC, SC, SC/CL) to depths of 7 to 10.5 feet bls. This was underlain by sand with silt (SP-SM) to depths of 12 to 13.5 feet bls, followed by clayrich soils (CL/CH) to the explored depth of 15 feet bls. Auger boring P-5 initially encountered 5.5 feet of clayey sand (SC) and 5 feet of sand with silt (SP-SM) overlying clay with sand (CL/CH) to a depth of 12.5 feet bls. This was underlain by sand with silt (SP-SM) to the explored depth of 15 feet bls.

The water table was encountered in the auger borings at depths of 7.5 to 8.8 feet bls at the time of our exploration. We anticipate the seasonal high groundwater table to be at depths of 1 to 2.5 feet bls.

The laboratory permeability tests indicate the surficial layers of silty sand (SM) has hydraulic conductivity values of 0.8 to 1.1 feet per day, and clayey sand (SC) has no flow. The deeper very clayey sand encountered below the surficial sandy soils is friable and will have permeability values at least one order of magnitude lower than the sandy soils. The underlying dense soils and clayrich soils are expected to be confining soils.

Mr. Cole Menhennett with CHW confirmed the proposed stormwater management facility as a dry pond via email. We understand that the current design will consider underdrains. We understand that imported clean sand will be used for the backfill for the underdrains. This revision includes soil parameters considering and underdrain design with clean sand backfill.

Based upon our findings and test results, our recommended soil parameters for the stormwater management design in the explored areas are presented below. The recommended parameters consider the results of the permeability tests, wash 200 determinations, and our experience with these types of soils. The parameters below do not consider a factor of safety.

Proposed Stormwater Management Facility

- 1. Base elevation of effective or mobilized aquifer (average depth of confining layer) equal to 8 feet bls.
- 2. Unsaturated vertical infiltration rate of 10 foot per day.
- 3. Horizontal hydraulic conductivity equal to 10 feet per day.
- 4. Specific yield (fillable porosity) of 20 percent.
- 5. Average seasonal high groundwater table depth equal to 2 feet bls.
- 6. Average seasonal low groundwater table depth equal to 6 feet bls.

In areas where clay-rich soils are present at the basin bottom, we recommend these soils be undercut a minimum of 2 feet and backfilled with the on-site sands and sands with silt (SP, SP-SM) having a maximum of 12 percent soil fines passing the No. 200 sieve. This fill should also be used above the bottom of the underdrains. The intent of this undercutting and replacement is to provide a more uniform sand "blanket" at the basin bottom that allows the migration of water to the underdrains. This sand blanket will also reduce the potential for clay-fines leaching out of the soils when water is present in the basin that can result in a thin layer of confining type material on the basin bottom that can reduce the effectiveness of the basin.

4.9 Fill Suitability

The soils encountered at this site within the explored depths range from sands (SP) to clays (CL/CH). A discussion of the suitability for reuse as structural fill for each soil classification according to the Unified Soil Classification System (USCS) designation is provided below.

SP, SP/SM – Sands (SP) and sand with silt (SP/SM) have less than 5 percent and 12 percent soil fines passing the No. 200 sieve, respectively, and are typically well draining soils that are suitable for reuse as structural fill. The sands with silt may require moisture conditioning (drying) to make the material more workable. These soils will require stockpiling and drying before they are reused if they are excavated from below the water table.

SM – Silty sands (SM) can have between 12 percent and 50 percent soil fines passing the No. 200 sieve. Silty sands are typically non-plastic or have low plasticity, and can be reused as structural fill with precautions. Silty sands can be moisture sensitive and difficult to work and compact and can rut if the moisture content is near or above the optimum moisture content. We recommend these soils be moisture conditioned (dried) so that the moisture content during use is at or below the optimum moisture content. Aerating and exposure to the sun is typically the most effective methods of drying these soils. It may not be practical to reuse these materials during the wet season, as frequent rain showers may not allow these soils to dry to a workable moisture content. Suitable silty sands are limited to soil having less than 30 percent soil fines passing the No. 200 sieve. Silty sands with more than 30 percent soil fines are especially moisture sensitive, and are not recommended for reuse as structural fill. These soils will behave more as sandy silt, and for this reason, very silty sands having more than 30 percent soil fines passing the No. 200 sieve have been assigned a dual classification of SM/ML. Silty sand soils that are excavated from below the water table are not recommended for reuse as structural fill due to the amount of time that will be required to dry these soils to a workable condition.

SC - Clayey sand (SC) soils can have between 12 percent and 50 percent soil fines passing the No. 200 sieve. Clayey sands can have a high range of plasticity, varying from a PI of 7 or greater and plotting above the A-line to highly plastic. Friable clayey sands are typically suitable for use as structural fill with precautions. Clayey sands will be moisture sensitive and difficult to work and compact and can rut during placement if the moisture content is near or above the natural moisture content. We recommend these soils be moisture conditioned (dried) so that the moisture content during use is at or below the optimum moisture content. Aerating and exposure to the sun is typically the most effective methods of drying these soils. It may not be practical to reuse these materials during the wet season, as frequent rain showers may not allow these soils to dry to a workable moisture content. Suitable clayey sands are limited to soil having less than 30 percent soil fines passing the No. 200 sieve. Clayey sands with more than 30 percent soil fines passing the No. 200 sieve are especially moisture sensitive and are typically highly plastic, and are not recommended for reuse as structural fill. These soils will behave more as sandy clay, and for this reason, very clayey sands having more than 30 percent soil fines passing the No. 200 sieve have been assigned a dual classification of SC/CH or SC/CL. Clayey sand soils that are excavated from below the water table are not recommended for reuse as structural fill due to the amount of time that will be required to dry these soils to a workable condition.

ML, MH, CL, CH - Silts and clays are not suitable materials for reuse as structural fill.

When using on-site soils as fill materials, we recommend the silty and clayey sand soils (SM, SC) be used in the lower depths of the fill. Sand and sand with silt (SP, SP-SM) should be used in the upper portions of the fill. We recommend a minimum of 2 feet of sand (SP, SP-SM) cover the silty and clayey sand fill materials to reduce the potential for soggy surface conditions due to the low permeability characteristics of the silty and clayey sand materials.

4.10 Surface Water Control and Landscaping

Roof gutters should be considered to divert runoff away from the building. The gutter downspouts should discharge a minimum of 10 feet from the structure to reduce the amount of water collecting around the foundations. Where possible, the gutter downspouts should discharge directly into the storm sewer system or onto the asphalt paved areas in order to reduce the amount of water collecting around the foundations. Grading of the site should be such that water is diverted away from the building on all sides to reduce the potential for erosion and water infiltration along the foundation.

With respect to landscaping, it is recommended that any trees and large "tree-like" shrubbery with potential for developing large root systems be planted a minimum distance of half their mature height, and preferably their expected final height, away from the structure. The purpose of this is to reduce the potential for foundation or slab movements from the growth of root systems as the landscaping matures.

5.0 FIELD DATA

5.1 Auger Boring Logs



AB 2 PORTRAIT CPT - GINT STD US.GDT - 10/11/23 09:53 - P./GENERAL/PROJECTS/16251 DOLLAR GENERAL – LAKE CITY SW MARVIN BURNETT/16251 BORINGS/16251 BORINGS.GPJ

GSE Engineering 5590 SW 64th St Gainesville, FL 32608

Telephone: 3523773233 Engineering & Consulting, Inc. PROJECT NAME Dollar General - Lake City SW Marvin Burnett CLIENT Concept Development, Inc. PROJECT LOCATION Lake City, Columbia County, Florida PROJECT NUMBER 16251 DATE PERFORMED 9/20/2023 BORING NUMBER A-2 DATE PERFORMED 9/20/2023 BORING NUMBER A-1 DRILLING CONTRACTOR Whitaker Drilling, Inc. DRILLING CONTRACTOR Whitaker Drilling, Inc. GROUND WATER LEVELS: LOGGED BY WDI **GROUND WATER LEVELS:** LOGGED BY WDI AT TIME OF DRILLING NE AT TIME OF DRILLING NE_ CHECKED BY AXL CHECKED BY AXL ☐ ESTIMATED SEASONAL HIGH 3.5 ft ☐ ESTIMATED SEASONAL HIGH 3.5 ft NOTES SAMPLE TYPE NUMBER (kg/cm²) CPT (kg/cm²) MATERIAL DESCRIPTION MATERIAL DESCRIPTION CPT ((SP-SM) Dark brown and gray SAND (SP) Brown and gray SAND with silt %PASS-200 = 11 MC = 8.72.0 (SP) Pale gray and brown SAND 2,5 (SP) Pale brown SAND ∇ 立 5.0 5 5.0 Bottom of borehole at 5.0 feet. Bottom of borehole at 5.0 feet.



GSE Engineering 5590 SW 64th St Gainesville, FL 32608 Telephone: 3523773233

PROJECT NAME Dollar General - Lake City SW Marvin Burnett CLIENT Concept Development, Inc. PROJECT LOCATION Lake City, Columbia County, Florida PROJECT NUMBER 16251 DATE PERFORMED 9/20/2023 BORING NUMBER A-4 DATE PERFORMED 9/20/2023 BORING NUMBER A-3 DRILLING CONTRACTOR Whitaker Drilling, Inc. DRILLING CONTRACTOR Whitaker Drilling, Inc. LOGGED BY WDI **GROUND WATER LEVELS:** LOGGED BY WDI **GROUND WATER LEVELS:** AT TIME OF DRILLING NE CHECKED BY AXL AT TIME OF DRILLING NE CHECKED BY AXL ☐ ESTIMATED SEASONAL HIGH 3.0 ft ☐ ESTIMATED SEASONAL HIGH 3.5 ft NOTES SAMPLE TYPE NUMBER CPT (kg/cm²) (kg/cm²) MATERIAL DESCRIPTION MATERIAL DESCRIPTION AB 2 PORTRAIT CPT - GINT STD US.GDT - 10/11/23 09:53 - P./GENERAL/PROJECTS/16251 DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT/16251 BORINGS/16251 BORINGS.GPJ CPT (SP-SM) Brown and gray SAND with silt (SP-SM) Brown and gray SAND with silt (SC/CL) Brown, gray, and orange very clayey SAND (SP) Pale gray and brown SAND %PASS-200 = 34 MC = 18 ∇ 5.0 5.0 Bottom of borehole at 5.0 feet. Bottom of borehole at 5.0 feet.



GSE Engineering 5590 SW 64th St

Gainesville, FL 32608 Telephone: 3523773233 PROJECT NAME Dollar General - Lake City SW Marvin Burnett CLIENT Concept Development, Inc. PROJECT LOCATION Lake City, Columbia County, Florida PROJECT NUMBER 16251 DATE PERFORMED 9/20/2023 BORING NUMBER A-5 DRILLING CONTRACTOR Whitaker Drilling, Inc. GROUND WATER LEVELS: LOGGED BY WDI X AT TIME OF DRILLING <u>NE</u> CHECKED BY AXL ☐ ESTIMATED SEASONAL HIGH 3.0 ft SAMPLE TYPE NUMBER CPT (kg/cm²) MATERIAL DESCRIPTION AB 2 PORTRAIT CPT - GINT STD US.GDT - 10/11/23 09:53 - P./GENERAL/PROJECTS/16251 DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT/16251 BORINGS/16251 BORINGS.GPJ (SP-SM) Brown and gray SAND with silt ∇ 3.5 (SC) Brown and gray clayey SAND 5.0 Bottom of borehole at 5.0 feet.



GSE Engineering 5590 SW 64th St Gainesville, FL 32608 Telephone: 3523773233

PROJECT NAME Dollar General - Lake City SW Marvin Burnett CLIENT Concept Development, Inc. PROJECT LOCATION Lake City, Columbia County, Florida PROJECT NUMBER <u>16251</u> DATE PERFORMED 9/20/2023 BORING NUMBER P-2 DATE PERFORMED 9/20/2023 BORING NUMBER P-1 DRILLING CONTRACTOR Whitaker Drilling, Inc. DRILLING CONTRACTOR Whitaker Drilling, Inc. **GROUND WATER LEVELS:** LOGGED BY WDI GROUND WATER LEVELS: LOGGED BY WDI CHECKED BY AXL AT TIME OF DRILLING 7.8 ft CHECKED BY AXL AT TIME OF DRILLING 7.5 ft ☐ ESTIMATED SEASONAL HIGH 2.5 ft ☑ ESTIMATED SEASONAL HIGH 2.5 ft NOTES NOTES SAMPLE TYPE NUMBER CPT (kg/cm²) (kg/cm²) (ft) GRAPHIC MATERIAL DESCRIPTION MATERIAL DESCRIPTION AB 2 PORTRAIT CPT - GINT STD US.GDT - 10/11/23 09:53 - P://GENERAL/PROJECTS/16251 DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT/16251 BORINGS/16251 BORINGS.GP. CPT (0.0 (SP) Brown and gray SAND (SM) Gray and brown silty SAND %PASS-200 = 14 2.5 $\bar{\Delta}$ MC = 7.8 $k_h = 1.1 \text{ ft/day}$ 3,5 (SC) Brown and orange clayey SAND 4.0 (SP) Brown and gray SAND with trace of 5.0 6.0 (SC/CL) Pale gray and brown very clayey SAND 6.5 (SM-SC) Gray, brown, and orange silty clayey SAND ¥ \mathbf{Y} 10.0 10,0 (SP-SM) Pale gray and brown SAND with 12.0 (CL/CH) Gray CLAY with sand 12.5 13.5 (CL/CH) Pale gray and brown CLAY with (SC) Brown and orange clayey SAND 15.0 15.0 15.0 15.0 Bottom of borehole at 15.0 feet. Bottom of borehole at 15.0 feet. (Continued Next Page)

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GSE Engineering 5590 SW 64th St Gainesville, FL 32608

Telephone: 3523773233 CLIENT Concept Development, Inc. PROJECT NAME Dollar General - Lake City SW Marvin Burnett PROJECT NUMBER 16251 PROJECT LOCATION Lake City, Columbia County, Florida DATE PERFORMED 9/20/2023 BORING NUMBER P-3 DATE PERFORMED 9/20/2023 BORING NUMBER P-4 DRILLING CONTRACTOR Whitaker Drilling, Inc. DRILLING CONTRACTOR Whitaker Drilling, Inc. **GROUND WATER LEVELS:** LOGGED BY WDI GROUND WATER LEVELS: LOGGED BY WDI AT TIME OF DRILLING 8.8 ft CHECKED BY AXL AT TIME OF DRILLING 7.5 ft CHECKED BY AXL ✓ ESTIMATED SEASONAL HIGH 2.5 ft ☑ ESTIMATED SEASONAL HIGH 1.5 ft NOTES NOTES SAMPLE TYPE NUMBER (kg/cm²) CPT (kg/cm² (ft) GRAPHIC MATERIAL DESCRIPTION MATERIAL DESCRIPTION AB 2 PORTRAIT CPT - GINT STD US.GDT - 10/11/23 09:53 - PXGENERAL/PROJECTS\16251 DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT\16251 BORINGS\16251 BORINGS\16251 BORINGS\16351 F. C DEP CPT (SP-SM) Brown and gray SAND with silt (SM) Brown silty SAND %PASS-200 = 15 ☑ MC = 9.7 $k_h = 0.8 \text{ ft/day}$ (SC) Brown, gray, and orange clayey 2.5 亙 5.0 5.0 (SC) Brown, gray, and orange clayey 7.0 (SP-SM) Pale gray and pale brown 7.5 SAND with silt (SC/CL) Brown, gray, and orange very clayey SAND Y 10.0 (SP-SM) Pale brown and pale gray SAND with silt 12.0 (CL/CH) Green CLAY with sand 12.5 12.5 (CL/CH) Brown and gray CLAY with sand 15.0 15.0 15.0 Bottom of borehole at 15.0 feet. Bottom of borehole at 15.0 feet.



GSE Engineering 5590 SW 64th St Gainesville, FL 32608 Telephone: 3523773233

CLIENT Concept Development, Inc. PROJECT NUMBER 16251

PROJECT NAME Dollar General - Lake City SW Marvin Burnett

PROJECT LOCATION Lake City, Columbia County, Florida

DATE PERFORMED 9/20/2023 BORING NUMBER P-5 DRILLING CONTRACTOR Whitaker Drilling, Inc. GROUND WATER LEVELS: LOGGED BY WDI

▼ AT TIME OF DRILLING 8.8 ft CHECKED BY AXL

☐ ESTIMATED SEASONAL HIGH 1.0 ft

NOTES ___

	140				=::
O DEPTH	GRAPHIC LOG	SAMPLE TYPE NUMBER	CPT (kg/cm²)	MATERIAL DESCRIPTION	
0.0	///			(SC) Brown and gray clayey SAND	
2.5				∑	
2		1 4		%PASS-200 = 30	
5.0		AP 1		$MC = 13$ $k_h = NF$	
					5.5
7.5		AU 2		(SP-SM) Brown, gray, and orange SAND with silt ▼	
		AU		(CL/CH) Gray and brown CLAY with	10.5
12.5		3		sand	12.5
		AU 4		(SP-SM) Brown and orange SAND with silt	
15.0				Pottom of horobolo at 15 0 foot	15.0
				Bottom of borehole at 15.0 feet.	

5.2 Standard Penetration Test Soil Boring Logs

CO	
UTD.	
Engineering & Const	ulting, Inc.

GSE Engineering 5590 SW 64th St Gainesville, FL 32608 Telephone: 352377323

BORING NUMBER B-1

		Telephone: 3523773233							.0307				
CLIEN	NT _Co	oncept Development, Inc.			NAME Dolla								
		UMBER 16251			OCATION								
		TED 9/20/23 COMPLETED 9/20/23		OUND E	LEVATION				HOL	E SIZ	E		
		ONTRACTOR Whitaker Drilling, Inc.			ATER LEV								
		ETHOD Flight Auger			ME OF DRI								
LOGG	ED BY	WDI CHECKED BY AXL	6	¥ ESTI	MATED SE	ASON	AL HI	GH _3	3.5 ft	_			
NOTE	s		_			,		_					
DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	CONTACT DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	LIQUID LIMIT, %	PLASTIC LIMIT, %	PLASTICITY INDEX	PERCENT PASS NO. 200 SIEVE	MOISTURE CONTENT, %		PT N VA 40 6	
0		(SP-SM) Very loose brown SAND with silt	Т								:		
			22	SPT 1	1-1-1 (2)					3	\		
-			3	SPT 2	1-2-3 (5)						1		
5				SPT 3	4-6-9 (15)	35	18	17	56	17	1	ıÐ.	
		Ţ	100	SPT 4	7-10-11 (21)						1		
_		(SP-SC) Medium dense brown, gray, and orange SAND with clay	7.5	SPT 5	8-11-12 (23)						1	92	
10		(SP) Medium dense pale gray and brown SAND	9.5	SPT 6	9-10-14 (24)						1	126 3 5 9	
i :												<u>ų.</u>	
S 12		(CL/CH) Firm to stiff green and orange CLAY	12										
15				SPT 7	3-4-5 (9)								
										8		gran	
1 E				SPT	2-3-4								
	1		20	8	(7)								
20	11 11												



GSE Engineering 5590 SW 64th St Gainesville, FL 32608 Telephone: 3523773233

BORING NUMBER B-2

			nsulting, Inc.													
	CLIEN	IT Co	ncept Development, Inc.	PR	OJE	CT N	AME Dolla	ar Gen	eral -	Lake	City S	W Ma	arvin Bu	rnett		
		_	UMBER 16251		_	_	OCATION						11.			
	DATE	STAR	TED <u>9/20/23</u> COMPLETED <u>9/20/23</u>	GR	OU	ND EI	LEVATION				HOL	E SIZ	E,		_	
	DRILL	ING C	ONTRACTOR Whitaker Drilling, Inc.				ATER LEV									
	DRILL	ING M	ETHOD Flight Auger		₩,	AT TI!	ME OF DRI	LLING	6.1	ft						
	LOGG	ED BY	WDI CHECKED BY AXL	e :	Vι	STIN	IATED SEA	ASON	AL HI	GH_3	3.5 ft					
	NOTE	s														
	O DEPTH	GRAPHIC LOG	MATERIAL DESCRIPTION	CONTACT DEPTH (ft)	SAMPLE TYPE	NUMBER	BLOW COUNTS (N VALUE)	LIQUID LIMIT, %	PLASTIC LIMIT, %	PLASTICITY INDEX	PERCENT PASS NO. 200 SIEVE	MOISTURE CONTENT, %		PT N \		IE ▲ 80
GP.			(SP-SM) Very loose gray and brown SAND with silt										*	:		
SING.						-								5		
S\16251 BOF	-				X	SPT 1	1-2-2 (4)					8	^	88.7		
251 BORING				3	X	SPT 2	1-2-2 (4)					15	†			
URNETT/162	5				X	SPT 3	2-4-6 (10)						<u>}</u>			
W MARVIN B	2		Ť		X	SPT 4	7-9-8 (17)						}			
AKE CITY SI	_			8,5	X	SPT 5	7-8-10 (18)						+	8		
GENERAL - L	10		(SP-SC) Very loose to medium dense pale gray and brown SAND with clay		X	SPT 6	7-9-10 (19)						<u> </u>			
BORINGS - GINT STD US.GDT - 10/11/23 09:54 - P./GENERAL/PROJECTS/16251 DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT/16251 BORINGS/16251 BORINGS.GPJ														E		
(GENERAL)			Weight-of-Rod from 13.5 to 14.5 ft bls.	14.5	Y	SPT 7	0-0-3 (3)						1	10		
4	15		(CL/CH) Soft gray sandy CLAY			-							-	- 17	<u>:</u>	
3 09:£		11											1 :		:	
1112	- d			16.5									1		:	:
10,10	l, j		(CL/CH) Firm green and orange CLAY		1									19	:	:
8		1											1			
Sing														:	:	
T ST		11				_								10	:	:
<u>S</u>				8	V	SPT	3-3-4							15	:	
NGS	20			20	A	8	(7)							12		
BOR	_20	11 1	Bottom of borehole at 20.0 feet.												:	1
PP														į.	<u>:</u>	

GSE	
CIDE	
Engineering & Consulting, Inc.	

GSE Engineering 5590 SW 64th St Gainesville, FL 32608

BORING NUMBER B-3

96

Engin	eering & Co	Telephone: 3523773233										
CLIE	NT _C	oncept Development, Inc.	_ PR	OJECT	NAME Dolla	ar Ger	neral -	Lake	City S	SW Ma	arvin Burnett	
-		UMBER _16251			LOCATION							
		TED 9/20/23 COMPLETED 9/20/23							HOL	.E SIZ	E	
		ONTRACTOR Whitaker Drilling, Inc.			VATER LEV			. 4				
		ETHOD Flight Auger										
		Y WDI CHECKED BY AXL	_	<u>*</u> E3111	MIATED 3E	HOON	AL III	GH_S).J IC			
1401												
O DEPTH	GRAPHIC	MATERIAL DESCRIPTION	CONTACT DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	LIQUID LIMIT, %	PLASTIC LIMIT, %	PLASTICITY INDEX	PERCENT PASS NO. 200 SIEVE	MOISTURE CONTENT, %	▲ SPT N VAI	
GP.		(SP-SM) Very loose gray and brown SAND with silt									10.	
SPT BORINGS - GINT STD US.GDT - 10/11/23 09:54 - P:\GENERALPROJECTS\16251 DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT\16251 BORINGS\16251 BORING\16251 BORINGS\16251 BORINGS\16251 BORINGS\16251 BORINGS\16251 BORINGS\16251 BORINGS\16251 BORI	=		2.5	SPT 1	1-1-1 (2)						N	el Je
51 BORINGS		(SP) Loose pale gray SAND ☑		SPT 2	1-2-4 (6)						À	ii
JRNETT/162		(SP-SC) Loose to gray and brown SAND with clay	4.5	SPT 3	2-4-5 (9)						*	
/ MARVIN BL		(SC/CL) Medium dense to dense gray, brown, and orange very clayey SAND	6	SPT 4	6-2-9 (11)						l d	
AKE CITY SV				SPT 5	7-9-11 (20)							
GENERAL - 10				SPT 6	14-16-22 (38))	
251 DOLLAR												
ROJECTS/16												
ZALIP!		(OL) Firm annual conduction	13.5									:
- P:/GENER		(CL) Firm gray sandy CLAY		SPT 7	2-3-3 (6)	41	15	26	62	23	1	
0/11/23 09:5		(CL/CH) Green and orange CLAY	16									23
JS.GDT - 1												
T STD (
N C	1											
20			20									
PT 80		Bottom of borehole at 20.0 feet.										4
U)						-	-					

CICIT	
GSE	
Engineering & Consulting, Inc	

GSE Engineering 5590 SW 64th St Gainesville, FL 32608

BORING NUMBER B-4

97

	-	ing & Co.	Gainesville, FL 32608 Telephone: 3523773233										
			ncept Development, Inc.	PR	OJEC	T N	AME Dolla	r Gen	eral -	Lake	City 5	SW Ma	arvin Burnett
- 1			UMBER 16251		OJEC	T LO	OCATION	Lake	City	Colur	mbia (Count	/, Florida
- 1-			TED 9/20/23 COMPLETED 9/20/23	GR	OUN	D EL	EVATION				HOI	E SIZ	E
1	DRILL	ING C	ONTRACTOR Whitaker Drilling, Inc.	GR	OUN	D W	ATER LEV	ELS:					
1	DRILL	ING M	ETHOD Flight Auger	. 9	▼ A1	T TIN	ME OF DRI	LLING	6.5	ft			
1	LOGG	ED BY	WDI CHECKED BY AXL			MITE	ATED SEA	SON	AL HI	GH_3	3.5 ft		
	NOTE	s											
İ								%	%		o		
	DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	CONTACT DEPTH (#)	SAMPLE TYPE	NICE.	BLOW COUNTS (N VALUE)	ПООІР LІМІТ,	PLASTIC LIMIT,	PLASTICITY INDEX	PERCENT PASS NO. 200 SIEVE	MOISTURE CONTENT, %	▲ SPT N VALUE ▲
	0	R		SH	SAMI		"ÖŚ	IIQUI	PLAST	~	PERC NO.	SOM	20 40 60 80
50.67			(SP-SM) Very loose gray and brown SAND with silt				_						
TIDZDI BORINGS.GP.						PT 1	1-1-2 (3)						1
SOI BURINGS ID				3		PT 2	4-7-11 (18))
	5			19		PT 3	7-5-6 (11)						A
ARE CILL SW MARVIN BURINELLING			▼ (SM-SC) Loose to medium dense gray, brown, and orange silty SAND with clay	6		PT 4	3-4-5 (9)						\
TANE CITY				19		PT 5	5-7-14 (21)				27	18	1
GENERAL	10		(SC) gray and brown clayey SAND	9		PT 6	12-10-9 (19)						A
CISHOZSI DULLAR													
2025			(SP-SC) Medium dense gray, brown, and orange SAND	13									
- P. WEINERAL PROJECT S11623	15		with clay	10	S	PT 7	4-9-12 (21)						1
- 1011173 08:04 -	10			17									
GINT STD US.6DT - 10/11/23 09:34			(CL/CH) Firm green and gray sandy CLAY		V								
5 - conii	20			20		PT 8	3-3-4 (7)						A .
20	_		Bottom of borehole at 20.0 feet.										

COL	
UDL	
Engineering & Consulting, Inc.	

SPT BORINGS - GINT STD US.GDT - 10/11/23 09:54 - P./GENERAL/PROJECTS/16251 DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT/16251 BORINGS/16251 BORINGS.GPJ

GSE Engineering 5590 SW 64th St Gainesville, FL 32608

BORING NUMBER B-5

Enginee	ring & Co	onsulting, Inc. Telephone: 3523773233									
		oncept Development, Inc.	PR	OJECT	NAME Dolla	ar Gen	ieral -	Lake	City S	SW Ma	arvin Burnett
		IUMBER _16251		OJECT	LOCATION	Lake	City,	Colur	nbia (County	, Florida
DATE	STAR	RTED 9/20/23 COMPLETED 9/20/23	GR	OUND E	LEVATION				HOL	E SIZ	E
DRILL	ING C	CONTRACTOR Whitaker Drilling, Inc.	GR	OUND V	VATER LEV	ELS:					
DRILL	ING N	METHOD Flight Auger									
LOGG	ED B	Y WDI CHECKED BY AXL	+7	☑ ESTI	MATED SEA	ASON	AL HI	GH 3	3.5 ft		
NOTE	s										
O DEPTH	GRAPHIC LOG	MATERIAL DESCRIPTION	CONTACT DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	LIQUID LIMIT, %	PLASTIC LIMIT, %	PLASTICITY INDEX	PERCENT PASS NO. 200 SIEVE	MOISTURE CONTENT, %	▲ SPT N VALUE ▲ 20 40 60 80
		(SP-SM) Very loose brown and gray SAND with silt									B
				SPT 1	1-1-2						1
5 6	111		3	SPT 2	4-7-8 (15)						À
5				SPT 3	10-11-13 (24)						\
- n-		_		SPT 4	10-8-9 (17)						4
	- 1/2	(SP-SC) Medium dense to dense brown and orange	8	SPT 5	7-8-11 (19)						
10		SAND with clay	10	SPT 6	17-21-24 (45)				11	17	\
10	1//	(SP) Medium dense pale brown and gray SAND	10								
15			O	SPT 7	5-7-9 (16)						
		(CL/CH) Hard pale gray sandy CLAY	17.5	SPT	8-14-19						
20		Bottom of borehole at 20.0 feet.	20	8	(33)						-

5.3 Laboratory Results



SUMMARY REPORT OF LABORATORY TEST RESULTS

16251 Project Number: Dollar General - Lake City SW Marvin Burnett Project Name:

Unified Soil Classification	SP-SM	sc/cr	ซ	CL	SM-SC	SP-SC	SM	SM	SC
Hydraulic Conductivity (ft/day)							1.1	0.8	NF
Organic Content (%)									
Percent Passing No. 200 Sieve	11	34	95	62	27	11	14	15	30
Plasticity			17	26					
Plastic Limit			18	15					
Liquid Limit			35	41					
Natural Moisture Content (%)	8.7	18	17	23	18	17	7.8	9.7	13
Soil Description	Dark brown and gray SAND with silt	Brown, gray, and orange very clayey SAND	Brown, gray, and orange sandy CLAY	Gray sandy CLAY	Gray, brown, and orange silty SAND with clay	Pale brown and gray SAND with clay	Gray and brown silty SAND	Brown silty SAND	Brown and gray clayey SAND
Depth (ft)	1-1.5	3-3.5	4-5.5	13.5-15	7-8.5	8.5-10	2-4	0-5	3-5
Boring	A-2	A-4	B-1	B-3	B-4	B-5	P-1	P-3	P-5

5.4 Key to Soil Classification

KEY TO SOIL CLASSIFICATION CHART

				SYM	BOLS	GROUP NAME
Criteria for	r Assigning Group Symbol	s and Group Names U	sing Laboratory Tests	GRAPHIC	LETTER	GROUP NAME
COARSE-GRAINED SOILS	Gravels	Clean Gravels	$Cu \ge 4$ and $1 \le Cc \le 3$	1	GW	Well graded GRAVEL
More than 50% retained	More than 50% of coarse	Less than 5% fines	Cu < 4 and/or 1 > Cc > 3	30.20	GP	Poorly graded GRAVEI
on No. 200 sieve	fraction retained on No. 4 sieve	Gravels with fines	Fines classify as ML or MH	X & Y	GM	Silty GRAVEL
	sieve	More than 12% fines	Fines classify as CL or CH	92	GC	Clayey GRAVEL
	Sands	Clean Sands	$Cu \ge 6$ and $1 \le Cc \le 3$		SW	Well graded SAND
	50% or more of coarse	Less than 5% fines	Cu < 6 and/or 1 > Cc > 3	Septimieneerings Luxus Selleys	SP	Poorly graded SAND
	fraction passes No. 4 sieve	Sand with fines	Fines classify as ML or MH		SP-SM	SAND with silt
		5% ≤ fines < 12%	Fines classify as CL or CH		SP-SC	SAND with clay
		Sand with fines	Fines classify as ML or MH		SM	Silty SAND
		12% ≤ fines < 30%	Fines classify as CL or CH		SC	Clayey SAND
		Sand with fines	Fines classify as ML or MH		SM	Very silty SAND
		30% fines or more	Fines classify as CL or CH		SC	Very clayey SAND
FINE-GRAINED SOILS	Clays	inorganic	50% ≤ fines < 70%		CL/CH	Sandy CLAY
50% or more passes the	•		70% ≤ fines < 85%		CL/CH	CLAY with sand
No. 200 sieve			fines ≥ 85%		CL/CH	CLAY
	Silts and Clays	inorganic	PI > 7 and plots on/above "A" line		CL	Lean CLAY
	Liquid Limit less than 50		PI < 4 or plots below "A" line		ML	SILT
	•	organic	Liquid Limit - oven dried		OT	Organic clay
			< 0.75 Liquid Limit - not dried		OL	Organic silt
	Silts and Clays	inorganic	PI plots on or above "A" line		СН	Fat CLAY
	Liquid Limit 50 or more	-	PI plots below "A" line		MH	Elastic SILT
	•	organic	Liquid Limit - oven dried		OII	Organic clay
		-	< 0.75 Liquid Limit - not dried		ОН	Organic silt
HIGHLY ORGANIC SOILS	Primaril	y organic matter, dark in	color, and organic odor	24 24 24 24	PT	PEAT

CORRELATION OF PENETRATION RESISTANCE WITH RELATIVE DENSITY AND CONSISTENCY

	No. OF BLOWS, N	RELATIVE DENSITY		No. OF BLOWS, N	CONSISTENCY
l	0 - 4	Very Loose		0 - 2	Very Soft
l	5 - 10	Loose	SILTS	3 - 4	Soft
SANDS:	11 - 30	Medium dense	&	5 - 8	Firm
	31 - 50	Dense	CLAYS:	9 - 15	Stiff
1	OVER 50	Very Dense		16 - 30	Very Stiff
		•		31 - 50	Hard
	No. OF BLOWS, N	RELATIVE DENSITY		OVER 50	Very Hard
l .	0.8	Very Soft			

 0 - 8
 Very Soft

 9 - 18
 Soft

 19 - 32
 Moderately Hard

 33 - 50
 Hard

 OVER 50
 Very Hard



Location of SPT Sample

SAMPLE GRAPHIC TYPE LEGEND

LABORATORY TEST LEGEND



Location of Auger Sample

PARTICLE SIZE IDENTIFICATION

LIMESTONE:

1	BOULDERS	S:	Greater than 300 mm			
	COBBLES:		75 mm to 300 mm	LL	=	Liquid Limit, %
ı	GRAVEL:	Coarse -	19.0 mm to 75 mm	PL	=	Plastic Limit, %
١		Fine -	4.75 mm to 19.0 mm	PI	=	Plasticity Index, %
١	SANDS:	Coarse -	2.00 mm to 4.75 mm	% PASS - 2	= 000	Percent Passing the No. 200 Sieve
١		Medium -	0.425 mm to 2.00 mm	MC	=	Moisture Content, %
١		Fine -	0.075 mm to 0.425 mm	ORG	=	Organic Content, %
SILTS & CLAYS:		AYS:	Less than 0.075 mm	k _h	=	Horizontal Hydraulic Conductivity, ft/day

6.0 LIMITATIONS

6.1 Warranty

This report has been prepared for our client for their exclusive use, in accordance with generally accepted soil and foundation engineering practices, and makes no other warranty either expressed or implied as to the professional advice provided in the report.

6.2 Auger and SPT Borings

The determination of soil type and conditions was performed from the ground surface to the maximum depth of the borings, only. Any changes in subsurface conditions that occur between or below the borings would not have been detected or reflected in this report.

Soil classifications that were made in the field are based upon identifiable textural changes, color changes, changes in composition or changes in resistance to penetration in the intervals from which the samples were collected. Abrupt changes in soil type, as reflected in boring logs and/or cross sections may not actually occur, but instead, be transitional.

Depth to the water table is based upon observations made during the performance of the auger and SPT borings. This depth is an estimate and does not reflect the annual variations that would be expected in this area due to fluctuations in rainfall and rates of evapotranspiration.

6.3 Site Figures

The measurements used for the preparation of the figures in this report were made using the provided site plan and by estimating distances from existing structures and site features. Figures in this report were not prepared by a licensed land surveyor and should not be interpreted as such.

6.4 Unanticipated Soil Conditions

The analysis and recommendations submitted in this report are based upon the data obtained from soil borings performed at the locations indicated on Figure 2. This report does not reflect any variations that may occur between these borings.

The nature and extent of variations between borings may not become known until excavation begins. If variations appear, we may have to re-evaluate our recommendations after performing on-site observations and noting the characteristics of any variations.

6.5 Misinterpretation of Soil Engineering Report

GSE Engineering & Consulting, Inc. is responsible for the conclusions and opinions contained within this report based upon the data relating only to the specific project and location discussed herein. If others make the conclusions or recommendations based upon the data presented, those conclusions or recommendations are not the responsibility of GSE.

FIGURES





DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT LAKE CITY, COLUMBIA COUNTY, FLORIDA GSE PROJECT NO. 16251

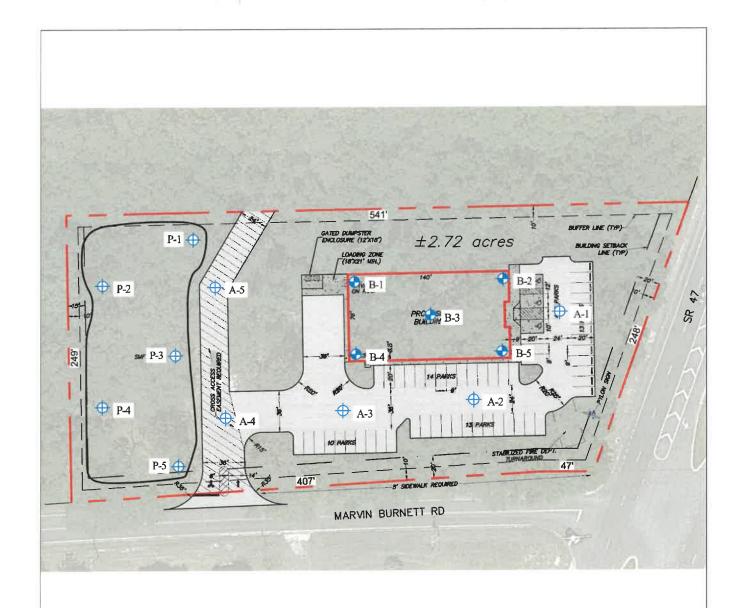
PROJECT SITE LOCATION MAP

DESIGNED BY: AXL CHECKED BY: JEG DRAWN BY: EEW



FIGURE

1



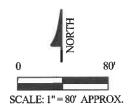
LEGEND:



SPT BORING



AUGER BORING



DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT LAKE CITY, COLUMBIA COUNTY, FLORIDA GSE PROJECT NO. 15396 SITE PLAN SHOWING APPROXIMATE LOCATIONS OF FIELD TESTS

DESIGNED BY: AXL CHECKED BY: JEG

DRAWN BY : AXL



FIGURE

2

LEGAL DESCRIPTION

23-0653



DATE: OCTOBER 25, 2023

PROJECT NAME: DG LAKE CITY ALTA

PROJECT NO: 23-0653

DESCRIPTION FOR: PARCEL A

A TRACT OF LAND BEING A PORTION OF LANDS AS DESCRIBED IN OFFICIAL RECORDS BOOK 1330 PAGE 1324 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA, SITUATED IN THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 7, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA, AND BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCE AT THE SOUTHWEST CORNER OF AFOREMENTIONED NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 7, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA; THENCE NORTH 02°08'35" WEST, ALONG WEST LINE OF SAID NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER, A DISTANCE OF 50.00 FEET TO THE NORTHERLY RIGHT OF WAY LINE OF S.W. MARVIN BURNETT ROAD (RIGHT OF WAY WIDTH VARIES); THENCE DEPARTING SAID WEST LINE. NORTH 85°44'05" EAST, ALONG SAID NORTHERLY RIGHT OF WAY LINE, A DISTANCE OF 130.73 FEET TO THE POINT OF BEGINNING; THENCE NORTH 02°10'29" WEST, A DISTANCE OF 249.00 FEET: THENCE NORTH 88°10'08" EAST, A DISTANCE OF 541.00 FEET TO THE WESTERLY RIGHT OF WAY LINE OF S.W. STATE ROAD NO. 47 (RIGHT OF WAY WIDTH VARIES); THENCE SOUTH 18°32'28" WEST, ALONG SAID WESTERLY RIGHT OF WAY LINE, A DISTANCE OF 248.00 FEET; THENCE, CONTINUE ALONG SAID WESTERLY RIGHT OF WAY LINE, SOUTH 89°06'47" WEST. A DISTANCE OF 46.52 FEET TO THE INTERSECTION OF SAID WESTERLY RIGHT OF WAY LINE AND AFOREMENTIONED NORTHERLY RIGHT OF WAY LINE OF S.W. MARVIN BURNETT ROAD; THENCE SOUTH 85°44'05" WEST. ALONG SAID NORTHERLY RIGHT OF WAY LINE A DISTANCE OF 407.00 FEET TO THE POINT OF BEGINNING.

SAID TRACT OF LAND CONTAINING 2.70 ACRES, MORE OR LESS.

PROPERTY OWNER AFFIDAVIT **Owner Mailing Address: Property Owner Name:** 13820 W. Newberry Road, Suite 100 St. Johns Limited Liability Company Newberry, FL 32669 Owner phone: Owner email: **Property County:** Columbia Parcel ID #: 07-4S-17-08127-005 Agent: Concept Development, Inc. and Concept Construction of North Florida, Inc. 1449 SW 74th Drive, Suite 200 Gainesville, FL 32607 (352) 333-3233 **Authorized Actions of Agent:** Design and submission of documentation, forms and plans and application for all permits as required from those regulatory agencies having jurisdiction over the Property (e.g. County, City, Water Management District, FDOT, FDEP, etc.) and on-site access for inspections, testing, data collection, I hereby certify that I am the owner of record. I hereby authorize the above listed agents to act on my behalf for the purposes of any and all applications and securing the above requested actions. St. Johns Limited Liability Company, a Florida limited liability company William B. Martin its: Manager STATE OF FLORIDA COUNTY OF HELACKUM The foregoing instrument was acknowledged before me this 3000 day of June, 2023, by William. B. Martin, as Manager of St. Johns Limited Liability Company, a Florida limited liability company, on behalf of said company who is

Page 15 of 15

personally known to me or has produced

JAMES D. SALTER Commission # HH 253573

Expires May 30, 2026

as identification.

Notary Public, State of Florida at Large

This instrument prepared by and after recording return to:

John C. Bovay, Attorney at Law 901 N.W. 57th Street Gainesville, Florida 32605

Inst:2005003121 Date:02/09/2005 Time:15:12

Doc Stamp-Deed:

0.70

DC, P. DeWitt Cason, Columbia County B: 1037 P: 1953

07-4S-

Property Appraiser's Parcel Identification Number(s)

WARRANTY DEED

The Grantor, William B. Martin, as Trustee of the William B. Martin Trust, dated July 31, 1990, in consideration of Ten and grants and conveys to the Grantee, St. Johns, LLC (a Florida limited liability company), whose mailing address is 2841 NW 41st Street, Gainesville, Florida 32606, the real property in Columbia County, Florida, described as follows:

The South ½ of the Northeast ¼ of the Northeast ¼ of Section 7, Township 4 South, Range 17 East as lies West of S.R. No. 47; LESS AND EXCEPT: Begin at the Southwest Corner of the Northeast ¼ of the Northeast ¼ of said Section 7, and run North along West Line of said Northeast ¼ of the Northeast ¼ 211 feet; thence run East 337 feet; thence run South 211 feet; thence run West 337 feet to the Point of Beginning.

The Grantor warrants that the property is free of all encumbrances, except the lien for real estate taxes not yet due and payable and restrictions, reservations, and easements of record, and that lawful seisin of and good right to convey the property are vested in the Grantor. The Grantor hereby fully warrants the title to the property and will defend the same against the lawful claims of all persons.

This deed was prepared without examination of title or legal opinion, but upon information, including the legal description and the ownership interest, supplied by the Grantor.

The interest conveyed is not the homestead of the Grantor.

Doc Stamp-Deed : DC,P.Dewitt Cason,Columbia County B:1037 P:1954 Signed on February $\frac{4}{2}$, 2005. Signed in the presence of: William B. Martin, as Trustee of the William B. Martin Trust, dated July 31, 1990 2841 NW 41st Street Gainesville, Florida 32606 Two witnesses as to William B. Martin, as Trustee of the William B. Martin Trust, dated July 31, 1990 STATE OF FLORIDA COUNTY OF ALACHUA by William B. Martin. Notary Public-State of Florida Personally Known ____ Produced Identification Print Notary Name: My Commission Number is: My Commission Expires: Type of Identification

Inst:2005003121 Date:02/09/2005 Time:15:12

JOHN C. BOVAY
Cummil 000309817
Expires 872/2008
Sended thru (800)432-4254
Floride Notary Asen., Inc.



GROWTH MANAGEMENT DEPARTMENT 205 North Marion Ave, Lake City, FL 32055

Phone: 386-719-5750 E-mail: growthmanagement@lcfla.com

AGENT AUTHORIZATION FORM

I,MatthewCason	(owner name), owner of property parcel
number 07-48-17-08127-005	(parcel number), do certify that
the below referenced person(s) listed on this for is an officer of the corporation; or, partner as do said person(s) is/are authorized to sign, speak relating to this parcel.	
Printed Name of Person Authorized	Signature of Authorized Person
1. Randall Olney (CHW)	1. 1/2/
2.	2.
3.	3.
4.	4.
5.	5.
I, the owner, realize that I am responsible for all with, and I am fully responsible for compliance Development Regulations pertaining to this partification and time the person(s) you have authorized officer(s), you must notify this department in what authorization form, which will supersede all prefunauthorized persons to use your name and/or	with all Florida Statutes, City Codes, and Land cel. d is/are no longer agents, employee(s), or iting of the changes and submit a new letter of vious lists. Failure to do so may allow
Owner Signature (Notarized)	Date
NOTARY INFORMATION: STATE OF: Florida COUNTY OF	Alachua
The above person, whose name is	
Notary So	Public State of Florida pria R. Hopewell commission HH 428758 kpires 8/2/2027



Department of State / Division of Corporations / Search Records / Search by Entity Name /

01/01/2005

Detail by Entity Name

Florida Limited Liability Company
ST. JOHNS LIMITED LIABILITY COMPANY

Filing Information

Document Number L05000000431

FEI/EIN Number 20-3739691

Date Filed 12/23/2004

State FL

Status ACTIVE

Principal Address

Effective Date

13820 W Newberry Rd

Suite 100

GAINESVILLE, FL 32669

Changed: 01/14/2018

Mailing Address

13820 W Newberry Rd

Suite 100

GAINESVILLE, FL 32669

Changed: 01/14/2018

Registered Agent Name & Address

Martin, William B

13820 W Newberry Road

Suite 100

Newberry, FL 32669

Name Changed: 03/10/2019

Address Changed: 03/10/2019

Authorized Person(s) Detail

Name & Address

Title MGR

Banks, Judith 13820 W Newberry Road Suite 100 Newberry, FL 32669

Title MGR

Martin, William B 13820 W Newberry Rd Suite 100 GAINESVILLE, FL 32669

Annual Reports

Report Year	Filed Date
2022	02/23/2022
2023	02/08/2023
2024	02/12/2024

Document Images

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02/27/2007 - ANNUAL REPORT	View image in PDF format
06/21/2006 ANNUAL REPORT	View image in PDF format
12/23/2004 Florida Limited Liabilites	View image in PDF format



Department of State / Division of Corporations / Search Records / Search by Entity Name /

Detail by Entity Name

Florida Profit Corporation CONCEPT COMPANIES, INC.

Filing Information

Document Number P14000067003

 FEI/EIN Number
 47-1672849

 Date Filed
 08/07/2014

Effective Date 07/08/2009

State FL

Status ACTIVE

Last Event CONVERSION

Event Date Filed 08/07/2014

Event Effective Date NONE

Principal Address

1449 SW 74th Drive

Suite 200

Gainesville, FL 32607

Changed: 04/18/2022

Mailing Address

1449 SW 74th Drive

Suite 200

Gainesville, FL 32607

Changed: 04/18/2022

Registered Agent Name & Address

Burch, Stephanie 1449 SW 74th Drive

Suite 200

Gainesville, FL 32607

Name Changed: 04/18/2022

Address Changed: 04/18/2022

Officer/Director Detail

Name & Address

Title Founder and Principal

Crawford, Brian S 1449 SW 74th Drive Suite 200 Gainesville, FL 32607

Title President

Cason, Matthew 1449 SW 74th Drive Suite 200 Gainesville, FL 32607

Title CEO

Banks, Nick 1449 SW 74th Drive Suite 200 Gainesville, FL 32607

Annual Reports

Report Year	Filed Date
2022	04/18/2022
2023	03/03/2023
2024	03/11/2024

Document Images

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04/18/2022 ANNUAL REPORT	View image in PDF format
04/02/2021 - ANNUAL REPORT	View image in PDF format
06/09/2020 – ANNUAL REPORT	View image in PDF format
02/13/2019 – ANNUAL REPORT	View image in PDF format
04/26/2018 – ANNUAL REPORT	View image in PDF format
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04/15/2016 ANNUAL REPORT	View image in PDF format
04/22/2015 ANNUAL REPORT	View image in PDF format
08/07/2014 Domestic Profit	View image in PDF format

Tax Record

Last Update: 3/20/2024 3:02:58 PM EDT



Account Number		Tax Ty	/ ре	Tax Year			
R08127-005		REAL ES	STATE	2	2023		
Mailing Address ST JOHNS LLC		Property	Address				
13820 W NEWBERRY RD STE	100						
NEWBERRY FL 32669		GEO Numb 074S17-0	98127-005				
Exempt Amount	· T	Taxable '	Value				
See Below		See Be	low				
Exemption Detail NO EXEMPTIONS Legal Description (clic 07-4s-17 0000/00009.69 LE 1318-991, DC 1327- 1 766, 894- 679, 912-1064	Acres S1/2 Ol 297, DC 1327	escription F NE1/4 O	<u>n)</u> F NE1/4 W O		82-143,		
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NO EXEMPTIONS Legal Description (clic 07-4s-17 0000/00009.69 LE 1318-991, DC 1327- 1	001 k for full de Acres \$1/2 01 297, DC 1327- , 1037-1953 Ad Valor Rate 4.9000 7.8150 0.7480 3.2170 1.5000 0.3113	escription F NE1/4 OT -1298, WD em Taxes Assessed Value 266,978 266,978 266,978 266,978 266,978 266,978 266,978 266,978 266,978 266,978 266,978 266,978 266,978 266,978 266,978	Exemption Amount 0 0 0 0 0 0 0	Taxable Value \$266,978 \$266,978 \$266,978 \$266,978 \$266,978 \$266,978 \$266,978 \$266,978	82-143, 804- Taxes Levied \$1,308.19 \$2,086.43 \$199.70 \$858.87 \$400.47 \$83.11		

Total Assessment	\$ \$61.26
Taxes & Assessment	\$4,998.06
If Paid By	Amount Due
	\$0.00

Date Paid	Transaction	Receipt	Item	Amount Paid
11/27/2023	PAYMENT	9921508.0001	2023	\$4,798.14

Price Year Taxes Due

NO DELINQUENT TAXES

TRAFFIC IMPACT STUDY

Variety Retail Store Marvin Burnett Road Lake City, Florida

October 3, 2023

prepared for:

FLORIDA DOT DISTRICT 2 and THE CITY OF LAKE CITY

submitted on behalf of: Concept Development, Inc.

prepared by:

AGEN

CONSULTING

SERVICES LICE



PROFESSIONAL ENGINEER ENDORSEMENT

I hereby certify that I am a Registered Professional Engineer in the State of Florida and currently practicing as the principal of Hagen Consulting Services, LLC.

Hagen Consulting Services, LLC is authorized via Registry No: 27955 to operate as an Engineering Business by the Florida Board of Professional Engineers, State of Florida, Department of Professional Regulation.

I have prepared or supervised the preparation of the evaluation, findings, conclusions, recommendations, and professional opinions/advice contained in this document. My endorsement constitutes my approval of these items.

PROJECT: Marvin Burnett Road Retail Store

LOCATION: Lake City, Florida **CLIENT:** Concept Development, Inc.

The results contained in this report were developed using procedures and references standard to the transportation engineering practice. These references and procedures were applied using professional judgment and experience.

Name: Lawrence T. Hagen, P.E., PTOE, RSP

Florida P.E. No.: 43968



Lawrence T Hagen

Digitally signed by Lawrence T Hagen Date: 2023.10.03 08:20:11 -04'00'

This item has been digitally signed and sealed by Lawrence T. Hagen on the date adjacent to the seal.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.



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TABLES Table 1 – Trip Generation Table 2 – Net Trip Generation with Pass-By Reduction Table 3 – AM Peak Hour Volumes. Table 4 – PM Peak Hour Volumes. Table 5 – Intersection Level Of Service (AM) Table 6 – Intersection Level Of Service (AM) Table 7 – AM Peak Hour Volumes.	1581213131414



EXECUTIVE SUMMARY

The results of the traffic impact analysis for the proposed variety retail store location at the intersection of State Road 47 and SW Marvin Burnett Road near the City of Lake City show that the traffic generated by the development will not have a significant impact on the operation of the roadway network surrounding it. The existing STOP-Controlled intersection adjacent to the project site will continue to operate well with the addition of the projected traffic from the development. Additionally, the nearby intersection of SW Bascom Norris Drive and SW Marvin Burnett Road will also continue to operate well.

The project location is within Columbia County south of the City of Lake City, Florida and State Road 47 is under the jurisdiction of the Florida DOT, District 2. This study utilized turning movement count data for the AM and PM Peak Hours collected by Hagen Consulting Services in July of 2023. The turning movement count information for the AM and PM Peak Hours of traffic were adjusted using a seasonal adjustment factor from FDOT's Peak Season Factor Category Report and a growth factor was applied to adjust traffic volumes to the build-out year (2024). The adjusted traffic volumes were then analyzed with and without the project traffic utilizing the Highway Capacity Manual (HCM) procedures.

The project traffic was developed using the Institute of Transportation Engineers (ITE) $Trip\ Generation-11^{th}$ Edition. The ITE Land Use Code for a variety retail store was used to estimate the trips generated by the proposed 12,480 square foot building. The trips were then distributed on the transportation network to estimate the traffic impacts.

The HCM analysis showed that the intersections, and hence the roadway network adjacent to the site, will be able to accommodate the traffic from the proposed development without a significant degradation in operational performance. Traffic conditions in the area will continue to operate at a very good level that meets the needs of the traveling public.



INTRODUCTION

Hagen Consulting Services, LLC is assisting Concept Development, Inc. with the transportation impacts for the proposed new 10,640 square foot variety retail store in Columbia County, Florida. The site will serve the southern Lake City area. The proposed retail store site is located on SW Marvin Burnett Road, at the intersection with State Road 47. State Road 47 is under the jurisdiction of the Florida Department of Transportation, District Two. The proposed site will have a connection to SW Marvin Burnett Road. The site currently is undeveloped and heavily wooded. There is a single family home foundation and accessory shed and propane tank on the site. The project location is shown in **Figure 1** below.

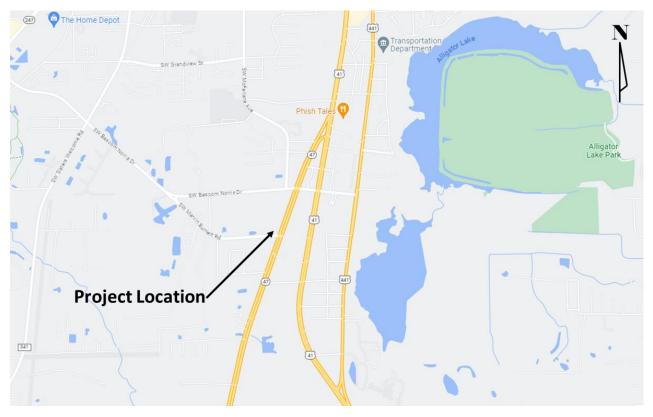


Figure 1 - Project Location Map

The preliminary site plan for the proposed retail store is shown in **Figure 2** on the following page.



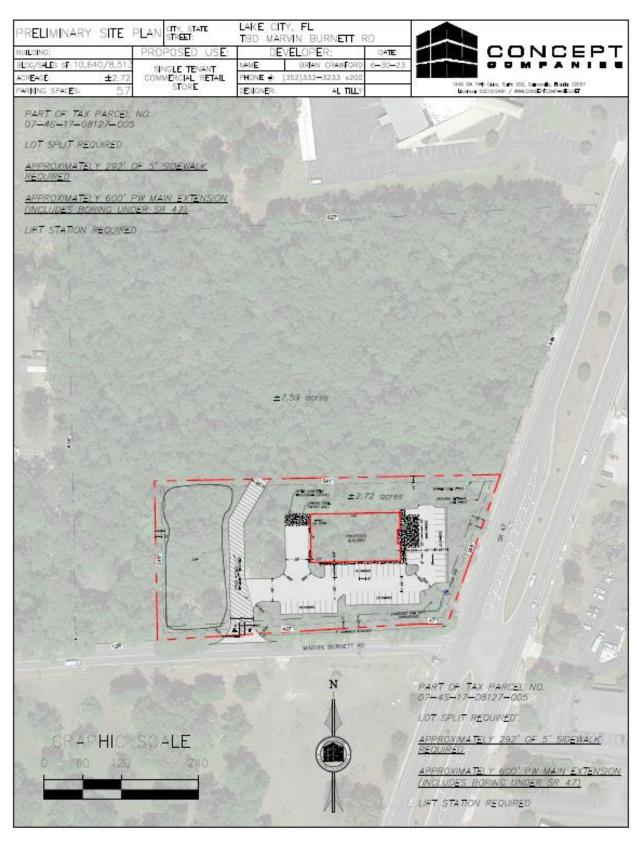


Figure 2 - Preliminary Site Plan



The 11th Edition of the Institute of Transportation Engineers (ITE) <u>Trip Generation</u> is the recognized authoritative source for estimating the trips generated by developments such as the proposed variety retail store facility. According to *Trip Generation*, a variety retail facility such as proposed here falls under ITE Land Use Code 814 – Variety Store. The assessment of the traffic impacts of the proposed variety retail store will be based on the impacts to traffic in the AM and PM peak hour periods.

The traffic impacts of the proposed development will be based on a Highway Capacity Software analysis of the operation of the signalized intersection adjoining the site both with and without the traffic generated by the development. A comparison of the delay and Level Of Service (LOS) with and without the project traffic will serve as the basis of the analysis.



EXISTING CONDITIONS

State Road 47 is a four-lane divided highway with an urban typical section (curb and gutter). The posted speed limit in the vicinity of the project site is 45 miles per hour. There are existing bike lanes and sidewalks on both sides of the roadway. State Road 47 is classified as an Urban Minor Arterial. There are existing NB and SB left turn lanes at the Marvin Burnett Road intersection. Marvin Burnett Road is a two-lane roadway that is functionally classified as a minor collector rural with a posted speed of 35 miles per hour. The cross-section features a flush shoulder on the north side of the road and the south side has raised curb. There are currently no bike lanes or sidewalks present.

Existing AM and PM Peak Hour turning movement counts were collected at the intersection of State Road 47 and Marvin Burnett Road. Two hours of AM Peak data (7:00 AM – 9:00 AM) and two hours of PM Peak data (4:00 PM – 6:00 PM) were collected. From these counts, the AM Peak Hour (7:30 – 8:30 AM) and PM Peak Hour (4:30 – 5:30 PM) turning movement counts were determined. The AM and PM Peak Hour turning movement counts are shown in **Figure 3** below.

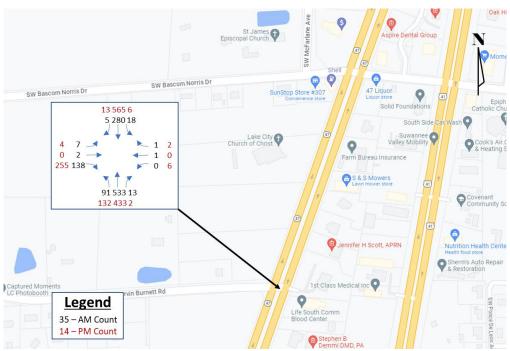


Figure 3 - Existing AM and PM Peak Hour Turning Movement Counts

The raw turning movement count data for the AM and PM Peak Hour is included in Appendix A.



TRIP GENERATION

The Institute of Transportation Engineers (ITE) *Trip Generation* 11th Edition was used to calculate the project trip estimates for the new land use at the project site. Trip generations estimates are shown in terms of daily traffic, as well as the AM and PM peak hours. The proposed Variety Retail Store falls under ITE Land Use Code 814 – Variety Store. The trip generation information for the proposed Variety Retail Store is shown in Table 1 below.

TABLE 1: Trip Generation Variety Retail Store – ITE Land Use 814 – 10,640 SF

Lake City, Florida

		Distrib	Distribution Trips					
Period	ITE Rate	Units Trips		% In	% Out	In	Out	Net
Weekday	T = 63.66 (X)	10.64	677	50%	50%	339	338	677
AM Peak	T = 3.04 (X)	10.64	32	55%	45%	18	14	32
PM Peak	T = 6.70 (X)	10.64	71	51%	49%	36	35	71

Source: ITE 11th Edition of Trip Generation - Units: 1,000 square feet Gross Floor Area

The 2021 Pass-By Tables for ITE's *Trip Generation* indicate a 34% pass-by rate for Land Use 814. This means that 34% of the trips generated are existing pass-by trips, and the net new trips represent 66% of the estimated *Trip Generation* number.

TABLE 2: Net Trip Generation with Pass-By Reduction

				Distribution Net Trips				
Period	Trips	Pass-By	Net Trips	% In	% Out	In	Out	Net
Weekday	677	34%	447	50%	50%	224	224	447
AM Peak	32	34%	21	55%	45%	12	10	21
PM Peak	71	34%	47	51%	49%	24	23	47

The trip generation data is then used to develop the external distribution of project trips onto the adjacent roadway network from the project site. The next section of the report presents information on the trip distribution.



TRIP DISTRIBUTION

The distribution of project trips on the roadway network is a manual assignment derived from the AM and PM peak period traffic data collected on the adjacent roadway and a review of existing locations of interacting land-uses. The distribution is based on engineering judgment of the expected routes that patrons would take to / from the proposed development. The project has access just on SW Marvin Burnett Road. The AM and PM Peak Hour Project Trip Distribution is shown in **Figure 4** below.

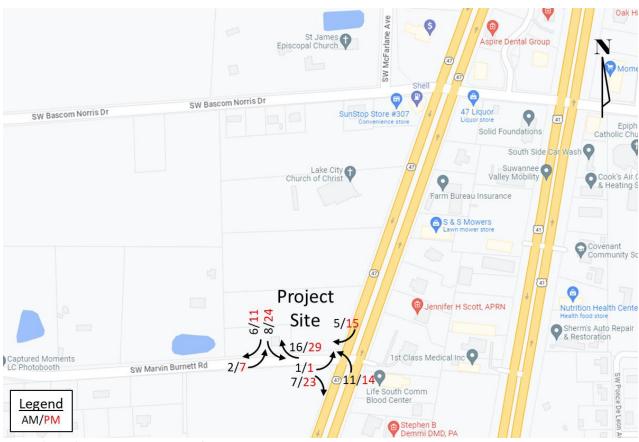


Figure 4 - Peak Hour Project Trip Distribution



LEFT TURN LANE ANALYSIS – Marvin Burnett Road

The criteria for evaluating left turn lanes are established in *NCHRP Report 457: Evaluating Intersection Improvements: An Engineering Study Guide*. The highest left turning volume into the project site from Marvin Burnett Road is the PM Peak Hour left turn volume of 7 vehicles. The left + through + right turn volumes are added together to compute the "advancing volume." The through + right turning volumes opposing the left turn are used as the "opposing volume."

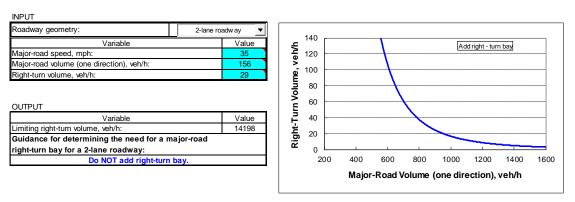
Value Variable veh/h 800 85th percentile speed, mph: Percent of left-turns in advancing volume (VA), % eft-turn treatment. warranted. 700 266 Advancing volume (V_A), veh/h: Opposing Volume (Vo), 600 Opposing volume (V_O), veh/h: 500 OUTPUT 400 Variable Value 300 imiting advancing volume (VA), veh/h 200 Left-turn treatment no Guidance for determining the need for a major-road left-turn bay: w arranted Left-turn treatment NOT warranted. 100 0 0 700 Advancing Volume (VA), veh/h CALIBRATION CONSTANTS Value Average time for making left-turn, s: Critical headway, s: Average time for left-turn vehicle to clear the advancing lane, s

Figure 2 - 5. Guideline for determining the need for a major-road left-turn bay at a two-way stop-controlled intersection.

RIGHT TURN LANE ANALYSIS - Marvin Barnett Road

Similarly, the criteria for evaluating right turn lanes are established in *NCHRP Report 457: Evaluating Intersection Improvements: An Engineering Study Guide.* For this analysis, we need to enter the major road speed, the major road volume (through + right), and the right turn volume.

Figure 2 - 6. Guideline for determining the need for a major-road right-turn bay at a two-way stop-controlled intersection.

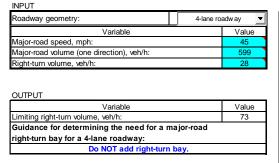


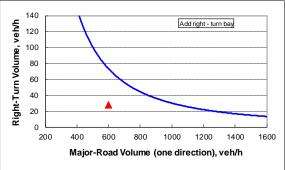


RIGHT TURN LANE ANALYSIS - State Road 47

As indicated previously, the criteria for evaluating right turn lanes are established in *NCHRP Report 457: Evaluating Intersection Improvements: An Engineering Study Guide*. For this analysis, we need to enter the major road speed, the major road volume (left + through + right), and the right turn volume.

Figure 2 - 6. Guideline for determining the need for a major-road right-turn bay at a two-way stop-controlled intersection.







INTERSECTION LEVEL OF SERVICE (LOS) ANALYSIS

The roadway Level Of Service (LOS) analysis is conducted using the procedures outlined in the Transportation Research Board's *Highway Capacity Manual* (HCM). The HCM procedures represent the state-of-the-practice for the analysis of transportation facilities.

Existing turning movement count data was collected on Tuesday, July 18, 2023 at the intersection of State Road 47 and SW Marvin Burnett Road. Two hours of turning movement count data were collected for both the AM peak period (7 AM to 9 AM) and the PM peak period (4 PM to 6 PM). Out of that two-hours of data collection in each period, the overall AM peak hour of 7:30 AM to 8:30 AM and the overall PM peak hour of 4:30 PM to 5:30 PM were used in the analysis. A seasonal adjustment of 1.02 is then applied based on FDOT Peak Season Factor Category Report for Columbia County (included in Appendix A). A growth factor of 3% is then added to the volumes to convert to 2024 (expected build-out year) volumes. The AM peak hour volumes along with the assigned new project trips are provided in **Table 3** below. The PM peak hour volumes along with the assigned new project trips are provided in **Table 4** below.

Table 3 – AM Peak Hour Volumes

Roadway	State Road 47							SW M	arvin E	Burnett	Road	
Approach	Northbound			So	Southbound			stbour	nd	W	estbou	nd
Movement	Lt	Thru	Rt	Lt	Thru	Rt	Lt	Thru	Rt	Lt	Thru	Rt
2023	91	533	13	18	280	5	7	2	138	0	1	1
Seasonal	93	544	13	18	286	5	7	2	141	0	1	1
2024	96	560	14	19	294	5	7	2	145	0	1	1
Project	11	0	0	0	0	5	1	0	7	0	0	0
Total	107	560	14	19	294	10	8	2	152	0	1	1

Table 4 – PM Peak Hour Volumes

Roadway		;	State R	Road 47	7			SW M	arvin E	Burnett	t Road	
Approach	No	rthbou	ınd	So	uthbou	ınd	Ea	stbour	nd	V	estbou	nd
Movement	Lt	Thru	Rt	Lt	Thru	Rt	Lt	Thru	Rt	Lt	Thru	Rt
2023	132	433	2	6	565	13	4	0	255	6	0	2
Seasonal	135	442	2	6 576 13			4	0	260	6	0	2
2024	139	455	2	6 594		14	4	0	268	6	0	2
Project	14	0	0	0	0	15	1	0	23	0	0	0
Total	153	455	2	6	594	29	5	0	291	6	0	2



The Highway Capacity Software (HCS) Two-Way Stop-Controlled intersection module was utilized in analyzing the no-build and the build-out traffic volumes at the intersection of SR 47 and Marvin Burnett Road. The results from the HCS analyses are summarized in **Table 5** and **Table 6** below. The outputs from HCS are included in Appendix B.

Table 5 – Intersection Level Of Service (AM)

	ı — — —								
Roadway		State F	Road 47	7	SW M	arvin E	Burnett	Road	
Approach	North	bound	South	bound	Eastb	ound	Westbound		
MOE	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	
No-Build	8.3	Α	9.0	Α	11.3	В	19.4	С	
Build	8.3	Α	9.0	Α	11.5	В	20.1	С	

Table 6 – Intersection Level Of Service (PM)

Roadway		State F	Road 47	7	SW M	arvin E	Burnett	Road	
Approach	North	bound	South	bound	Eastb	ound	Westbound		
MOE	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	
No-Build	9.8	Α	8.5	Α	15.9	С	28.8	D	
Build	10.0	В	8.5	Α	17.2 C		35.0	Е	

The HCS analyses show that the impacts of the proposed variety retail store development on the operation of the intersection are minimal in the AM period. In the PM period, the westbound approach degrades from LOS D to LOS E, despite the fact that the project assigns no trips to the westbound approach. The westbound approach is a minor approach to the intersection and even in the LOS E scenario is operating at a volume to capacity ratio of 0.07 with an hourly flow rate of just nine vehicles. The HCS two-way stop-controlled analysis is well known for being overly pessimistic, and this is an example of that. An average delay of 35 seconds is not an intolerable scenario for those vehicles.



The City of Lake City requested that the intersection of SW Bascom Norris Drive and SW Marvin Burnett Road also be analyzed for this project. Existing turning movement count data was also collected on Tuesday, July 18, 2023 at this intersection. Two hours of turning movement count data were collected for both the AM peak period (7 AM to 9 AM) and the PM peak period (4 PM to 6 PM). Out of that two-hours of data collection in each period, the overall AM peak hour of 7:30 AM to 8:30 AM and the overall PM peak hour of 4:45 PM to 5:45 PM were used in the analysis. A seasonal adjustment of 1.02 is then applied based on FDOT Peak Season Factor Category Report for Columbia County. A growth factor of 3% is then added to the volumes to convert to 2024 (expected build-out year) volumes. The AM peak hour volumes along with the assigned new project trips are provided in **Table 7** below. The PM peak hour volumes along with the assigned new project trips are provided in **Table 8** below. For this analysis, a worst-case scenario where all of the project trips from the proposed retail site are presumed to make a northbound left at the intersection.

Table 7 – AM Peak Hour Volumes

Roadway		SW M	arvin E	Burnett	t Road			SW Ba	ascom	Norris	Drive	
Approach	No	rthbou	ınd	So	uthbou	ınd	Ea	astbour	nd	W	estbou	nd
Movement	Lt	Thru	Rt	Lt	Thru	Rt	Lt	Thru	Rt	Lt	Thru	Rt
2023	102	0	0	-	-	-	0	234	0	0	249	0
Seasonal	104	0	0			0	239	0	0	254	0	
2024	107	0	0	-	-	-	0	246	0	0	262	0
Project	6	0	0	-	-	-	0	0	0	0	0	0
Total	113	0	0	-	-	-	0	246	0	0	262	0

Table 8 – PM Peak Hour Volumes

Roadway		SW M	arvin E	Burnett	Road			SW Ba	ascom	Norris	Drive	
Approach	No	rthbou	ınd	So	uthbou	ınd	Ea	stbour	nd	W	estbou	nd
Movement	Lt	Thru	Rt	Lt	Thru	Rt	Lt	Thru	Rt	Lt	Thru	Rt
2023	145	0	2	-	-	ı	0	205	0	0	422	0
Seasonal	148	0	2			ı	0	209	0	0	430	0
2024	152	0	2	-	-	•	0	215	0	0	443	0
Project	11	0	0	-	-	ı	0	0	0	0	0	0
Total	163	0	2	-	-	-	0	215	0	0	443	0



The intersection of Bascom Norris Drive and Marvin Burnett Road is a somewhat unusual T-intersection: Bascom Norris Drive is the major street that does not stop, and Marvin Burnett Road intersects and is controlled by a STOP sign. However, the left turn from Bascom Norris Drive WB onto Marvin Burnett Road is prohibited, and the right turn from Bascom Norris Drive EB onto Marvin Burnett Road is a free-flowing movement that is channelized and unimpeded. Thus, the only movement that has any control delay is the northbound left or right turn from Marvin Barnett Road onto Bascom Norris Drive. The layout of the intersection is shown in **Figure 5** below.



Figure 5 - SW Bascom Norris Dr & SW Marvin Burnett Rd

The results from the HCS analyses are summarized in **Table 9** and **Table 10** below. The outputs from HCS are included in Appendix B.



Table 9 – Intersection Level Of Service (AM)

Roadway	SW M	larvin E	Burnett	Road	SW Ba	ascom	Norris	Drive	
Approach	North	bound	South	bound	Eastb	ound	Westbound		
MOE	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	
No-Build	15.8	С	-	-	-	-	-	-	
Build	16.1	С	-	-	-	-	-	-	

Table 10 – Intersection Level Of Service (PM)

Roadway	SW M	larvin E	Burnett	Road	SW Ba	ascom	Norris	Drive
Approach	North	bound	South	bound	Eastb	ound	West	oound
MOE	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
No-Build	24.6	С	-	-	-	-	-	-
Build	26.0	D	-	-	-	-	-	-

The HCS analyses show that the impact of the project traffic on the intersection of SW Marvin Burnett Road and SW Bascom Norris Drive is minimal. Although in the PM period the LOS does go from C to D, it is only an increase of 1.4 seconds of delay per vehicle. This movement operates with a volume to capacity ratio of just 0.54 in the PM period with the project traffic.



CONCLUSIONS AND RECOMMENDATIONS

Based on the foregoing data and analysis provided, the following conclusions and recommendations are offered:

Conclusions:

- The proposed variety retail store is estimated to generate 32 trips in the AM Peak Hour and 71 trips in the PM Peak Hour. To be conservative in the analyses, these numbers were used. If the pass-by reductions from ITE are used, the net trips would be 21 in the AM Peak Hour and 47 in the PM Peak Hour.
- The additional traffic generated by the proposed variety retail store will not have a
 noticeable impact on the adjoining STOP-controlled intersections and will not
 degrade the performance of the transportation network.
- Neither left-turn lanes nor right-turn lanes are warranted on either State Road 47 or
 on SW Marvin Burnett Road. There is very little disruption to traffic with the
 addition of the project driveways and the generated project traffic.

Recommendations:

 Approve the project for construction and approve the associated driveway connection onto SW Marvin Burnett Road.



Tue Jul 18, 2023

Full Length (7 AM-9 AM, 4 PM-6 PM)

All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1091860, Location: 30.160196, -82.645384, Site Code: SR 47 & Marvin Burnett



Provided by: Hagen Consulting Services 361 Strawder Road, Ray City, GA, 31645, US

Leg	Marvin	Burne	tt Road	l		Radiati	on Onc	ology (Grp		SR 47					SR 47					
Direction	Eastbo	und				Westbo	ound				Northb	ound				South	oound				
Time	L	T	R	U	App	L	T	R	U	App	L	Т	R	U	App	L	T	R	U	App	Int
2023-07-18 7:00AM	0	0	17	0	17	0	0	0	0	0	13	91	2	0	106	0	58	1	1	60	183
7:15AM	1	0	16	0	17	0	0	0	0	0	16	140	2	0	158	0	42	0	0	42	217
7:30AM	2	0	34	0	36	0	0	0	0	0	20	163	3	0	186	2	63	1	4	70	292
7:45AM	0	0	35	0	35	0	1	0	0	1	27	150	3	0	180	1	82	2	4	89	305
Hourly Total	3	0	102	0	105	0	1	0	0	1	76	544	10	0	630	3	245	4	9	261	997
8:00AM	2	0	31	0	33	0	0	1	0	1	23	121	5	0	149	0	63	2	4	69	252
8:15AM	3	2	38	0	43	0	0	0	0	0	20	99	2	1	122	1	72	0	2	75	240
8:30AM	0	1	33	0	34	0	1	2	0	3	28	117	1	0	146	0	54	1	2	57	240
8:45AM	3	0	40	1	44	0	0	2	0	2	32	137	1	0	170	1	65	0	2	68	284
Hourly Total	8	3	142	1	154	0	1	5	0	6	103	474	9	1	587	2	254	3	10	269	1016
4:00PM	2	0	51	0	53	1	0	0	0	1	33	90	0	1	124	0	124	3	2	129	307
4:15PM	3	0	71	0	74	1	1	0	0	2	27	92	0	1	120	0	103	3	1	107	303
4:30PM	1	0	64	0	65	3	0	0	0	3	21	117	1	1	140	0	141	3	4	148	356
4:45PM	0	0	67	0	67	0	0	1	0	1	35	96	0	3	134	0	137	0	1	138	340
Hourly Total	6	0	253	0	259	5	1	1	0	7	116	395	1	6	518	0	505	9	8	522	1306
5:00PM	1	0	63	0	64	3	0	0	0	3	36	133	1	9	179	0	147	7	0	154	400
5:15PM	2	0	61	0	63	0	0	1	0	1	21	97	0	6	124	0	140	3	1	144	332
5:30PM	2	0	66	1	69	3	0	0	0	3	36	88	0	6	130	1	148	4	1	154	356
5:45PM	0	0	79	0	79	0	0	0	0	0	24	76	1	1	102	0	99	1	1	101	282
Hourly Total	5	0	269	1	275	6	0	1	0	7	117	394	2	22	535	1	534	15	3	553	1370
Total	22	3	766	2	793	11	3	7	0	21	412	1807	22	29	2270	6	1538	31	30	1605	4689
% Approach	2.8%	0.4%	96.6%	0.3%	-	52.4%	14.3%	33.3%	0%	-	18.1%	79.6%	1.0%	1.3%	-	0.4%	95.8%	1.9%	1.9%	-	-
% Total	0.5%	0.1%	16.3%	0%	16.9%	0.2%	0.1%	0.1%	0%	0.4%	8.8%	38.5%	0.5%	0.6%	48.4%	0.1%	32.8%	0.7%	0.6%	34.2%	-
Lights and Motorcycles	21	3	759	2	785	11	2	7	0	20	410	1743	22	28	2203	6	1489	28	29	1552	4560
% Lights and Motorcycles	95.5%	100%	99.1%	100%	99.0%	100%	66.7%	100%	0% 9	95.2%	99.5%	96.5%	100%	96.6%	97.0%	100%	96.8%	90.3%	96.7%	96.7%	97.2%
Heavy	1	0	7	0	8	0	1	0	0	1	2	64	0	1	67	0	49	3	1	53	129
% Heavy	4.5%	0%	0.9%	0%	1.0%	0%	33.3%	0%	0%	4.8%	0.5%	3.5%	0%	3.4%	3.0%	0%	3.2%	9.7%	3.3%	3.3%	2.8%

^{*}L: Left, R: Right, T: Thru, U: U-Turn

Tue Jul 18, 2023 Full Length (7 AM-9 AM, 4 PM-6 PM)

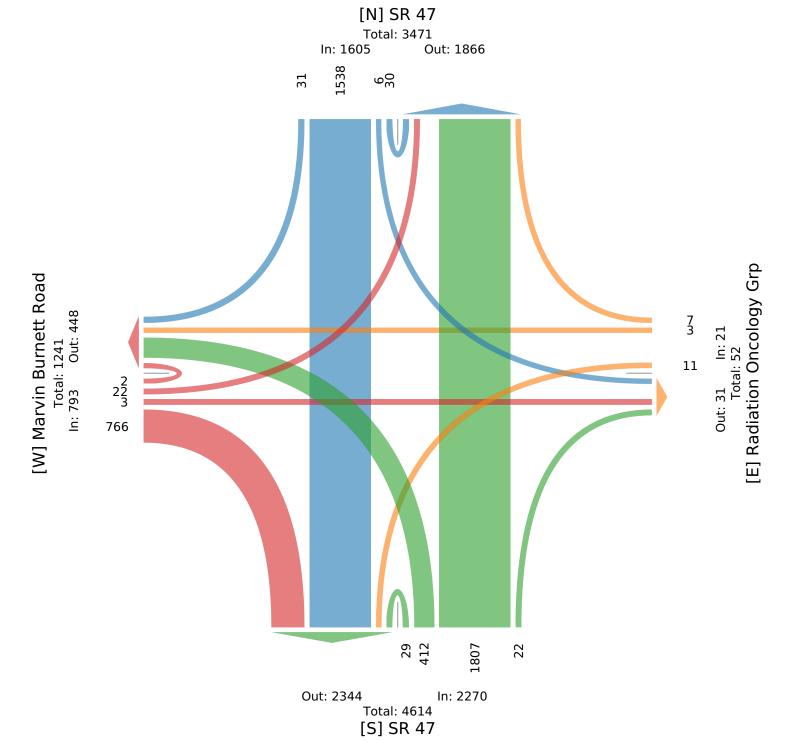
All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1091860, Location: 30.160196, -82.645384, Site Code: SR 47 & Marvin Burnett



Provided by: Hagen Consulting Services 361 Strawder Road, Ray City, GA, 31645, US



Tue Jul 18, 2023

AM Peak (7:30 AM - 8:30 AM)

All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1091860, Location: 30.160196, -82.645384, Site Code: SR 47 & Marvin Burnett



Provided by: Hagen Consulting Services 361 Strawder Road, Ray City, GA, 31645, US

Leg	Marvi	n Burn	ett Road	l		Rad	iation C	Oncolog	y G	rp	SR 47					SR 47					
Direction	Eastbo	ound				Wes	stbound				Northb	ound				South	ound				
Time	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	Int
2023-07-18 7:30AM	2	0	34	0	36	0	0	0	0	0	20	163	3	0	186	2	63	1	4	70	292
7:45AM	0	0	35	0	35	0	1	0	0	1	27	150	3	0	180	1	82	2	4	89	305
8:00AM	2	0	31	0	33	0	0	1	0	1	23	121	5	0	149	0	63	2	4	69	252
8:15AM	3	2	38	0	43	0	0	0	0	0	20	99	2	1	122	1	72	0	2	75	240
Total	7	2	138	0	147	0	1	1	0	2	90	533	13	1	637	4	280	5	14	303	1089
% Approach	4.8%	1.4%	93.9%	0%	-	0%	50.0%	50.0%	0%	-	14.1%	83.7%	2.0%	0.2%	-	1.3%	92.4%	1.7%	4.6%	-	-
% Total	0.6%	0.2%	12.7%	0%	13.5%	0%	0.1%	0.1%	0%	0.2%	8.3%	48.9%	1.2%	0.1%	58.5%	0.4%	25.7%	0.5%	1.3%	27.8%	-
PHF	0.583	0.250	0.908	-	0.855	-	0.250	0.250	-	0.500	0.833	0.817	0.650	0.250	0.856	0.500	0.854	0.625	0.875	0.851	0.893
Lights and Motorcycles	7	2	137	0	146	0	0	1	0	1	89	515	13	1	618	4	272	3	13	292	1057
% Lights and Motorcycles	100%	100%	99.3%	0%	99.3%	0%	0%	100%	0%	50.0%	98.9%	96.6%	100%	100%	97.0%	100%	97.1%	60.0%	92.9%	96.4%	97.1%
Heavy	0	0	1	0	1	0	1	0	0	1	1	18	0	0	19	0	8	2	1	11	32
% Heavy	0%	0%	0.7%	0%	0.7%	0%	100%	0%	0%	50.0%	1.1%	3.4%	0%	0%	3.0%	0%	2.9%	40.0%	7.1%	3.6%	2.9%

^{*}L: Left, R: Right, T: Thru, U: U-Turn

Tue Jul 18, 2023

AM Peak (7:30 AM - 8:30 AM)

All Classes (Lights and Motorcycles, Heavy)

All Movements

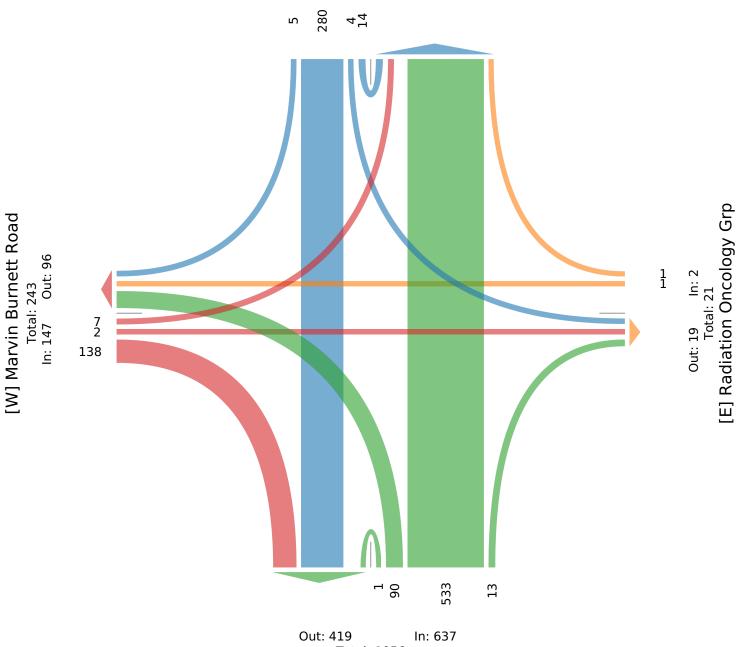
ID: 1091860, Location: 30.160196, -82.645384, Site Code: SR 47 & Marvin Burnett



Provided by: Hagen Consulting Services 361 Strawder Road, Ray City, GA, 31645, US



Total: 858 In: 303 Out: 555



Out: 419 In: 63 Total: 1056 [S] SR 47

142

Tue Jul 18, 2023

PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour

All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1091860, Location: 30.160196, -82.645384, Site Code: SR 47 & Marvin Burnett



Provided by: Hagen Consulting Services 361 Strawder Road, Ray City, GA, 31645, US

Leg	Marvii	ı Bu	rnett Roa	ıd		Radiatio	n O	ncology	/ Grj)	SR 47					SR -	47				
Direction	Eastbo	und				Westbo	und				Northbo	ound				Sou	thbound				
Time	I	. Т	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	Int
2023-07-18 4:30PM	1	. 0	64	0	65	3	0	0	0	3	21	117	1	1	140	0	141	3	4	148	356
4:45PM	(0	67	0	67	0	0	1	0	1	35	96	0	3	134	0	137	0	1	138	340
5:00PM	1	. 0	63	0	64	3	0	0	0	3	36	133	1	9	179	0	147	7	0	154	400
5:15PM	2	2 0	61	0	63	0	0	1	0	1	21	97	0	6	124	0	140	3	1	144	332
Total	4	- 0	255	0	259	6	0	2	0	8	113	443	2	19	577	0	565	13	6	584	1428
% Approach	1.5%	0%	98.5%	0%	-	75.0%	0%	25.0%	0%	-	19.6%	76.8%	0.3%	3.3%	-	0%	96.7%	2.2%	1.0%	-	-
% Total	0.3%	0%	17.9%	0%	18.1%	0.4%	0%	0.1%	0%	0.6%	7.9%	31.0%	0.1%	1.3%	40.4%	0%	39.6%	0.9%	0.4%	40.9%	-
PHF	0.500	-	0.951	-	0.966	0.500	-	0.500	-	0.667	0.785	0.833	0.500	0.528	0.806	-	0.961	0.464	0.375	0.948	0.893
Lights and Motorcycles	3	3 0	253	0	256	6	0	2	0	8	113	430	2	18	563	0	549	12	6	567	1394
% Lights and Motorcycles	75.0%	0%	99.2%	0%	98.8%	100%	0%	100%	0%	100%	100%	97.1%	100%	94.7%	97.6%	0%	97.2%	92.3%	100%	97.1%	97.6%
Heavy	1	. 0	2	0	3	0	0	0	0	0	0	13	0	1	14	0	16	1	0	17	34
% Heavy	25.0%	0%	0.8%	0%	1.2%	0%	0%	0%	0%	0%	0%	2.9%	0%	5.3%	2.4%	0%	2.8%	7.7%	0%	2.9%	2.4%

^{*}L: Left, R: Right, T: Thru, U: U-Turn

Tue Jul 18, 2023

PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour

All Classes (Lights and Motorcycles, Heavy)

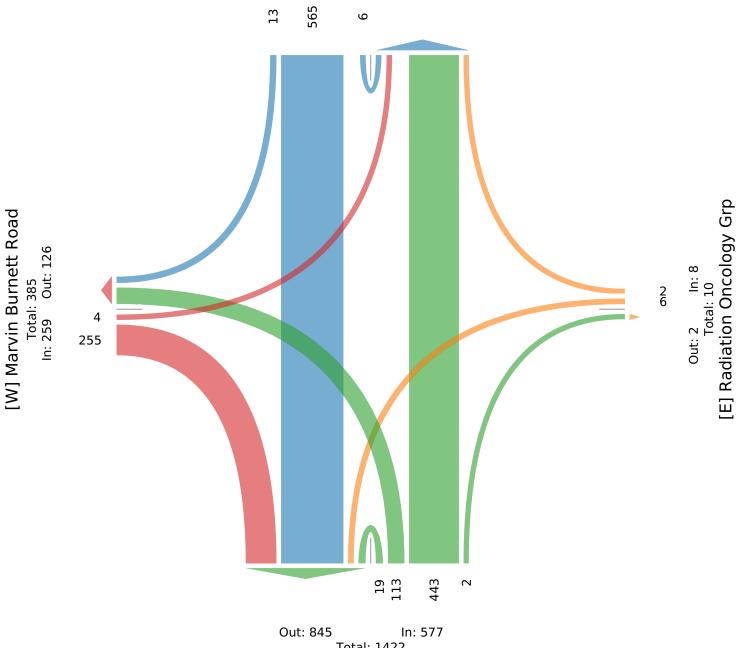
All Movements

ID: 1091860, Location: 30.160196, -82.645384, Site Code: SR 47 & Marvin Burnett



Provided by: Hagen Consulting Services 361 Strawder Road, Ray City, GA, 31645, US





Total: 1422

[S] SR 47

Tue Jul 18, 2023 Full Length (7 AM-9 AM, 4 PM-6 PM) All Classes (Lights and Motorcycles, Heavy) All Movements

ID: 1091861, Location: 30.163397, -82.655082, Site Code: Marvin Burnett & Bascom

Norris



Provided by: Hagen Consulting Services 361 Strawder Road, Ray City, GA, 31645, US

Leg	SW Bascom	Norris	Dr			om Norris	Dr		SW Marvin E	Burnett Rd			
Direction	Eastbound				Westbou				Northbound				
Time	T	R	U	App	L	T	U	App	L	R	U	App	Int
2023-07-18 7:00AM	33	0	0	33	0	27	0	27	16	0	0	16	76
7:15AM	31	0	0	31	0	48	0	48	16	0	0	16	95
7:30AM	59	0	0	59	0	63	0	63	21	0	0	21	143
7:45AM	78	0	0	78	0	63	0	63	34	0	0	34	175
Hourly Total	201	0	0	201	0	201	0	201	87	0	0	87	489
8:00AM	41	0	0	41	0	58	0	58	24	0	0	24	123
8:15AM	56	0	0	56	0	65	0	65	23	0	0	23	144
8:30AM	49	0	0	49	0	57	0	57	26	0	0	26	132
8:45AM	41	0	0	41	0	74	0	74	38	0	0	38	153
Hourly Total	187	0	0	187	0	254	0	254	111	0	0	111	552
4:00PM	43	0	0	43	0	103	0	103	36	0	0	36	182
4:15PM	52	0	0	52	0	107	0	107	38	0	0	38	197
4:30PM	48	0	0	48	0	97	0	97	20	0	0	20	165
4:45PM	48	0	0	48	0	95	0	95	32	0	0	32	175
Hourly Total	191	0	0	191	0	402	0	402	126	0	0	126	719
5:00PM	52	0	0	52	0	142	0	142	35	0	0	35	229
5:15PM	58	0	0	58	0	98	0	98	22	1	0	23	179
5:30PM	47	0	0	47	0	87	0	87	56	1	0	57	191
5:45PM	44	0	0	44	0	62	0	62	29	0	0	29	135
Hourly Total	201	0	0	201	0	389	0	389	142	2	0	144	734
Total	780	0	0	780	0	1246	0	1246	466	2	0	468	2494
% Approach	100%	0%	0%	-	0%	100%	0%	-	99.6%	0.4%	0%	-	-
% Total	31.3%	0%	0%	31.3%	0%	50.0%	0%	50.0%	18.7%	0.1%	0%	18.8%	-
Lights and Motorcycles	759	0	0	759	0	1221	0	1221	461	2	0	463	2443
% Lights and Motorcycles	97.3%	0%	0%	97.3%	0%	98.0%	0%	98.0%	98.9%	100%	0%	98.9%	98.0%
Heavy	21	0	0	21	0	25	0	25	5	0	0	5	51
% Heavy	2.7%	0%	0%	2.7%	0%	2.0%	0%	2.0%	1.1%	0%	0%	1.1%	2.0%

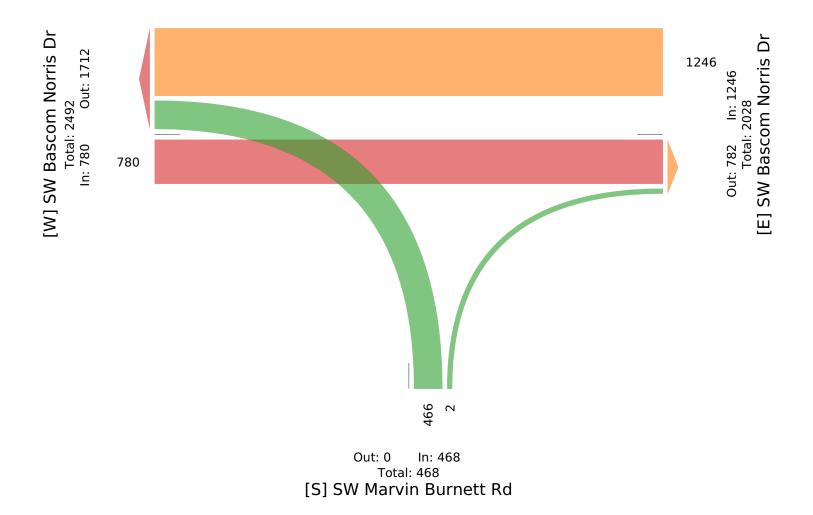
^{*}L: Left, R: Right, T: Thru, U: U-Turn

Tue Jul 18, 2023
Full Length (7 AM-9 AM, 4 PM-6 PM)
All Classes (Lights and Motorcycles, Heavy)
All Movements

ID: 1001961 Legation: 20 162207 - 92 65509

Provided by: Hagen Consulting Services
361 Strawder Road,
Ray City, GA, 31645, US

ID: 1091861, Location: 30.163397, -82.655082, Site Code: Marvin Burnett & Bascom Norris



Tue Jul 18, 2023 AM Peak (7:30 AM - 8:30 AM) All Classes (Lights and Motorcycles, Heavy) All Movements

ID: 1091861, Location: 30.163397, -82.655082, Site Code: Marvin Burnett & Bascom Norris

HAGEN CONSULTING SERVICES, LLC

Provided by: Hagen Consulting Services 361 Strawder Road, Ray City, GA, 31645, US

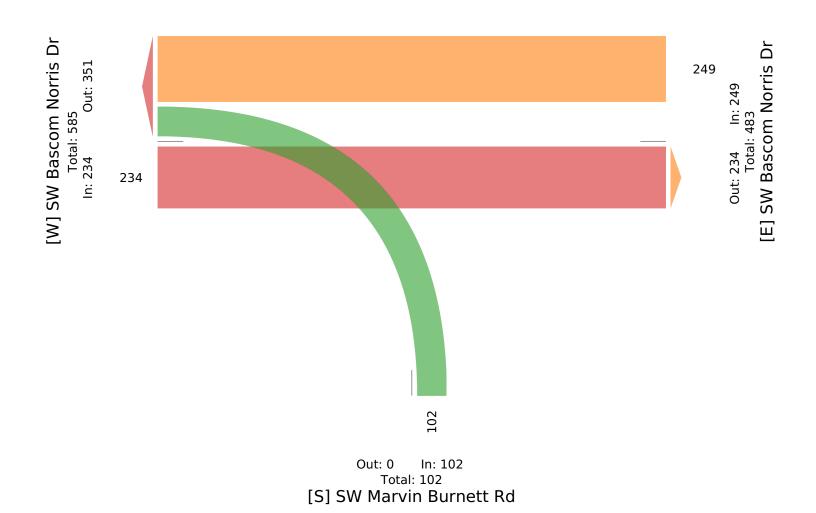
Leg	SW Bascom	Norris	Dr		SW Base	om Norris	Dr		SW Marvin E	Burnett	Rd		
Direction	Eastbound				Westbou	ınd			Northbound				
Time	T	R	U	Арр	L	T	U	App	L	R	U	App	Int
2023-07-18 7:30AM	59	0	0	59	0	63	0	63	21	0	0	21	143
7:45AM	78	0	0	78	0	63	0	63	34	0	0	34	175
8:00AM	41	0	0	41	0	58	0	58	24	0	0	24	123
8:15AM	56	0	0	56	0	65	0	6 5	23	0	0	23	144
Total	234	0	0	234	0	249	0	249	102	0	0	102	585
% Approach	100%	0%	0%	-	0%	100%	0%	-	100%	0%	0%	-	-
% Total	40.0%	0%	0%	40.0%	0%	42.6%	0%	42.6%	17.4%	0%	0%	17.4%	-
PHF	0.750	-	-	0.750	-	0.958	-	0.958	0.750	-	-	0.750	0.836
Lights and Motorcycles	222	0	0	222	0	240	0	240	99	0	0	99	561
% Lights and Motorcycles	94.9%	0%	0%	94.9%	0%	96.4%	0%	96.4%	97.1%	0%	0%	97.1%	95.9%
Heavy	12	0	0	12	0	9	0	9	3	0	0	3	24
% Heavy	5.1%	0%	0%	5.1%	0%	3.6%	0%	3.6%	2.9%	0%	0%	2.9%	4.1%

^{*}L: Left, R: Right, T: Thru, U: U-Turn

Tue Jul 18, 2023 AM Peak (7:30 AM - 8:30 AM) All Classes (Lights and Motorcycles, Heavy) All Movements

CONSULTING SERVICES, LLC Provided by: Hagen Consulting Services 361 Strawder Road, Ray City, GA, 31645, US

ID: 1091861, Location: 30.163397, -82.655082, Site Code: Marvin Burnett & Bascom Norris



Tue Jul 18, 2023 PM Peak (4:45 PM - 5:45 PM) - Overall Peak Hour All Classes (Lights and Motorcycles, Heavy)

All Movements

ID: 1091861, Location: 30.163397, -82.655082, Site Code: Marvin Burnett & Bascom Norris

AGEN CONSULTING SERVICES, LLC

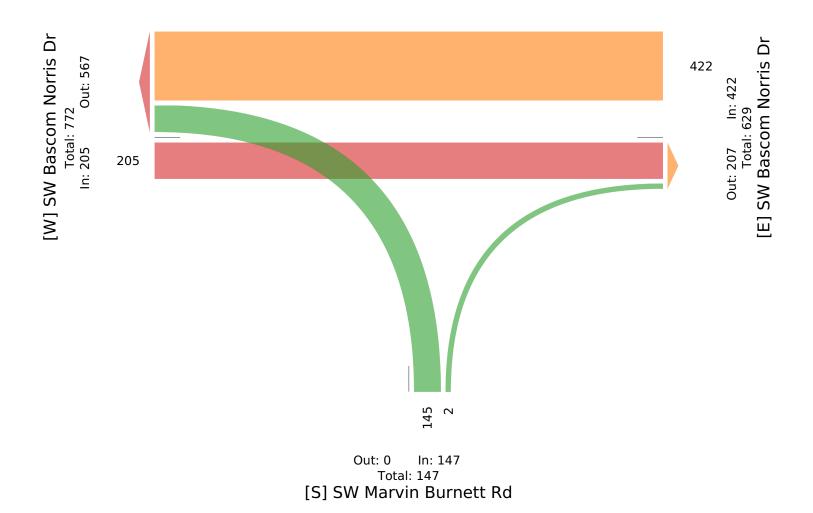
Provided by: Hagen Consulting Services 361 Strawder Road, Ray City, GA, 31645, US

Leg	SW Bascom	Norris	Dr		SW Base	com Norris	Dr		SW Marvin I	Burnett Rd			
Direction	Eastbound				Westbou	ınd			Northbound				
Time	T	R	U	Арр	L	T	U	Арр	L	R	U	App	Int
2023-07-18 4:45PM	48	0	0	48	0	95	0	95	32	0	0	32	175
5:00PM	52	0	0	52	0	142	0	142	35	0	0	35	229
5:15PM	58	0	0	58	0	98	0	98	22	1	0	23	179
5:30PM	47	0	0	47	0	87	0	87	56	1	0	57	191
Total	205	0	0	205	0	422	0	422	145	2	0	147	774
% Approach	100%	0%	0%	-	0%	100%	0%	-	98.6%	1.4%	0%	-	-
% Total	26.5%	0%	0%	26.5%	0%	54.5%	0%	54.5%	18.7%	0.3%	0%	19.0%	-
PHF	0.884	-	-	0.884	-	0.743	-	0.743	0.647	0.500	-	0.645	0.845
Lights and Motorcycles	204	0	0	204	0	416	0	416	144	2	0	146	766
% Lights and Motorcycles	99.5%	0%	0%	99.5%	0%	98.6%	0%	98.6%	99.3%	100%	0%	99.3%	99.0%
Heavy	1	0	0	1	0	6	0	6	1	0	0	1	8
% Heavy	0.5%	0%	0%	0.5%	0%	1.4%	0%	1.4%	0.7%	0%	0%	0.7%	1.0%

^{*}L: Left, R: Right, T: Thru, U: U-Turn

Tue Jul 18, 2023 PM Peak (4:45 PM - 5:45 PM) - Overall Peak Hour All Classes (Lights and Motorcycles, Heavy) All Movements Provided by: Hagen Consulting Services 361 Strawder Road, Ray City, GA, 31645, US

ID: 1091861, Location: 30.163397, -82.655082, Site Code: Marvin Burnett & Bascom Norris



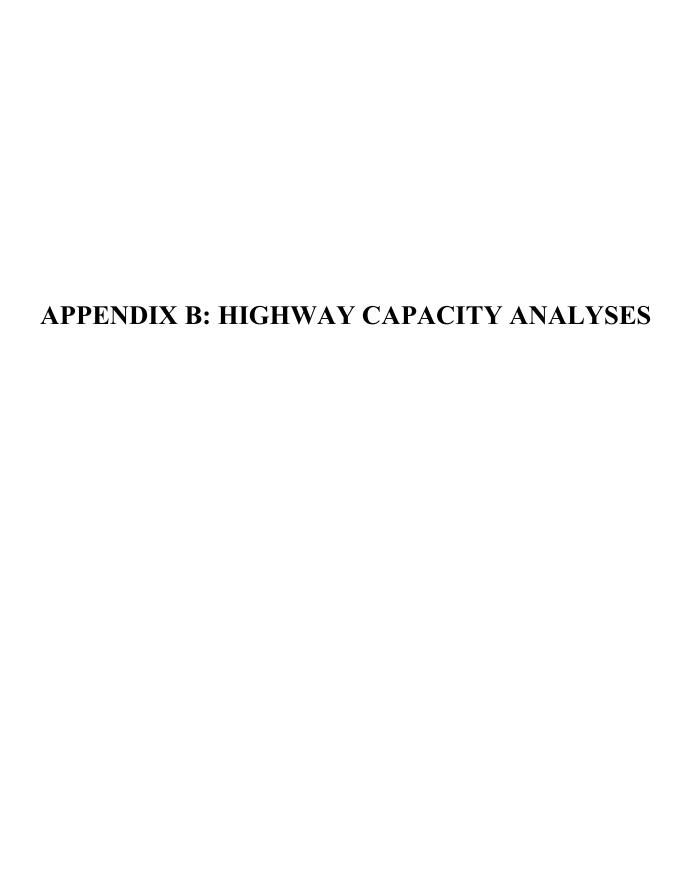
2022 PEAK SEASON FACTOR CATEGORY REPORT - REPORT TYPE: ALL CATEGORY: 2900 COLUMBIA COUNTYWIDE

CITIEC			MOCF: 0.98
WEEK	DATES	SF	PSCF
			1 04
1 2	01/01/2022 - 01/01/2022 01/02/2022 - 01/08/2022	1.02 1.05	1.04 1.07
3	01/02/2022 - 01/08/2022	1.08	1.10
4	01/16/2022 - 01/22/2022	1.07	1.09
5	01/23/2022 - 01/29/2022	1.05	1.07
6	01/30/2022 - 02/05/2022	1.03	1.05
7	02/06/2022 - 02/12/2022	1.02	1.04
8	02/13/2022 - 02/19/2022	1.00	1.02
9	02/20/2022 - 02/26/2022	1.00	1.02
10	02/27/2022 - 03/05/2022	0.99	1.01
11	03/06/2022 - 03/12/2022	0.99	1.01
*12 *13	03/13/2022 - 03/19/2022 03/20/2022 - 03/26/2022	0.98 0.98	1.00
*14	03/20/2022 - 03/26/2022	0.98	1.00
*15	04/03/2022 - 04/09/2022	0.97	0.99
*16	04/10/2022 - 04/16/2022	0.97	0.99
*17	04/17/2022 - 04/23/2022	0.97	0.99
*18	04/24/2022 - 04/30/2022	0.97	0.99
*19	05/01/2022 - 05/07/2022	0.97	0.99
*20	05/08/2022 - 05/14/2022	0.97	0.99
*21	05/15/2022 - 05/21/2022	0.98	1.00
*22	05/22/2022 - 05/28/2022	0.98	1.00
*23 *24	05/29/2022 - 06/04/2022	0.99	1.01
^24 25	06/05/2022 - 06/11/2022 06/12/2022 - 06/18/2022	0.99 1.00	1.01 1.02
26	06/19/2022 - 06/25/2022	1.00	1.02
27	06/26/2022 - 07/02/2022	1.01	1.03
28	07/03/2022 - 07/09/2022	1.02	1.04
29	07/10/2022 - 07/16/2022	1.03	1.05
30	07/17/2022 - 07/23/2022	1.02	1.04
31	07/24/2022 - 07/30/2022	1.01	1.03
32	07/31/2022 - 08/06/2022	1.01	1.03
33	08/07/2022 - 08/13/2022	1.00	1.02
34 35	08/14/2022 - 08/20/2022 08/21/2022 - 08/27/2022	0.99 1.00	1.01 1.02
36	08/28/2022 - 09/03/2022	1.00	1.02
37	09/04/2022 - 09/10/2022	1.01	1.03
38	09/11/2022 - 09/17/2022	1.01	1.03
39	09/18/2022 - 09/24/2022	1.00	1.02
40	09/25/2022 - 10/01/2022	0.99	1.01
41	10/02/2022 - 10/08/2022	0.98	1.00
42	10/09/2022 - 10/15/2022	0.97	0.99
43	10/16/2022 - 10/22/2022	0.98	1.00
44	10/23/2022 - 10/29/2022	0.99	1.01
45 46	10/30/2022 - 11/05/2022 11/06/2022 - 11/12/2022	1.00	1.02
47	11/13/2022 - 11/12/2022	1.01 1.02	1.03 1.04
48	11/20/2022 - 11/26/2022	1.02	1.04
49	11/27/2022 - 12/03/2022	1.02	1.04
50	12/04/2022 - 12/10/2022	1.02	1.04
51	12/11/2022 - 12/17/2022	1.02	1.04
52	12/18/2022 - 12/24/2022	1.05	1.07
53	12/25/2022 - 12/31/2022	1.08	1.10

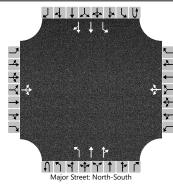
* PEAK SEASON

23-FEB-2023 09:11:19

830UPD 2_2900_PKSEASON.TXT

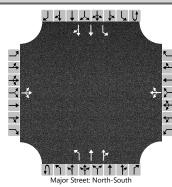


	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	L. Hagen	Intersection	SR 47 & Marvin Burnett Road
Agency/Co.	Hagen Consulting Services	Jurisdiction	Columbia County
Date Performed	10/2/2023	East/West Street	SW Marvin Burnett Road
Analysis Year	2024	North/South Street	SR 47
Time Analyzed	AM Peak Period	Peak Hour Factor	0.89
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	No-build scenario		



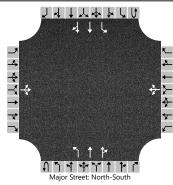
Vehicle Volumes and Ad	_																
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	T	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	1	0	0	1	2	0	0	1	2	0	
Configuration			LTR				LTR			L	Т	TR		L	Т	TR	
Volume (veh/h)		7	2	145		0	1	1	0	96	560	14	0	19	294	5	
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3			
Proportion Time Blocked																	
Percent Grade (%)		(0			()										
Right Turn Channelized																	
Median Type Storage				Left	Only								1				
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1				4.1			
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16				4.16			
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2			
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23				2.23			
Delay, Queue Length, an	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)			173				2			108				21			
Capacity, c (veh/h)			743				252			1213				929			
v/c Ratio			0.23				0.01			0.09				0.02			
95% Queue Length, Q ₉₅ (veh)	Ì	Ì	0.9				0.0			0.3				0.1			
Control Delay (s/veh)			11.3				19.4			8.3				9.0			
Level of Service (LOS)	Ì	Ì	В				С			Α				А			
Approach Delay (s/veh)		11.3				19.4				1.2				0.5			
Approach LOS		В				С				A				A			

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	L. Hagen	Intersection	SR 47 & Marvin Burnett Road
Agency/Co.	Hagen Consulting Services	Jurisdiction	Columbia County
Date Performed	10/2/2023	East/West Street	SW Marvin Burnett Road
Analysis Year	2024	North/South Street	SR 47
Time Analyzed	AM Peak Period	Peak Hour Factor	0.89
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Build scenario		



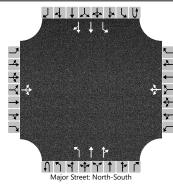
				Majoi	r Street: Nor	tn-Soutn										
ustme	nts															
	Eastb	ound			Westl	oound			North	bound			South	bound		
U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
	10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
	0	1	0		0	1	0	0	1	2	0	0	1	2	0	
		LTR				LTR			L	T	TR		L	Т	TR	
	8	2	152		0	1	1	0	107	560	14	0	19	294	10	
	3	3	3		3	3	3	3	3			3	3			
		0				0										
			Left	Only								1				
adwa	ys															
	7.5	6.5	6.9		7.5	6.5	6.9		4.1				4.1			
	7.56	6.56	6.96		7.56	6.56	6.96		4.16				4.16			
	3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2			
	3.53	4.03	3.33		3.53	4.03	3.33		2.23				2.23			
d Leve	l of S	ervice														
		182				2			120				21			
		732				241			1207				929			
		0.25				0.01			0.10				0.02			
		1.0				0.0			0.3				0.1			
		11.5				20.1			8.3				9.0			
		В		С			A					А				
	11.5				20.1				1.3				0.5			
	В				С				А				A			
	U U	U L 10 0 0 8 8 3 3 7.5 7.56 3.5 3.53 4 Level of So	Eastbound U L T 10 11 0 1 LTR 8 2 3 3 0 0 0 eadways 7.5 6.5 7.56 6.56 3.5 4.0 3.53 4.03 Level of Service 182 732 0.25 1.0 11.5 B B	Eastbound U L T R 10 11 12 0 1 0 LTR 8 2 152 3 3 3 3 0 0 0 Left Cadways 7.5 6.5 6.9 7.56 6.56 6.96 3.5 4.0 3.3 3.53 4.03 3.33 Level of Service 182 732 0.25 1.0 11.5 B B	Eastbound U L T R U 10 11 12 0 1 0 1 0 LTR 8 2 152 3 3 3 3 0 0 0 Left Only Padways 7.5 6.5 6.9 7.56 6.56 6.96 3.5 4.0 3.3 3.53 4.03 3.33 Level of Service 182	Eastbound Westl U L T R U L 10 11 12 7 0 1 0 0 LTR 8 2 152 0 3 3 3 3 3 3 0 0 0 Left Only Padways 7.5 6.5 6.9 7.5 7.56 6.56 6.96 7.56 3.5 4.0 3.3 3.53 3 Level of Service 182 732	Eastbound U L T R U L T 10 11 12 7 8 0 0 1 0 0 1 LTR	Eastbund Westbund	Eastbound Westbound U	Eastbound Westbound Northing	Eastbound Westbound Northbound	Eastbound Westbound Northbound U	Eastbound Westbound Northbound	Eastbound Westbound Northbound South	Eastborn Westborn Northborn Southborn	

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	L. Hagen	Intersection	SR 47 & Marvin Burnett Road
Agency/Co.	Hagen Consulting Services	Jurisdiction	Columbia County
Date Performed	10/2/2023	East/West Street	SW Marvin Burnett Road
Analysis Year	2024	North/South Street	SR 47
Time Analyzed	PM Peak Period	Peak Hour Factor	0.89
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	No-build scenario		



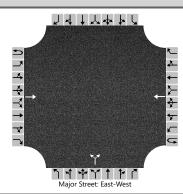
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0	0	1	2	0	0	1	2	0
Configuration			LTR				LTR			L	Т	TR		L	Т	TR
Volume (veh/h)		4	0	268		6	0	2	0	139	455	2	0	6	594	14
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3		
Proportion Time Blocked																
Percent Grade (%)			0			. ()									
Right Turn Channelized																
Median Type Storage				Left	Only								1			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1				4.1		
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16				4.16		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23				2.23		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	T		306				9			156				7		
Capacity, c (veh/h)			634				160			899				1041		
v/c Ratio			0.48				0.06			0.17				0.01		
95% Queue Length, Q ₉₅ (veh)			2.6				0.2			0.6				0.0		
Control Delay (s/veh)			15.9				28.8			9.8				8.5		
Level of Service (LOS)			С				D			А				А		
Approach Delay (s/veh)		1!	5.9	_	28.8				2.3				0.1			
Approach LOS		(С		D				A				А			

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	L. Hagen	Intersection	SR 47 & Marvin Burnett Road
Agency/Co.	Hagen Consulting Services	Jurisdiction	Columbia County
Date Performed	10/2/2023	East/West Street	SW Marvin Burnett Road
Analysis Year	2024	North/South Street	SR 47
Time Analyzed	PM Peak Period	Peak Hour Factor	0.89
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Build scenario		



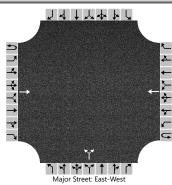
					iviajo	Jueet. Noi	tii-30utii										
Vehicle Volumes and Adju	ustme	nts															
Approach		Eastb	ound			Westl	bound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	1	0	0	1	2	0	0	1	2	0	
Configuration			LTR				LTR			L	Т	TR		L	Т	TR	
Volume (veh/h)		5	0	291		6	0	2	0	153	455	2	0	6	594	29	
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3			
Proportion Time Blocked																	
Percent Grade (%)			0				0										
Right Turn Channelized																	
Median Type Storage				Left	Only								1				
Critical and Follow-up He	adwa	adways															
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1				4.1			
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16				4.16			
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2			
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23				2.23			
Delay, Queue Length, and	l Leve	l of S	ervice	•													
Flow Rate, v (veh/h)			333				9			172				7			
Capacity, c (veh/h)			622				129			886				1041			
v/c Ratio			0.53				0.07			0.19				0.01			
95% Queue Length, Q ₉₅ (veh)			3.2				0.2			0.7				0.0			
Control Delay (s/veh)			17.2				35.0			10.0				8.5			
Level of Service (LOS)			С		E			В					А				
Approach Delay (s/veh)	17.2				35.0				2.5				0.1				
Approach LOS		С				E				А				А			

	HCS Two-Way Stop-Control Report										
General Information		Site Information									
Analyst	L. Hagen	Intersection	Bascom Norris & Marvin Burnett								
Agency/Co.	Hagen Consulting Services	Jurisdiction	Columbia County								
Date Performed	10/2/2023	East/West Street	Bascom Norris Drive								
Analysis Year	2024	North/South Street	Marvin Burnett Road								
Time Analyzed	AM Peak	Peak Hour Factor	0.84								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description	No-build scenario										



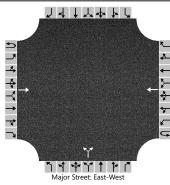
Vehicle Volumes and Ad	justme	nts														
Approach	Т	Eastl	oound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			Т				Т				LR					
Volume (veh/h)			246				262			107		0				
Percent Heavy Vehicles (%)										3		3				
Proportion Time Blocked																
Percent Grade (%))					
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	Т									7.1		6.2				
Critical Headway (sec)										6.43		6.23				
Base Follow-Up Headway (sec)										3.5		3.3				
Follow-Up Headway (sec)										3.53		3.33				
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	Т										127					
Capacity, c (veh/h)											459					
v/c Ratio											0.28					
95% Queue Length, Q ₉₅ (veh)											1.1					
Control Delay (s/veh)											15.8					
Level of Service (LOS)											С					
Approach Delay (s/veh)		•	•			•			15.8							
Approach LOS										(C					

	HCS Two-Way Stop-Control Report										
General Information		Site Information									
Analyst	L. Hagen	Intersection	Bascom Norris & Marvin Burnett								
Agency/Co.	Hagen Consulting Services	Jurisdiction	Columbia County								
Date Performed	10/2/2023	East/West Street	Bascom Norris Drive								
Analysis Year	2024	North/South Street	Marvin Burnett Road								
Time Analyzed	AM Peak	Peak Hour Factor	0.84								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description	Build scenario										



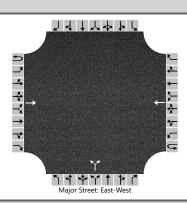
		Major Street: East-We														
Vehicle Volumes and Ad	justme	nts														
Approach	Τ	Eastk	oound			Westl	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			Т				Т				LR					
Volume (veh/h)			246				262			113		0				
Percent Heavy Vehicles (%)										3		3				
Proportion Time Blocked																
Percent Grade (%)										()					
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)										7.1		6.2				
Critical Headway (sec)										6.43		6.23				
Base Follow-Up Headway (sec)										3.5		3.3				
Follow-Up Headway (sec)										3.53		3.33				
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	T										135					
Capacity, c (veh/h)											459					
v/c Ratio											0.29					
95% Queue Length, Q ₉₅ (veh)											1.2					
Control Delay (s/veh)											16.1					
Level of Service (LOS)											С					
Approach Delay (s/veh)									16.1							
Approach LOS										(С					

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	L. Hagen	Intersection	Bascom Norris & Marvin Burnett
Agency/Co.	Hagen Consulting Services	Jurisdiction	Columbia County
Date Performed	10/2/2023	East/West Street	Bascom Norris Drive
Analysis Year	2024	North/South Street	Marvin Burnett Road
Time Analyzed	PM Peak	Peak Hour Factor	0.84
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	No-build scenario		



					Мај	or Street: Ea	ist-West									
Vehicle Volumes and Adj	justme	nts														
Approach	Т	Eastk	oound			Westl	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration			Т				Т				LR					
Volume (veh/h)			215				443			152		0				
Percent Heavy Vehicles (%)										3		3				
Proportion Time Blocked																
Percent Grade (%))					
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)										7.1		6.2				
Critical Headway (sec)										6.43		6.23				
Base Follow-Up Headway (sec)										3.5		3.3				
Follow-Up Headway (sec)										3.53		3.33				
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)											181					
Capacity, c (veh/h)											361					
v/c Ratio											0.50					
95% Queue Length, Q ₉₅ (veh)						Ì					2.7		Ì	Ì		
Control Delay (s/veh)											24.6					
Level of Service (LOS)						Ì					С		Ì	Ì		
Approach Delay (s/veh)								•		24	1.6					
Approach LOS										(<u> </u>					

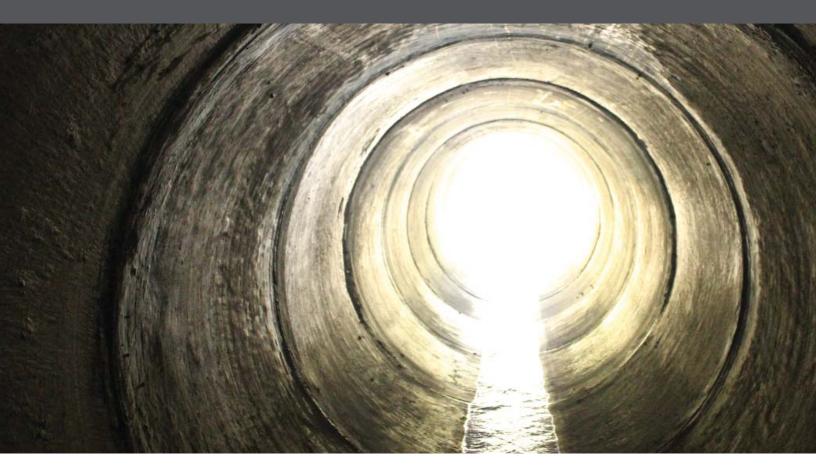
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	L. Hagen	Intersection	Bascom Norris & Marvin Burnett
Agency/Co.	Hagen Consulting Services	Jurisdiction	Columbia County
Date Performed	10/2/2023	East/West Street	Bascom Norris Drive
Analysis Year	2024	North/South Street	Marvin Burnett Road
Time Analyzed	PM Peak	Peak Hour Factor	0.84
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Build scenario		



Vehicle Volumes and Adj	justme	nts															
Approach		Eastk	oound			Westl	bound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0	
Configuration			Т				Т				LR						
Volume (veh/h)			215				443			163		0					
Percent Heavy Vehicles (%)										3		3					
Proportion Time Blocked																	
Percent Grade (%)											0						
Right Turn Channelized																	
Median Type Storage				Undi	vided												
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)										7.1		6.2					
Critical Headway (sec)										6.43		6.23					
Base Follow-Up Headway (sec)										3.5		3.3					
Follow-Up Headway (sec)										3.53		3.33					
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)											194						
Capacity, c (veh/h)											361						
v/c Ratio											0.54						
95% Queue Length, Q ₉₅ (veh)											3.0						
Control Delay (s/veh)											26.0						
Level of Service (LOS)											D						
Approach Delay (s/veh)									26.0								
Approach LOS										I	D						

Stormwater Management System Report

CRS Marvin Burnett



Prepared For: Concept Development, Inc.

Submitted To: City of Lake City and Suwannee River Water Management District

Date: 03/20/2024 PN# 23-0653 PM: Randall S. Olney,

Suite 200 Gainesville, Florida 32607

Address: 1449 SW 74th Drive

www.chw-inc.com



	Engineer's	Certification	Statement
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I hereby certify that the design of the stormwater management systems for the project known as CRS Marvin Burnett has been designed substantially in accordance with the City of Lake City, the Suwannee River Water Management District, and the Florida Department of Transportation applicable rules and regulations.

> Randall Scott Olney, State of Florida, Professional Engineer, License No. 68382

This item has been electronically signed and sealed by Randall Scott Olney, PE. On 03/21/2024 using a Digital Signature.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Digitally signed by Randall Scott Olney
DN: E=randyo@chw-inc.com, CN=Randall Scott Olney,
O=Randall Scott Olney,
L=Alachua, S=Florida, C=US Date: 2024.03.21
13:52:58-04'00'

Randall S. Olney, FL PE No. 68382

03/21/2024

Date

Cont	rents Page
Intro	duction1
Desig	gn Criteria1
Site (Characteristics2
Drain	nage Analysis3
Sumi	mary and Conclusions6
Figu	res
1	Project Location Map
2	USGS Quadrangle Map
3	Aerial Map
4	NRCS Soils Map
5	FEMA Flood Map
6	Pre-Development Drainage Map
7	Post-Development Drainage Map
Appe	endices
A.	Drainage Calculations and Computer Model Output
B.	Operation and Maintenance Requirements and Erosion and Sedimentation Control Requirements
C.	Geotechnical Report

Introduction

The CRS Marvin Burnett project proposes the development of a $\pm 10,640$ sf commercial retail store with associated parking, stormwater, and utility infrastructure. The total proposed site area is ± 2.70 acres, located along the northwest corner of the intersection of State Road 47 and SW Marvin Burnett Road in Lake City, Florida.

The project site is located on a portion of tax parcel #07-4S-17-08127-005 according to the Columbia County Property Appraiser's website. Figure 1 provides a Location Map and Figure 2 depicts the site on a portion of the Lake City West USGS Quadrangle Map. The site is located in Section 7, Township 4 South, Range 17 East in Columbia County, Florida.

Refer to the accompanying engineering plans for details about the proposed construction and demolition regarding this project.

Design Criteria

The design criteria for the proposed stormwater management facility (SMF) is based upon the criteria set forth by the City of Lake City (CLC), the Suwannee River Water Management District (SRWMD), and the Florida Department of Transportation (FDOT) for a dry retention system design in a closed watershed. The criteria are as follows:

- 1. <u>Provide Peak Discharge Rate Attenuation</u>: Attenuate the post-development peak discharge rates to be less than the pre-development peak discharge rates for:
 - a. The 100 year 1 hour, 100 year 2 hour, 100 year 4 hour, 100 year 8 hour, 100 year 24 hour storm events (SRWMD).
 - b. The 3, 5, 10, 25, 50, and 100-year frequency analysis of the 1, 2, 4, 8, 24, 72, 168, and 240-hr storm events (FDOT).
- 2. <u>Provide Peak Discharge Volume Attenuation</u>: Attenuate the post-development peak discharge volumes to be less than the pre-development peak discharge volume for:
 - a. The 100 year 1 hour, 100 year 2 hour, 100 year 4 hour, 100 year 8 hour, 100 year 24 hour storm events (SRWMD).
 - b. The 3, 5, 10, 25, 50, and 100-year frequency analysis of the 1, 2, 4, 8, 24, 72, 168, and 240-hr storm events (FDOT).
- 3. <u>Provide Water Quality Treatment Volume (WQTV)</u>: The minimum stormwater treatment volume shall be the runoff from the first 2.0 inch of runoff from the design storm. WQTV must be recovered within 72 hours (SRWMD).
- 4. <u>Freeboard:</u> Retention ponds shall have a freeboard of 1 foot above the maximum stage in order to function properly during storms greater than the design storm (SRWMD).
- 5. <u>Provide Volume Recovery:</u> Retention systems must have one-half of the total volume available within 7 days following the end of the design storm event, and the total volume must be recovered within 30 days following the end of the storm event (SRWMD and FDOT).

- Alternatively, if recovery requirements cannot be met, back-to-back storms can be routed through the system (SRWMD).
- 6. <u>Fencing:</u> Any water retention areas that have a potential of holding water in excess of one (1) foot depth to be fenced with a four (4) foot high fence and screened by trees or shrubbery (CLC).

City of Lake City, SRWMD, and FDOT also require that best management practices be employed to control erosion, sedimentation, and that an operation and maintenance entity be established.

Site Characteristics

Physical characteristics of the site are described in the following sections. Additional details are provided in the accompanying Engineering plans.

Site Topography

The existing site is undeveloped and heavily wooded with existing pavement and structures that are to be removed. The project site is bordered by a single-family residence to the west, a church to the north, State Road 47 to the east, and Marvin Burnett Road to the south. The site is sloped from the northeast to the southwest. Site topography ranges from EL. \pm 167.00' (NAVD 88) in the northeast corner of the site to EL. \pm 152.60' (NAVD 88) in the southwest corner.

Please refer to the accompanying engineering plans for details.

Pre-Development Drainage

Pre-development drainage consists of two watersheds: Pre-Development Watershed #1 (Pre DA-1) and Pre-Development Watershed #2 (Pre DA-2). Pre DA-1 is ± 2.03 acres in size and includes a portion of offsite area to the north of the site as well as most of the western portion of the project site. Stormwater runoff from Pre DA-1 flows via sheet flow and shallow concentrated flow to a natural low area along the western boundary of the site. Pre DA-2 is ± 1.79 acres in size and includes offsite area to the north as well as the eastern portion of the project site. Stormwater runoff from Pre DA-2 flows via sheet flow and shallow concentrated flow into the SR-47 (FDOT) storm sewer system.

Refer to Figure 4 for a NRCS Soils Map. Refer to Figure 6 for more information on the predevelopment watershed.

Post-Development Drainage

Post-Development drainage consists of two watersheds: Post-Development Watershed #1 (Post DA-1) and Post-Development Watershed #2 (Post DA-2). Post DA-1 comprises ±3.29 acres including ±0.93 acres of impervious area as well as a portion of offsite area. Stormwater runoff from Post DA-1 will be routed via sheet flow and shallow concentrated flow to a stormwater pipe conveyance system and into the proposed stormwater management facility (SMF-1). Post DA-2 comprises

 ± 0.53 acres including ± 0.02 acres of impervious area from a small portion of sidewalk. Stormwater runoff from Post DA-2 will be routed via sheet flow and shallow concentrated flow to the SR-47 (FDOT) storm sewer system as in the pre-development condition. The drainage area discharging to the FDOT system is greatly reduced in comparison to pre-development. Additionally, the CN of this area did not increase. Therefore, it is assumed that runoff rates and volumes have been reduced for each design storm event and these watersheds were not included in the drainage model.

SMF-1 is designed as a dry retention facility that will retain and infiltrate the difference between pre-development and post-development runoff volume. The top of bank for SMF-1 is set at EL. 160.00° while the bottom of pond is at EL. 157.00° with 4:1 side slope. The resulting total storage volume is $\pm 49,744$ cf. An underdrain system is proposed to lower the seasonal high-water table and meet recovery requirements. An outfall structure has been provided, which enables discharge to the existing depression beyond the western border of the site, mimicking the pre-development drainage patterns.

Refer to Figure 7 for more information on the post-development watershed.

Soils Information

The National Resource Conservation Service (NRCS) Soil Survey for Columbia County describes the near surface soil profile for the project area as *Blanton fine sand* (0-5% slopes) of hydrologic soil group rating of 'A', *Ichetucknee fine sand* (5-8% slopes) of hydrologic soil group rating of 'D', *Mascotte fine sand* of hydrologic soil group rating of 'B/D', *Pelham fine sand* (0-2% slopes) of hydrologic soil group rating of 'B/D'. Refer to Figure 4 for the NRCS Soils Map.

A site-specific soils investigation was conducted by GSE Engineering & Consulting, Inc. on October 11th, 2023 and the report was later revised on December 7th, 2023. Based on the Summary Report of Geotechnical Site Exploration, the following design parameters were recommended for the stormwater management facility calculations. Refer to Appendix C for further details.

SMF-1

- Average ground elevation of borings within proposed SMF-1 area: 156.70' (NAVD 88)
- Base elevation of effective or mobilized aquifer: 148.70' (NAVD 88)
- Average seasonal high groundwater table elevation: *152.99' (NAVD 88)
- Horizontal hydraulic conductivity: 10 feet per day (5 feet per day used in calculations)
- Unsaturated vertical infiltration rate: 10 feet per day (5 feet per day used in calculations)
- Specific yield (fillable porosity): 20%

Drainage Analysis

The proposed stormwater management system (SMF-1) has been designed to provide attenuation of the discharge rates and volumes for the 100 year - 1 hour, 100 year - 2 hour, 100 year - 4 hour, 100 year - 8 hour, and 100 year - 24 hour storm events. Since the portion of the site draining towards the FDOT ROW (Post DA-2) is minimal and has been reduced from its pre-development

^{*}Seasonal high-water table established based on highest invert of the underdrain system.

condition (Pre DA-2), the FDOT storms were not modeled. SMF-1 should recover one-half of the total volume available within 7 days following the end of the design storm event, and the total volume must be recovered within 30 days. Additionally, the stormwater management system is designed to retain the water quality treatment volume and recover this volume within 72 hours.

Appendix A contains details and calculations as well as a section for routing results, recovery analysis, hydraulic calculations, and general drainage calculations.

Analysis Methodology

The drainage analysis was conducted using the computer program PONDS (v3.3) to generate runoff hydrographs and route the runoff hydrographs through the proposed stormwater system. The required storm events were analyzed using SRWMD rainfall amounts for the predevelopment and post-development watersheds.

Unit Hydrograph Parameters

Unit hydrograph parameters required for the drainage analysis include run-off curve number (CN), time of concentration (Tc), and drainage area. Values used in the analysis are summarized as follows:

Pre-L	Jeve	ors	pment	Watershed #1	(Pre DA-1)):
		_	*.			

Watershed Area =	2.03 ac.
Impervious Area (Existing) =	0.02 ac.
Woods (Good, Type 'A' Soil) =	0.47 ac.
Woods (Good, Type 'D' Soil) =	1.55 ac.

CN = 66Tc = 29 min.

Post-Development Watershed #1 (Post DA-1):

Watershed Area =	3.29 ac.
Impervious Area =	0.93 ac.
Stormwater Management Facility =	0.45 ac.
Open Space (Good, Type 'A' Soil) =	1.29 ac.
Open Space (Good, Type 'D' Soil) =	0.62 ac.

CN = 72Tc = 10 min.*

Pond Storage

Stage-storage values for the proposed stormwater management facilities are provided in Appendix A.

^{*}Time of Concentration is assumed to be 10 minutes.

Water Quality Treatment Volume (WQTV)

Per SRWMD, the required water quality treatment volume (WQTV) required for a dry retention system is 2.0 inch of runoff over the drainage area, that must draw down within 72 hours. The WQTV calculations and modeling results are summarized in Table 1, additional details can be found in appendix A.

Table 1: Post Development Watershed Water Quality Treatment

Stormwater Management Facility	Required WQTV (cf)	Peak Elevation at WQTV (ft)	Time to Recover WQTV (hours)
SMF-1	12,483	157.87	< 6

Run-off and Facility Routing Results

The routing results for Pre DA-1 and Post DA-1 (SMF-1) are summarized in Tables 2 and 3. Table 2 displays the peak stage, freeboard, and recovery time for the analyzed storm events, while Table 3 displays the discharge rates and volumes for pre and post-development. Detailed results and calculations are provided in Appendix A.

Table 2: Pre DA-1 vs. Post DA-1 Routing Results

Storm Event	Peak Stage (ft.)	Freeboard (ft)	Full Volume Recovery (days after storm)
SRWMD 100YR-1HR	158.14	1.86	< 1
SRWMD 100YR-2HR	158.35	1.65	< 1
SRWMD 100YR-4HR	158.46	1.54	< 1
SRWMD 100YR-8HR	158.61	1.39	< 4
SRWMD 100YR-24HR	158.78	1.22	< 7

Table 3: Pre DA-1 vs. Post DA-1 Attenuation Results

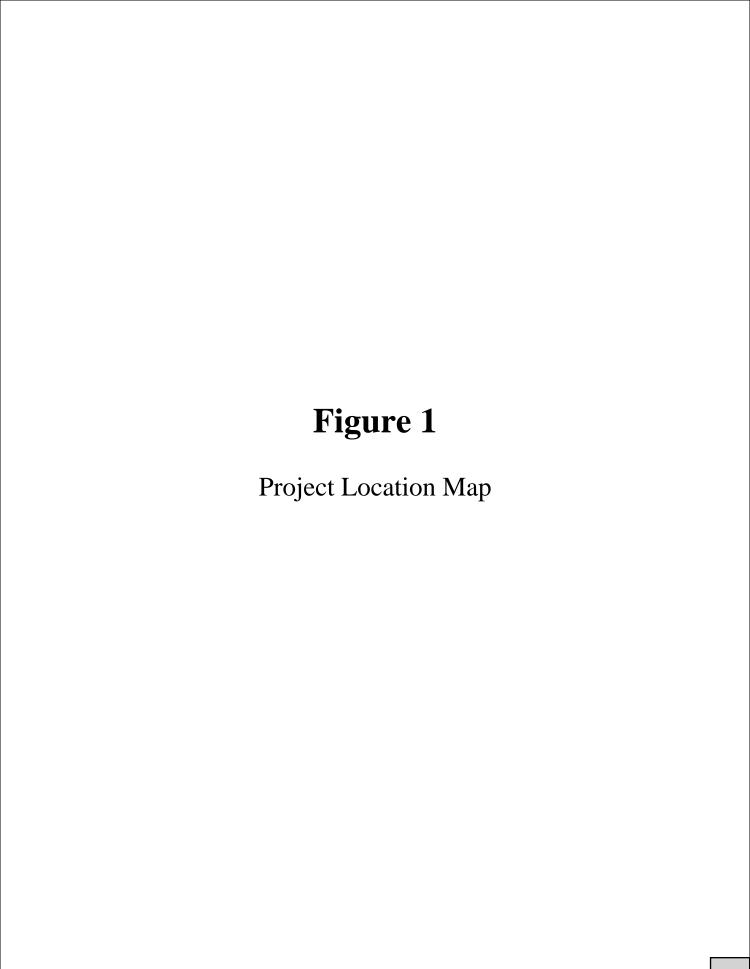
Storm Event	Discharge Rates (cfs)		Discharge Volumes (cf)			
	Pre	Post	Change	Pre	Post	Change
SRWMD 100YR-1HR	4.27	0.31	-3.96	8,901	463	-8439
SRWMD 100YR-2HR	5.47	1.28	-4.19	13,242	4,512	-8730
SRWMD 100YR-4HR	6.46	1.93	-4.53	18,428	10,552	-7876
SRWMD 100YR-8HR	7.25	3.00	-4.25	25,727	17,171	-8556
SRWMD 100YR-24HR	6.67	4.28	-2.39	40,983	37,974	-3009

Summary and Conclusions

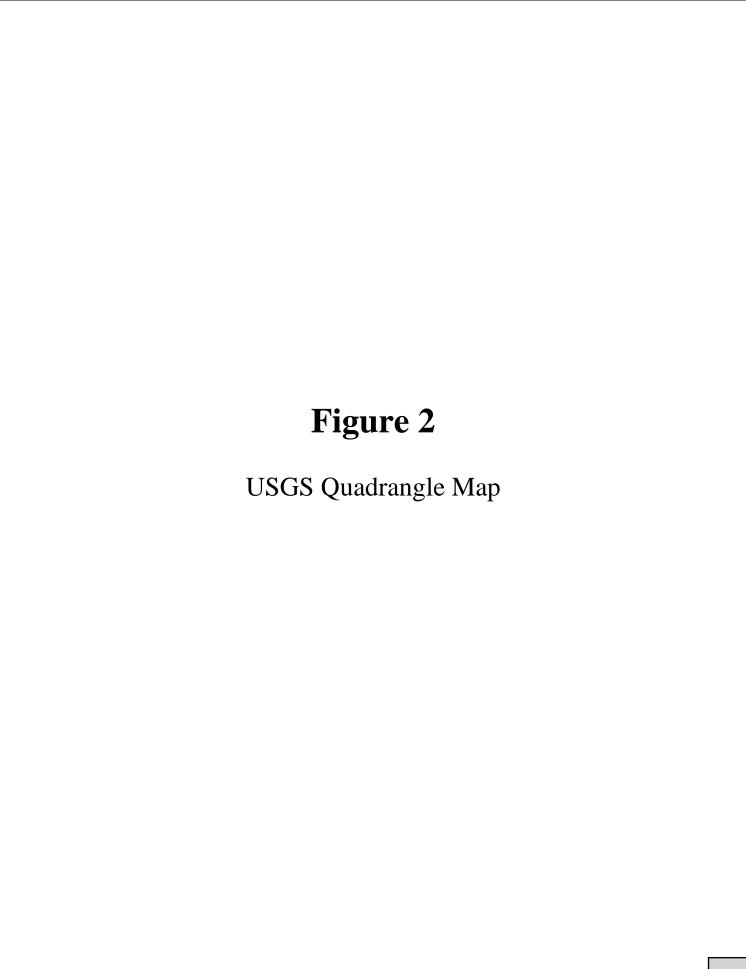
The proposed drainage system meets CLC, SRWMD, and FDOT criteria for dry retention system designs in a closed watershed. The criteria are as follows:

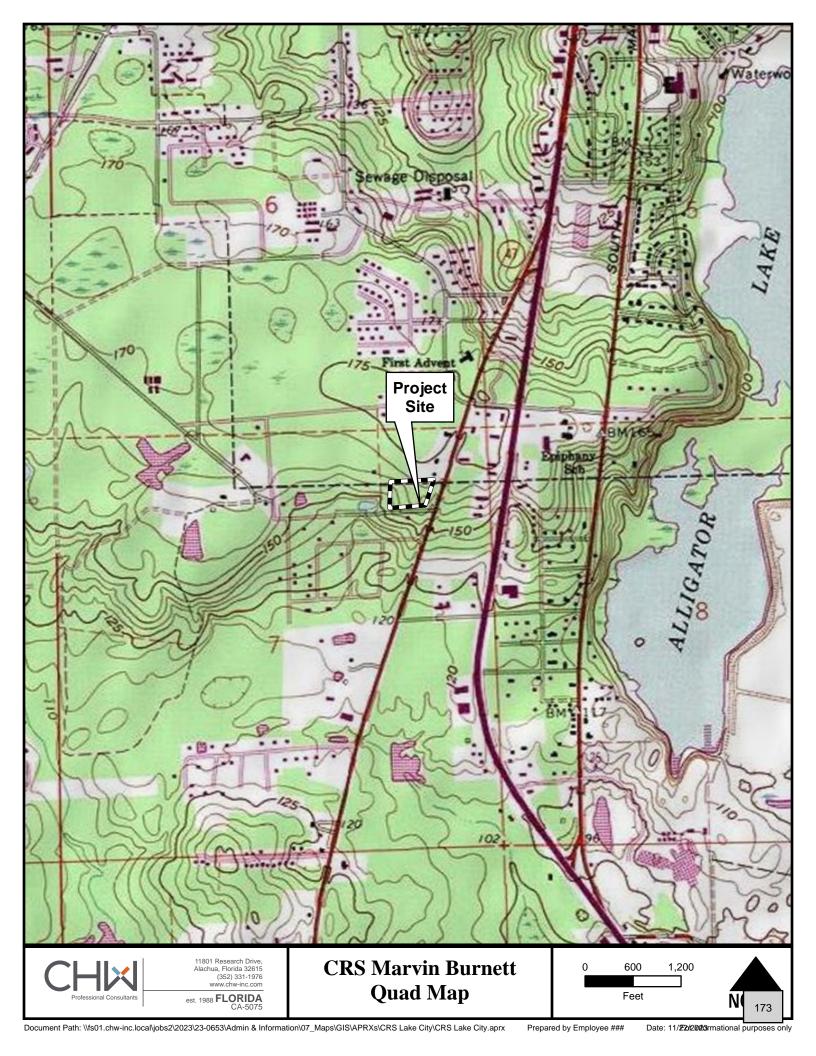
- 1. <u>Provide Peak Discharge Rate Attenuation</u>: SMF-1 attenuates the post-development peak discharge rates to be less than the pre-development peak discharge rates for:
 - a. The 100 year 1 hour, 100 year 2 hour, 100 year 4 hour, 100 year 8 hour, 100 year 24 hour storm events (SRWMD).
 - b. The 3, 5, 10, 25, 50, and 100-year frequency analysis of the 1, 2, 4, 8, 24, 72, 168, and 240-hr storm events (FDOT).
- 2. <u>Provide Peak Discharge Volume Attenuation</u>: SMF-1 attenuates the post-development peak discharge volumes to be less than the pre-development peak discharge volume for:
 - a. The 100 year 1 hour, 100 year 2 hour, 100 year 4 hour, 100 year 8 hour, 100 year 24 hour storm events (SRWMD).
 - b. The 3, 5, 10, 25, 50, and 100-year frequency analysis of the 1, 2, 4, 8, 24, 72, 168, and 240-hr storm events (FDOT).
- 3. <u>Provide Water Quality Treatment Volume (WQTV)</u>: SMF-1 has been designed to retain the runoff from the first 2.0 inch of runoff from the design storm. WQTV is recovered within 72 hours. (SRWMD).
- 4. <u>Freeboard:</u> SMF-1 provides 1 foot of freeboard above the maximum stage in order to function properly during storms greater than the design storm (SRWMD).
- 5. <u>Provide Volume Recovery:</u> SMF-1 provides half of the total available volume within 7 days after the end of all storm events, and provides the total available volume within 30 days after the end of all storm events (SRWMD and FDOT).
- 6. <u>Fencing:</u> SMF-1 has the potential to hold water in excess of one (1) foot depth, therefore a (4) foot high fence and sufficient screening by trees and shrubbery is proposed. (CLC).

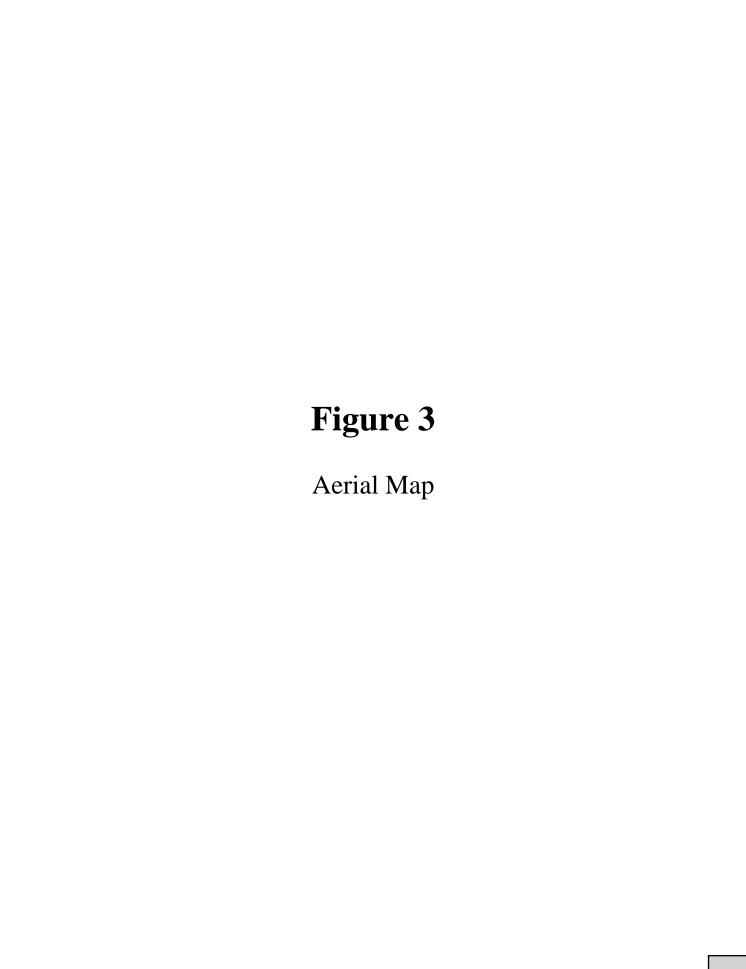
Based on the information provided, the project is eligible for approval by City of Lake City, SRWMD, and FDOT.

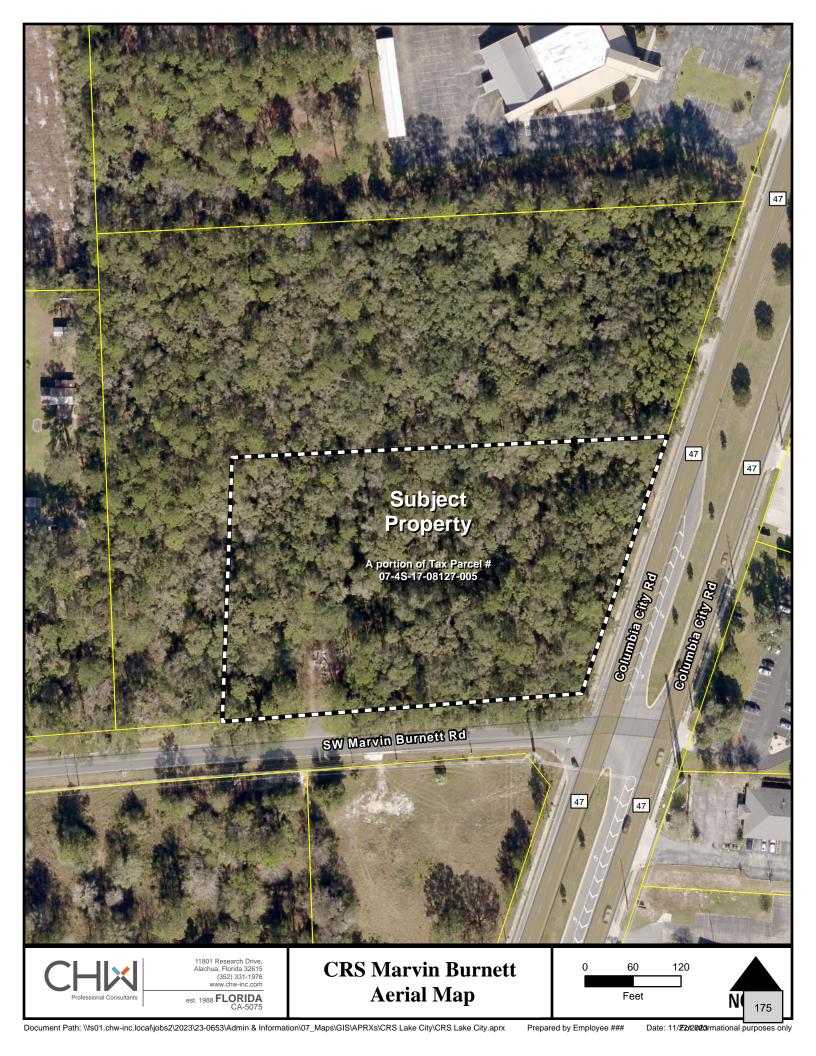


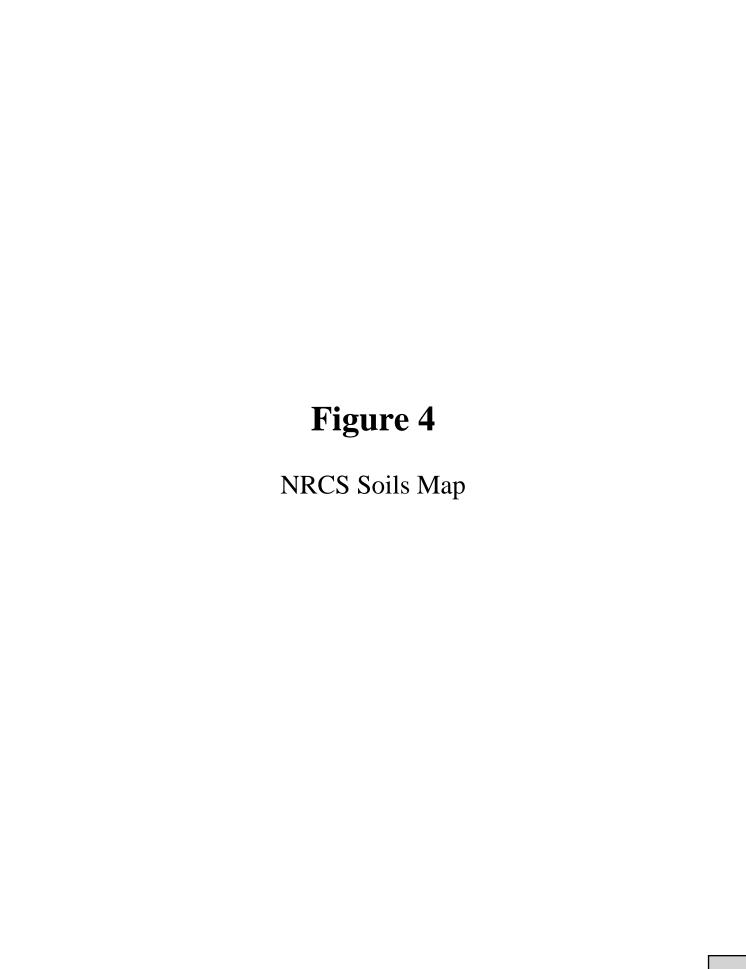
Project Location Map CRS Marvin Burnett Columbia County SW Marvin Burnett Rd

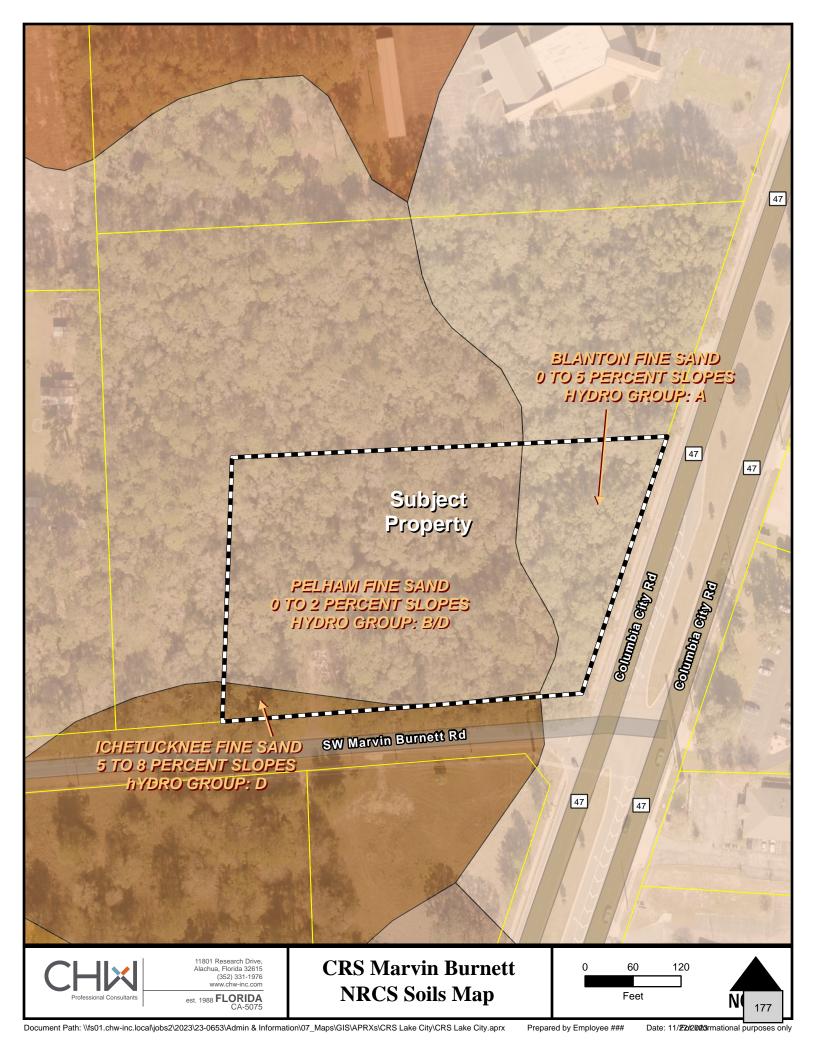


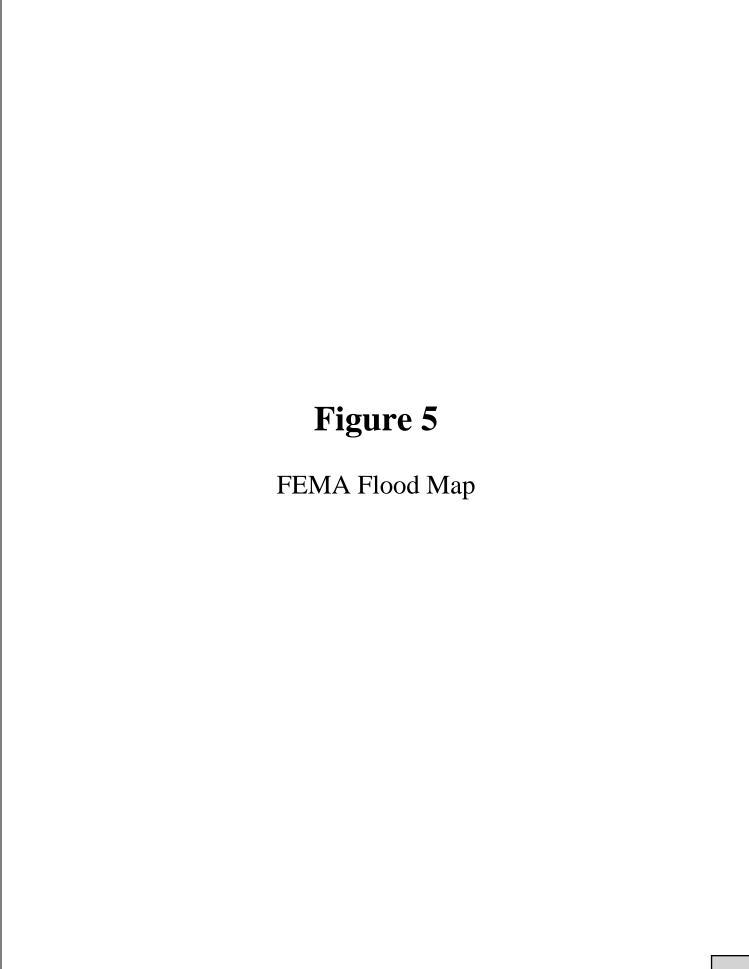


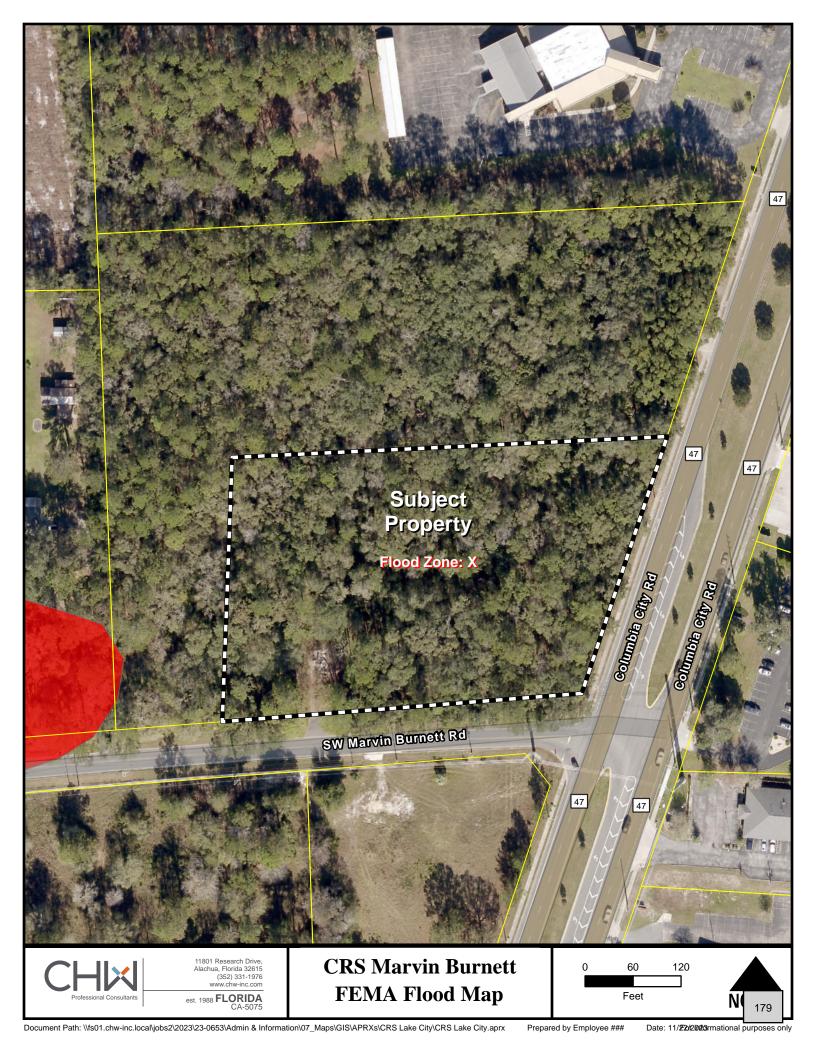


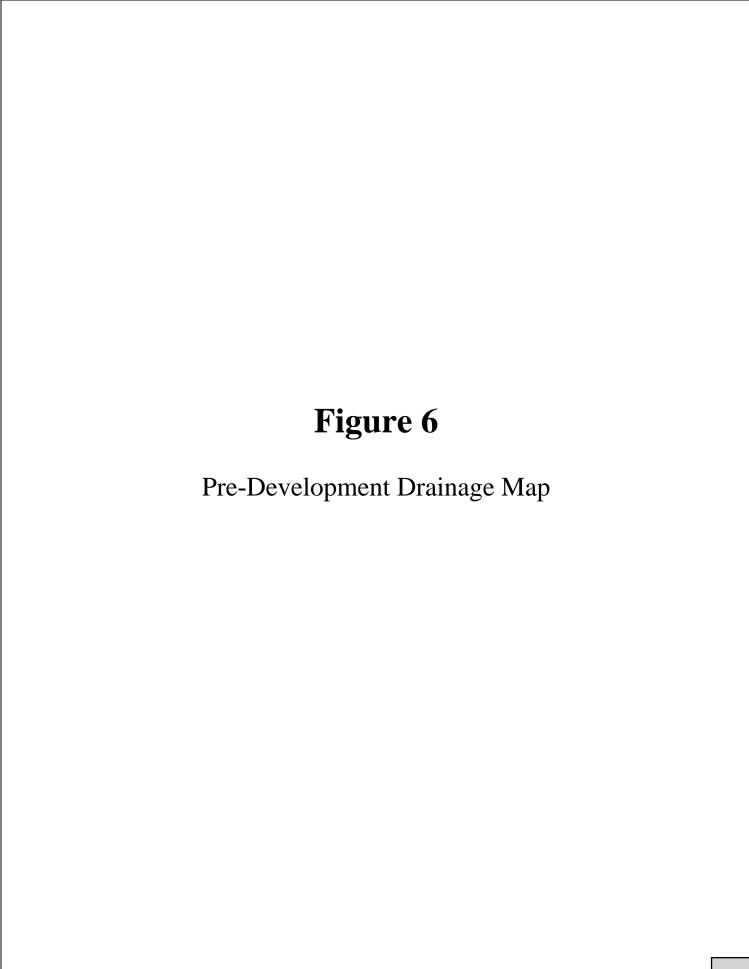


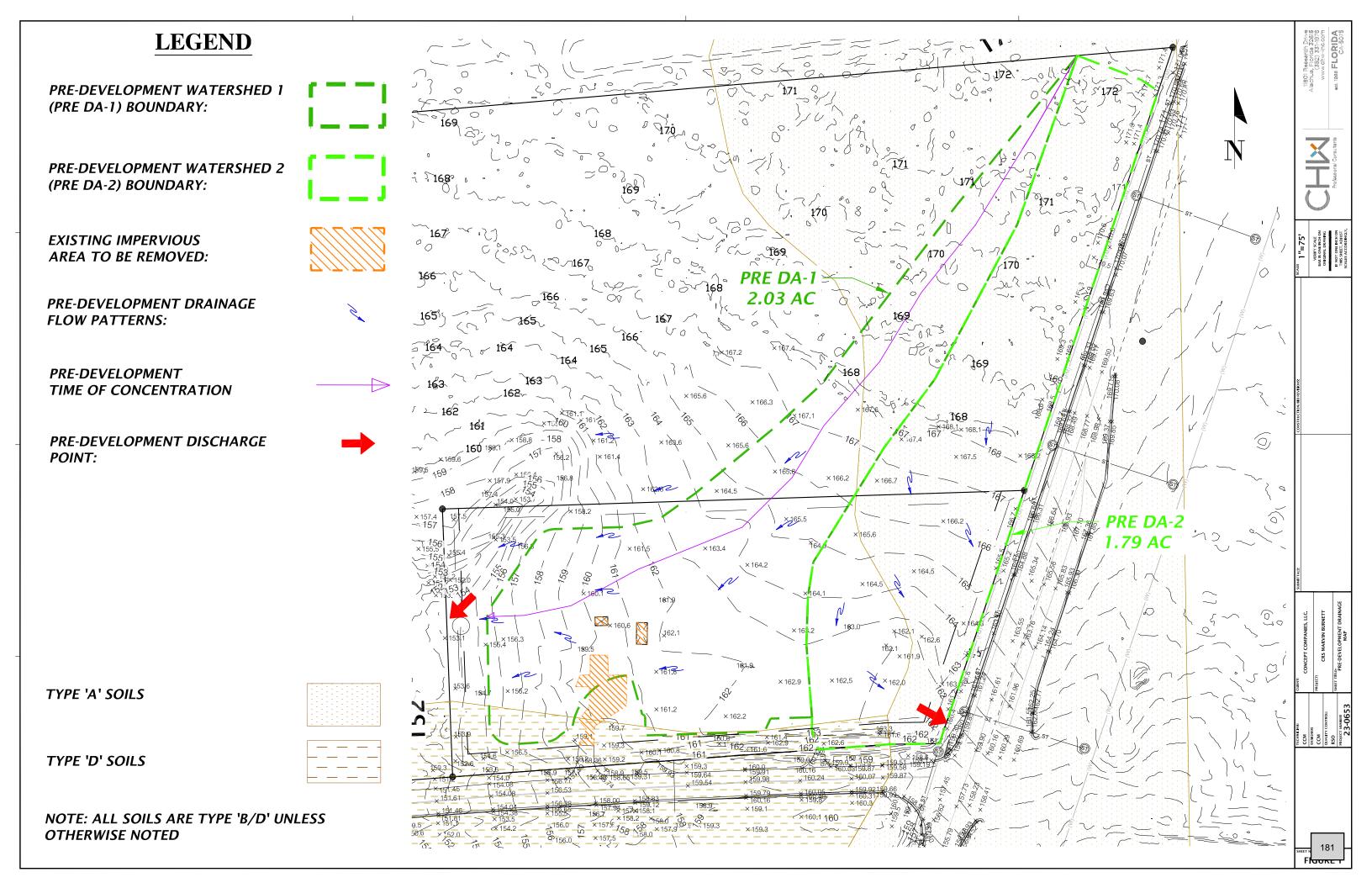


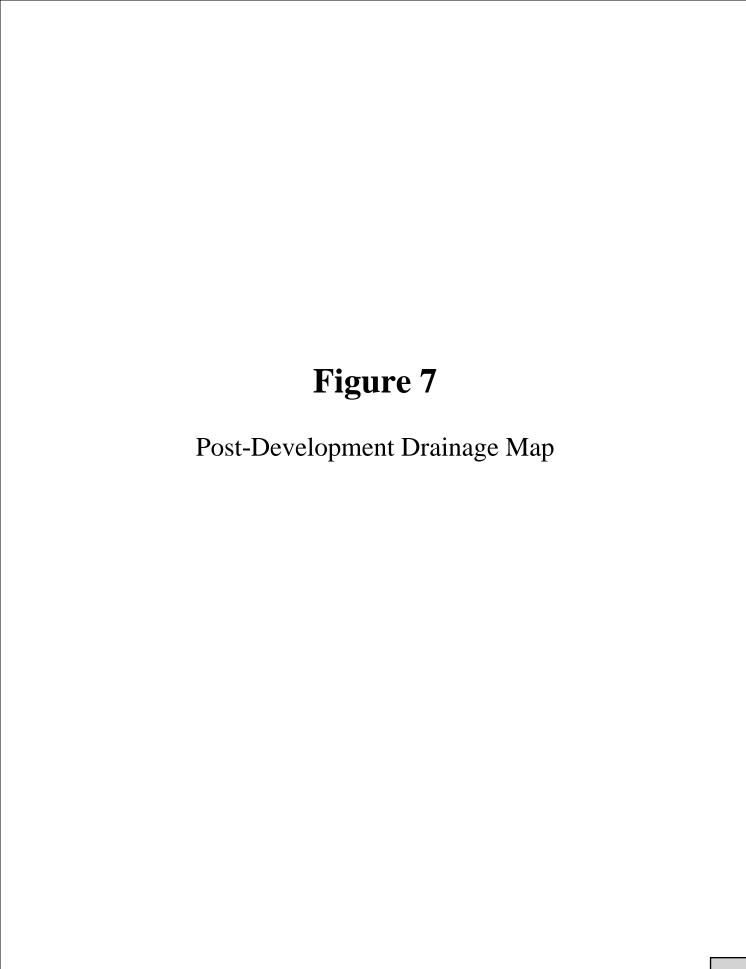


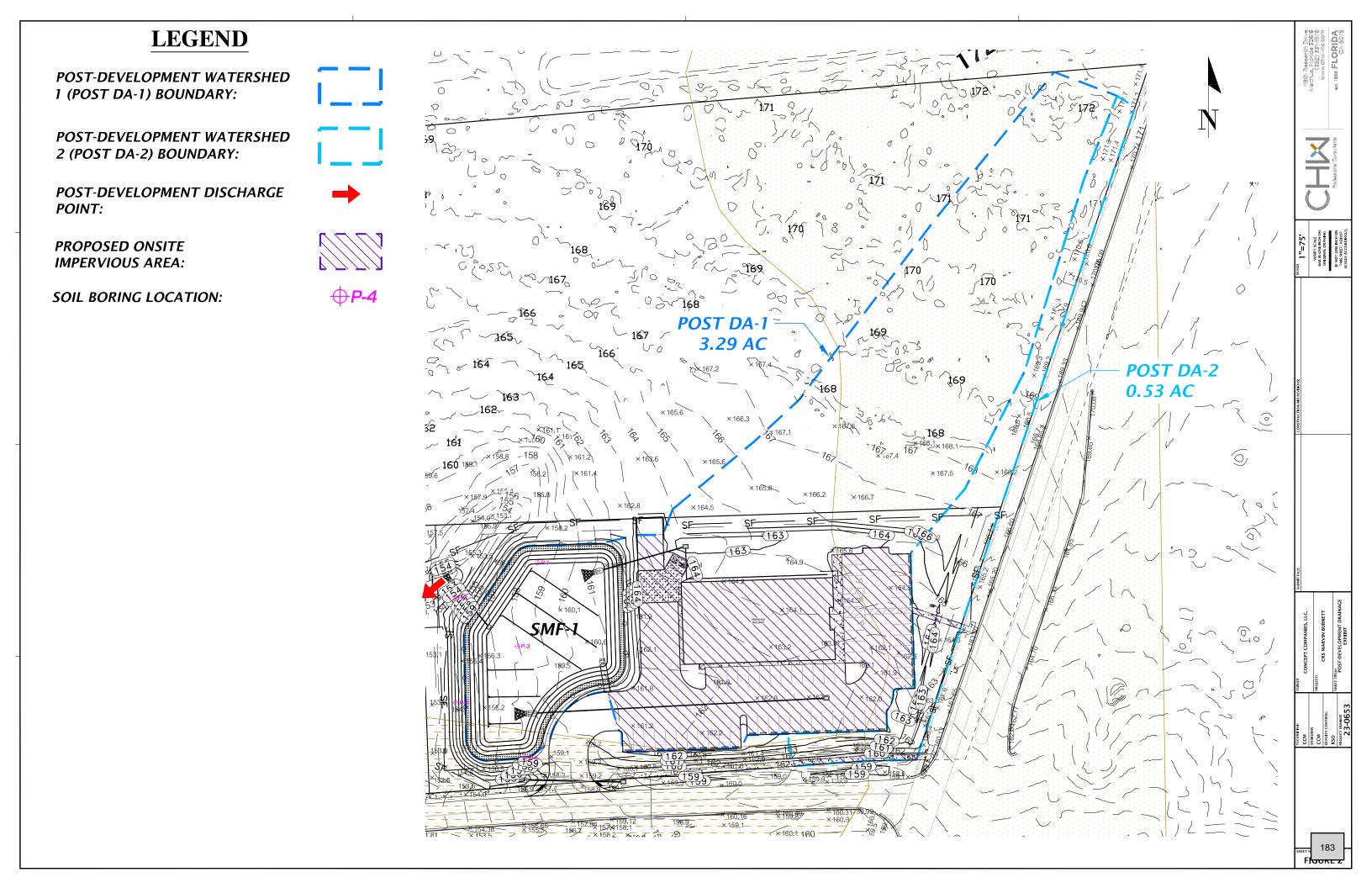


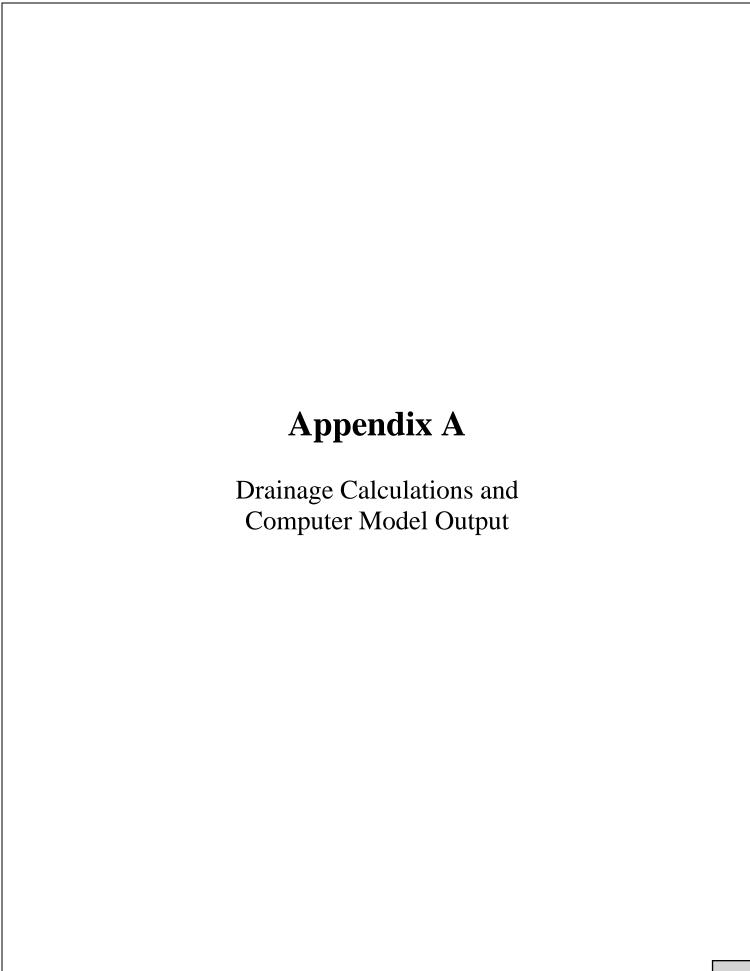














Project Number: 23-0653
Project Name: CRS Marvin Burnett

Calculated by: JHP Checked by: CCM Date: 1/4/2024

CURVE NUMBER CALCULATIONS:

Pre DA-1								
Total Area:	88,582	s.f.	2.03	ac.	CN	CN * Area	С	C * Area
Woods (Good, Group "A" Soil)	20,340	s.f.	0.47	ac.	30	610200	0.2	4068
Woods (Good, Group "D" Soil)	67,311	s.f.	1.55	ac.	77	5182947	0.2	13462.2
Exisitng Impervious Area	931	s.f.	0.02	ac.	98	91238	0.95	884.45

Weighted C: **0.21** Weighted CN: **66**

Time of Concentration: 29 minutes

Post DA-1								
Total Area:	143,411	s.f.	3.29	ac.	CN	CN * Area	С	C * Area
Open Space (Good, Group "A" Soil)	56,007	s.f.	1.29	ac.	39	2184273	0.2	11201.4
Open Space (Good, Group "D" Soil)	27,102	s.f.	0.62	ac.	80	2168160	0.2	5420.4
Impervious Area	40,483	s.f.	0.93	ac.	98	3967334	0.95	38458.85
Stormwater Management Facility	19,819	s.f.	0.45	ac.	100	1981900	1	19819

Weighted C: **0.52** Weighted CN: **72**

Time of Concentration: 10 minutes

WQTV CALCULATIONS: SMF-1 (Dry Retention)

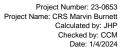
SRWMD WQTV Calculation:							
Runoff from the first 2.0" of rainfall							
2" x Drainage Area:	23901.83	c.f.					
C =	0.52						
SRWMD WQTV:	12,483	c.f.					

		Pre DA-2	2				
Total Area:		77,911	s.f.	1.79	ac.	CN	CN * Area
	Woods (Good, Group "A" Soil)	62,699	s.f.	1.44	ac.	30	1880970
	Woods (Good, Group "D" Soil)	15,212	s.f.	0.35	ac.	77	1171324

Weighted CN: 39

Post DA-2							
Total Area:	23,090	s.f.	0.53	ac.	CN	CN * Area	
Woods (Good, Group "A" Soil)	9,873	s.f.	0.23	ac.	30	296190	
Open Space (Good, Group "A" Soil)	11,246	s.f.	0.26	ac.	39	438594	
Open Space (Good, Group "D" Soil)	1,162	s.f.	0.03	ac.	80	92960	
Impervious Area	809	s.f.	0.02	ac.	98	79282	

Weighted CN: 39





Tc CALCULATIONS:

		SHEE	T FLOW			SHALLOW CONCENTRATED FLOW						CHAI	NNEL / P	IPE FLOW	ı			
		Flow	2-Year	Land		Paved	Flow	Water-	Avg.		Cross-	Wetted	Hydraulic	Pipe		Avg.	Flow	
BASIN	Manning's	Length	24-Hour	Slope	Tt1	or	Length	course	Velocity		Section	Perim.	Radius	Slope	Manning	Velocity	Length	Tt3
	n		Rain, P2			Unpvd.	L	Slope, s	V		Area, a	Pw	r		n		L	
	()	(ft)	(in)	(ft/ft)	(hr)	(P or U)	(ft)	(ft/ft)	(ft/s)	(hr)	(ft^2)	(ft)	(ft)	(ft/ft)	()	(ft/s)	(ft)	(hr)
Pre DA-1	0.4	100	4.2	0.011	0.40	U	694	0.023	2.44	0.08	-		-	-	-	-	-	-

ID	Тс	Тс
	(hr)	
PRE DA-1	0.48	29

If Tc less than 10 minutes, 10 minutes was assumed per FDOT standards

TIME OF CONCENTRATION VALUES DETERMINED USING TR-55 METHODOLOGY.

SHEET FLOW:

$$Tt = \frac{0.007 \text{ (nL)}^{0.8}}{(P2)^{0.5} \text{s}^{0.4}}$$

SHALLOW CONCENTRATED FLOW:

1. For slopes < 0.005 ft/ft
Unpaved V=16.1345 s^{0.5}
Paved V=20.3282 s^{0.5}

2. For slopes > 0.005 ft/ft Velocity per Figure 3-1, TR-55 CHANNEL/PIPE FLOW:

$$V = \frac{1.49r^{2/3}s^{1/2}}{n}$$

$$Tt = \frac{L}{3600 \text{ V}}$$



Project Number: 23-0653
Project Name: CRS Marvin Burnett

Calculated by: JHP Checked by: CCM Date: 1/4/2024

STAGE-STORAGE CALCULATIONS:

Post-Development: SMF-1 Stage-Storage Relationship											
ELEV.	AREA (SF)	AREA (AC.)	STORAGE (CF)	STORAGE VOLUME (AC-FT)							
157.00	13,469	0.3092	0	0.00							
158.00	15,494	0.3557	14,482	0.33							
159.00	159.00 17,606 0.4042 31,032 0.71										
160.00	19,819	0.4550	49,744	1.14							

Geote	ch Borings
Boring #	Ex. Grade EL.
P-1	158.50
P-2	154.00
P-3	158.00
P-4	154.75
P-5	158.25
Avg.	156.70

WQTV =	12,483 cf	SHWT =	*152.99	ft
WQTV EL. =	157.86 ft	Confining Layer =	148.70	ft
		Kv =	5.00	ft/day
Weir Elevation =	158.00 ft	Kh =	5.00	ft/day
		Porosity =	20	%
Eq. Length =	200 ft			
Eq. Width =	83 ft	Depth =	3.00	ft
		Perimeter =	566	ft

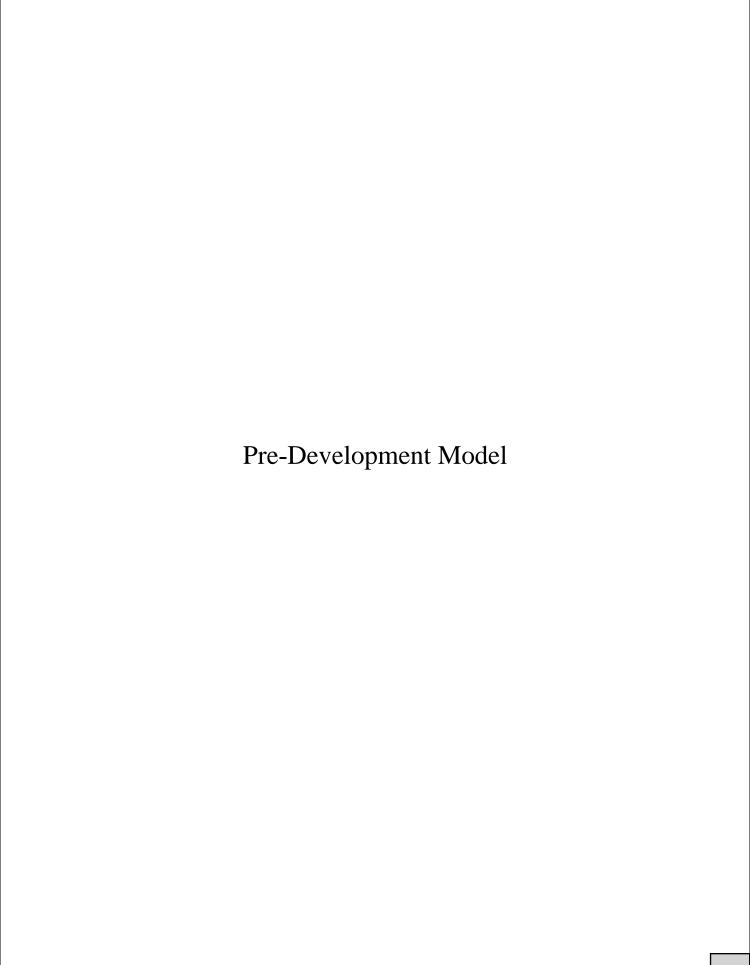
*Established based on Invert of Underdrain System



PIPE CALCS: CRS Marvin Burnett (23-0653)

														Q		V - Full		Minor		l .			ToG/	
Structu	ure No.	Invert	Elev.	Length	Slope	Dia.	C	Tc	i	Α	Α	Q (cfs	Actual	Allowed	Pipe A	Flow	Pipe R	Loss	Loss	Loss	H	GL	EoP	F.B.
From	To	U.S.	D.S.	(ft)	(ft/foot)	(in)		(min)	(in/hr)	(sf)	(ac)	Inc	Cumul	(cfs)	(sq-ft)	(fps)	(ft)	Coeff.	(ft)	(ft)	U.S.	D.S.		(in)
S-2	S-1	158.38	157.00	277	0.0050	15	0.95	10	6.2	18163	0.42	2.5	2.5	4.94	1.2	4.0	0.31	0.5	0.03	0.3	158.51	158.14	162.23	45
C/O-1	C/O-2	160.61	159.96	65	0.0100	12	0.95	10	6.2	3547	0.08	0.5	0.5	3.86	0.8	4.9	0.25	0.5	0.00	0.0	160.93	160.91	163.72	34
C/O-2	C/O-3	159.96	159.30	65	0.0102	12	0.95	10	6.2	3547	0.08	0.5	1.0	3.89	0.8	5.0	0.25	0.5	0.01	0.0	160.91	160.86	163.85	35
C/O-3	S-4	159.30	159.00	30	0.0100	12	0.95	10	6.2	3547	0.08	0.5	1.4	3.86	0.8	4.9	0.25	0.8	0.04	0.0	160.86	160.78	164.03	38
S-4	S-3	159.00	157.00	86	0.0233	15	0.80	10	6.2	76339	1.75	8.7	10.1	10.67	1.2	8.7	0.31	0.8	0.85	1.8	160.78	158.14	162.12	16

- 1. ToG = Top of Grate/EoP = Edge of Pavement
- 2. FB = Free Board
- 3. Rainfall intensity is based on the FDOT Zone 3 Rainfall Intensity-Duration-Frequency curve for the 3 year 10 min storm event (6.2 inches/hr)
- 4. The tailwater condition was set at the peak stage for the 100 year 1 hour storm event of the receiving SMF.



Project Data

Project Name: CRS Marvin Burnett

Simulation Description: Pre-Development

Project Number: 23-0653

Engineer: Jarrett Pearson

Supervising Engineer: Cole Menhennett

Date: 12-28-2023

Scenario Input Data

Scenario 1 :: SRWMD 100YR-1HR

Hydrograph Type: Inline SCS
• Modflow Routing: Not routed

Repetitions: 1

Basin Area (acres) 2.030
Time Of Concentration (minutes) 29.0
DCIA (%) 0.0
Curve Number 66
Design Rainfall Depth (inches) 4.2
Design Rainfall Duration (hours) 1.0
Shape Factor UHG 484

Rainfall Distribution SCS Type II Florida Modified

Initial ground water level (ft datum) 154.70 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.250	6.250	12.250	18.250	24.250
0.500	6.500	12.500	18.500	24.500
0.750	6.750	12.750	18.750	24.750
1.000	7.000	13.000	19.000	25.000
1.250	7.250	13.250	19.250	25.250
1.500	7.500	13.500	19.500	25.500
1.750	7.750	13.750	19.750	25.750
2.000	8.000	14.000	20.000	26.000
2.250	8.250	14.250	20.250	26.250
2.500	8.500	14.500	20.500	26.500
2.750	8.750	14.750	20.750	26.750
3.000	9.000	15.000	21.000	27.000
3.250	9.250	15.250	21.250	27.250
3.500	9.500	15.500	21.500	27.500
3.750	9.750	15.750	21.750	27.750
4.000	10.000	16.000	22.000	28.000
4.250	10.250	16.250	22.250	28.250
4.500	10.500	16.500	22.500	28.500
4.750	10.750	16.750	22.750	28.750
5.000	11.000	17.000	23.000	29.000
5.250	11.250	17.250	23.250	29.250
5.500	11.500	17.500	23.500	29.500
5.750	11.750	17.750	23.750	29.750
6.000	12.000	18.000	24.000	30.000

Scenario Input Data (cont'd.)

Scenario 2 :: SRWMD 100YR-2HR

Hydrograph Type: Inline SCSModflow Routing: Not routed

Repetitions: 1

Basin Area (acres) 2.030
Time Of Concentration (minutes) 29.0
DCIA (%) 0.0
Curve Number 66
Design Rainfall Depth (inches) 5.1
Design Rainfall Duration (hours) 2.0
Shape Factor UHG 484

Rainfall Distribution SCS Type II Florida Modified

Initial ground water level (ft datum) 154.70 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.250	6.250	12.250	18.250	24.250
0.500	6.500	12.500	18.500	24.500
0.750	6.750	12.750	18.750	24.750
1.000	7.000	13.000	19.000	25.000
1.250	7.250	13.250	19.250	25.250
1.500	7.500	13.500	19.500	25.500
1.750	7.750	13.750	19.750	25.750
2.000	8.000	14.000	20.000	26.000
2.250	8.250	14.250	20.250	26.250
2.500	8.500	14.500	20.500	26.500
2.750	8.750	14.750	20.750	26.750
3.000	9.000	15.000	21.000	27.000
3.250	9.250	15.250	21.250	27.250
3.500	9.500	15.500	21.500	27.500
3.750	9.750	15.750	21.750	27.750
4.000	10.000	16.000	22.000	28.000
4.250	10.250	16.250	22.250	28.250
4.500	10.500	16.500	22.500	28.500
4.750	10.750	16.750	22.750	28.750
5.000	11.000	17.000	23.000	29.000
5.250	11.250	17.250	23.250	29.250
5.500	11.500	17.500	23.500	29.500
5.750	11.750	17.750	23.750	29.750
6.000	12.000	18.000	24.000	30.000

Scenario Input Data (cont'd.)

Scenario 3 :: SRWMD 100YR-4HR

Hydrograph Type: Inline SCSModflow Routing: Not routed

Repetitions: 1

Basin Area (acres) 2.030
Time Of Concentration (minutes) 29.0
DCIA (%) 0.0
Curve Number 66
Design Rainfall Depth (inches) 6.1
Design Rainfall Duration (hours) 4.0
Shape Factor UHG 484

Rainfall Distribution SCS Type II Florida Modified

Initial ground water level (ft datum) 154.70 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.250	6.250	12.250	18.250	24.250
0.500	6.500	12.500	18.500	24.500
0.750	6.750	12.750	18.750	24.750
1.000	7.000	13.000	19.000	25.000
1.250	7.250	13.250	19.250	25.250
1.500	7.500	13.500	19.500	25.500
1.750	7.750	13.750	19.750	25.750
2.000	8.000	14.000	20.000	26.000
2.250	8.250	14.250	20.250	26.250
2.500	8.500	14.500	20.500	26.500
2.750	8.750	14.750	20.750	26.750
3.000	9.000	15.000	21.000	27.000
3.250	9.250	15.250	21.250	27.250
3.500	9.500	15.500	21.500	27.500
3.750	9.750	15.750	21.750	27.750
4.000	10.000	16.000	22.000	28.000
4.250	10.250	16.250	22.250	28.250
4.500	10.500	16.500	22.500	28.500
4.750	10.750	16.750	22.750	28.750
5.000	11.000	17.000	23.000	29.000
5.250	11.250	17.250	23.250	29.250
5.500	11.500	17.500	23.500	29.500
5.750	11.750	17.750	23.750	29.750
6.000	12.000	18.000	24.000	30.000

Scenario Input Data (cont'd.)

Scenario 4 :: SRWMD 100YR-8HR

Hydrograph Type: Inline SCSModflow Routing: Not routed

Repetitions: 1

Basin Area (acres) 2.030
Time Of Concentration (minutes) 29.0
DCIA (%) 0.0
Curve Number 66
Design Rainfall Depth (inches) 7.4
Design Rainfall Duration (hours) 8.0
Shape Factor UHG 484

Rainfall Distribution SCS Type II Florida Modified

Initial ground water level (ft datum) 154.70 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.250	6.250	12.250	18.250	24.250
0.500	6.500	12.500	18.500	24.500
0.750	6.750	12.750	18.750	24.750
1.000	7.000	13.000	19.000	25.000
1.250	7.250	13.250	19.250	25.250
1.500	7.500	13.500	19.500	25.500
1.750	7.750	13.750	19.750	25.750
2.000	8.000	14.000	20.000	26.000
2.250	8.250	14.250	20.250	26.250
2.500	8.500	14.500	20.500	26.500
2.750	8.750	14.750	20.750	26.750
3.000	9.000	15.000	21.000	27.000
3.250	9.250	15.250	21.250	27.250
3.500	9.500	15.500	21.500	27.500
3.750	9.750	15.750	21.750	27.750
4.000	10.000	16.000	22.000	28.000
4.250	10.250	16.250	22.250	28.250
4.500	10.500	16.500	22.500	28.500
4.750	10.750	16.750	22.750	28.750
5.000	11.000	17.000	23.000	29.000
5.250	11.250	17.250	23.250	29.250
5.500	11.500	17.500	23.500	29.500
5.750	11.750	17.750	23.750	29.750
6.000	12.000	18.000	24.000	30.000

Scenario Input Data (cont'd.)

Scenario 5 :: SRWMD 100YR-24HR

Hydrograph Type: Inline SCSModflow Routing: Not routed

Repetitions: 1

Basin Area (acres) 2.030
Time Of Concentration (minutes) 29.0
DCIA (%) 0.0
Curve Number 66
Design Rainfall Depth (inches) 9.8
Design Rainfall Duration (hours) 24.0
Shape Factor UHG 484

Rainfall Distribution SCS Type II Florida Modified

Initial ground water level (ft datum) 154.70 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.250	6.250	12.250	18.250	24.250
0.500	6.500	12.500	18.500	24.500
0.750	6.750	12.750	18.750	24.750
1.000	7.000	13.000	19.000	25.000
1.250	7.250	13.250	19.250	25.250
1.500	7.500	13.500	19.500	25.500
1.750	7.750	13.750	19.750	25.750
2.000	8.000	14.000	20.000	26.000
2.250	8.250	14.250	20.250	26.250
2.500	8.500	14.500	20.500	26.500
2.750	8.750	14.750	20.750	26.750
3.000	9.000	15.000	21.000	27.000
3.250	9.250	15.250	21.250	27.250
3.500	9.500	15.500	21.500	27.500
3.750	9.750	15.750	21.750	27.750
4.000	10.000	16.000	22.000	28.000
4.250	10.250	16.250	22.250	28.250
4.500	10.500	16.500	22.500	28.500
4.750	10.750	16.750	22.750	28.750
5.000	11.000	17.000	23.000	29.000
5.250	11.250	17.250	23.250	29.250
5.500	11.500	17.500	23.500	29.500
5.750	11.750	17.750	23.750	29.750
6.000	12.000	18.000	24.000	30.000

Sort-By-Category Report

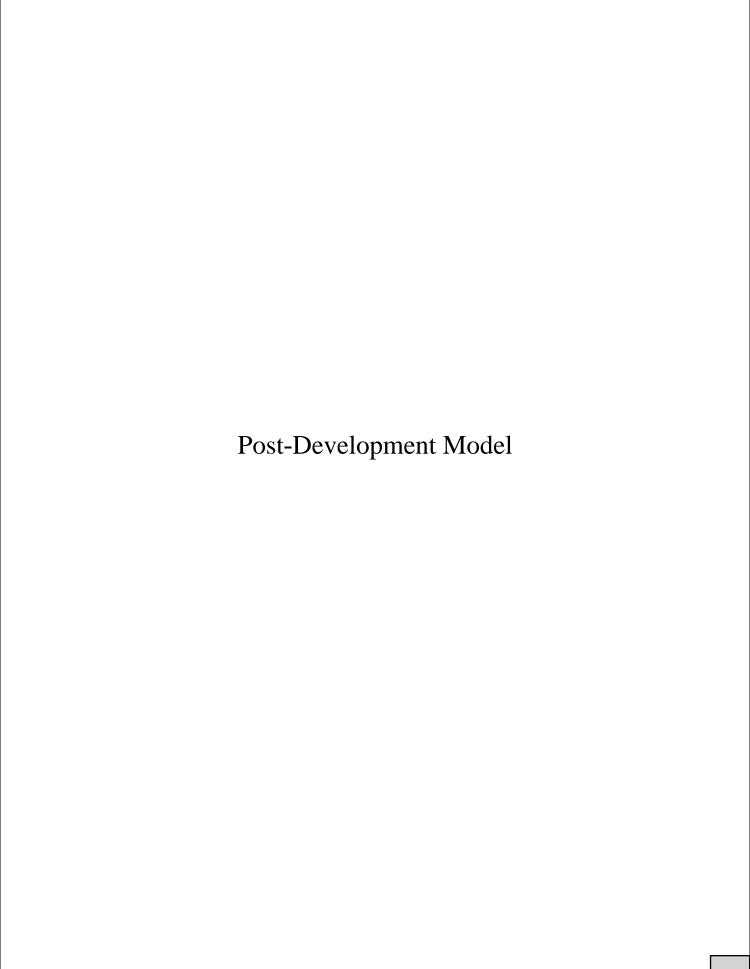
Scenarios Considered: 1 to 5

Discharge - Rate - Maximum Positive

Rank	Scenario Number	Maximum Positive Discharge Rate (ft³/s)	Time (hours)	Description
1	4	7.25	4.25	SRWMD 100YR-8HR
2	5	6.67	12.12	SRWMD 100YR-24HR
3	3	6.46	2.32	SRWMD 100YR-4HR
4	2	5.47	1.35	SRWMD 100YR-2HR
5	1	4.27	0.84	SRWMD 100YR-1HR

Discharge - Cumulative Volume - Maximum Positive

Rank	Scenario Number	Maximum Positive Cumulative Discharge Volume (ft³)	Time (hours)	Description
1	5	40983.17	25.58	SRWMD 100YR-24HR
2	4	25726.74	9.60	SRWMD 100YR-8HR
3	3	18428.31	5.54	SRWMD 100YR-4HR
4	2	13241.67	3.61	SRWMD 100YR-2HR
5	1	8901.35	2.58	SRWMD 100YR-1HR



Project Data

Project Name: **CRS Marvin Burnett**

Simulation Description: Post-Development DA-1

Project Number: 23-0653

Engineer: Jarrett Pearson

Supervising Engineer: Cole Menhennett

Date: 01-04-2024

Aquifer Data

Base Of Aquifer Elevation, [B] (ft datum):	148.70
Water Table Elevation, [WT] (ft datum):	152.99
Horizontal Saturated Hydraulic Conductivity, [Kh] (ft/day):	5.00
Fillable Porosity, [n] (%):	20.00
Unsaturated Vertical Infiltration Rate, [Iv] (ft/day):	5.0
Maximum Area For Unsaturated Infiltration, [Av] (ft²):	19819.0

Geometry Data

Equivalent Pond Length, [L] (ft): 200.0

Equivalent Pond Width, [W] (ft): 83.0

Ground water mound is expected to intersect the pond bottom

Stage vs Area Data

Stage	Area
(ft datum)	(ft²)
157.00	13469.0
158.00	15494.0
159.00	17606.0
160.00	19819.0

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PONDS Version 3.3.0241 **Retention Pond Recovery - Refined Method** Copyright 2011

Devo Seereeram, Ph.D., P.E.

Discharge Structures

Discharge Structure #1 is active as weir

Structure Parameters

Description: WQTV

Weir elevation, (ft datum): 158.00 3.13 2 Weir coefficient: Weir length, (ft): Weir exponent: 1.5

Tailwater - disabled, free discharge

Discharge Structure #2 is inactive

Discharge Structure #3 is inactive

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Scenario Input Data

Scenario 1 :: SRWMD 100YR-1HR

Hydrograph Type: Inline SCS

Modflow Routing: Routed with infiltration

Repetitions: 1

Basin Area (acres) 3.290
Time Of Concentration (minutes) 10.0
DCIA (%) 0.0
Curve Number 72
Design Rainfall Depth (inches) 4.2
Design Rainfall Duration (hours) 1.0
Shape Factor UHG 484

Shape Factor UHG 484
Rainfall Distribution SCS Type II Florida Modified

Initial ground water level (ft datum) 152.99 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.250	6.250	12.250	18.250	24.250
0.500	6.500	12.500	18.500	24.500
0.750	6.750	12.750	18.750	24.750
1.000	7.000	13.000	19.000	25.000
1.250	7.250	13.250	19.250	25.250
1.500	7.500	13.500	19.500	25.500
1.750	7.750	13.750	19.750	25.750
2.000	8.000	14.000	20.000	26.000
2.250	8.250	14.250	20.250	26.250
2.500	8.500	14.500	20.500	26.500
2.750	8.750	14.750	20.750	26.750
3.000	9.000	15.000	21.000	27.000
3.250	9.250	15.250	21.250	27.250
3.500	9.500	15.500	21.500	27.500
3.750	9.750	15.750	21.750	27.750
4.000	10.000	16.000	22.000	28.000
4.250	10.250	16.250	22.250	28.250
4.500	10.500	16.500	22.500	28.500
4.750	10.750	16.750	22.750	28.750
5.000	11.000	17.000	23.000	29.000
5.250	11.250	17.250	23.250	29.250
5.500	11.500	17.500	23.500	29.500
5.750	11.750	17.750	23.750	29.750
6.000	12.000	18.000	24.000	30.000

Scenario Input Data (cont'd.)

Scenario 2 :: SRWMD 100YR-2HR

Hydrograph Type: Inline SCS

Modflow Routing: Routed with infiltration

Repetitions: 1

Basin Area (acres) 3.290
Time Of Concentration (minutes) 10.0
DCIA (%) 0.0
Curve Number 72
Design Rainfall Depth (inches) 5.1
Design Rainfall Duration (hours) 2.0
Shape Factor UHG 484

Shape Factor UHG 484
Rainfall Distribution SCS Type II Florida Modified

Initial ground water level (ft datum) 152.99 (default)

Time After Storm Event	Time After Storm Event	Time After Storm Event	Time After Storm Event	Time After Storm Event
(days)	(days)	(days)	(days)	(days)
0.250	6.250	12.250	18.250	24.250
0.500	6.500	12.500	18.500	24.500
0.750	6.750	12.750	18.750	24.750
1.000	7.000	13.000	19.000	25.000
1.250	7.250	13.250	19.250	25.250
1.500	7.500	13.500	19.500	25.500
1.750	7.750	13.750	19.750	25.750
2.000	8.000	14.000	20.000	26.000
2.250	8.250	14.250	20.250	26.250
2.500	8.500	14.500	20.500	26.500
2.750	8.750	14.750	20.750	26.750
3.000	9.000	15.000	21.000	27.000
3.250	9.250	15.250	21.250	27.250
3.500	9.500	15.500	21.500	27.500
3.750	9.750	15.750	21.750	27.750
4.000	10.000	16.000	22.000	28.000
4.250	10.250	16.250	22.250	28.250
4.500	10.500	16.500	22.500	28.500
4.750	10.750	16.750	22.750	28.750
5.000	11.000	17.000	23.000	29.000
5.250	11.250	17.250	23.250	29.250
5.500	11.500	17.500	23.500	29.500
5.750	11.750	17.750	23.750	29.750
6.000	12.000	18.000	24.000	30.000

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Scenario Input Data (cont'd.)

Scenario 3 :: SRWMD 100YR-4HR

Hydrograph Type: Inline SCS

Modflow Routing: Routed with infiltration

Repetitions: 1

Basin Area (acres) 3.290
Time Of Concentration (minutes) 10.0
DCIA (%) 0.0
Curve Number 72
Design Rainfall Depth (inches) 6.1
Design Rainfall Duration (hours) 4.0
Shape Factor UHG 484

Shape Factor UHG 484
Rainfall Distribution SCS Type II Florida Modified

Initial ground water level (ft datum) 152.99 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.250	6.250	12.250	18.250	24.250
0.500	6.500	12.500	18.500	24.500
0.750	6.750	12.750	18.750	24.750
1.000	7.000	13.000	19.000	25.000
1.250	7.250	13.250	19.250	25.250
1.500	7.500	13.500	19.500	25.500
1.750	7.750	13.750	19.750	25.750
2.000	8.000	14.000	20.000	26.000
2.250	8.250	14.250	20.250	26.250
2.500	8.500	14.500	20.500	26.500
2.750	8.750	14.750	20.750	26.750
3.000	9.000	15.000	21.000	27.000
3.250	9.250	15.250	21.250	27.250
3.500	9.500	15.500	21.500	27.500
3.750	9.750	15.750	21.750	27.750
4.000	10.000	16.000	22.000	28.000
4.250	10.250	16.250	22.250	28.250
4.500	10.500	16.500	22.500	28.500
4.750	10.750	16.750	22.750	28.750
5.000	11.000	17.000	23.000	29.000
5.250	11.250	17.250	23.250	29.250
5.500	11.500	17.500	23.500	29.500
5.750	11.750	17.750	23.750	29.750
6.000	12.000	18.000	24.000	30.000

Scenario Input Data (cont'd.)

Scenario 4 :: SRWMD 100YR-8HR

Hydrograph Type: Inline SCS

Modflow Routing: Routed with infiltration

Repetitions: 1

Basin Area (acres) 3.290
Time Of Concentration (minutes) 10.0
DCIA (%) 0.0
Curve Number 72
Design Rainfall Depth (inches) 7.4
Design Rainfall Duration (hours) 8.0
Shape Factor UHG 484

Shape Factor UHG 484
Rainfall Distribution SCS Type II Florida Modified

Initial ground water level (ft datum) 152.99 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.250	6.250	12.250	18.250	24.250
0.500	6.500	12.500	18.500	24.500
0.750	6.750	12.750	18.750	24.750
1.000	7.000	13.000	19.000	25.000
1.250	7.250	13.250	19.250	25.250
1.500	7.500	13.500	19.500	25.500
1.750	7.750	13.750	19.750	25.750
2.000	8.000	14.000	20.000	26.000
2.250	8.250	14.250	20.250	26.250
2.500	8.500	14.500	20.500	26.500
2.750	8.750	14.750	20.750	26.750
3.000	9.000	15.000	21.000	27.000
3.250	9.250	15.250	21.250	27.250
3.500	9.500	15.500	21.500	27.500
3.750	9.750	15.750	21.750	27.750
4.000	10.000	16.000	22.000	28.000
4.250	10.250	16.250	22.250	28.250
4.500	10.500	16.500	22.500	28.500
4.750	10.750	16.750	22.750	28.750
5.000	11.000	17.000	23.000	29.000
5.250	11.250	17.250	23.250	29.250
5.500	11.500	17.500	23.500	29.500
5.750	11.750	17.750	23.750	29.750
6.000	12.000	18.000	24.000	30.000

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Scenario Input Data (cont'd.)

Scenario 5 :: SRWMD 100YR-24HR

Hydrograph Type: Inline SCS

Modflow Routing: Routed with infiltration

Repetitions: 1

Basin Area (acres) 3.290
Time Of Concentration (minutes) 10.0
DCIA (%) 0.0
Curve Number 72
Design Rainfall Depth (inches) 9.8
Design Rainfall Duration (hours) 24.0
Shape Factor UHG 484

Rainfall Distribution SCS Type II Florida Modified

Initial ground water level (ft datum) 152.99 (default)

Time After	Time After	Time After	Time After	Time After
Storm Event	Storm Event	Storm Event	Storm Event	Storm Event
(days)	(days)	(days)	(days)	(days)
0.250	6.250	12.250	18.250	24.250
0.500	6.500	12.500	18.500	24.500
0.750	6.750	12.750	18.750	24.750
1.000	7.000	13.000	19.000	25.000
1.250	7.250	13.250	19.250	25.250
1.500	7.500	13.500	19.500	25.500
1.750	7.750	13.750	19.750	25.750
2.000	8.000	14.000	20.000	26.000
2.250	8.250	14.250	20.250	26.250
2.500	8.500	14.500	20.500	26.500
2.750	8.750	14.750	20.750	26.750
3.000	9.000	15.000	21.000	27.000
3.250	9.250	15.250	21.250	27.250
3.500	9.500	15.500	21.500	27.500
3.750	9.750	15.750	21.750	27.750
4.000	10.000	16.000	22.000	28.000
4.250	10.250	16.250	22.250	28.250
4.500	10.500	16.500	22.500	28.500
4.750	10.750	16.750	22.750	28.750
5.000	11.000	17.000	23.000	29.000
5.250	11.250	17.250	23.250	29.250
5.500	11.500	17.500	23.500	29.500
5.750	11.750	17.750	23.750	29.750
6.000	12.000	18.000	24.000	30.000

Scenario 6 :: WQTV

Hydrograph Type: Slug Load

Modflow Routing: Routed with infiltration

Treatment Volume (ft³) 12483

Initial ground water level (ft datum) 152.99 (default)

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Scenario Input Data (cont'd.)

Scenario 6 (cont'd.) :: Slug Load :: WQTV

Time After	Time After
Storm Event	Storm Event
(days)	(days)
0.100	2.000
0.250	2.500
0.500	3.000
1.000	3.500
1.500	4.000

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Sort-By-Category Report

Scenarios Considered: 1 to 6

Stage - Maximum

Rank	Scenario Number	Maximum Stage (ft datum)	Time (hours)	Description
1	5	158.78	12.58	SRWMD 100YR-24HR
2	4	158.61	4.47	SRWMD 100YR-8HR
3	3	158.46	2.64	SRWMD 100YR-4HR
4	2	158.35	1.89	SRWMD 100YR-2HR
5	1	158.14	1.13	SRWMD 100YR-1HR
6	6	157.87	0.00	WQTV

Discharge - Rate - Maximum Positive

Rank	Scenario Number	Maximum Positive Discharge Rate (ft³/s)	Time (hours)	Description
1	5	4.28	12.58	SRWMD 100YR-24HR
2	4	3.00	4.47	SRWMD 100YR-8HR
3	3	1.93	2.64	SRWMD 100YR-4HR
4	2	1.28	1.89	SRWMD 100YR-2HR
5	1	0.31	1.13	SRWMD 100YR-1HR
6	6	None	N.A.	WQTV

Discharge - Cumulative Volume - Maximum Positive

Rank	Scenario Number	Maximum Positive Cumulative Discharge Volume (ft³)	Time (hours)	Description
1	5	37974.45	30.58	SRWMD 100YR-24HR
2	4	17170.97	8.47	SRWMD 100YR-8HR
3	3	10552.48	4.58	SRWMD 100YR-4HR
4	2	4512.00	2.58	SRWMD 100YR-2HR
5	1	462.72	1.58	SRWMD 100YR-1HR
6	6	None	N.A.	WQTV

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<u>Detailed Results</u> :: Scenario 1 :: SRWMD 100YR-1HR

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
0.000	0.0000	0.0000	152.990	0.00000	0.00000	0.0	0.0	0.0	N.A.
0.022	0.0000	0.0000	152.990	0.00000	0.00000	0.0	0.0	0.0	U
0.044	0.0000	0.0000	152.990	0.00000	0.00000	0.0	0.0	0.0	U
0.067 0.089	0.0000 0.0000	0.0000 0.0000	152.990 152.990	0.00000 0.00000	0.00000 0.00000	0.0 0.0	0.0 0.0	0.0 0.0	U U
0.111	0.0000	0.0000	152.990	0.00000	0.00000	0.0	0.0	0.0	Ü
0.133	0.0000	0.0000	152.990	0.00000	0.00000	0.0	0.0	0.0	Ü
0.156	0.0000	0.0000	152.990	0.00000	0.00000	0.0	0.0	0.0	U
0.178	0.0000	0.0000	152.990	0.00000	0.00000	0.0	0.0	0.0	U
0.200	0.0000	0.0000	152.990	0.00000	0.00000	0.0	0.0	0.0	U U
0.222 0.244	0.0000 0.0000	0.0000 0.0000	152.990 152.990	0.00000 0.00000	0.00000 0.00000	0.0 0.0	0.0 0.0	0.0 0.0	U
0.267	0.0000	0.0000	152.990	0.00000	0.00000	0.0	0.0	0.0	Ŭ
0.289	0.0000	0.0000	152.990	0.00000	0.00000	0.0	0.0	0.0	Ü
0.311	0.0000	0.0000	152.990	0.00000	0.00000	0.0	0.0	0.0	U
0.333	0.0000	0.0000	152.990	0.00000	0.00000	0.0	0.0	0.0	U
0.356 0.378	0.0000 0.0000	0.0000 0.0000	152.990 152.990	0.00000 0.00000	0.00000 0.00000	0.0 0.0	0.0 0.0	0.0 0.0	U U
0.400	0.0000	0.0000	152.990	0.00120	0.00000	0.0	0.0	0.0	U
0.422	0.0048	0.0000	152.990	0.01123	0.00000	0.2	0.2	0.0	Ü
0.444	0.0353	0.0000	152.990	0.05641	0.00000	1.8	1.8	0.0	Ü
0.467	0.1503	0.0000	152.992	0.28887	0.00000	9.2	9.2	0.0	U
0.489	0.8197	0.0000	153.002	0.63289	0.00000	48.0	48.0	0.0	U
0.511 0.533	3.0541 7.3661	0.0000 0.0000	157.007 157.033	0.78234 0.78715	0.00000 0.00000	203.0 619.8	110.5 173.2	0.0 0.0	U/P U/P
0.556	13.2100	0.0000	157.033	0.79551	0.00000	1442.8	236.4	0.0	U/P
0.578	18.1747	0.0000	157.176	0.80685	0.00000	2698.2	300.5	0.0	U/P
0.600	20.6736	0.0000	157.283	0.81971	0.00000	4252.1	365.5	0.0	U/P
0.622	20.8533	0.0000	157.395	0.83258	0.00000	5913.2	431.6	0.0	U/P
0.644	19.2460	0.0000	157.502	0.84432	0.00000	7517.2	498.7	0.0	U/P
0.667 0.689	16.5416 13.9483	0.0000 0.0000	157.596 157.674	0.85437 0.86274	0.00000 0.00000	8948.7 10168.3	566.7 635.4	0.0 0.0	U/P U/P
0.711	11.8984	0.0000	157.738	0.86972	0.00000	11202.2	704.8	0.0	U/P
0.733	10.2784	0.0000	157.793	0.87561	0.00000	12089.2	774.6	0.0	U/P
0.756	8.9361	0.0000	157.839	0.88063	0.00000	12857.8	844.9	0.0	U/P
0.778	7.8660	0.0000	157.879	0.88496	0.00000	13529.9	915.5	0.0	U/P
0.800	6.9880	0.0000	157.913	0.88872	0.00000	14124.1	986.4	0.0	U/P
0.822 0.844	6.2744 5.6997	0.0000 0.0000	157.943 157.969	0.89202 0.89496	0.00000 0.00000	14654.6 15133.5	1057.7 1129.2	0.0 0.0	U/P U/P
0.867	5.2272	0.0000	157.993	0.89764	0.00000	15570.6	1200.9	0.0	U/P
0.889	4.8274	0.0000	158.014	0.90011	0.01044	15972.8	1272.8	0.4	U/P
0.911	4.4846	0.0000	158.033	0.90236	0.03797	16345.3	1344.9	2.4	U/P
0.933	4.1987	0.0000	158.051	0.90441	0.07132	16692.6	1417.2	6.7	U/P
0.956	3.9662	0.0000	158.067	0.90628	0.10723	17019.2	1489.6	13.9	U/P
0.978 1.000	3.7655 3.5795	0.0000 0.0000	158.081 158.094	0.90800 0.90957	0.14413 0.18099	17328.5 17622.2	1562.2 1634.9	23.9 36.9	U/P U/P
1.022	3.3573	0.0000	158.106	0.91097	0.21671	17899.7	1707.7	52.8	U/P
1.044	3.0563	0.0000	158.117	0.91215	0.24959	18156.3	1780.6	71.5	U/P
1.067	2.6439	0.0000	158.125	0.91306	0.27735	18384.3	1853.6	92.6	U/P
1.089	2.1539	0.0000	158.131	0.91365	0.29779	18576.2	1926.7	115.6	U/P
1.111	1.6654	0.0000 0.0000	158.135	0.91394	0.30981	18729.0	1999.8	139.9	U/P
1.133 1.156	1.2315 0.8803	0.0000	158.136 158.135	0.91396 0.91376	0.31371 0.31073	18844.8 18929.3	2073.0 2146.1	164.8 189.8	U/P U/P
1.178	0.6289	0.0000	158.133	0.91341	0.30262	18989.7	2219.2	214.3	U/P
1.200	0.4528	0.0000	158.129	0.91294	0.29108	19032.9	2292.2	238.1	U/P
1.222	0.3260	0.0000	158.125	0.91240	0.27735	19064.1	2365.2	260.8	U/P
1.244	0.2330	0.0000	158.121	0.91182	0.26226	19086.5	2438.2	282.4	U/P
1.267 1.289	0.1669 0.1190	0.0000 0.0000	158.116 158.111	0.91119 0.91055	0.24642 0.23026	19102.5 19113.9	2511.1 2584.0	302.7 321.8	U/P U/P
1.311	0.0845	0.0000	158.105	0.90990	0.21408	19122.0	2656.8	339.6	U/P
1.333	0.0599	0.0000	158.100	0.90923	0.19809	19127.8	2729.6	356.1	U/P
1.356	0.0421	0.0000	158.095	0.90857	0.18244	19131.9	2802.3	371.3	U/P
1.378	0.0293	0.0000	158.089	0.90791	0.16723	19134.7	2874.9	385.3	U/P
1.400	0.0201	0.0000	158.084	0.90725	0.15251	19136.7	2947.5	398.1	U/P
1.422 1.444	0.0134 0.0086	0.0000 0.0000	158.079 158.074	0.90660 0.90595	0.13835	19138.0 19138.9	3020.1 3092.6	409.7 420.2	U/P U/P
1.444	0.0086	0.0000	158.074	0.90595	0.12475 0.11175	19139.5	3092.6 3165.1	420.2 429.7	U/P
1.489	0.0049	0.0000	158.063	0.90468	0.09934	19139.8	3237.5	438.1	U/P
1.511	0.0007	0.0000	158.058	0.90405	0.08755	19139.9	3309.8	445.6	U/P
1.533	0.0000	0.0000	158.053	0.90343	0.07638	19139.9	3382.1	452.2	U/P
1.556	0.0000	0.0000	158.048	0.90282	0.06584	19139.9	3454.4	457.9	U/P
1.578 7.578	0.0000	0.0000	158.043 156.339	0.90177 0.35071	0.05593 0.00000	19139.9 19139.9	3526.6 18677.2	462.7 462.7	U/P U/S
13.578	0.0000	0.0000	156.032	0.00000	0.00000	19139.9	18677.2	462.7	S
	2.0000	2.0000		2.50000	2.00000	. 5 . 5 . 5	. 50		-

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Detailed Results (cont,d.) :: Scenario 2 :: SRWMD 100YR-2HR

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
1.644	2.8768	0.0000	158.329	0.93836	1.18168	23236.3	2444.0	1097.9	U/P
1.667	2.8113	0.0000	158.333	0.93878	1.20074	23463.9	2519.1	1193.2	U/P
1.689	2.7507	0.0000	158.336	0.93914	1.21770	23686.3	2594.2	1289.9	U/P
1.711 1.733	2.6872	0.0000	158.339	0.93946	1.23263	23903.9 24115.8	2669.4	1387.9	U/P U/P
1.756	2.6123 2.5262	0.0000 0.0000	158.341 158.343	0.93973 0.93993	1.24535 1.25561	24321.4	2744.5 2819.7	1487.1 1587.1	U/P
1.778	2.4400	0.0000	158.344	0.94008	1.26331	24520.0	2894.9	1687.9	U/P
1.800	2.3649	0.0000	158.345	0.94018	1.26864	24712.2	2970.1	1789.1	U/P
1.822	2.3054	0.0000	158.346	0.94024	1.27203	24899.0	3045.4	1890.8	U/P
1.844	2.2636	0.0000	158.346	0.94027	1.27397	25081.8	3120.6	1992.6	U/P
1.867	2.2352	0.0000 0.0000	158.346	0.94029	1.27492	25261.7	3195.8	2094.6	U/P U/P
1.889 1.911	2.2139 2.1931	0.0000	158.346 158.346	0.94029 0.94027	1.27517 1.27485	25439.7 25616.0	3271.0 3346.2	2196.6 2298.6	U/P
1.933	2.1665	0.0000	158.346	0.94024	1.27390	25790.4	3421.5	2400.5	U/P
1.956	2.1306	0.0000	158.346	0.94019	1.27214	25962.3	3496.7	2502.4	U/P
1.978	2.0834	0.0000	158.345	0.94011	1.26931	26130.8	3571.9	2604.0	U/P
2.000	2.0249	0.0000	158.344	0.94000	1.26515	26295.1	3647.1	2705.4	U/P
2.022	1.9305	0.0000	158.343	0.93982	1.25906	26453.4	3722.3 3797.5	2806.4 2906.7	U/P U/P
2.044 2.067	1.7813 1.5542	0.0000 0.0000	158.342 158.339	0.93956 0.93917	1.24989 1.23598	26601.8 26735.3	3872.6	3006.2	U/P
2.089	1.2738	0.0000	158.335	0.93863	1.21577	26848.4	3947.7	3104.2	U/P
2.111	0.9890	0.0000	158.330	0.93793	1.18876	26938.9	4022.8	3200.4	U/P
2.133	0.7342	0.0000	158.324	0.93710	1.15562	27007.8	4097.8	3294.2	U/P
2.156	0.5253	0.0000	158.317	0.93616	1.11765	27058.2	4172.7	3385.1	U/P
2.178 2.200	0.3755 0.2706	0.0000 0.0000	158.309 158.301	0.93515 0.93410	1.07648 1.03363	27094.2 27120.1	4247.6 4322.4	3472.9 3557.3	U/P U/P
2.222	0.1954	0.0000	158.293	0.93303	0.99016	27138.7	4397.0	3638.2	U/P
2.244	0.1399	0.0000	158.284	0.93195	0.94675	27152.1	4471.6	3715.7	U/P
2.267	0.1004	0.0000	158.275	0.93087	0.90387	27161.7	4546.2	3789.7	U/P
2.289	0.0717	0.0000	158.267	0.92980	0.86184	27168.6	4620.6	3860.4	U/P
2.311	0.0511	0.0000	158.258	0.92874	0.82086	27173.5	4694.9	3927.7	U/P
2.333 2.356	0.0363 0.0255	0.0000 0.0000	158.250 158.241	0.92770 0.92667	0.78106 0.74251	27177.0 27179.5	4769.2 4843.4	3991.7 4052.7	U/P U/P
2.378	0.0233	0.0000	158.233	0.92566	0.70526	27181.2	4917.5	4110.6	U/P
2.400	0.0121	0.0000	158.225	0.92467	0.66930	27182.4	4991.5	4165.6	U/P
2.422	0.0081	0.0000	158.217	0.92370	0.63462	27183.2	5065.4	4217.7	U/P
2.444	0.0052	0.0000	158.210	0.92275	0.60121	27183.8	5139.3	4267.2	U/P
2.467 2.489	0.0030 0.0014	0.0000 0.0000	158.202 158.195	0.92182 0.92090	0.56904 0.53807	27184.1 27184.3	5213.0 5286.8	4314.0 4358.3	U/P U/P
2.469	0.0005	0.0000	158.188	0.92090	0.50827	27184.3	5360.4	4400.1	U/P
2.533	0.0000	0.0000	158.180	0.91913	0.47960	27184.3	5434.0	4439.6	U/P
2.556	0.0000	0.0000	158.173	0.91826	0.45203	27184.3	5507.4	4476.9	U/P
2.578	0.0000	0.0000	158.167	0.91737	0.42553	27184.3	5580.9	4512.0	U/P
8.578	0.0000	0.0000	156.340	0.39564	0.00000	27184.3	22672.3	4512.0	U/S
14.578 20.578	0.0000 0.0000	0.0000 0.0000	156.033 155.832	0.00000 0.00000	0.00000 0.00000	27184.3 27184.3	22672.3 22672.3	4512.0 4512.0	S S
26.578	0.0000	0.0000	155.680	0.00000	0.00000	27184.3	22672.3	4512.0	S
32.578	0.0000	0.0000	155.558	0.00000	0.00000	27184.3	22672.3	4512.0	S
38.578	0.0000	0.0000	155.455	0.00000	0.00000	27184.3	22672.3	4512.0	S S
44.578	0.0000	0.0000	155.366	0.00000	0.00000	27184.3	22672.3	4512.0	S
50.578 56.578	0.0000 0.0000	0.0000 0.0000	155.288 155.219	0.00000 0.00000	0.00000 0.00000	27184.3 27184.3	22672.3 22672.3	4512.0 4512.0	S S
62.578	0.0000	0.0000	155.156	0.00000	0.00000	27184.3	22672.3	4512.0	S
68.578	0.0000	0.0000	155.099	0.00000	0.00000	27184.3	22672.3	4512.0	S
74.578	0.0000	0.0000	155.047	0.00000	0.00000	27184.3	22672.3	4512.0	S
80.578	0.0000	0.0000	154.998	0.00000	0.00000	27184.3	22672.3	4512.0	S
86.578	0.0000	0.0000	154.954	0.00000	0.00000 0.00000	27184.3 27184.3	22672.3 22672.3	4512.0 4512.0	S
92.578 98.578	0.0000 0.0000	0.0000 0.0000	154.912 154.873	0.00000 0.00000	0.00000	27184.3 27184.3	22672.3	4512.0 4512.0	S S
104.578	0.0000	0.0000	154.836	0.00000	0.00000	27184.3	22672.3	4512.0	S
110.578	0.0000	0.0000	154.801	0.00000	0.00000	27184.3	22672.3	4512.0	S
116.578	0.0000	0.0000	154.768	0.00000	0.00000	27184.3	22672.3	4512.0	S
122.578	0.0000	0.0000	154.737	0.00000	0.00000	27184.3	22672.3	4512.0	S
128.578	0.0000	0.0000	154.708 154.680	0.00000	0.00000	27184.3	22672.3	4512.0 4512.0	S
134.578 140.578	0.0000 0.0000	0.0000 0.0000	154.680 154.653	0.00000 0.00000	0.00000 0.00000	27184.3 27184.3	22672.3 22672.3	4512.0 4512.0	S S
146.578	0.0000	0.0000	154.628	0.00000	0.00000	27184.3	22672.3	4512.0	S
152.578	0.0000	0.0000	154.603	0.00000	0.00000	27184.3	22672.3	4512.0	S
158.578	0.0000	0.0000	154.580	0.00000	0.00000	27184.3	22672.3	4512.0	S
164.578	0.0000	0.0000	154.557	0.00000	0.00000	27184.3	22672.3	4512.0	S
170.578 176.578	0.0000	0.0000	154.536	0.00000	0.00000 0.00000	27184.3 27184.3	22672.3	4512.0 4512.0	S
182.578	0.0000 0.0000	0.0000 0.0000	154.515 154.495	0.00000 0.00000	0.00000	27184.3 27184.3	22672.3 22672.3	4512.0 4512.0	S S
188.578	0.0000	0.0000	154.476	0.00000	0.00000	27184.3	22672.3	4512.0	S
								-	

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Detailed Results (cont,d.) :: Scenario 3 :: SRWMD 100YR-4HR

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
3.289	1.6884	0.0000	158.378	0.94412	1.45726	32414.7	5467.0	6451.7	U/P
3.311	1.6908	0.0000	158.375	0.94370	1.43741	32549.9	5542.5	6567.5	U/P
3.333	1.6865	0.0000	158.372	0.94328	1.41816	32685.0	5618.0	6681.7	U/P
3.356	1.6727	0.0000	158.368	0.94286	1.39929	32819.4	5693.5	6794.4	U/P
3.378	1.6482	0.0000	158.365	0.94245	1.38050	32952.2	5768.9	6905.6	U/P
3.400 3.422	1.6107 1.5651	0.0000 0.0000	158.362 158.358	0.94202 0.94157	1.36146 1.34190	33082.5 33209.6	5844.2 5919.6	7015.3 7123.4	U/P U/P
3.444	1.5196	0.0000	158.355	0.94112	1.32172	33333.0	5994.9	7230.0	U/P
3.467	1.4789	0.0000	158.351	0.94065	1.30104	33452.9	6070.2	7334.9	U/P
3.489	1.4456	0.0000	158.347	0.94017	1.28002	33569.9	6145.4	7438.1	U/P
3.511	1.4219	0.0000	158.343	0.93969	1.25892	33684.6	6220.6	7539.7	U/P
3.533	1.4055	0.0000	158.339	0.93921	1.23796	33797.7	6295.8	7639.6	U/P
3.556	1.3939	0.0000	158.336	0.93874	1.21731	33909.6	6370.9	7737.8	U/P
3.578	1.3855	0.0000	158.332	0.93828	1.19706	34020.8	6445.9	7834.3 7929.3	U/P U/P
3.600 3.622	1.3799 1.3760	0.0000 0.0000	158.328 158.325	0.93782 0.93737	1.17726 1.15798	34131.4 34241.7	6521.0 6596.0	7929.3 8022.7	U/P
3.644	1.3736	0.0000	158.321	0.93694	1.13730	34351.7	6671.0	8114.6	U/P
3.667	1.3720	0.0000	158.318	0.93651	1.12099	34461.5	6745.9	8205.0	U/P
3.689	1.3711	0.0000	158.314	0.93610	1.10330	34571.2	6820.8	8294.0	U/P
3.711	1.3706	0.0000	158.311	0.93569	1.08615	34680.9	6895.7	8381.6	U/P
3.733	1.3704	0.0000	158.308	0.93530	1.06951	34790.5	6970.5	8467.8	U/P
3.756	1.3702	0.0000	158.305	0.93492	1.05338	34900.1	7045.3	8552.7	U/P
3.778 3.800	1.3677 1.3606	0.0000 0.0000	158.302 158.299	0.93454 0.93417	1.03771 1.02239	35009.7 35118.8	7120.1 7194.9	8636.4	U/P U/P
3.822	1.3466	0.0000	158.296	0.93380	1.02239	35227.1	7194.9	8718.8 8799.9	U/P
3.844	1.3273	0.0000	158.293	0.93342	0.99218	35334.0	7344.3	8879.9	U/P
3.867	1.3057	0.0000	158.290	0.93305	0.97704	35439.4	7418.9	8958.7	U/P
3.889	1.2832	0.0000	158.287	0.93267	0.96182	35542.9	7493.6	9036.3	U/P
3.911	1.2614	0.0000	158.284	0.93228	0.94650	35644.7	7568.2	9112.6	U/P
3.933	1.2401	0.0000	158.281	0.93190	0.93112	35744.8	7642.7	9187.7	U/P
3.956	1.2168	0.0000	158.278	0.93150	0.91566	35843.0	7717.3	9261.6	U/P
3.978 4.000	1.1871 1.1521	0.0000 0.0000	158.274 158.271	0.93110 0.93069	0.90001 0.88406	35939.2 36032.8	7791.8 7866.2	9334.2 9405.5	U/P U/P
4.022	1.1018	0.0000	158.268	0.93025	0.86757	36122.9	7940.7	9475.6	U/P
4.044	1.0235	0.0000	158.264	0.92977	0.85005	36207.9	8015.1	9544.3	U/P
4.067	0.8987	0.0000	158.260	0.92923	0.83066	36284.8	8089.4	9611.5	U/P
4.089	0.7409	0.0000	158.256	0.92861	0.80857	36350.4	8163.8	9677.1	U/P
4.111	0.5789	0.0000	158.250	0.92790	0.78349	36403.2	8238.0	9740.8	U/P
4.133	0.4312	0.0000	158.244	0.92713	0.75569	36443.6	8312.2	9802.4	U/P
4.156	0.3085	0.0000	158.238	0.92629	0.72579	36473.2	8386.4 8460.4	9861.6	U/P U/P
4.178 4.200	0.2204 0.1588	0.0000 0.0000	158.231 158.224	0.92542 0.92453	0.69459 0.66288	36494.3 36509.5	8534.4	9918.4 9972.7	U/P
4.222	0.1147	0.0000	158.217	0.92363	0.63120	36520.4	8608.3	10024.5	U/P
4.244	0.0820	0.0000	158.209	0.92273	0.59988	36528.3	8682.2	10073.7	U/P
4.267	0.0589	0.0000	158.202	0.92183	0.56915	36533.9	8756.0	10120.5	U/P
4.289	0.0420	0.0000	158.195	0.92094	0.53917	36538.0	8829.7	10164.8	U/P
4.311	0.0299	0.0000	158.188	0.92007	0.51003	36540.9	8903.3	10206.8	U/P
4.333	0.0213	0.0000	158.181	0.91920	0.48180	36542.9	8976.9	10246.5	U/P
4.356 4.378	0.0150 0.0104	0.0000 0.0000	158.174 158.167	0.91835 0.91751	0.45450 0.42815	36544.4 36545.4	9050.4 9123.8	10283.9 10319.2	U/P U/P
4.400	0.0071	0.0000	158.161	0.91668	0.40273	36546.1	9197.2	10313.2	U/P
4.422	0.0048	0.0000	158.154	0.91587	0.37825	36546.6	9270.5	10383.7	U/P
4.444	0.0031	0.0000	158.148	0.91507	0.35468	36546.9	9343.7	10413.0	U/P
4.467	0.0018	0.0000	158.141	0.91429	0.33200	36547.1	9416.9	10440.5	U/P
4.489	0.0009	0.0000	158.135	0.91351	0.31019	36547.2	9490.0	10466.2	U/P
4.511	0.0003	0.0000	158.129	0.91276	0.28922 0.26909	36547.2	9563.1	10490.2	U/P U/P
4.533 4.556	0.0000 0.0000	0.0000 0.0000	158.123 158.117	0.91201 0.91128	0.24976	36547.2 36547.2	9636.1 9709.0	10512.5 10533.3	U/P
4.578	0.0000	0.0000	158.111	0.90990	0.23121	36547.2	9781.9	10552.5	U/P
10.578	0.0000	0.0000	157.182	0.35377	0.00000	36547.2	23509.0	10552.5	U/S
16.578	0.0000	0.0000	157.069	0.05754	0.00000	36547.2	25065.0	10552.5	S
22.578	0.0000	0.0000	156.950	0.02152	0.00000	36547.2	25994.7	10552.5	S
28.578	0.0000	0.0000	156.719	0.00000	0.00000	36547.2	25994.7	10552.5	S
34.578 40.578	0.0000 0.0000	0.0000 0.0000	156.537 156.387	0.00000 0.00000	0.00000 0.00000	36547.2 36547.2	25994.7 25994.7	10552.5 10552.5	S
46.578	0.0000	0.0000	156.259	0.00000	0.00000	36547.2 36547.2	25994.7 25994.7	10552.5	S S
52.578	0.0000	0.0000	156.259	0.00000	0.00000	36547.2	25994.7	10552.5	S
58.578	0.0000	0.0000	156.048	0.00000	0.00000	36547.2	25994.7	10552.5	Š
64.578	0.0000	0.0000	155.958	0.00000	0.00000	36547.2	25994.7	10552.5	S
70.578	0.0000	0.0000	155.878	0.00000	0.00000	36547.2	25994.7	10552.5	S
76.578	0.0000	0.0000	155.804	0.00000	0.00000	36547.2	25994.7	10552.5	S
82.578	0.0000	0.0000	155.736	0.00000	0.00000	36547.2 36547.2	25994.7	10552.5	S S
88.578 94.578	0.0000 0.0000	0.0000 0.0000	155.673 155.614	0.00000 0.00000	0.00000 0.00000	36547.2 36547.2	25994.7 25994.7	10552.5 10552.5	S
			. 55.517		2.00000	550 IT.E	2500 1.7	. 5002.0	

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Detailed Results (cont,d.) :: Scenario 4 :: SRWMD 100YR-8HR

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
8.222	0.0696	0.0000	158.052	0.90330	0.07386	49421.5	16989.3	17144.9	U/P
8.244	0.0498	0.0000	158.047	0.90272	0.06409	49426.3	17061.5	17150.4	U/P
8.267	0.0358	0.0000	158.042	0.90214	0.05471	49429.7	17133.7	17155.2	U/P
8.289	0.0255	0.0000	158.038	0.90156	0.04582	49432.1	17205.8	17159.2	U/P
8.311	0.0182	0.0000	158.033	0.90097	0.03747	49433.9	17277.9	17162.5	U/P
8.333	0.0129	0.0000	158.028	0.90039	0.02971	49435.1	17350.0	17165.2	U/P
8.356	0.0091	0.0000	158.024	0.89981	0.02260	49436.0	17422.0	17167.3	U/P
8.378	0.0064	0.0000	158.019	0.89924	0.01619	49436.6	17494.0	17168.8	U/P
8.400	0.0044	0.0000	158.014	0.89866	0.01056	49437.1	17565.9	17169.9	U/P
8.422	0.0030	0.0000	158.010	0.89809	0.00581	49437.4	17637.8	17170.6 17170.9	U/P U/P
8.444 8.467	0.0020 0.0011	0.0000 0.0000	158.005 158.000	0.89752 1.15253	0.00213 0.00002	49437.5 49437.7	17709.6 17781.4	17170.9	U/P
8.489	0.0006	0.0000	157.993	1.05693	0.00002	49437.7	17894.0	17171.0	U/S
8.511	0.0002	0.0000	157.989	0.70395	0.00000	49437.8	17950.5	17171.0	S
8.533	0.0000	0.0000	157.986	0.69816	0.00000	49437.8	18006.6	17171.0	S
8.556	0.0000	0.0000	157.982	0.68940	0.00000	49437.8	18062.2	17171.0	S
8.578	0.0000	0.0000	157.979	0.68240	0.00000	49437.8	18116.9	17171.0	S S S
14.578	0.0000	0.0000	157.750	0.12199	0.00000	49437.8	21590.3	17171.0	
20.578	0.0000	0.0000	157.630	0.07204	0.00000	49437.8	23387.1	17171.0	S S S
26.578	0.0000	0.0000	157.540	0.05534	0.00000	49437.8	24702.6	17171.0	S
32.578	0.0000	0.0000	157.466	0.04630	0.00000	49437.8	25777.7	17171.0	S
38.578	0.0000	0.0000	157.401	0.04039	0.00000	49437.8	26702.7	17171.0	S S
44.578 50.578	0.0000 0.0000	0.0000 0.0000	157.343 157.291	0.03613 0.03286	0.00000 0.00000	49437.8 49437.8	27522.5 28263.3	17171.0 17171.0	0
56.578	0.0000	0.0000	157.242	0.03026	0.00000	49437.8	28942.2	17171.0	S
62.578	0.0000	0.0000	157.197	0.02812	0.00000	49437.8	29570.6	17171.0	S S S
68.578	0.0000	0.0000	157.155	0.02632	0.00000	49437.8	30157.0	17171.0	Š
74.578	0.0000	0.0000	157.115	0.02478	0.00000	49437.8	30707.7	17171.0	S S S
80.578	0.0000	0.0000	157.077	0.02345	0.00000	49437.8	31227.6	17171.0	S
86.578	0.0000	0.0000	157.040	0.02227	0.00000	49437.8	31720.5	17171.0	S
92.578	0.0000	0.0000	157.006	0.01265	0.00000	49437.8	32189.6	17171.0	S
98.578	0.0000	0.0000	156.905	0.00179	0.00000	49437.8	32266.8	17171.0	S
104.578	0.0000	0.0000	156.803	0.00000	0.00000	49437.8	32266.8	17171.0	S
110.578 116.578	0.0000 0.0000	0.0000 0.0000	156.711 156.626	0.00000 0.00000	0.00000 0.00000	49437.8 49437.8	32266.8 32266.8	17171.0 17171.0	S
122.578	0.0000	0.0000	156.548	0.00000	0.00000	49437.8	32266.8	17171.0	S S S
128.578	0.0000	0.0000	156.475	0.00000	0.00000	49437.8	32266.8	17171.0	S
134.578	0.0000	0.0000	156.407	0.00000	0.00000	49437.8	32266.8	17171.0	S
140.578	0.0000	0.0000	156.343	0.00000	0.00000	49437.8	32266.8	17171.0	Š
146.578	0.0000	0.0000	156.283	0.00000	0.00000	49437.8	32266.8	17171.0	S S S
152.578	0.0000	0.0000	156.226	0.00000	0.00000	49437.8	32266.8	17171.0	S
158.578	0.0000	0.0000	156.172	0.00000	0.00000	49437.8	32266.8	17171.0	S S S
164.578	0.0000	0.0000	156.121	0.00000	0.00000	49437.8	32266.8	17171.0	S
170.578	0.0000	0.0000	156.072	0.00000	0.00000	49437.8	32266.8	17171.0	S S S
176.578	0.0000	0.0000	156.026	0.00000	0.00000	49437.8	32266.8	17171.0	S
182.578 188.578	0.0000 0.0000	0.0000 0.0000	155.981 155.938	0.00000 0.00000	0.00000 0.00000	49437.8 49437.8	32266.8 32266.8	17171.0 17171.0	5
194.578	0.0000	0.0000	155.897	0.00000	0.00000	49437.8	32266.8	17171.0	S S
200.578	0.0000	0.0000	155.858	0.00000	0.00000	49437.8	32266.8	17171.0	S
206.578	0.0000	0.0000	155.820	0.00000	0.00000	49437.8	32266.8	17171.0	S S S
212.578	0.0000	0.0000	155.783	0.00000	0.00000	49437.8	32266.8	17171.0	S
218.578	0.0000	0.0000	155.748	0.00000	0.00000	49437.8	32266.8	17171.0	S
224.578	0.0000	0.0000	155.714	0.00000	0.00000	49437.8	32266.8	17171.0	S
230.578	0.0000	0.0000	155.681	0.00000	0.00000	49437.8	32266.8	17171.0	S
236.578	0.0000	0.0000	155.650	0.00000	0.00000	49437.8	32266.8	17171.0	S
242.578	0.0000	0.0000	155.619	0.00000	0.00000	49437.8	32266.8	17171.0	S
248.578 254.578	0.0000 0.0000	0.0000 0.0000	155.589 155.560	0.00000 0.00000	0.00000 0.00000	49437.8 49437.8	32266.8 32266.8	17171.0 17171.0	S
260.578	0.0000	0.0000	155.532	0.00000	0.00000	49437.8	32266.8	17171.0	S S
266.578	0.0000	0.0000	155.505	0.00000	0.00000	49437.8	32266.8	17171.0	S
272.578	0.0000	0.0000	155.479	0.00000	0.00000	49437.8	32266.8	17171.0	S
278.578	0.0000	0.0000	155.453	0.00000	0.00000	49437.8	32266.8	17171.0	S
284.578	0.0000	0.0000	155.428	0.00000	0.00000	49437.8	32266.8	17171.0	S
290.578	0.0000	0.0000	155.404	0.00000	0.00000	49437.8	32266.8	17171.0	S
296.578	0.0000	0.0000	155.380	0.00000	0.00000	49437.8	32266.8	17171.0	S
302.578	0.0000	0.0000	155.357	0.00000	0.00000	49437.8	32266.8	17171.0	S
308.578	0.0000	0.0000	155.335	0.00000	0.00000	49437.8	32266.8	17171.0	S
314.578	0.0000	0.0000	155.313	0.00000	0.00000	49437.8	32266.8	17171.0	S
320.578 326.578	0.0000	0.0000	155.292 155.271	0.00000	0.00000	49437.8	32266.8	17171.0 17171.0	S S
326.578 332.578	0.0000 0.0000	0.0000 0.0000	155.271 155.251	0.00000 0.00000	0.00000 0.00000	49437.8 49437.8	32266.8 32266.8	17171.0 17171.0	S
338.578	0.0000	0.0000	155.231	0.00000	0.00000	49437.8 49437.8	32266.8	17171.0	S
344.578	0.0000	0.0000	155.211	0.00000	0.00000	49437.8	32266.8	17171.0	S
350.578	0.0000	0.0000	155.192	0.00000	0.00000	49437.8	32266.8	17171.0	S

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Detailed Results (cont,d.) :: Scenario 5 :: SRWMD 100YR-24HR

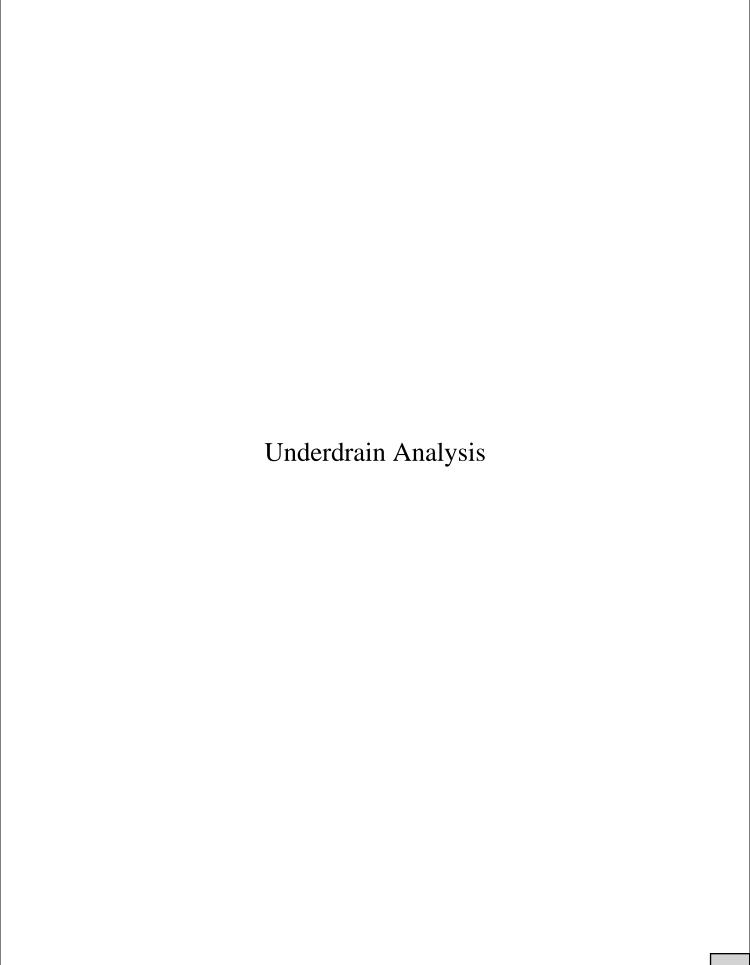
Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type	
48.578	0.0000	0.0000	157.790	0.04142	0.00000	75765.1	26523.4	37974.5	S	
54.578	0.0000	0.0000	157.733	0.03772	0.00000	75765.1	27373.4	37974.5	Š	
60.578	0.0000	0.0000	157.681	0.03474	0.00000	75765.1	28152.7	37974.5	S	
66.578	0.0000	0.0000	157.632	0.03228	0.00000	75765.1	28874.2	37974.5	S	
72.578	0.0000	0.0000	157.586	0.03021	0.00000	75765.1	29547.2	37974.5	S	
78.578	0.0000	0.0000	157.543	0.02843	0.00000	75765.1	30179.2	37974.5	S	
84.578	0.0000	0.0000	157.502	0.02689	0.00000	75765.1	30775.5	37974.5	S	
90.578	0.0000	0.0000	157.463	0.02554	0.00000	75765.1	31340.8	37974.5	S	
96.578	0.0000	0.0000	157.425	0.02433	0.00000	75765.1	31878.7	37974.5	S	
102.578	0.0000	0.0000	157.389	0.02325	0.00000	75765.1	32392.0	37974.5	S	
108.578	0.0000	0.0000	157.355	0.02228	0.00000	75765.1	32883.3	37974.5	S	
114.578	0.0000	0.0000	157.322	0.02140	0.00000	75765.1	33354.6	37974.5	S	
120.578	0.0000	0.0000	157.289	0.02059	0.00000	75765.1	33807.7	37974.5	S	
126.578	0.0000	0.0000	157.258	0.01985	0.00000	75765.1	34244.2	37974.5	S	
132.578	0.0000	0.0000	157.228	0.01917	0.00000	75765.1	34665.4	37974.5	S	
138.578	0.0000	0.0000	157.199	0.01854	0.00000	75765.1	35072.5	37974.5	S	
144.578	0.0000	0.0000	157.170	0.01795	0.00000	75765.1	35466.4	37974.5	S S	
150.578	0.0000 0.0000	0.0000	157.143 157.116	0.01741	0.00000	75765.1 75765.1	35848.2	37974.5	5	
156.578	0.0000	0.0000 0.0000	157.116	0.01690 0.01642	0.00000	75765.1 75765.1	36218.5	37974.5	S S	
162.578 168.578	0.0000	0.0000	157.069	0.01597	0.00000 0.00000	75765.1	36578.1 36927.7	37974.5 37974.5	S	
174.578	0.0000	0.0000	157.039	0.01554	0.00000	75765.1	37267.9	37974.5	S	
180.578	0.0000	0.0000	157.014	0.01210	0.00000	75765.1	37599.1	37974.5	S	
186.578	0.0000	0.0000	156.967	0.00443	0.00000	75765.1	37790.6	37974.5	S	
192.578	0.0000	0.0000	156.889	0.00000	0.00000	75765.1	37790.6	37974.5	S	
198.578	0.0000	0.0000	156.818	0.00000	0.00000	75765.1	37790.6	37974.5	S	
204.578	0.0000	0.0000	156.752	0.00000	0.00000	75765.1	37790.6	37974.5	S	
210.578	0.0000	0.0000	156.690	0.00000	0.00000	75765.1	37790.6	37974.5	S	
216.578	0.0000	0.0000	156.632	0.00000	0.00000	75765.1	37790.6	37974.5	S	
222.578	0.0000	0.0000	156.577	0.00000	0.00000	75765.1	37790.6	37974.5	S	
228.578	0.0000	0.0000	156.525	0.00000	0.00000	75765.1	37790.6	37974.5	S	
234.578	0.0000	0.0000	156.475	0.00000	0.00000	75765.1	37790.6	37974.5	S	
240.578	0.0000	0.0000	156.428	0.00000	0.00000	75765.1	37790.6	37974.5	S	
246.578	0.0000	0.0000	156.382	0.00000	0.00000	75765.1	37790.6	37974.5	S	
252.578	0.0000	0.0000	156.338	0.00000	0.00000	75765.1	37790.6	37974.5	S	
258.578	0.0000	0.0000	156.296	0.00000	0.00000	75765.1	37790.6	37974.5	S	
264.578	0.0000	0.0000	156.256	0.00000	0.00000	75765.1	37790.6	37974.5	S	
270.578	0.0000	0.0000	156.216	0.00000	0.00000	75765.1	37790.6	37974.5	S S	
276.578	0.0000	0.0000	156.179	0.00000	0.00000	75765.1	37790.6	37974.5	5	
282.578 288.578	0.0000 0.0000	0.0000 0.0000	156.142 156.107	0.00000 0.00000	0.00000 0.00000	75765.1 75765.1	37790.6 37790.6	37974.5 37974.5	S S	
294.578	0.0000	0.0000	156.073	0.00000	0.00000	75765.1	37790.6	37974.5	S	
300.578	0.0000	0.0000	156.040	0.00000	0.00000	75765.1	37790.6	37974.5	s	
306.578	0.0000	0.0000	156.008	0.00000	0.00000	75765.1	37790.6	37974.5	s	
312.578	0.0000	0.0000	155.976	0.00000	0.00000	75765.1	37790.6	37974.5	Š	
318.578	0.0000	0.0000	155.946	0.00000	0.00000	75765.1	37790.6	37974.5	S	
324.578	0.0000	0.0000	155.917	0.00000	0.00000	75765.1	37790.6	37974.5	S	
330.578	0.0000	0.0000	155.888	0.00000	0.00000	75765.1	37790.6	37974.5	S	
336.578	0.0000	0.0000	155.860	0.00000	0.00000	75765.1	37790.6	37974.5	S S	
342.578	0.0000	0.0000	155.833	0.00000	0.00000	75765.1	37790.6	37974.5	S	
348.578	0.0000	0.0000	155.807	0.00000	0.00000	75765.1	37790.6	37974.5	S	
354.578	0.0000	0.0000	155.781	0.00000	0.00000	75765.1	37790.6	37974.5	S	
360.578	0.0000	0.0000	155.756	0.00000	0.00000	75765.1	37790.6	37974.5	S	
366.578	0.0000	0.0000	155.731	0.00000	0.00000	75765.1	37790.6	37974.5	S	
372.578	0.0000	0.0000	155.707	0.00000	0.00000	75765.1	37790.6	37974.5 37974.5	S S	
378.578 384.578	0.0000 0.0000	0.0000 0.0000	155.684 155.661	0.00000 0.00000	0.00000 0.00000	75765.1 75765.1	37790.6 37790.6		S	
390.578	0.0000	0.0000	155.638	0.00000	0.00000	75765.1	37790.6	37974.5 37974.5	S	
396.578	0.0000	0.0000	155.616	0.00000	0.00000	75765.1	37790.6	37974.5	S	
402.578	0.0000	0.0000	155.595	0.00000	0.00000	75765.1	37790.6	37974.5	S	
408.578	0.0000	0.0000	155.574	0.00000	0.00000	75765.1	37790.6	37974.5	Š	
414.578	0.0000	0.0000	155.554	0.00000	0.00000	75765.1	37790.6	37974.5	S	
420.578	0.0000	0.0000	155.534	0.00000	0.00000	75765.1	37790.6	37974.5	Š	
426.578	0.0000	0.0000	155.514	0.00000	0.00000	75765.1	37790.6	37974.5	Š	
432.578	0.0000	0.0000	155.495	0.00000	0.00000	75765.1	37790.6	37974.5	S	
438.578	0.0000	0.0000	155.476	0.00000	0.00000	75765.1	37790.6	37974.5	S	
444.578	0.0000	0.0000	155.457	0.00000	0.00000	75765.1	37790.6	37974.5	S	
450.578	0.0000	0.0000	155.439	0.00000	0.00000	75765.1	37790.6	37974.5	S	
456.578	0.0000	0.0000	155.421	0.00000	0.00000	75765.1	37790.6	37974.5	S	
462.578	0.0000	0.0000	155.404	0.00000	0.00000	75765.1	37790.6	37974.5	S	
468.578	0.0000	0.0000	155.386	0.00000	0.00000	75765.1	37790.6	37974.5	S	
474.578	0.0000	0.0000	155.370	0.00000	0.00000	75765.1	37790.6	37974.5	S	
480.578	0.0000	0.0000	155.353	0.00000	0.00000	75765.1	37790.6	37974.5	S S	
486.578	0.0000	0.0000	155.337	0.00000	0.00000	75765.1	37790.6	37974.5	5	

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Detailed Results :: Scenario 6 :: WQTV

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
0.000	2080.5000	0.0000	152.990	0.00000	0.00000	0.0	0.0	0.0	N.A.
0.002	2080.5000	0.0000	157.870	0.88138	0.00000	12483.0	5.3	0.0	U/P
2.400	0.0000	0.0000	157.369	0.51210	0.00000	12483.0	7372.3	0.0	U/P
6.000	0.0000	0.0000				12483.0	12483.0	0.0	dry
12.000	0.0000	0.0000				12483.0	12483.0	0.0	dry
24.000	0.0000	0.0000				12483.0	12483.0	0.0	dry
36.000	0.0000	0.0000				12483.0	12483.0	0.0	dry
48.000	0.0000	0.0000				12483.0	12483.0	0.0	dry
60.000	0.0000	0.0000				12483.0	12483.0	0.0	dry
72.000	0.0000	0.0000				12483.0	12483.0	0.0	dry
84.000	0.0000	0.0000				12483.0	12483.0	0.0	dry
96.000	0.0000	0.0000				12483.0	12483.0	0.0	dry

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Project Number: 23-0653 Project Name: CRS Marvin Burnett

Calculated by: JHP Checked by: CCM Date: 1/16/2024

BACKGROUND SEEPAGE PONDS INPUTS

Aquifer Data

Base of Aquifer: 148.7 ft
Seasonal High Water Table: 154.7 ft
Hydraulic Conductivity 5 ft/day
Fillable Porosity 20 %

Geometry: Underdrain Stage Storage

Based on the theoretical volume of water the underdrains could draw down.

ELEV.	AREA (SF)	STORAGE (CF)	STORAGE VOLUME (AC- FT)
152.00	13,469	0	0.000
153.00	15,494	14,482	0.332
154.00	17,606	31,032	0.712
155.00	19,819	49,744	1.142

*Based on minimum Underdrain Orifice EL. 152.28'

*Set above the measured SHWT EL. 154.7'

Volume: 49,744 cf Area: 19819 sf Depth: 3.00 ft Perimeter: 566 ft Eq. Length: 200 ft Eq: Width: 83 ft

Project Data

Project Name: CRS Marvin Burnett

Simulation Description: Background Seepage

Project Number: 23-0653

Engineer: JHP

Supervising Engineer: CCM

Date: 01-18-2024

Aquifer Data

Base Of Aquifer Elevation, [B] (ft datum): 148.70

Water Table Elevation, [WT] (ft datum): 154.70

Horizontal Saturated Hydraulic Conductivity, [Kh] (ft/day): 5.00

Fillable Porosity, [n] (%):

Vertical infiltration was not considered.

Geometry Data

Equivalent Pond Length, [L] (ft): 200.0

Equivalent Pond Width, [W] (ft): 83.0

Ground water mound is expected to intersect the pond bottom

Stage vs Area Data

Stage	Area
(ft datum)	(ft²)
152.00	13469.0
153.00	15494.0
154.00	17606.0
155.00	19819.0

Discharge Structures

Discharge Structure #1 is active as orifice

Structure Parameters

Description: 12" Underdrain

Orifice elevation, (ft datum): 152.27
Orifice coefficient: 4.9
Orifice area, (ft²): 0.785
Orifice exponent: 0.5

Tailwater - disabled, free discharge

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Scenario Input Data

Scenario 1 :: 12" Underdrain

Hydrograph Type: Baseflow

Modflow Routing: Routed with infiltration

Seasonal Water Table Fluctuation (ft) 0.01 Duration of Wet Season (days) 120.0 **Number of Increments** 240

Initial (seasonal low) ground water level (ft datum) 154.69

Recharge is applied inside pond (in addition to outside pond)? No

Note: when this option is selected, water will be added to the pond to synchronize the rise in the pond level with the rise in the groundwater. Otherwise, no water will be added directly to the pond, and the pond water level will rise as a result of infiltration only.

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PONDS Version 3.3.0241 **Retention Pond Recovery - Refined Method** Copyright 2011 Devo Seereeram, Ph.D., P.E.

Detailed Results (cont,d.) :: Scenario 1 :: 12" Underdrain

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
2664.000	0.0000	0.0000	152.270	-0.00331	0.00331	0.0	-178509.4	218681.4	S
2676.000	0.0000	0.0000	152.270	-0.00331	0.00331	0.0	-178652.3	218824.3	S
2688.000	0.0000	0.0000	152.270	-0.00330	0.00330	0.0	-178795.1	218967.1	S
2700.000	0.0000	0.0000	152.270	-0.00330	0.00330	0.0	-178937.7	219109.7	S
2712.000	0.0000	0.0000	152.270	-0.00329	0.00329	0.0	-179080.1	219252.1	S
2724.000	0.0000	0.0000	152.270	-0.00329	0.00329	0.0	-179222.4	219394.4	S
2736.000	0.0000	0.0000	152.270	-0.00329	0.00329	0.0	-179364.5	219536.4	S
2748.000	0.0000	0.0000	152.270	-0.00328	0.00328	0.0	-179506.3	219678.3	S
2760.000	0.0000	0.0000	152.270	-0.00328	0.00328	0.0	-179648.1	219820.1	S
2772.000	0.0000	0.0000	152.270	-0.00327	0.00327	0.0	-179789.6	219961.6	S
2784.000	0.0000	0.0000	152.270	-0.00327	0.00327	0.0	-179931.0	220103.0	S
2796.000	0.0000	0.0000	152.270	-0.00327	0.00327	0.0	-180072.2	220244.2	S
2808.000	0.0000	0.0000	152.270	-0.00326	0.00326	0.0	-180213.2	220385.2	S
2820.000	0.0000	0.0000	152.270	-0.00326	0.00326	0.0	-180354.1	220526.1	S
2832.000	0.0000	0.0000	152.270	-0.00325	0.00325	0.0	-180494.8	220666.8	S
2844.000	0.0000	0.0000	152.270	-0.00325	0.00325	0.0	-180635.3	220807.3	S
2856.000	0.0000	0.0000	152.270	-0.00325	0.00325	0.0	-180775.7	220947.7	S
2868.000	0.0000	0.0000	152.270	-0.00324	0.00324	0.0	-180915.9	221087.9	S
2880.000	0.0000	0.0000	152.270		<u></u>	0.0	-181055.9	221227.9	N.A.

FLOW RATE WHEN SYSTEM ACHIEVES STEADY STATE (120 DAYS) WITH SEASONAL HIGH WATER TABLE ELEVATION

CRS Marvin Burnett 01-18-2024 15:29:32 Page 15:29 Page 1

Job Information

Job Name: 23-0653 CRS Marvin Burnett

Engineer: JHP Date: 01-18-2024

Input Data

Area at top of pond, [ATOP]:	19819	ft²
Depth of basin, [d]:	3	ft
Aquifer depth below pond bottom, [B]:	8	ft
Desired depth to water table below pond bottom, [R]:	0.5	ft
Hydraulic conductivity of soil, [K]:	10	ft/day
Drain diameter, [D]:	12	in
Thickness of gravel envelope, [t]:	9	in
Thickness of soil cover, [H]:	2	ft
Treatment volume, [PAV]:	31032	ft³
Recovery time, [T]:	30	days
Factor of safety, [FS]:	2	
Background seepage, [qb]:	1.45	gpm 🖊

0.00324 ft^3/s converted to gallons per minute

Free discharge / no tailwater

Results

Computed underdrain spacing, [S]: 106.6505 ft Computed total length of laterals, [L]: 185.8312 ft Computed flow rate through outfall, [Q]: 2.717507E-02 ft³/sec Computed flow rate per lineal foot of lateral, [ql]: 1.462352E-04 ft³/sec/ft

23-0653 CRS Marvin Burnett 01-18-2024 15:33:05

Notes

- 1. Laterals should be no farther than S/2 from the top of the basin.
- 2. A gravel envelope at least 3 inches thick is recommended around the underdrain pipes. If a gravel envelope is used, a filter fabric will be required around this envelope.
- 3. The underdrain pipe should have a filter fabric sock to prevent fines from moving into and clogging the perforated pipe.
- 4. Ensure outfall elevation for system will allow gravity flow without tailwater backpressure to the underdrains.
- 5. Theory is applicable where ground water flow is largely in a horizontal direction (i.e., natural gradients less than 1%).
- 6. Capped and sealed inspection and cleanout ports which extend to the ground surface are recommended at the following locations for each drain pipe:
 - a. the terminus
 - b. at every 400 feet or every bend of 45 or more degrees, whichever is shortest
- 7. Underdrain basin should be stabilized with permanent vegetative cover.

Warnings

None.

23-0653 CRS Marvin Burnett 01-18-2024 15:33:05 P

Job Information

Job Name: 23-0653 CRS Marvin Burnett

Engineer: JHP Date: 01-18-2024

Input Data

Area at top of pond, [ATOP]:	19819	ft²	
Depth of basin, [d]:	3	ft	
Aquifer depth below pond bottom, [B]:	8	ft	
Desired depth to water table below pond bottom, [R]:	0.5	ft	
Hydraulic conductivity of soil, [K]:	10	ft/day	
Drain diameter, [D]:	12	in	
Thickness of gravel envelope, [t]:	9	in	
Thickness of soil cover, [H]:	2	ft	
Treatment volume, [PAV]:	12483	ft³	
Recovery time, [T]:	3	days	0.00004 ((40)
Factor of safety, [FS]:	2	_	0.00324 ft^3/s converted
Background seepage, [qb]:	1.45	gpm 🖊	to gallons per minute
Free discharge / no tailwater			

Results

Computed underdrain spacing, [S]: 55.72212 ft
Computed total length of laterals, [L]: 355.6756 ft
Computed flow rate through outfall, [Q]: 9.955007E-02 ft³/sec
Computed flow rate per lineal foot of lateral, [ql]: 2.798901E-04 ft³/sec/ft

<u>Note</u>	
Maximum Underdrain Spacing:	55.7 ft
Provided Underdrain Spacing:	25.0 ft
Minimum Lateral Length:	355 ft
Provided Lateral Length:	375 ft

23-0653 CRS Marvin Burnett 01-18-2024 15:31:41 P

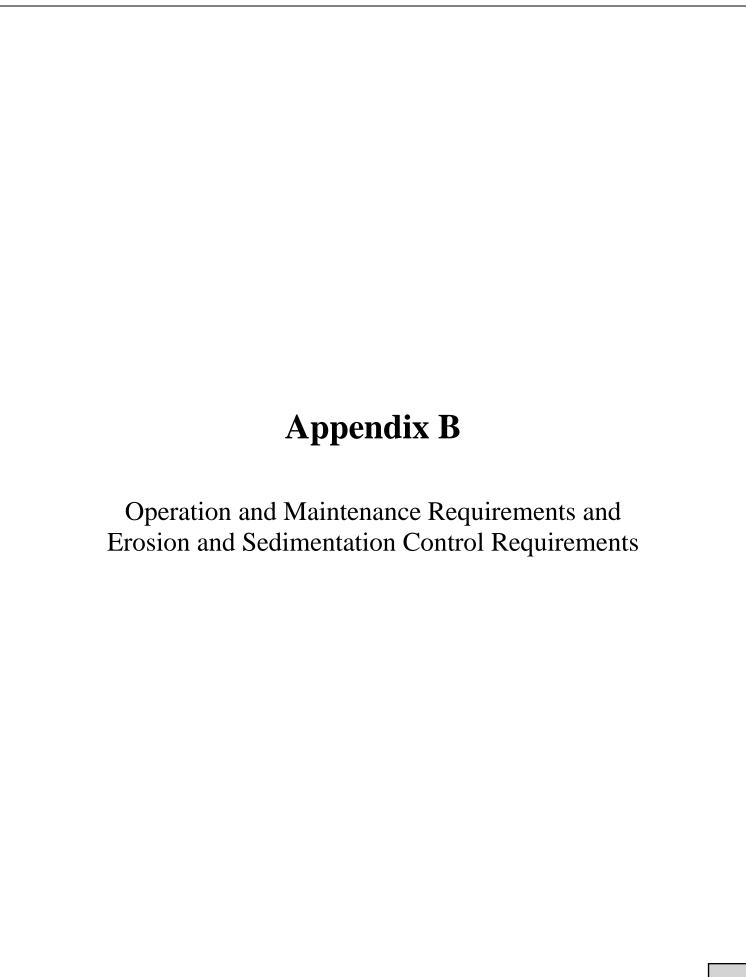
Notes

- 1. Laterals should be no farther than S/2 from the top of the basin.
- 2. A gravel envelope at least 3 inches thick is recommended around the underdrain pipes. If a gravel envelope is used, a filter fabric will be required around this envelope.
- 3. The underdrain pipe should have a filter fabric sock to prevent fines from moving into and clogging the perforated pipe.
- 4. Ensure outfall elevation for system will allow gravity flow without tailwater backpressure to the underdrains.
- 5. Theory is applicable where ground water flow is largely in a horizontal direction (i.e., natural gradients less than 1%).
- 6. Capped and sealed inspection and cleanout ports which extend to the ground surface are recommended at the following locations for each drain pipe:
 - a. the terminus
 - b. at every 400 feet or every bend of 45 or more degrees, whichever is shortest
- 7. Underdrain basin should be stabilized with permanent vegetative cover.

Warnings

None.

23-0653 CRS Marvin Burnett 01-18-2024 15:31:41 P



Proposed operation and maintenance and soil erosion and sediment control practices are outlined in the following paragraphs.

Surface water Management Facilities

The man-made surface water facility shall be maintained free of sediments and debris. Areas shall be inspected on a routine basis and nuisance plants shall be removed a minimum of twice annually. Grassed areas shall be mowed a minimum of 6 times per year. The natural systems shall be least disturbed as possible. Minimal maintenance is required for the natural and undisturbed areas. All ponds shall be inspected monthly. Monthly documentation shall be noted based upon the inspection findings.

Erosion Control

All erosion damage at spillways, outfall structures, and along pond side slopes shall be repaired (grading and grassing) as conditions occur. All side slopes and other areas disturbed by construction shall be stabilized by sodding, hydro-mulching or other appropriate vegetative or non-vegetative erosion control measures.

Swale/Ditch

All swales, if any, shall be maintained free of debris and sediment. Sediments shall be removed when the depth has been reduced by 20 percent. Sediments removed from swales/ditches should be evenly spread over grassed areas away from the stormwater management facilities.

Culverts, Pipes and Structures

All pipes, if any, shall be inspected bi-annually. Culverts and pipes shall be maintained free of debris and sediment. Sediments removed from culverts and pipes should be evenly spread over grassed areas away from the stormwater management facilities.

The structures and paved flow lines, if any, shall be maintained clear of debris. Remove any debris and silt collected in inlets and pipes as routine inspections dictates.

Inspection Reporting

Annual inspection reports, prepared by a properly licensed professional engineer, should be submitted to the water management district as appropriate. The engineer shall inspect the site and report on the status and function of the system. Noted deficiencies and/or maintenance requirements shall be reported to the owner with recommendations for repairs. Repairs shall be executed.

Limerock/Sinkhole

If continuous limerock is encountered during excavation of the swales/pond or if a sinkhole forms in the area of a drainage swale/pond the engineer of record shall be notified by either the contractor or the established operation and maintenance entity. The engineer of record shall inspect the repaired area upon completion of the repair.

Where continuous limerock is encountered during excavation of the swales/ponds, the limerock shall be over excavated by 2 feet and replaced with clayey soils that extend 2 feet beyond the perimeter of the limerock outcropping. The clayey soil shall have at least 20% passing the no. 200 sieve, compacted to 95% of standard proctor, and compacted in a wet condition with moisture 2% - 4% above optimum.

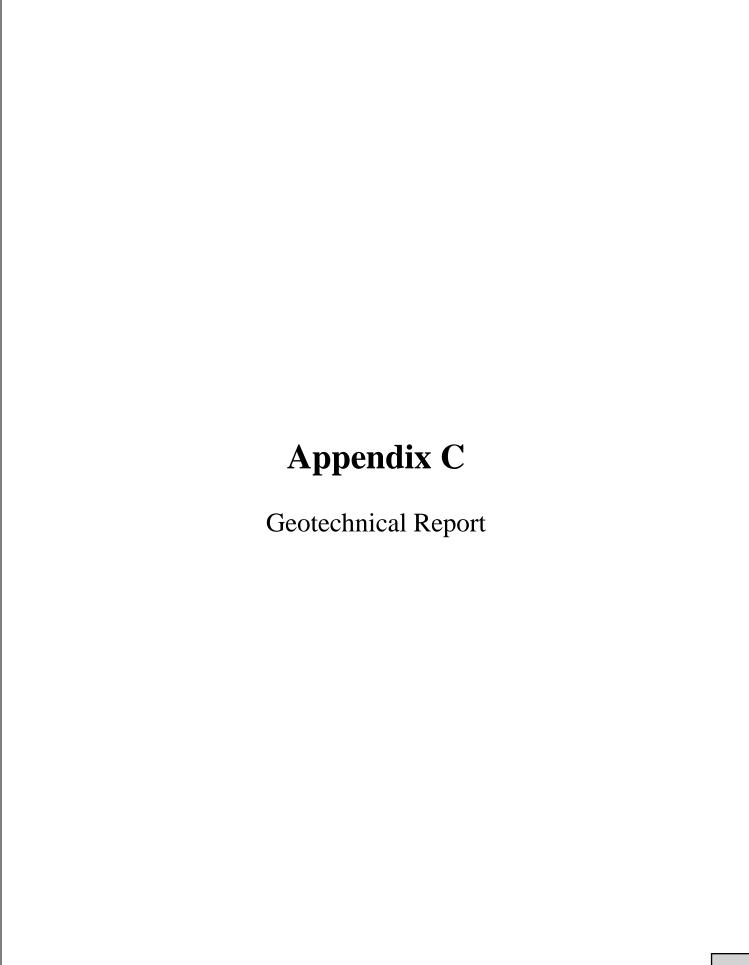
All swales/ponds shall be inspected monthly for sinkhole occurrence. Should a sinkhole occur, the area shall be repaired as soon as possible. Repair shall include filling (limerock such as road base material, clay/sand mixture, or concrete if necessary). A 2-foot deep cap that extends 2 feet beyond the perimeter of the sinkhole shall be constructed with clayey soils. The clayey soil shall have at least 20% passing the no. 200 sieve, compacted to 95% of standard proctor, and compacted in a wet condition with moisture 2% - 4% above optimum. The clay soil cap shall be re-graded to prevent concentration of waters (ponding) and re-vegetated.

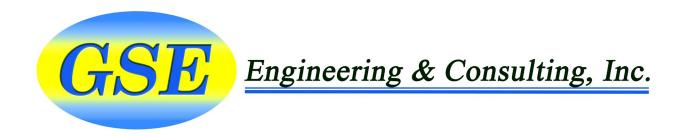
Outfall Structures

All outfall and drawdown orifices are to be inspected bi-annually for sediment or debris in the flow line of weirs or orifices. All sediment and debris should be removed and disposed of in an approved manner.

Operation & Maintenance Entity:

Concept Development, Inc. 1449 SW 74th Drive. Suite 200 Gainesville, FL 32607





SUMMARY REPORT OF A GEOTECHNICAL SITE EXPLORATION – REVISION 1

DOLLAR GENERAL – LAKE CITY SW MARVIN BURNETT LAKE CITY, COLUMBIA COUNTY, FLORIDA

GSE PROJECT NO. 16251

Prepared For:

CONCEPT DEVELOPMENT, INC.

DECEMBER 2023



December 7, 2023

Andrea Barnett Concept Development, Inc. 1449 SW 74th Drive, Suite 200 Gainesville, Florida 32607

Subject: Summary Report of a Geotechnical Site Exploration - Revision 1

Dollar General - Lake City SW Marvin Burnett

Lake City, Columbia County, Florida

GSE Project No. 16251

GSE Engineering & Consulting, Inc. (GSE) is pleased to submit this geotechnical site exploration report for the above referenced project.

Presented herein are the findings and conclusions of our exploration, including the geotechnical parameters and recommendations to assist with building foundation, pavement, and stormwater management designs. This revision includes recommended soil parameters for stormwater management design with underdrains.

GSE appreciates this opportunity to have assisted you on this project. If you have any questions or comments concerning this report, please contact us.

Sincerely,

GSE Engineering & Consulting, Inc.

This item has been digitally signed and sealed by

15:39:40 -05'00' on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on any

Digitally signed by Jason E Gowland

Jason E. Gowland, P.E. Principal Engineer Florida Registration No. 66467

electronic copies.

Angelina X. Liu, E.I. Staff Engineer

AXL / JEG: tlf Q:\Projects\16251 Dollar General - Lake City SW Marvin Burnett\16251 Rev.1.docx

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File (1)

GSE Project No. 16251

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- 1. Project Site Location Map
- 2. Site Plan Showing Approximate Locations of Field Tests

1.0 INTRODUCTION

1.1 General

GSE Engineering & Consulting, Inc. (GSE) has completed this geotechnical exploration for the proposed commercial retail store located on SW Marvin Burnett Road in Lake City, Columbia County, Florida. This exploration was performed in accordance with GSE Proposal No. 2023-589 dated September 12, 2023. Ms. Andrea Barnett authorized our services on September 15, 2023.

1.2 Project Description

We understand that you are coordinating due diligence related work related to the development of this site into a commercial retail store. The site is located on the northwest corner of the State Road 47 and SW Marvin Burnett Road intersection in Lake City, Columbia County, Florida. The site is approximately +/-2.72 acres.

You provided GSE with information about the project. We understand the project will consist of an approximate 10,640 square foot building, a parking lot, and a stormwater management facility.

The structure is expected to be a single-story, high wall concrete masonry unit (CMU) and steel frame construction. Structural loads have not been provided but are expected to be on the order of 1 to 2 kips per foot for non-load bearing CMU walls, and less than 50 kips for columns. The finished floor of the structure is anticipated to be constructed within 1 to 2 feet of the existing site grades.

The building will be located in the northern portion of the site. The parking lot will be located west, south, and east of the structure. The stormwater management facility will be located on the western portion of the site.

A recent aerial photograph of the site was obtained and reviewed. The site plan and aerial photograph were used in preparation of this exploration and report.

1.3 Purpose

The purpose of this geotechnical exploration was to determine the general subsurface conditions, evaluate these conditions with respect to the proposed construction, and prepare geotechnical parameters and recommendations to assist with building foundation, stormwater management, and pavement designs.

2.0 FIELD AND LABORATORY TESTS

2.1 General Description

The procedures used for field sampling and testing are in general accordance with industry standards of care and established geotechnical engineering practices for this geographic region. This exploration consisted of performing five (5) Standard Penetration Test (SPT) borings to a depth of 20 feet below land surface (bls) within the proposed building area, five (5) auger borings to a depth of 5 feet bls in the area of the parking lot and driveways, and five (5) auger borings to depths of 15 feet bls in the area of the stormwater management facility.

The soil borings were performed at the approximate locations as shown on Figure 2. The borings were located at the site using the provided site plan, Global Positioning System (GPS) coordinates, and obvious site features as reference. The boring locations should be considered approximate. The soil borings were performed on September 20, 2023.

2.2 Auger Borings

The auger borings were performed in accordance with ASTM D1452. The borings were performed with flight auger equipment that was rotated into the ground in a manner that reduces soil disturbance. After penetrating to the required depth, the auger was retracted and the soils collected on the auger flights were field classified and placed in sealed containers. Representative samples of each stratum were retained from the auger boring. Results from the auger borings are provided in Section 5.1.

2.3 Standard Penetration Test Borings

The soil borings were performed with a drill rig employing mud rotary drilling techniques and Standard Penetration Testing (SPT) in accordance with ASTM D1586. The SPTs were performed continuously to 10 feet and at 5-foot intervals thereafter. Soil samples were obtained at the depths where the SPTs were performed. The soil samples were classified in the field, placed in sealed containers, and returned to our laboratory for further evaluation.

After drilling to the sampling depth, the standard two-inch O.D. split-barrel sampler was seated by driving it 6 inches into the undisturbed soil. The sampler was then driven an additional 12 inches by blows of a 140-pound hammer falling 30 inches. The number of blows required to produce the next 12 inches of penetration were recorded as the penetration resistance (N-value). These values and the complete SPT boring logs are provided in Section 5.2.

Upon completion of the sampling, the boreholes were abandoned in accordance with Water Management District guidelines.

2.4 Soil Laboratory Tests

The soil samples recovered from the soil borings were returned to our laboratory, and examined to confirm the field descriptions. Representative samples were then selected for laboratory testing. The laboratory tests consisted of nine (9) percent soil fines passing the No. 200 sieve, nine (9) natural moisture content determinations, two (2) Atterberg Limits tests, and three (3) constant head hydraulic conductivity tests. These tests were performed in order to aid in classifying the soils and to further evaluate their engineering properties. The laboratory tests are provided in Section 5.3.

3.0 FINDINGS

3.1 Surface Conditions

Karen Roylos with GSE visited the site on September 18, 2023 to observe the site conditions and mark the boring locations. Mr. Jason Kite with Jason Kite, LLC was retained by GSE to clear lanes to allow access to the boring locations for drilling equipment.

The majority of the site is densely vegetated with trees, scattered saw palmettos, shrubs, vines and weedy groundcover. Portions of the site were densely vegetated and more difficult to traverse. To the south of the site is SW Marvin Burnett Road. State Road 47 is located east of the site. Undeveloped wooded land borders the site to the north and west.

The topography at the site is moderately sloping from northeast towards southwest. Regional topography can be characterized as gently to moderately sloping. The Lake City West USGS Topographic Map indicates the ground surface elevations at the site are near 155 to 165 feet¹ NAVD 88.

3.2 Subsurface Conditions

The locations of the auger and SPT borings are provided on Figure 2. Complete logs for the borings are provided in Sections 5.1 and 5.2. Descriptions for the soils encountered are accompanied by the Unified Soil Classification System symbol (SM, SP-SM, etc.) and are based on visual examination of the recovered soil samples and the laboratory tests performed. Stratification boundaries between the soil types should be considered approximate, as the actual transition between soil types may be gradual.

The auger borings located within the proposed parking lot and driveways encountered relatively similar soil conditions. Auger borings A-1 to A-3 encountered poorly graded sand, and sand with silt (SP, SP-SM) to the explored depths of 5 feet bls. Auger borings A-4 and A-5 initially encountered sand with silt (SP-SM) to depths of 1.5 to 3.5 feet bls. This was underlain by clayey to very clayey sand (SC, SC/CL) to the explored depths of 5 feet bls.

The auger borings located within the stormwater management facility encountered relatively consistent soil conditions. Auger boring P-1 encountered 6 feet of silty sand, and poorly graded sand (SM, SP) overlying clayey to very clayey sand, and clay with sand (SC, SC/CL, CL/CH) to the explored depth of 15 feet bls. Auger borings P-2 to P-4 initially encountered poorly graded sand, sand with silt, and silty sand (SP, SP-SM, SM) to depths of 2 to 5 feet bls, overlying silty clayey sand, and clayey to very clayey sand (SM-SC, SC, SC/CL) to depths of 7 to 10.5 feet bls. This was underlain by sand with silt (SP-SM) to depths of 12 to 13.5 feet bls, followed by clayrich soils (CL/CH) to the explored depth of 15 feet bls. Auger boring P-5 initially encountered 5.5 feet of clayey sand (SC) and 5 feet of sand with silt (SP-SM) overlying clay with sand (CL/CH) to a depth of 12.5 feet bls. This was underlain by sand with silt (SP-SM) to the explored depth of 15 feet bls.

-

¹ United States Geological Survey, Lake City West Quadrangle, 2021.

The SPT borings located within the proposed building footprint indicate the soils across these areas are relatively consistent. SPT boring B-1 initially encountered 3 feet of sand with silt (SP-SM), and 4.5 feet of sandy clay (CL) overlying sand with clay, and poorly graded sand (SP-SC, SP) to a depth of 12 feet bls. This was underlain by clay (CL/CH) to the explored depth of 20 feet bls. SPT borings B-2 to B-5 encountered poorly graded sand, sand with silt, sand with clay, silty sand, and silty clayey sand (SP, SP-SM, SP-SC, SM-SC) with some interbedded layers of clayey to very clayey sand (SC, SC/CL) to depths of 13.5 to 17.5 feet bls. This was underlain by clay-rich (CL, CL/CH) soils to the explored depths of 20 feet bls.

The sandy soils (SP, SP-SM, SP-SC) encountered are generally in a very loose to dense condition with N-values ranging from 2 to 45 blows per foot. The silty sand, silty clayey sand, and clayey to very clayey sands (SM, SM-SC, SC, SC/CL) encountered are generally in a very loose to dense condition with N-values ranging from 4 to 38 blows per foot. The sandy clay, clay with sand, and clay (CL/CH, CL) encountered are generally in a very soft to hard condition with N-values ranging from 3 to 33 blows per foot.

Weight-of-rod strength material was encountered in SPT boring B-2 at depth range from 13.5 to 14.5 feet bls. This isolated occurrence is likely related to depositional characteristics of the soil materials and transitions between material types.

The groundwater table was encountered in the auger and SPT borings at depths of 6.1 to 8.8 feet bls at the time of our investigation.

3.3 Review of Published Data

The majority of the site is mapped as three soil series by the Soil Conservation Service (SCS) Soil Survey for Columbia County². The following soil descriptions are from the Soil Survey.

Blanton fine sand, 0 to 5 percent slopes

Map Unit Setting

- National map unit symbol: 2w0q2
- *Elevation:* 30 to 200 feet
- Mean annual precipitation: 51 to 59 inches
- *Mean annual air temperature:* 64 to 72 degrees F
- Frost-free period: 258 to 310 days
- Farmland classification: Not prime farmland

Map Unit Composition

- Blanton and similar soils: 85 percent
- *Minor components:* 15 percent
- Estimates are based on observations, descriptions, and transects of the map unit.

² Soil Survey of Hamilton County, Florida. Soil Conservation Service, U.S. Department of Agriculture.

Description of Blanton

Setting

- Landform: Knolls on marine terraces, ridges on marine terraces
- Landform position (two-dimensional): Backslope
- Landform position (three-dimensional): Side slope, interfluve, riser
- *Down-slope shape:* Convex
- Across-slope shape: Linear
- Parent material: Sandy and loamy marine deposits

Typical profile

- A 0 to 7 inches: fine sand
- E 7 to 52 inches: fine sand
- Bt 52 to 80 inches: fine sandy loam

Properties and qualities

- Slope: 0 to 5 percent
- *Depth to restrictive feature:* More than 80 inches
- Drainage class: Moderately well drained
- Runoff class: Negligible
- Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.20 to 6.00 in/hr)
- *Depth to water table:* About 42 to 72 inches
- Frequency of flooding: None
- Frequency of ponding: None
- *Maximum salinity:* Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
- Sodium adsorption ratio, maximum: 4.0
- Available water supply, 0 to 60 inches: Low (about 3.6 inches)

Interpretive groups

- Land capability classification (irrigated): None specified
- Land capability classification (nonirrigated): 3s
- Hydrologic Soil Group: A
- Forage suitability group: Sandy soils on rises, knolls, and ridges of mesic uplands (G138XA121FL)
- Other vegetative classification: Sandy soils on rises, knolls, and ridges of mesic uplands (G138XA121FL)
- *Hydric soil rating:* No

Minor Components

Albany

- *Percent of map unit:* 6 percent
- Landform: Ridges on marine terraces
- Landform position (two-dimensional): Shoulder
- *Landform position (three-dimensional):* Interfluve, talf
- Down-slope shape: Convex
- *Across-slope shape:* Linear
- Other vegetative classification: Sandy soils on rises and knolls of mesic uplands (G138XA131FL), North Florida Flatwoods (R138XY004FL)
- Hydric soil rating: No

Troup

- Percent of map unit: 4 percent
- Landform: Ridges, knolls
- Landform position (two-dimensional): Summit
- Landform position (three-dimensional): Interfluve
- Down-slope shape: Convex
- *Across-slope shape:* Linear
- Other vegetative classification: Sandy soils on ridges and dunes of xeric uplands (G133AA111FL), Longleaf Pine-Turkey Oak Hills (R133AY002FL)
- Hydric soil rating: No

Chipley

- Percent of map unit: 3 percent
- *Landform:* Knolls on marine terraces, rises on marine terraces, flats on marine terraces
- Landform position (two-dimensional): Shoulder, footslope
- Landform position (three-dimensional): Interfluve
- Down-slope shape: Convex
- *Across-slope shape:* Linear
- Other vegetative classification: Sandy soils on rises and knolls of mesic uplands (G138XA131FL)
- *Hydric soil rating:* No

Alpin

- Percent of map unit: 2 percent
- *Landform:* Flatwoods on marine terraces, knolls on marine terraces, ridges on marine terraces
- *Landform position (two-dimensional):* Shoulder, backslope
- Landform position (three-dimensional): Interfluve
- *Down-slope shape:* Convex
- *Across-slope shape:* Linear
- Other vegetative classification: Sandy soils on ridges and dunes of xeric uplands (G138XA111FL), Sand Pine Scrub (R153AY001FL)
- *Hydric soil rating:* No

Ichetucknee fine sand, 5 to 8 percent slopes

Map Unit Setting

- National map unit symbol: vrt4
- Elevation: 330 to 660 feet
- *Mean annual precipitation:* 50 to 58 inches
- *Mean annual air temperature:* 64 to 72 degrees F
- Frost-free period: 258 to 288 days
- Farmland classification: Not prime farmland

Map Unit Composition

- *Ichetucknee and similar soils:* 80 percent
- *Minor components:* 20 percent
- Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Ichetucknee

Setting

- Landform: Hills on marine terraces, ridges on marine terraces
- Landform position (three-dimensional): Interfluve, side slope
- *Down-slope shape:* Convex
- *Across-slope shape:* Linear
- Parent material: Sandy and clayey marine deposits over limestone

Typical profile

- A 0 to 4 inches: fine sand
- E 4 to 7 inches: fine sand
- *Bg 7 to 75 inches:* clay
- 2R 75 to 79 inches: weathered bedrock

Properties and qualities

- *Slope:* 5 to 8 percent
- *Depth to restrictive feature:* 50 to 75 inches to lithic bedrock
- Drainage class: Somewhat poorly drained
- Runoff class: Negligible
- Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
- Depth to water table: About 18 to 36 inches
- Frequency of flooding: None
- Frequency of ponding: None
- *Maximum salinity:* Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
- Sodium adsorption ratio, maximum: 4.0
- Available water supply, 0 to 60 inches: Moderate (about 8.4 inches)

Interpretive groups

- Land capability classification (irrigated): None specified
- Land capability classification (nonirrigated): 6e
- *Hydrologic Soil Group:* D
- Forage suitability group: Loamy and clayey soils on rises, knolls, and ridges of mesic uplands (G138XA322FL)
- Other vegetative classification: Loamy and clayey soils on rises, knolls, and ridges of mesic uplands (G138XA322FL)
- *Hydric soil rating:* No

Minor Components

Goldsboro

- Percent of map unit: 10 percent
- Landform: Knolls on marine terraces, ridges on marine terraces
- Landform position (three-dimensional): Interfluve
- *Down-slope shape:* Convex
- *Across-slope shape:* Linear
- Other vegetative classification: Loamy and clayey soils on flats and rises of mesic lowlands (G138XA331FL)
- *Hydric soil rating:* No

Ocilla

- Percent of map unit: 10 percent
- Landform: Rises on marine terraces
- *Landform position (three-dimensional):* Interfluve
- *Down-slope shape:* Convex
- *Across-slope shape:* Linear
- Other vegetative classification: Sandy over loamy soils on rises and knolls of mesic uplands (G138XA231FL)
- *Hydric soil rating:* No

Pelham fine sand, 0 to 2 percent slopes

Map Unit Setting

- *National map unit symbol:* 2tg56
- Elevation: 0 to 190 feet
- *Mean annual precipitation:* 48 to 63 inches
- *Mean annual air temperature:* 57 to 79 degrees F
- Frost-free period: 251 to 293 days
- Farmland classification: Not prime farmland

Map Unit Composition

- Pelham and similar soils: 75 percent
- *Minor components:* 25 percent
- Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Pelham

Setting

- *Landform:* Flatwoods
- Landform position (three-dimensional): Talf
- *Down-slope shape:* Linear
- *Across-slope shape:* Linear
- Parent material: Sandy and loamy marine deposits

Typical profile

- A 0 to 6 inches: fine sand
- Eg 6 to 26 inches: fine sand
- Btg1 26 to 42 inches: sandy clay loam
- Btg2 42 to 83 inches: sandy clay loam

Properties and qualities

- Slope: 0 to 2 percent
- Depth to restrictive feature: More than 80 inches
- Drainage class: Poorly drained
- Runoff class: High
- Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.20 to 5.95 in/hr)
- Depth to water table: About 6 to 12 inches
- Frequency of flooding: None
- Frequency of ponding: None
- *Maximum salinity:* Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
- Sodium adsorption ratio, maximum: 4.0
- Available water supply, 0 to 60 inches: Moderate (about 7.0 inches)

Interpretive groups

- Land capability classification (irrigated): None specified
- Land capability classification (nonirrigated): 3w
- *Hydrologic Soil Group:* B/D
- Ecological site: F153AY060NC Wet Loamy Flats and Depressions
- Forage suitability group: Sandy over loamy soils on flats of hydric or mesic lowlands (G153AA241FL)
- Other vegetative classification: Sandy over loamy soils on flats of hydric or mesic lowlands (G153AA241FL)
- *Hydric soil rating:* No

Minor Components

Unnamed

- Percent of map unit: 13 percent
- *Landform:* Flatwoods
- Landform position (three-dimensional): Talf
- *Down-slope shape:* Linear
- *Across-slope shape:* Linear
- Other vegetative classification: Sandy over loamy soils on flats of hydric or mesic lowlands (G153AA241FL)
- Hydric soil rating: Yes

Albany

- Percent of map unit: 6 percent
- Landform: Flatwoods
- Landform position (three-dimensional): Talf
- *Microfeatures of landform position:* Rises
- *Down-slope shape:* Convex
- Across-slope shape: Convex
- Ecological site: F153AY040NC Moist Loamy Rises and Flats
- Other vegetative classification: Sandy soils on rises and knolls of mesic uplands (G153AA131FL)
- *Hydric soil rating:* No

Meggett

- Percent of map unit: 3 percent
- Landform: Flatwoods
- Landform position (three-dimensional): Talf
- *Down-slope shape:* Linear
- Across-slope shape: Linear
- Ecological site: F153AY090NC Flooded Mineral Soil Floodplains and Terraces
- Other vegetative classification: Loamy and clayey soils on flats of hydric or mesic lowlands (G153AA341FL)
- *Hydric soil rating:* Yes

Surrency

- Percent of map unit: 3 percent
- Landform: Drainageways, depressions
- Landform position (three-dimensional): Dip
- *Down-slope shape:* Linear, concave
- *Across-slope shape:* Convex, concave
- Ecological site: F153AY060NC Wet Loamy Flats and Depressions
- Other vegetative classification: Sandy over loamy soils on stream terraces, flood plains, or in depressions (G153AA245FL)
- *Hydric soil rating:* Yes

3.4 Laboratory Soil Analysis

Selected soil samples recovered from the soil borings were analyzed for the percent soil fines passing the No. 200 sieve, natural moisture content, Atterberg Limits, and hydraulic conductivity. Samples selected for laboratory testing were collected at depths ranging from near-surface to 15 feet bls. These tests were performed to confirm visual soil classification and evaluate their engineering properties. The complete laboratory report is provided in Section 5.3.

The laboratory tests indicate the tested soils consist sand with silt, silty sand, silty sand with clay, sand with clay, clayey sand, very clayey sand, and sandy clay. The tested sand with silt (SP-SM) contains approximately 11 percent soil fines passing the No. 200 sieve with a natural moisture content of about 8.7 percent. The tested silty sand, and silty sand with clay (SM, SM-SC) contains approximately 14 to 27 percent soil fines passing the No. 200 sieve with natural moisture contents of about 7.8 to 18 percent. The tested sand with clay (SP-SC) contains approximately 11 percent soil fines passing the No. 200 sieve with a natural moisture content of about 17 percent. The tested clayey sand (SC) contains approximately 30 percent soil fines passing the No. 200 sieve with a natural moisture content of about 13 percent. The tested very clayey sand (SC/CL) contains approximately 34 percent soil fines passing the No. 200 sieve with a natural moisture content of about 18 percent. The tested sandy clay (CL) contains approximately 56 to 62 percent soil fines passing the No. 200 sieve with natural moisture contents of about 17 to 23 percent.

Atterberg Limits tests indicate the tested sandy clay (CL) has Liquid Limit (LL) values of 35 and 41, Plastic Limit (PL) values of 15 to 18, and Plasticity Index (PI) values of 17 and 26. These values correspond to materials with low potential (LL < 50) to marginal potential (PI \leq 35) for expansive behavior³.

The constant head hydraulic conductivity test results indicate the near-surface silty sand (SM) has hydraulic conductivity values of 0.8 to 1.1 feet per day. The tested clayey sand (SC) has no flow. Tests were not conducted on the deeper very clayey sand due to the limitations of the test method on soils having moderate to high fines content, but these soils are expected to have permeability values at least one order of magnitude lower than the sandy soils.

³ U.S. Department of the Army USA, 1983, Foundations in Expansive Soils, TM 5-818-7, p. 4-1.

4.0 EVALUATION AND RECOMMENDATIONS

4.1 General

The following recommendations are made based upon our understanding of the proposed construction, a review of the attached soil borings and laboratory test data, and experience with similar projects and subsurface conditions. If plans or the location of proposed construction changes from those discussed previously, GSE requests the opportunity to review and possibly amend our recommendations with respect to those changes.

The final design of a foundation system is dependent upon adequate integration of geotechnical and structural engineering considerations. Consequently, GSE must review the final foundation design in order to evaluate the effectiveness and applicability of our initial analyses, and to determine if additional recommendations may be warranted. Without such a review, the recommendations presented herein could be misinterpreted or misapplied resulting in potentially unacceptable performance of the foundation system.

The performance of site improvements may be sensitive to their post-construction relationship to site groundwater levels, seepage zones, or soil/rock characteristics exposed at final site grades. GSE recommends that use of boring information for final design of all site improvements be predicated on proper horizontal and vertical control of borings.

In this section of the report, we present our geotechnical parameters and recommendations to assist with building foundation, stormwater management, and pavement designs as well as our general site preparation guidelines.

4.2 Groundwater

The groundwater table was encountered in the borings at depths of 6.1 to 8.8 feet bls at the time of our exploration. The Soil Survey indicates the groundwater table is typically at a depth of near-surface to 6 feet bls. We anticipate the seasonal high groundwater table will be near depths of 1 to 3.5 feet bls. Estimates for the seasonal high groundwater table are shown on the individual boring logs.

4.3 **Building Foundations**

The SPT borings located within the proposed building footprint indicate the soils across these areas are relatively consistent. SPT boring B-1 initially encountered 3 feet of sand with silt (SP-SM), and 4.5 feet of sandy clay (CL) overlying sand with clay, and poorly graded sand (SP-SC, SP) to a depth of 12 feet bls. This was underlain by clay (CL/CH) to the explored depth of 20 feet bls. SPT borings B-2 to B-5 encountered poorly graded sand, sand with silt, sand with clay, silty sand, and silty clayey sand (SP, SP-SM, SP-SC, SM-SC) with some interbedded layers of clayey to very clayey sand (SC, SC/CL) to depths of 13.5 to 17.5 feet bls. This was underlain by clay-rich (CL, CL/CH) soils to the explored depths of 20 feet bls.

Based upon the soil conditions encountered and our limited understanding of the structural loads and site grading, we recommend the building be supported by conventional, shallow strip and/or spread foundations. We recommend the shallow foundations be designed for a maximum allowable gross bearing pressure of 2,500 psf. The gross bearing pressure is defined as the soil contact pressure that can be imposed from the maximum structural loads, weight of the concrete foundations, and weight of the soil above the foundations. The foundations should be designed based upon the maximum load that could be imposed by all loading conditions.

The foundations should be embedded a minimum of 18 inches below the lowest adjacent grade. Interior foundations or thickened sections should be embedded a minimum of 12 inches. The foundations should have minimum widths of 18 inches for strip footings, and 24 inches for columns, even though the maximum soil bearing pressure may not be fully developed.

Due to the mostly sandy nature of the majority of the near-surface soils, we expect settlement to be mostly elastic in nature. The majority of the settlement will occur on application of the loads, during and immediately following construction. Using the recommended maximum bearing pressure, the assumed maximum structural loads, and the field and laboratory test data which we have correlated into the strength and compressibility characteristics of the subsurface soils, we estimate the total settlements of the structure to be 1 inch or less, with approximately half of it occurring upon load application (during construction).

Differential settlement results from differences in applied bearing pressures and the variations in the compressibility characteristics of the subsurface soils. For the building pad prepared as recommended, we anticipate differential settlement of less than 1/2 inch.

Post-construction settlement of the structures will be influenced by several interrelated factors, such as (1) subsurface stratification and strength/compressibility characteristics of the bearing soils; (2) footing size, bearing level, applied loads, and resulting bearing pressures beneath the foundation; (3) site preparation and earthwork construction techniques used by the contractor, and (4) external factors, including but not limited to vibration from off-site sources and groundwater fluctuations beyond those normally anticipated for the naturally-occurring site and soil conditions which are present.

Our settlement estimates for the structure are based upon our limited understanding of the structural loads and site grading and the use of successful adherence to the site preparation recommendations presented later in this report. Any deviation from our project understanding and/or our site preparation recommendations could result in an increase in the estimated post-construction settlement of the structure.

4.4 Flexible Pavement

Overall soil conditions encountered by our borings at this site are suitable for supporting conventional limerock base and asphalt wearing surface pavements. We have not been provided the anticipated traffic loading conditions; therefore, the following pavement component recommendations should be used only as guidelines. The below recommendations are intended to be minimums. Increasing base course and asphalt thicknesses would increase the design life of the pavement.

The seasonal high groundwater table is estimated to be approximately 12 inches to about 3.5 feet beneath existing grade across the site. We recommend a minimum of either 12 to 24 inches of separation (depending upon the pavement section design) be present between the bottom of the base course and the estimated seasonal high groundwater table. If this separation cannot be achieved by site grading, GSE recommends underdrains be used beneath the base course.

4.4.1 Stabilized Subgrade

If a crushed limerock or recycled concrete base is used, we recommend a stabilized subgrade be located beneath the base. The stabilized subgrade should have a minimum Limerock Bearing Ratio (LBR) of 40, with minimum thicknesses of 6 inches for automobile parking areas and 12 inches for driveways.

The stabilized subgrade can be imported material or a mixture of imported and on-site material. If a mix is proposed, a mix design should be performed to determine the optimum mix proportions. The stabilized subgrade should be compacted to a minimum of 98 percent of the Modified Proctor maximum dry density (ASTM D1557) for soils with less than 15 percent fines content. Soils with 15 percent or greater fines content should be compacted to 100 percent of the Standard Proctor maximum dry density (ASTM D698).

4.4.2 Base Course

The base course can consist of either crushed limerock, soil cement, or recycled concrete. If you should use a soil cement base course, a stabilized subgrade is not required.

Limerock should have an LBR of at least 100, be obtained from a FDOT approved source and meet FDOT gradation requirements. The base course thickness should be a minimum of 6 inches in automobile parking areas, and 8 inches in driveway areas. The base course should be compacted to at least 98 percent of the Modified Proctor maximum dry density (ASTM D1557). We recommend a minimum 24 inches separation between the bottom of the limerock base course and the estimated seasonal high-water table. If site grading does not allow for this separation, we recommend underdrains be considered.

Soil cement can consist of an imported material or a blend of the on-site soils and cement. A mix design should be performed to determine the optimum cement content. We recommend the soil cement have a minimum 28-day compressive strength of 500 psi. Soil cement can be blended offsite (in a pug mill) or on site. Soil cement pills should be cast from each day's production to verify the recommended compressive strength has been achieved at 28 days. We recommend the soil cement base course be a minimum of 8 inches thick throughout the project. We recommend a minimum 18 inches separation between the bottom of the soil cement base course and the estimated seasonal high-water table. If site grading does not allow for this separation, we recommend underdrains be considered.

Recycled concrete should have an LBR of at least 150, be obtained from a FDOT approved source and meet FDOT gradation requirements. The base course thickness should be a minimum of 8 inches. The base course should be compacted to at least 98 percent of the Modified Proctor maximum dry density (ASTM D1557). We recommend a minimum 12 inches separation between the bottom of the recycled concrete base course and the estimated seasonal high-water table. If site grading does not allow for this separation, we recommend underdrains be considered.

4.4.3 Wearing Surface

The asphalt-wearing surface should consist of an FDOT Type SP Hot Mix Asphalt mixture. For automobile parking areas, the thickness should be a minimum of 1.5 inches. For driveway areas, the thickness should be a minimum of 2 inches. The asphalt-wearing surface should consist of an SP-12.5 mix. The asphalt should be compacted to at least 95 percent of the mix design density.

The constructability of differing asphalt thicknesses may be difficult, and having a uniform 2-inch thick asphalt wearing surface may be more practical.

4.5 Rigid Pavement

Concrete pavement is a rigid pavement that results in smaller load transfers to the subgrade soils than flexible pavement. For concrete pavement subgrade, we recommend using the existing surficial sands or recommended clean sand (SP) fill, compacted to at least 98 percent of the Modified Proctor maximum dry density without additional stabilization with the following stipulations:

- 1. Subgrade soils must be compacted to at least 98 percent of Modified Proctor maximum dry density to a depth of at least 2 feet prior to placement of concrete.
- 2. The surface of the subgrade soils must be smooth and any disturbances or wheel rutting corrected prior to placement of the concrete.
- 3. The subgrade soils must be moistened prior to placement of concrete.
- 4. Concrete pavement thickness should be uniform throughout, with the exception of thickened edges (curb or footing).
- 5. The bottom of the pavement should be separated from the estimated seasonal high groundwater level by at least 18 inches.
- 6. Limerock or any other impermeable base is not suitable unless it meets the minimum recommended permeability of 10 ft/day.
- 7. The upper 12 inches of subgrade underlying the base course must also be "free-draining" and water that enters the base and subgrade must be allowed to seep out by gravity or if this is not possible, underdrains must be incorporated into the subgrade. A "bathtub" condition within the base/subgrade must be avoided.

Our recommendations for slab thickness for both light-duty and heavy-duty concrete pavements is based on a.) subgrade soils are compacted to 98 percent of the Modified Proctor maximum dry density, b.) modulus of subgrade reaction (k) of 200 pounds per cubic inch, c.) a 20-year design life, and d.) previously stated design parameters. For an anticipated light-duty traffic group, a minimum pavement thickness of 5.5 inches is recommended, using Table 2.4 from the ACI 330 Guide for Design and Construction of Concrete Parking Lots, ACI 330R-01. For an anticipated heavy-duty traffic group, a minimum pavement thickness of 8 inches is recommended, using Table 3.4 from the FDOT *Rigid Pavement Design Manual*, January 2019.

We recommend using concrete with a minimum 28-day compressive strength of 4,000 pounds per square inch and a minimum 28-day flexural strength (modulus of rupture) of at least 600 pounds per square inch based on the third point loading of concrete beam test samples. Maximum control joint spacing of 12.5 by 12.5 feet is suggested for light-duty concrete pavements. Maximum control joint spacing of 15 by 15 feet is suggested for heavy-duty concrete pavements. Layout of sawcut control joints should form square panels, and the depth of sawcut joint should be at least 1/4 of the concrete slab thickness. The joints should be sawed within six hours of concrete placement or as soon as the concrete has developed sufficient strength to support workers and equipment.

For further details on concrete pavement construction, refer to "Guide to Jointing Non-reinforced Concrete Pavements" published by the Florida Concrete and Products Associates, Inc. and "Building Quality Concrete Parking Areas", published by the Portland Cement Association.

4.6 Site Preparation

The soils at this site should be suitable for supporting the proposed construction using normal, good practice site preparation procedures. The following recommendations are our general guidelines for site preparation.

4.6.1 Stripping

Strip the construction limits and 10 feet beyond the perimeter of all grass, roots, topsoil, and other deleterious materials. You should expect to strip to depths of 12 or more inches. Deeper stripping will likely be necessary due to major root systems present at the site.

4.6.2 Dewatering

Temporary dewatering may be necessary for this project. If needed, we anticipate dewatering can be accomplished with sumps placed near the construction area, or with underdrains connected to a vacuum pump.

In any case, the site should always be graded to promote runoff and limit the amount of ponding. Localized ponding of stormwater is expected without proper grading during construction, and could render previously acceptable surfaces unacceptable.

4.6.3 **Proof-Rolling**

Proof-roll the subgrade with heavy rubber-tired equipment, such as a loaded front-end loader or dump truck, to identify any loose or soft zones not found by the soil borings. The proof-rolling should be monitored by a geotechnical engineer or qualified technician. Undercut or otherwise treat these zones as recommended by the geotechnical engineer in this report.

4.6.4 **Proof Compaction**

Compact the subgrade to a density of at least 95 percent of the Modified Proctor maximum dry density (ASTM D1557). The specified compaction should be obtained to a depth of 1 foot below the foundation bottoms and the existing grade prior to placing fill. Vibratory roller equipment should not be used within approximately 100 feet of existing structures. Lighter "walk-behind" compaction equipment may be used to achieve the degree of compaction.

Should clayey sand be encountered at the bearing surface, this material should be probed and visually confirmed to be unyielding in the upper 12 inches in lieu of density testing. If the foundation excavations penetrate the clayey sand, the excavation should be performed in a manner that reduces soil disturbance. Clayey sand soils (with fines content in excess of 15 percent) that are removed and replaced or appreciably disturbed need to be re-compacted to 98 percent of the Standard Proctor maximum dry density (ASTM D698).

4.6.5 Fill Placement

Imported fill placed to raise the site grades should consist of clean sand having less than 10 percent passing the No. 200 sieve. On-site soils meeting the requirements of Section 4.9 may also be used as structural fill. The fill should be placed in maximum 12-inch loose lifts that are compacted to at least 95 percent of the Modified Proctor maximum dry density (ASTM D1557). If lighter "walkbehind" compaction equipment is used, this may require lifts of 4 inches or less to achieve the required degree of compaction.

4.7 Quality Control and Construction Materials Testing

It should be noted that the geotechnical engineering design does not end with the advertisement of the construction documents. As the geotechnical engineer of record, GSE is the most qualified to perform the construction materials testing that will be required for this project. The benefits of having the geotechnical engineer of record also perform the construction materials testing are numerous. If GSE continues to be involved with the project through construction, we will be able to constantly re-evaluate and possibly alter our geotechnical recommendations in a timely and cost effective manner once final design and construction techniques are developed. This often results in cost savings for the project.

We recommend performing compaction testing beneath the concrete floor slab and the building foundations. We recommend one test be performed every 50 linear feet of continuous footing and every other column footing, per foot depth of fill or native material. We recommend a compaction test be performed for each 2,500 square feet of floor area or 10,000 square feet of pavement area per foot of fill or native material, or a minimum of three tests each, whichever is greater. Test all footing excavations to a depth of 12 inches at the frequencies stated above.

4.8 Stormwater Management

The auger borings located within the stormwater management facility encountered relatively consistent soil conditions. Auger boring P-1 encountered 6 feet of silty sand, and poorly graded sand (SM, SP) overlying clayey to very clayey sand, and clay with sand (SC, SC/CL, CL/CH) to the explored depth of 15 feet bls. Auger borings P-2 to P-4 initially encountered poorly graded sand, sand with silt, and silty sand (SP, SP-SM, SM) to depths of 2 to 5 feet bls, overlying silty clayey sand, and clayey to very clayey sand (SM-SC, SC, SC/CL) to depths of 7 to 10.5 feet bls. This was underlain by sand with silt (SP-SM) to depths of 12 to 13.5 feet bls, followed by clayrich soils (CL/CH) to the explored depth of 15 feet bls. Auger boring P-5 initially encountered 5.5 feet of clayey sand (SC) and 5 feet of sand with silt (SP-SM) overlying clay with sand (CL/CH) to a depth of 12.5 feet bls. This was underlain by sand with silt (SP-SM) to the explored depth of 15 feet bls.

The water table was encountered in the auger borings at depths of 7.5 to 8.8 feet bls at the time of our exploration. We anticipate the seasonal high groundwater table to be at depths of 1 to 2.5 feet bls.

The laboratory permeability tests indicate the surficial layers of silty sand (SM) has hydraulic conductivity values of 0.8 to 1.1 feet per day, and clayey sand (SC) has no flow. The deeper very clayey sand encountered below the surficial sandy soils is friable and will have permeability values at least one order of magnitude lower than the sandy soils. The underlying dense soils and clayrich soils are expected to be confining soils.

Mr. Cole Menhennett with CHW confirmed the proposed stormwater management facility as a dry pond via email. We understand that the current design will consider underdrains. We understand that imported clean sand will be used for the backfill for the underdrains. This revision includes soil parameters considering and underdrain design with clean sand backfill.

Based upon our findings and test results, our recommended soil parameters for the stormwater management design in the explored areas are presented below. The recommended parameters consider the results of the permeability tests, wash 200 determinations, and our experience with these types of soils. The parameters below do not consider a factor of safety.

Proposed Stormwater Management Facility

- 1. Base elevation of effective or mobilized aquifer (average depth of confining layer) equal to 8 feet bls.
- 2. Unsaturated vertical infiltration rate of 10 foot per day.
- 3. Horizontal hydraulic conductivity equal to 10 feet per day.
- 4. Specific yield (fillable porosity) of 20 percent.
- 5. Average seasonal high groundwater table depth equal to 2 feet bls.
- 6. Average seasonal low groundwater table depth equal to 6 feet bls.

In areas where clay-rich soils are present at the basin bottom, we recommend these soils be undercut a minimum of 2 feet and backfilled with the on-site sands and sands with silt (SP, SP-SM) having a maximum of 12 percent soil fines passing the No. 200 sieve. This fill should also be used above the bottom of the underdrains. The intent of this undercutting and replacement is to provide a more uniform sand "blanket" at the basin bottom that allows the migration of water to the underdrains. This sand blanket will also reduce the potential for clay-fines leaching out of the soils when water is present in the basin that can result in a thin layer of confining type material on the basin bottom that can reduce the effectiveness of the basin.

4.9 Fill Suitability

The soils encountered at this site within the explored depths range from sands (SP) to clays (CL/CH). A discussion of the suitability for reuse as structural fill for each soil classification according to the Unified Soil Classification System (USCS) designation is provided below.

SP, SP/SM – Sands (SP) and sand with silt (SP/SM) have less than 5 percent and 12 percent soil fines passing the No. 200 sieve, respectively, and are typically well draining soils that are suitable for reuse as structural fill. The sands with silt may require moisture conditioning (drying) to make the material more workable. These soils will require stockpiling and drying before they are reused if they are excavated from below the water table.

SM – Silty sands (SM) can have between 12 percent and 50 percent soil fines passing the No. 200 sieve. Silty sands are typically non-plastic or have low plasticity, and can be reused as structural fill with precautions. Silty sands can be moisture sensitive and difficult to work and compact and can rut if the moisture content is near or above the optimum moisture content. We recommend these soils be moisture conditioned (dried) so that the moisture content during use is at or below the optimum moisture content. Aerating and exposure to the sun is typically the most effective methods of drying these soils. It may not be practical to reuse these materials during the wet season, as frequent rain showers may not allow these soils to dry to a workable moisture content. Suitable silty sands are limited to soil having less than 30 percent soil fines passing the No. 200 sieve. Silty sands with more than 30 percent soil fines are especially moisture sensitive, and are not recommended for reuse as structural fill. These soils will behave more as sandy silt, and for this reason, very silty sands having more than 30 percent soil fines passing the No. 200 sieve have been assigned a dual classification of SM/ML. Silty sand soils that are excavated from below the water table are not recommended for reuse as structural fill due to the amount of time that will be required to dry these soils to a workable condition.

SC – Clayey sand (SC) soils can have between 12 percent and 50 percent soil fines passing the No. 200 sieve. Clayey sands can have a high range of plasticity, varying from a PI of 7 or greater and plotting above the A-line to highly plastic. Friable clayey sands are typically suitable for use as structural fill with precautions. Clayey sands will be moisture sensitive and difficult to work and compact and can rut during placement if the moisture content is near or above the natural moisture content. We recommend these soils be moisture conditioned (dried) so that the moisture content during use is at or below the optimum moisture content. Aerating and exposure to the sun is typically the most effective methods of drying these soils. It may not be practical to reuse these materials during the wet season, as frequent rain showers may not allow these soils to dry to a workable moisture content. Suitable clayey sands are limited to soil having less than 30 percent soil fines passing the No. 200 sieve. Clayey sands with more than 30 percent soil fines passing the No. 200 sieve are especially moisture sensitive and are typically highly plastic, and are not recommended for reuse as structural fill. These soils will behave more as sandy clay, and for this reason, very clayey sands having more than 30 percent soil fines passing the No. 200 sieve have been assigned a dual classification of SC/CH or SC/CL. Clayey sand soils that are excavated from below the water table are not recommended for reuse as structural fill due to the amount of time that will be required to dry these soils to a workable condition.

ML, MH, CL, CH – Silts and clays are not suitable materials for reuse as structural fill.

When using on-site soils as fill materials, we recommend the silty and clayey sand soils (SM, SC) be used in the lower depths of the fill. Sand and sand with silt (SP, SP-SM) should be used in the upper portions of the fill. We recommend a minimum of 2 feet of sand (SP, SP-SM) cover the silty and clayey sand fill materials to reduce the potential for soggy surface conditions due to the low permeability characteristics of the silty and clayey sand materials.

4.10 Surface Water Control and Landscaping

Roof gutters should be considered to divert runoff away from the building. The gutter downspouts should discharge a minimum of 10 feet from the structure to reduce the amount of water collecting around the foundations. Where possible, the gutter downspouts should discharge directly into the storm sewer system or onto the asphalt paved areas in order to reduce the amount of water collecting around the foundations. Grading of the site should be such that water is diverted away from the building on all sides to reduce the potential for erosion and water infiltration along the foundation.

With respect to landscaping, it is recommended that any trees and large "tree-like" shrubbery with potential for developing large root systems be planted a minimum distance of half their mature height, and preferably their expected final height, away from the structure. The purpose of this is to reduce the potential for foundation or slab movements from the growth of root systems as the landscaping matures.

5.0 FIELD DATA

5.1 Auger Boring Logs



GSE Engineering 5590 SW 64th St Gainesville, FL 32608 Telephone: 3523773233

CLIENT Concept Development, Inc. PROJECT NAME Dollar General - Lake City SW Marvin Burnett PROJECT NUMBER 16251 PROJECT LOCATION Lake City, Columbia County, Florida DATE PERFORMED 9/20/2023 BORING NUMBER A-1 DATE PERFORMED 9/20/2023 BORING NUMBER A-2 DRILLING CONTRACTOR Whitaker Drilling, Inc. DRILLING CONTRACTOR Whitaker Drilling, Inc. GROUND WATER LEVELS: LOGGED BY WDI GROUND WATER LEVELS: LOGGED BY WDI AT TIME OF DRILLING NE ▼ AT TIME OF DRILLING NE CHECKED BY AXL CHECKED BY AXL ∇ ESTIMATED SEASONAL HIGH <u>3.5 ft</u> NOTES NOTES SAMPLE TYPE NUMBER SAMPLE TYPE NUMBER (kg/cm²) (kg/cm² DEPTH MATERIAL DESCRIPTION MATERIAL DESCRIPTION AB 2 PORTRAIT CPT - GINT STD US.GDT - 10/11/23 09:53 - P:\GENERAL\PROJECTS\16251 DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT\16251 BORINGS\16251 BORINGS.GPJ CPT (CPT ((SP) Brown and gray SAND (SP-SM) Dark brown and gray SAND with silt %PASS-200 = 11 MC = 8.72.0 (SP) Pale gray and brown SAND 2.5 (SP) Pale brown SAND 3 ∇ ∇ 4 5 5.0 5.0 Bottom of borehole at 5.0 feet. Bottom of borehole at 5.0 feet. 251

(Continued Next Page)



GSE Engineering 5590 SW 64th St Gainesville, FL 32608 Telephone: 3523773233

CLIENT Concept Development, Inc. PROJECT NAME Dollar General - Lake City SW Marvin Burnett PROJECT NUMBER 16251 PROJECT LOCATION Lake City, Columbia County, Florida DATE PERFORMED 9/20/2023 BORING NUMBER A-3 DATE PERFORMED 9/20/2023 BORING NUMBER A-4 DRILLING CONTRACTOR Whitaker Drilling, Inc. DRILLING CONTRACTOR Whitaker Drilling, Inc. GROUND WATER LEVELS: LOGGED BY WDI GROUND WATER LEVELS: LOGGED BY WDI ▼ AT TIME OF DRILLING NE ▼ AT TIME OF DRILLING NE CHECKED BY AXL CHECKED BY AXL ∇ ESTIMATED SEASONAL HIGH <u>3.0 ft</u> NOTES NOTES SAMPLE TYPE NUMBER SAMPLE TYPE NUMBER · (kg/cm^²· (kg/cm² DEPTH MATERIAL DESCRIPTION MATERIAL DESCRIPTION AB 2 PORTRAIT CPT - GINT STD US.GDT - 10/11/23 09:53 - P:\GENERAL\PROJECTS\16251 DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT\16251 BORINGS\16251 BORINGS.GPJ CPT (CPT (SP-SM) Brown and gray SAND with silt (SP-SM) Brown and gray SAND with silt (SC/CL) Brown, gray, and orange very clayey SAND (SP) Pale gray and brown SAND %PASS-200 = 34 MC = 18 ∇ 5.0 5.0 Bottom of borehole at 5.0 feet. Bottom of borehole at 5.0 feet. 252

(Continued Next Page)



GSE Engineering 5590 SW 64th St Gainesville, FL 32608 Telephone: 3523773233

CLIENT Concept Development, Inc.

PROJECT NAME Dollar General - Lake City SW Marvin Burnett

PROJECT LOCATION Lake City, Columbia County, Florida

PROJECT NUMBER 16251

DATE PERFORMED 9/20/2023 BORING NUMBER A-5

DRILLING CONTRACTOR Whitaker Drilling, Inc.

GROUND WATER LEVELS: LOGGED BY WDI

AT TIME OF DRILLING NE CHECKED BY AXL



GSE Engineering 5590 SW 64th St

Gainesville, FL 32608 Telephone: 3523773233 CLIENT Concept Development, Inc. PROJECT NAME Dollar General - Lake City SW Marvin Burnett PROJECT NUMBER 16251 PROJECT LOCATION Lake City, Columbia County, Florida DATE PERFORMED 9/20/2023 BORING NUMBER P-1 DATE PERFORMED 9/20/2023 BORING NUMBER P-2 DRILLING CONTRACTOR Whitaker Drilling, Inc. DRILLING CONTRACTOR Whitaker Drilling, Inc. **GROUND WATER LEVELS:** GROUND WATER LEVELS: LOGGED BY WDI LOGGED BY WDI AT TIME OF DRILLING 7.5 ft AT TIME OF DRILLING 7.8 ft CHECKED BY AXL CHECKED BY AXL ∇ ESTIMATED SEASONAL HIGH <u>2.5 ft</u> ∇ ESTIMATED SEASONAL HIGH <u>2.5 ft</u> NOTES NOTES SAMPLE TYPE NUMBER SAMPLE TYPE NUMBER (kg/cm²) (kg/cm² (ft) GRAPHIC LOG DEPTH MATERIAL DESCRIPTION MATERIAL DESCRIPTION AB 2 PORTRAIT CPT - GINT STD US.GDT - 10/11/23 09:53 - P:\GENERAL\PROJECTS\16251 DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETTY16251 BORINGS\16251 BORINGS.GPJ CPT (CPT (SM) Gray and brown silty SAND (SP) Brown and gray SAND %PASS-200 = 14 2.5 2.5 ∇ MC = 7.8 $k_h = 1.1 \text{ ft/day}$ AP 3.5 (SC) Brown and orange clayey SAND 4.0 (SP) Brown and gray SAND with trace of clay 5.0 5.0 6.0 (SC/CL) Pale gray and brown very clayey 6.5 (SM-SC) Gray, brown, and orange silty clayey SAND Ţ Ţ 10.0 10.5 (SP-SM) Pale gray and brown SAND with 12.0 (CL/CH) Gray CLAY with sand 12.5 13.5 13.5 (CL/CH) Pale gray and brown CLAY with (SC) Brown and orange clayey SAND 15.0 15.0 15.0 15.0 Bottom of borehole at 15.0 feet. Bottom of borehole at 15.0 feet.

254



GSE Engineering 5590 SW 64th St Gainesville, FL 32608 Telephone: 3523773233

Telephone: 3523773233 CLIENT Concept Development, Inc. PROJECT NAME Dollar General - Lake City SW Marvin Burnett PROJECT NUMBER 16251 PROJECT LOCATION Lake City, Columbia County, Florida DATE PERFORMED 9/20/2023 BORING NUMBER P-3 DATE PERFORMED 9/20/2023 BORING NUMBER P-4 DRILLING CONTRACTOR Whitaker Drilling, Inc. DRILLING CONTRACTOR Whitaker Drilling, Inc. **GROUND WATER LEVELS: GROUND WATER LEVELS:** LOGGED BY WDI LOGGED BY WDI AT TIME OF DRILLING 7.5 ft CHECKED BY AXL ▼ AT TIME OF DRILLING 8.8 ft CHECKED BY AXL abla ESTIMATED SEASONAL HIGH <u>2.5 ft</u> NOTES NOTES SAMPLE TYPE NUMBER SAMPLE TYPE NUMBER (kg/cm² (kg/cm² DEPTH MATERIAL DESCRIPTION MATERIAL DESCRIPTION AB 2 PORTRAIT CPT - GINT STD US.GDT - 10/11/23 09:53 - P.(GENERAL/PROJECTS)16251 DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT116251 BORINGS\16251 BORINGS GPJ CPT (CPT 0.0 (SM) Brown silty SAND (SP-SM) Brown and gray SAND with silt AP %PASS-200 = 15 $\sqrt{\frac{1}{2}}$ MC = 9.7 $k_h = 0.8 \text{ ft/day}$ (SC) Brown, gray, and orange clayey 2 2.5 ∇ 5.0 5.0 (SC) Brown, gray, and orange clayey 7.0 (SP-SM) Pale gray and pale brown 7.5 SAND with silt (SC/CL) Brown, gray, and orange very clayey SAND AU 3 Ţ 10.0 10.0 (SP-SM) Pale brown and pale gray SAND with silt 12.0 (CL/CH) Green CLAY with sand 12.5 12.5 (CL/CH) Brown and gray CLAY with sand 15.0 15.0 15.0 15.0 Bottom of borehole at 15.0 feet. Bottom of borehole at 15.0 feet. 255

(Continued Next Page)



GSE Engineering 5590 SW 64th St Gainesville, FL 32608 Telephone: 3523773233

CLIENT Concept Development, Inc.

GROUND WATER LEVELS:

PROJECT NAME Dollar General - Lake City SW Marvin Burnett

PROJECT LOCATION Lake City, Columbia County, Florida

PROJECT NUMBER 16251

DATE PERFORMED 9/20/2023 BORING NUMBER P-5

DRILLING CONTRACTOR Whitaker Drilling, Inc.

LOGGED BY WDI

▼ AT TIME OF DRILLING 8.8 ft

CHECKED BY AXL

abla ESTIMATED SEASONAL HIGH <u>1.0 ft</u>

NOTES ___

		NO					_
30RINGS.GPJ	0.0 (#)	GRAPHIC LOG	SAMPLE TYPE NUMBER	CPT (kg/cm²)		MATERIAL DESCRIPTION	
TY SW MARVIN BURNETT/16251 BORINGS/16251 I	2.5 		AP 1		Ā	(SC) Brown and gray clayey SAND %PASS-200 = 30 MC = 13 k _h = NF	
LLAR GENERAL - LAKE	5.0 7.5 		AU 2		Ţ	(SP-SM) Brown, gray, and orange SAND with silt	5.5
- I	10.0		AU 3			(CL/CH) Gray and brown CLAY with sand	10.5
AT CPT - GINT STD US.C	12.5 15.0		AU 4			(SP-SM) Brown and orange SAND with silt Bottom of borehole at 15.0 feet.	15.0

Summary Report of a Geotechnical Site Exploration – Revision 1 **Dollar General – Lake City SW Marvin Burnett**Lake City, Columbia County, Florida

GSE Project No. 16251

5.2 Standard Penetration Test Soil Boring Logs

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UNDE
Engineering & Consulting, Inc.

GSE Engineering 5590 SW 64th St Gainesville, FL 32608

BORING NUMBER B-1

CLERT Concept Development, Inc. PROJECT NUMBER 16251 DATE STARTED 9/20/23 COMPLETED 9/20/23 GROUND ELEVATION HOLE SIZE DRILLING CONTRACTOR Whitelete Drilling, Inc. DRILLING CONTRACTOR Whitelete Drilling, Inc. LOGGED BY WD1 CHECKED BY AXI. NOTES ### ATTIME OF DRILLING METHOD 15/15/14 Along ### ATTIME OF DRILLING METHOD 15/15/14 Along ### ATTIME OF DRILLING 9.5.11 ### ATTIME OF DRILLING 9.5.	Engine	ering & Co	Gainesville, FL 32608 Telephone: 3523773233										
DATE STARTED 9/20/23 COMPLETED 9/20/23 GROUND ELEVATION HOLE SIZE	CLIE	NT _C	oncept Development, Inc.	PR	OJECT I	NAME Dolla	ar Ger	eral -	Lake	City S	SW Ma	arvin Burr	nett
DRILLING CONTRACTOR Whitaker Drilling, Inc. DRILLING METHOD Flight Auger LOGGED BY WDI CHECKED BY AXL VESTIMATED SEASONAL HIGH 3.5 ft VESTIMATED SEASONAL HIGH 3													
DRILLING METHOD Flight Auger		DATE STARTED 9/20/23 COMPLETED 9/20/23											
CHECKED BY AXL SETIMATED SEASONAL HIGH 3.5 ft													
## A SPT N VALUE A SPT 1-1-1 (SP-SK) Very loose brown SAND with slit SPT 1-1-1 (2)													
SP-SC Medium dense brown, gray, and orange SAND SPT 1-1-1 (21) (24)					¥ ESTII	MATED SEA	ASON	AL HI	GH _3	3.5 ft			
MATERIAL DESCRIPTION Septendent Septen	NOTE	:S							1	ı	ı		
(SP-SM) Very loose brown SAND with silt SPT 1-1.1 (2) (2) (5) (5) (6) (7) (1) (1) (1) (1) (1) (1) (2) (3) (1)		GRAPHIC LOG	MATERIAL DESCRIPTION	CONTACT DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)			PLASTICITY INDEX	PERCENT PASS NO. 200 SIEVE	MOISTURE CONTENT, %		
1			(SP-SM) Very loose brown SAND with silt									:	
SPT 1-2-3 (5) CLAY SPT 1-2-3 (5) SPT (1-5) (15) SPT (1-5) SPT SPT (1-5) SPT		-										lack	
3 (15) 35 18 17 56 17 (15) 35 18 17 56 17 (21) 7.5 (SPT 7-10-111 4 (21) 7.5 (SPT) Medium dense brown, gray, and orange SAND 9.5 (SP) Medium dense pale gray and brown SAND 10 (SP) Medium dense pale gray and brown SAND 12 (SP) Medium dense pale gray and brown SAND 12 (SP) Medium dense pale gray and brown SAND 12 (SP) Medium dense pale gray and brown SAND 15 (SP) Medium dense pale gray and brown SAND 12 (SP) Medium dense pale gray and brown SAND 15 (SP) Medium dense pale gray and brown SAND 15 (SP) Medium dense pale gray and brown SAND 15 (SP) Medium dense pale gray and brown SAND 15 (SP) Medium dense pale gray and brown SAND 15 (SP) Medium dense pale gray and brown SAND 15 (SP) Medium dense pale gray and brown SAND 15 (SP) Medium dense pale gray and brown SAND 16 (SP) Medium dense pale gray and brown SAND 16 (SP) Medium dense pale gray and brown SAND 16 (SP) Medium dense pale gray and brown SAND 16 (SP) Medium dense pale gray and brown SAND 16 (SP) Medium dense pale gray and brown SAND 17 (SP) Medium dense pale gray and brown SAND 17 (SP) Medium dense pale gray and brown SAND 18 (SP) Medium dense pale gray and brown SAND 18 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and brown SAND 19 (SP) Medium dense pale gray and				3									
(SP-SC) Medium dense brown, gray, and orange SAND (\$2) spt (23) spt (23) (23) (23) (23) (23) (23) (23) (24) (24) (24) (24) (24) (25) (26) (26) (26) (26) (26) (26) (26) (26	5						35	18	17	56	17		
(SP-SC) Medium dense brown, gray, and orange SAND with clay (SP) Medium dense pale gray and brown SAND (SP) Medium dense pale gray and brown SAND (CL/CH) Firm to stiff green and orange CLAY SPT (23) SPT 9-10-14 (24) SPT 3-4-5 (9) SPT 7 3-4-5 (9) SPT 8 2-3-4 (7)			Ţ										
(CL/CH) Firm to stiff green and orange CLAY (CL/CH) Firm to stiff green and orange CLAY SPT 3-4-5 (9) SPT 2-3-4 (7) Bottom of borehole at 20.0 feet.				7.5									
(CL/CH) Firm to stiff green and orange CLAY SPT 3-4-5 (9) 15 SPT 2-3-4 8 (7) Bottom of borehole at 20.0 feet.			(SP) Medium dense pale gray and brown SAND	9.5									
(CL/CH) Firm to stiff green and orange CLAY SPT 3-4-5 7 (9) SPT 8 2-3-4 (7) Bottom of borehole at 20.0 feet.													
7 (9) SPT 2-3-4 (7) Bottom of borehole at 20.0 feet.			(CL/CH) Firm to stiff green and orange CLAY	12									
20 Bottom of borehole at 20.0 feet.													
20 Bottom of borehole at 20.0 feet.	15				7	(9)							
20 Bottom of borehole at 20.0 feet.													
20 Bottom of borehole at 20.0 feet.													
Bottom of borehole at 20.0 feet.				00			1						
, , , , , , , , , , , , , , , , , , ,	20		Bottom of borehole at 20.0 feet.	20									258

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UTOL
Engineering & Consulting, Inc.

GSE Engineering 5590 SW 64th St Gainesville, FL 32608

BORING NUMBER B-2

	eering & Co	Gainesville, FL 32608 Telephone: 3523773233											
CLIE	NT C	oncept Development, Inc.	PR	OJECT I	NAME Dolla	ar Ger	eral -	Lake	City S	SW Ma	arvin B	urnett	
		UMBER 16251		OJECT L	OCATION	Lake	City	, Colur	nbia (County	, Florid	da	
DATE STARTED 9/20/23 COMPLETED 9/20/23													
	DRILLING CONTRACTOR Whitaker Drilling, Inc.				VATER LEV								
		IETHOD Flight Auger											
		Y WDI CHECKED BY AXL		¥ ESTII	MATED SEA	ASON	AL HI	GH _3	3.5 ft				
NOT	<u> </u>												
O DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	CONTACT DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	LIQUID LIMIT, %	PLASTIC LIMIT, %	PLASTICITY INDEX	PERCENT PASS NO. 200 SIEVE	MOISTURE CONTENT, %		SPT N V/	ALUE ▲ 60 80
		(SP-SM) Very loose gray and brown SAND with silt									:	:	
_	- 11					1						:	
_	-			SPT 1	1-2-2 (4)							:	
_			3	SPT 2	1-2-2 (4)						\		
5				SPT 3	2-4-6 (10)								
-	-	¥		SPT 4	7-9-8 (17)								
_			8.5	SPT 5	7-8-10 (18)	-							
- - - - - - -	-	(SP-SC) Very loose to medium dense pale gray and brown SAND with clay	0.0	SPT 6	7-9-10 (19)	-					<u> </u>		
_													
_	- <i>////</i>	Weight-of-Rod from 13.5 to 14.5 ft bls.		V and	0-0-3	-							
15		(CL/CH) Soft gray sandy CLAY	14.5	SPT 7	(3)							:	
10						1							
-			10.5									:	
		(CL/CH) Firm green and orange CLAY	16.5									:	
_												:	
-	-											:	
						-						:	
			20	SPT 8	3-3-4 (7)						A		
		Bottom of borehole at 20.0 feet.										:	250
											<u> </u>	:	259



GSE Engineering 5590 SW 64th St Gainesville, FL 32608

BORING NUMBER B-3

Gainesville, FL 32608 Telephone: 3523773233 Engineering & Consulting, Inc. PROJECT NAME Dollar General - Lake City SW Marvin Burnett CLIENT Concept Development, Inc. PROJECT NUMBER 16251 PROJECT LOCATION Lake City, Columbia County, Florida GROUND ELEVATION ___ **COMPLETED** 9/20/23 **HOLE SIZE DATE STARTED** 9/20/23 **GROUND WATER LEVELS:** DRILLING CONTRACTOR Whitaker Drilling, Inc. **TAT TIME OF DRILLING** 6.1 ft DRILLING METHOD Flight Auger $\sqrt{2}$ ESTIMATED SEASONAL HIGH $\sqrt{3.5}$ ft LOGGED BY WDI CHECKED BY AXL NOTES SAMPLE TYPE NUMBER PERCENT PASS NO. 200 SIEVE PLASTICITY INDEX MOISTURE CONTENT, % PLASTIC LIMIT, GRAPHIC LOG LIQUID LIMIT, DEPTH (ft) ▲ SPT N VALUE ▲ MATERIAL DESCRIPTION 20 60 80 40 SPT BORINGS - GINT STD US.GDT - 10/11/23 09:54 - P./GENERAL/PROJECTS\16251 DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT\16251 BORINGS\16251 BORINGS.GR. (SP-SM) Very loose gray and brown SAND with silt 1-1-1 (2) (SP) Loose pale gray SAND 1-2-4 SPT ∇ (6) 4.5 SPT 2-4-5 (SP-SC) Loose to gray and brown SAND with clay (9) 6 6-2-9 SPT (SC/CL) Medium dense to dense gray, brown, and orange very clayey SAND 7-9-11 SPT (20)SPT 14-16-22 (38)13.5 (CL) Firm gray sandy CLAY 2-3-3 SPT 15 26 62 23 15 16 (CL/CH) Green and orange CLAY 20 20 Bottom of borehole at 20.0 feet. 260



GSE Engineering 5590 SW 64th St Gainesville, FL 32608 Telephone: 3523773233

BORING NUMBER B-4

Engine	eering & Co	Gainesville, FL 32608 Telephone: 3523773233											
		oncept Development, Inc.	PRO	OJECT I	NAME Dolla	ar Gen	eral -	Lake	City S	SW Ma	arvin Burne	ett	
		UMBER 16251			LOCATION								
	DATE STARTED 9/20/23 COMPLETED 9/20/23												
		ONTRACTOR Whitaker Drilling, Inc.	,		VATER LEV								
		IETHOD Flight Auger	-		ME OF DRI								
		Y WDI CHECKED BY AXL	-	¥ ESTII	MATED SEA	ASON	AL HI	GH _3	3.5 ft				
NOT	ES												
O DEPTH	GRAPHIC LOG	MATERIAL DESCRIPTION	CONTACT DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	LIQUID LIMIT, %	PLASTIC LIMIT, %	PLASTICITY INDEX	PERCENT PASS NO. 200 SIEVE	MOISTURE CONTENT, %		「 N VALUE ▲ 40 60 80	
LAY GENERAL - LAKE CITY SW MARVIN BURNE ITTIGZST BURINGS. 1820 I BURINGS. 1870 1		(SP-SM) Very loose gray and brown SAND with silt											
N -	- 11												
5				SPT 1	1-1-2 (3)						A		
20102					(0)								
-	1111	$_{igstyle igstyle igytz{igstyle igstyle igstyle igytyle igstyle igytyle igstyle igstyle igstyle igstyle igstyle igstyle igstyle igytyle igstyle igytyle igstyle igstyle igstyle igytyle igytyle igytyle igytyle igytyle igytyle igstyle igytyle $	3	SPT	4-7-11								
5		\(\frac{1}{2}\) (6. \(\frac{1}{1}\) (6. \(\frac{1}\) (6. \(1		2	(18)						<i>T</i>		
				SPT	7-5-6								
5	-			3	(11)						†		
			6										
-		(SM-SC) Loose to medium dense gray, brown, and		SPT 4	3-4-5 (9)								
		orange silty SAND with clay		A .	(0)								
				SPT	5-7-14				27	18	\ <u>`</u>		
<u> </u>	111/			5	(21)				21	10	T		
-			9	SPT	12-10-9								
		(SC) gray and brown clayey SAND		6	(19)						🛉		
10						-							
-													
			13										
<u> </u>		(SP-SC) Medium dense gray, brown, and orange SAND	1.0										
	1 1/	with clay		SPT	4-9-12								
15				7	(21)						1		
15	1 1/					1							
222] [//												
2			_										
-		(CL/CH) Firm green and gray sandy CLAY	17										
S L													
						-							
5 -				SPT	3-3-4								
20			20	8	(7)								
SPT BORINGS - GINT STD US.GDT - 10/11/23 09:54 - P:/GENERALIPROJECT S/16251 DOL	1	Bottom of borehole at 20.0 feet.				1							
SPT												261	

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Engineering & Consulting, Inc.

SPT BORINGS - GINT STD US.GDT - 10/11/23 09:54 - P:\GENERAL\PROJECTS\16251 DOLLAR GENERAL – LAKE CITY SW MARVIN BURNETT\16251 BORINGS\16251 BORINGS\16251 BORINGS\16251

GSE Engineering 5590 SW 64th St Gainesville, FL 32608 Telephone: 3523773233

BORING NUMBER B-5

Telephone: 3523773233 PROJECT NAME Dollar General - Lake City SW Marvin Burnett **CLIENT** Concept Development, Inc. PROJECT NUMBER 16251 PROJECT LOCATION Lake City, Columbia County, Florida GROUND ELEVATION _ **COMPLETED** 9/20/23 **HOLE SIZE DATE STARTED** 9/20/23 **GROUND WATER LEVELS:** DRILLING CONTRACTOR Whitaker Drilling, Inc. **TAT TIME OF DRILLING** 6.5 ft DRILLING METHOD Flight Auger **<u>∑</u> ESTIMATED SEASONAL HIGH** 3.5 ft LOGGED BY WDI CHECKED BY AXL NOTES SAMPLE TYPE NUMBER PERCENT PASS NO. 200 SIEVE PLASTICITY INDEX MOISTURE CONTENT, % PLASTIC LIMIT, GRAPHIC LOG LIQUID LIMIT, DEPTH (ft) ▲ SPT N VALUE ▲ MATERIAL DESCRIPTION 60 80 40 (SP-SM) Very loose brown and gray SAND with silt 1-1-2 (3) 3 4-7-8 $_{igtriangledown}$ (SP) Medium dense pale brown and pale gray SAND SPT (15)SPT 10-11-13 5 (24)10-8-9 SPT ▼ 7-8-11 SPT 8 (19)(SP-SC) Medium dense to dense brown and orange SAND with clay SPT 17-21-24 11 17 (45)10 10 (SP) Medium dense pale brown and gray SAND 5-7-9 SPT (16)15 17.5 (CL/CH) Hard pale gray sandy CLAY 8-14-19 8 (33)20 20 Bottom of borehole at 20.0 feet. 262 Summary Report of a Geotechnical Site Exploration – Revision 1 **Dollar General – Lake City SW Marvin Burnett**Lake City, Columbia County, Florida
GSE Project No. 16251

5.3 Laboratory Results



SUMMARY REPORT OF LABORATORY TEST RESULTS

Project Number: 16251

Project Name: Dollar General - Lake City SW Marvin Burnett

Boring Number	Depth (ft)	Soil Description	Natural Moisture Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	Percent Passing No. 200 Sieve	Organic Content (%)	Hydraulic Conductivity (ft/day)	Unified Soil Classification
A-2	1-1.5	Dark brown and gray SAND with silt	8.7				11			SP-SM
A-4	3-3.5	Brown, gray, and orange very clayey SAND	18				34			SC/CL
B-1	4-5.5	Brown, gray, and orange sandy CLAY	17	35	18	17	56			CL
B-3	13.5-15	Gray sandy CLAY	23	41	15	26	62			CL
B-4	7-8.5	Gray, brown, and orange silty SAND with clay	18				27			SM-SC
B-5	8.5-10	Pale brown and gray SAND with clay	17				11			SP-SC
P-1	2-4	Gray and brown silty SAND	7.8				14		1.1	SM
P-3	0-2	Brown silty SAND	9.7				15		0.8	SM
P-5	3-5	Brown and gray clayey SAND	13				30		NF	SC

Summary Report of a Geotechnical Site Exploration – Revision 1 **Dollar General – Lake City SW Marvin Burnett**Lake City, Columbia County, Florida
GSE Project No. 16251

5.4 Key to Soil Classification

KEY TO SOIL CLASSIFICATION CHART

Critaria fa	n A saismin a Cusum Samulasl	o and Crown Names II	in a Labourtour Tasta	SYM	BOLS	GROUP NAME	
Criteria 10	r Assigning Group Symbol	s and Group Names U	sing Laboratory Tests	GRAPHIC	LETTER	GROUP NAME	
COARSE-GRAINED SOILS	Gravels	Clean Gravels	$Cu \ge 4$ and $1 \le Cc \le 3$	74.73	GW	Well graded GRAVEL	
More than 50% retained	More than 50% of coarse	Less than 5% fines	Cu < 4 and/or 1 > Cc > 3		GP	Poorly graded GRAVEL	
on No. 200 sieve	fraction retained on No. 4 sieve	Gravels with fines	Fines classify as ML or MH		GM	Silty GRAVEL	
		More than 12% fines	Fines classify as CL or CH		GC	Clayey GRAVEL	
	Sands	Clean Sands	$Cu \ge 6$ and $1 \le Cc \le 3$		SW	Well graded SAND	
	50% or more of coarse	Less than 5% fines	Cu < 6 and/or 1 > Cc > 3		SP	Poorly graded SAND	
	fraction passes No. 4 sieve	Sand with fines	Fines classify as ML or MH		SP-SM	SAND with silt	
		$5\% \le \text{fines} < 12\%$	Fines classify as CL or CH		SP-SC	SAND with clay	
		Sand with fines	Fines classify as ML or MH		SM	Silty SAND	
		$12\% \le \text{fines} < 30\%$	Fines classify as CL or CH		SC	Clayey SAND	
		Sand with fines	Fines classify as ML or MH		SM	Very silty SAND	
		30% fines or more	Fines classify as CL or CH		SC	Very clayey SAND	
FINE-GRAINED SOILS	Clays	inorganic	50% ≤ fines < 70%		CL/CH	Sandy CLAY	
50% or more passes the			70% ≤ fines < 85%		CL/CH	CLAY with sand	
No. 200 sieve			fines $\geq 85\%$		CL/CH	CLAY	
	Silts and Clays	inorganic	PI > 7 and plots on/above "A" line		CL	Lean CLAY	
	Liquid Limit less than 50		PI < 4 or plots below "A" line		ML	SILT	
		organic	Liquid Limit - oven dried		O.I.	Organic clay	
			< 0.75 Liquid Limit - not dried		OL	Organic silt	
	Silts and Clays	inorganic	PI plots on or above "A" line		СН	Fat CLAY	
	Liquid Limit 50 or more		PI plots below "A" line		MH	Elastic SILT	
		organic	Liquid Limit - oven dried		OII	Organic clay	
			< 0.75 Liquid Limit - not dried		ОН	Organic silt	
HIGHLY ORGANIC SOILS	Primaril	y organic matter, dark in	color, and organic odor	**************************************	РТ	PEAT	

CORRELATION OF PENETRATION RESISTANCE WITH RELATIVE DENSITY AND CONSISTENCY

	No. OF BLOWS, N	RELATIVE DENSITY		No. OF BLOWS, N	CONSISTENCY
	0 - 4	Very Loose		0 - 2	Very Soft
	5 - 10	Loose	SILTS	3 - 4	Soft
SANDS:	11 - 30	Medium dense	&	5 - 8	Firm
	31 - 50	Dense	CLAYS:	9 - 15	Stiff
	OVER 50	Very Dense		16 - 30	Very Stiff
				31 - 50	Hard
	No. OF BLOWS, N	RELATIVE DENSITY		OVER 50	Very Hard
	0 - 8	Very Soft			

Very Soft

9 - 18 Soft

LIMESTONE: 19 - 32 Moderately Hard

33 - 50 Hard OVER 50 Very Hard

SAMPLE GRAPHIC TYPE LEGEND

LABORATORY TEST LEGEND



Location of SPT Sample



Location of Auger Sample

PARTICLE SIZE IDENTIFICATION

BOULDER	S:	Greater than 300 mm			
COBBLES:		75 mm to 300 mm	LL	=	Liquid Limit, %
GRAVEL:	Coarse -	19.0 mm to 75 mm	PL	=	Plastic Limit, %
	Fine -	4.75 mm to 19.0 mm	PI	=	Plasticity Index, %
SANDS:	Coarse -	2.00 mm to 4.75 mm	% PASS - 20	00 =	Percent Passing the No. 200 Sieve
	Medium -	0.425 mm to 2.00 mm	MC	=	Moisture Content, %
	Fine -	0.075 mm to 0.425 mm	ORG	=	Organic Content, %
SILTS & CI	LAYS:	Less than 0.075 mm	k ₁ ,	=	Horizontal Hydraulic Conductivity, ft/dal

6.0 LIMITATIONS

6.1 Warranty

This report has been prepared for our client for their exclusive use, in accordance with generally accepted soil and foundation engineering practices, and makes no other warranty either expressed or implied as to the professional advice provided in the report.

6.2 Auger and SPT Borings

The determination of soil type and conditions was performed from the ground surface to the maximum depth of the borings, only. Any changes in subsurface conditions that occur between or below the borings would not have been detected or reflected in this report.

Soil classifications that were made in the field are based upon identifiable textural changes, color changes, changes in composition or changes in resistance to penetration in the intervals from which the samples were collected. Abrupt changes in soil type, as reflected in boring logs and/or cross sections may not actually occur, but instead, be transitional.

Depth to the water table is based upon observations made during the performance of the auger and SPT borings. This depth is an estimate and does not reflect the annual variations that would be expected in this area due to fluctuations in rainfall and rates of evapotranspiration.

6.3 Site Figures

The measurements used for the preparation of the figures in this report were made using the provided site plan and by estimating distances from existing structures and site features. Figures in this report were not prepared by a licensed land surveyor and should not be interpreted as such.

6.4 Unanticipated Soil Conditions

The analysis and recommendations submitted in this report are based upon the data obtained from soil borings performed at the locations indicated on Figure 2. This report does not reflect any variations that may occur between these borings.

The nature and extent of variations between borings may not become known until excavation begins. If variations appear, we may have to re-evaluate our recommendations after performing on-site observations and noting the characteristics of any variations.

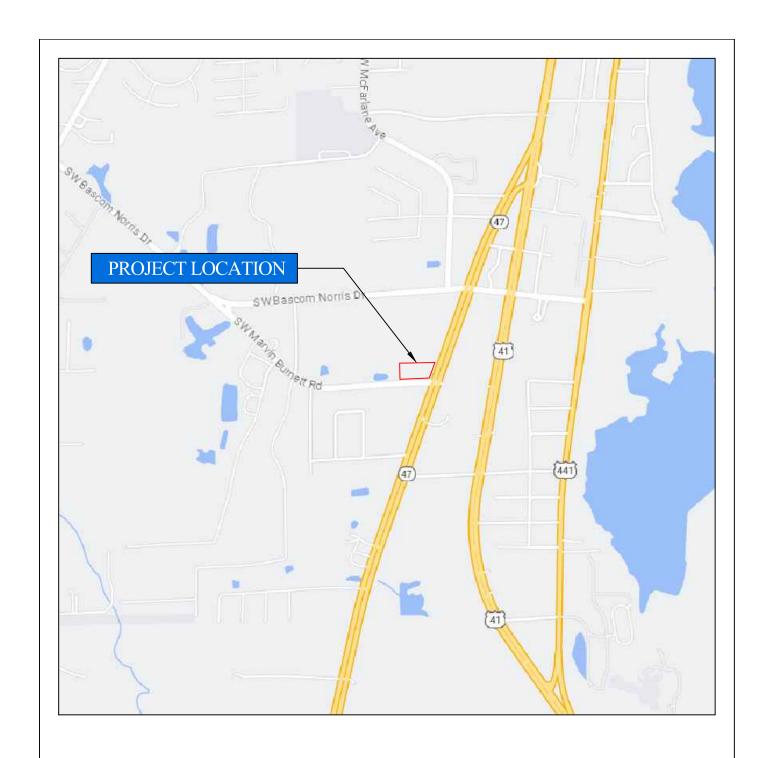
6.5 Misinterpretation of Soil Engineering Report

GSE Engineering & Consulting, Inc. is responsible for the conclusions and opinions contained within this report based upon the data relating only to the specific project and location discussed herein. If others make the conclusions or recommendations based upon the data presented, those conclusions or recommendations are not the responsibility of GSE.

Summary Report of a Geotechnical Site Exploration – Revision 1 **Dollar General – Lake City SW Marvin Burnett**Lake City, Columbia County, Florida

GSE Project No. 16251

FIGURES





DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT LAKE CITY, COLUMBIA COUNTY, FLORIDA GSE PROJECT NO. 16251

PROJECT SITE LOCATION MAP

DESIGNED BY: AXL CHECKED BY: JEG DRAWN BY: EEW



FIGURE

1







SPT BORING



AUGER BORING



DOLLAR GENERAL - LAKE CITY SW MARVIN BURNETT LAKE CITY, COLUMBIA COUNTY, FLORIDA GSE PROJECT NO. 15396

SITE PLAN SHOWING APPROXIMATE LOCATIONS OF FIELD TESTS

DESIGNED BY: AXL CHECKED BY: JEG DRAWN BY: AXL



FIGURE

2



DEPARTMENT OF GROWTH MANAGEMENT

205 North Marion Avenue Lake City, Florida 32055 Telephone: (386) 719-5750 growthmanagement@lcfla.com

REVIEW REPORT TO PLANNING AND ZONING, BOARD OF ADJUSTMENT AND HISTORICAL COMMITTEES' BY STAFF FOR SITE PLAN REVIEW, SPECIAL EXCEPTIONS, VARIANCES, COMPREHENSIVE PLAN AMENDMENTS/ ZONING AND CERTIFICATE OF APPROPRIATENESS

Date:
Request Type: Site Plan Review (SPR) Special Exception (SE) Variances (V)
Comprehensive Plan Amendment/Zoning (CPA/Z) Certificate of Appropriateness (COA)
Project Number: SPR24-05
Project Name: Commercial Retail Store- Dollar General
Project Address: TBD
Project Parcel Number: 08127-005
Owner Name: Concept Companies
Owner Address: 1449 SW 74th Dr. Suite 200
Owner Contact Information: Telephone Number: 352-333-3233 Email:
Owner Agent Name: Randall Olney, P.E.
Owner Agent Address: 11801 Research Drive, Alaucha, FL 32615
Owner Agent Contact Information: Telephone: 352-331-1976 Email: randyo@chw-inc.com

The City of Lake City staff has reviewed the application and documents provided for the above request and have determined the following.

Growth Management – Building Department, Planning and Zoning, Code Enforcement, Permitting

uilding Department: Reviewed by:	Date:
o comments at this time	
anning and Zoning: Reviewed by: Robert Argelo FOOTED33886E4BE.	Date:
The property is zoned Commercial Intensive. All Commercial General are permitted in Commercial 1.13.2. Retail stores are a permitted use per s	Intensive per section
— DocuSigned by:	3/22/2024
usiness License: Reviewed by:	Date:
ill need to apply for occupational license	
DocuSigned by:	Date:
ode Enforcement: Reviewed by: Marshall Sova	Date:
No liens, codes or violations	
ermitting. Beviewed by:	Date: 3/22/2024
ermitting: Reviewed by:	Date: ^{3/22/2024}
ermitting: Reviewed by:	Date: ^{3/22/2024}
	Date: ^{3/22/2024}
ermitting: Reviewed by:	Date: ^{3/22/2024}

Utilities – Water, Sewer, Gas, Water Distribution/Collections, Customer Service

Water Department: Reviewed by: Nike Oslow	Date:
None at this time	
ewer Department: Reviewed by: (oly fridgen	Date:
None	
Gas Department: Reviewed by: Stw Brown Eastnot comment wit no address.	Date:
Vater Distribution/Collection: Reviewed by:	Date: 4/2/2024
no comment at this time	
Customer Service: Reviewed by: Slasta Pellam	Date: 4/9/2024
A tap application will need to be submitted in order request city utilities. The utility fees will be can of the tap application.	

Public Safety – Public Works, Fire Department, Police Department

Public Works: Reviewed by: Stw Brown Bastrocestras.	Date:
No comment.	
ire Department: Reviewed by:	Date:
I have no issues	
Police Department: Reviewed by: Sw twll	Date: 3/22/2024
no concerns at this time	

NOTE: Please provide separate pages for comments that will not fit in provided spaces and please label the pages for your department and for the project.

State and County-FDOT, Suwannee River Water Management, School Board, Columbia County FDOT: Reviewed by:__ Date: _Date: 3/25/2024 Suwannee River Water Management: Reviewed by: ____ Garntt Spencer_ The project will require an ERP Permit. It is recommended that the applicant schedule a pre-application meeting with SRWMD staff to go over the permitting requirements. School Board: Reviewed by: __kuith Hatcher No comments at this time. Date: 4/9/2024 County: Reviewed by: Chad Williams A driveway permit will be required. The County is currently in the process of permitting several developments west of this location along Bascom

A driveway permit will be required. The County is currently in the process of permitting several developments west of this location along Bascom Norris Drive. For that reason, we ask that the applicant and agencies pay careful attention to the traffic issue. This comment is provided by the County Engineer based only on the information contained in the application provided. This response does not constitute the engineer's professional opinion with respect to the project and does not constitute approval of any committee or board for Columbia County.

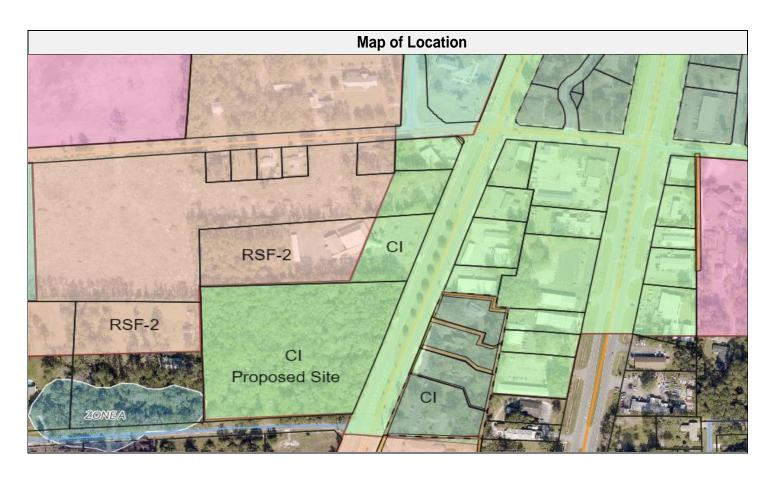
NOTE: Please provide separate pages for comments that will not fit in provided spaces and please label the pages for your department and for the project.

LAKE CITY GROWTH MANAGEMENT STAFF ANALYSIS REPORT

	Project Information
Project Name and Case No.	Dollar General site plan review
Applicant	Randall Olney, PE
Owner	Concept Companies
Requested Action	Site plan review for Dollar General, retail store, on parcel 08127-005
Hearing Date	05-14-2024
Staff Analysis/Determination	Sufficient for Review
Prepared By	Robert Angelo

Subject Property Information		
Size	+/- 2.70 Acres	
Location	Corner of Marvin Burnett and Hwy 47	
Parcel Number	08127-000	
Future Land Use	Commercial	
Proposed Future Land Use	Commercial	
Current Zoning District	Commercial Intensive	
Proposed Zoning	Commercial Intensive	
Flood Zone-BFE	Flood Zone X and A Base Flood Elevation-N/A	

	Land Use Table			
Direction	Future Land Use	Zoning	Existing Use	Comments
N	Residential Moderate	RSF-2	Residential	
Е	Residential Moderate	RSF-2	Residential	
S	County		Vacant	County Jurisdiction
W	Commercial	CI	Medical Office	





Summary of Request
Applicant has petitioned for a site plan review for the above parcels to build a retail store.

Commercial Retail Store SW Marvin Burnett Rd. Site Plan Application



City of Lake City
Planning and Zoning Board May 14, 2024



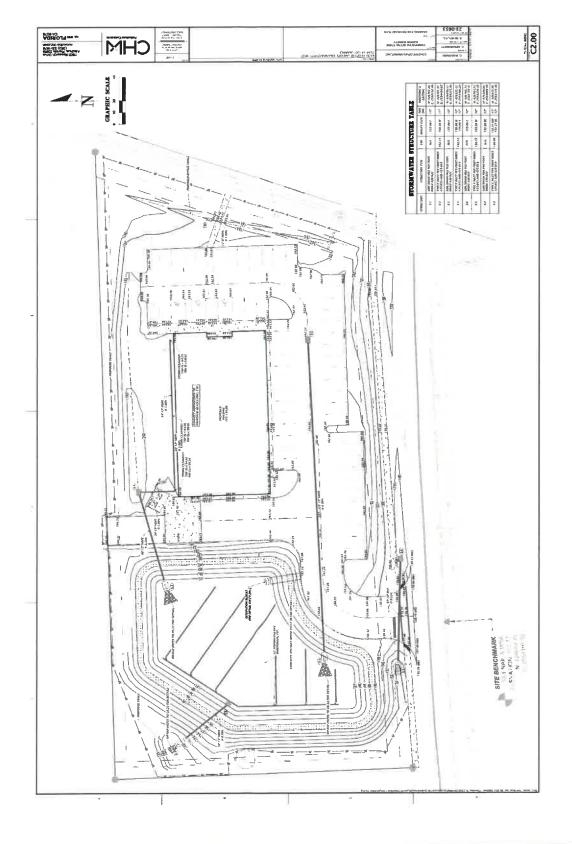
APPLICATION TYPE:

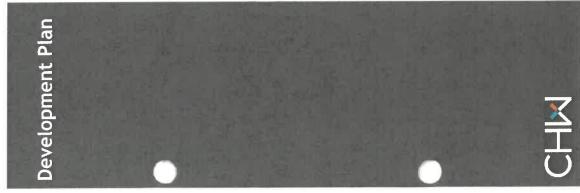
Site Plan

NTENT:

Construct a 10,640 SF Commercial Retail Store with associated parking, utility infrastructure, and stormwater management system on Columbia County tax parcel #08127-005.







Contact Information

Cole Menhennett, El

Address: 11801 Research Drive Alachua, FL 32615

Phone: 386.518.6514

Email: cole.menhennett@nv5.com





NOTICE LAND USE ACION

A PUBLIC HEARING IS SCHEDULED TO CONCIDER A REQUEST FOR:

SPR24.05, a petition by Randall Olney, P.E., as agent, to request a Site Plan Review approval be granted as provided for in Section 4.13 of the Land Development Regulations, to get approval on site plan for Dollar General for a property located in the Commercial Intensive zoning district, in accordance with the submittal of the petition dated March 21, 2024, to be located on parcels 08127-005

WHEN;	June 11, 2024 at 5:30pm or as soon after.
WHERE:	City Council Meeting Room, Second Floor, City Hall, located at 205 North Marion Avenue, Lake City, Florida. Members of the public may also view the meeting on our YouTube channel at: https://www.youtube.com/c/CityofLakeCity .

Copies of the site plan application are available for public inspection by contacting the Growth Management office at growthmanagement@lcfla.com or by calling 386-719-5820.

At the aforementioned public hearing, all interested parties may be heard with respect to the amendment.

FOR MORE INFORMAITON CONTACT
ROBERT ANGELO
PLANNING AND ZONING TECHNICIAN
AT 386-719-5820

Angelo, Robert

From:

LCR-Classifieds <classifieds@lakecityreporter.com>

Sent:

Tuesday, May 28, 2024 11:00 AM

To:

Angelo, Robert

Subject:

RE: 78430 78431 78432 RE: Non-Legal Ad for P&Z, BOA, and HPA for 06-11-2024

Confirmed!

Thank you much, Kymberlee Harrison 386-754-0401

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1086 SW Main Blvd. Ste 103, Lake City, FL 32055

PH 386-754-0401

Why Local Newsprint Advertising?

1 Newspaper readers are ENGAGED

2 Newspapers are viewed as TRUSTWORTHY

From: Angelo, Robert <AngeloR@lcfla.com> Sent: Tuesday, May 28, 2024 10:59 AM

To: LCR-Classifieds <classifieds@lakecityreporter.com>

Subject: RE: 78430 78431 78432 RE: Non-Legal Ad for P&Z, BOA, and HPA for 06-11-2024

Looks Good

Thank You Robert Angelo City of Lake City Growth Management growthmanagement@lcfla.com 386-719-5820



PLEASE NOTE: Florida has a very broad public records law. Most written communications to or from City officials regarding City business are public records available to the public and media upon request. Your email communications may be subject to public disclosure.

From: LCR-Classifieds <classifieds@lakecityreporter.com>

Sent: Tuesday, May 28, 2024 10:37 AM
To: Angelo, Robert < AngeloR@lcfla.com >

Subject: 78430 78431 78432 RE: Non-Legal Ad for P&Z, BOA, and HPA for 06-11-2024

Here you go! P&Z 2x8 247.50 Historic: 2x6.25 206.25

BOA: 2x6 198.00

Thank you much,

Kymberlee Harrison 386-754-0401

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PH 386-754-0401

Why Local Newsprint Advertising?

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From: Angelo, Robert < Angelo R@lcfla.com >

Sent: Friday, May 24, 2024 3:49 PM

To: LCR-Classifieds <<u>classifieds@lakecityreporter.com</u>>

Subject: Non-Legal Ad for P&Z, BOA, and HPA for 06-11-2024

Kym

Please publish this ad in the body of the paper as a display ad in the May 30, 2024 paper.

Thank You Robert Angelo City of Lake City Growth Management growthmanagement@lcfla.com 386-719-5820



PLEASE NOTE: Florida has a very broad public records law. Most written communications to or from City officials regarding City business are public records available to the public and media upon request. Your email communications may be subject to public disclosure.

? TICE OF PUBLIC MEETIN ? CITY OF LAKE CITY PLANNING AND ZONING BOARD

THIS SERVES AS PUBLIC NOTICE the Planning and Zoning Board will hold a meeting on Tuesday, June 11, 2024 at 5:30 PM or as soon after.

Agenda items-

- 1. SPR 24-05, Petition submitted by Randall Olney, P.E., (agent) for Concept Companies, (owner), for a Site Plan Review for Dollar General, in a Commercial Intensive zoning district, and located on parcel 08127-005, which is regulated by the Land Development Regulations Section 4.13.
- 2. SPR 24-06, Petition submitted by Christopher A. Gmuer, P.E., (agent) for ERA Investments, LLC, (owner), for a Site Plan Review for Lake City Hotels Phase 2, in a Commercial Intensive zoning district, and located on parcel 02582-002, which is regulated by the Land Development Regulations Section 4.13.
- 3. SPR 23-10, Petition submitted by Carol Chadwick, P.E., (agent) for Affiliated Property Management, (owner), for a Site Plan Review for Aspire Dental Addition, in a Residential Office zoning district, and located on parcel 07604-102, which is regulated by the Land Development Regulations Section 4.10.

Meeting Location: City Council Chambers located on the 2nd Floor of City Hall at 205 North Marion Avenue, Lake City, FL 32055.

Members of the public may also view the meeting on our YouTube channel at:

https://www.youtube.com/c/CityofLakeCity

Pursuant to 286.0105, Florida Statutes, the City hereby advises the public if a person decides to appeal any decision made by the City with respect to any matter considered at its meetings or hearings, he or she will need a record of the proceedings, and that, for such purpose, he or she may need to ensure that a verbatim record of the proceedings is made, which record includes the testimony and evidence upon which the appeal is to be based.

SPECIAL REQUIREMENTS: Pursuant to 286.26, Florida Statutes, persons needing special accommodations to participate in this meeting should contact the City Manager's Office at (386) 719-5768.

Robert Angelo Planning and Zoning Tech.

CITY OF LAKE CITY CUSTOMER SERVICE BUILDING 173 NW HILLSBORO STREET LAKE CITY, FL 32055

NOTICE OF PUBLIC MEETING CITY OF LAKE CITY PLANNING AND ZONING BOARD

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Robert Angelo
Planning and Zoning Tech.





NOTICE LAND USE ACTION

A PUBLIC HEARING IS SCHEDULED TO CONSIDER A REQUEST FOR:

SPR24.05, a petition by Randall Olney, P.E., as agent, to request a Site Plan Review approval be granted as provided for in Section 4.13 of the Land Development Regulations, to get approval on site plan for Dollar General for a property located in the Commercial Intensive zoning district, in accordance with the submittal of the petition dated March 21, 2024, to be located on parcels 08127-005

WHEN: May 14, 2024

5:30 p.m.

WHERE: City Council Meeting Room, Second Floor, City Hall, located at 205 North Marion Avenue,

Lake City, Florida.

Members of the public may also view the meeting on our YouTube channel at:

https://www.youtube.com/c/CityofLakeCity.

Copies of the site plan review application are available for public inspection by contacting the Office of Growth Management at growthmanagement@lcfla.com or by calling 386.719.5820.

At the aforementioned public hearing, all interested parties may be heard with respect to the Certificate of Appropriateness.

FOR MORE INFORMATION CONTACT ROBERT ANGELO PLANNING & ZONING TECHNICIAN AT 386.719.5820

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- LDR 24-04. Text amendment to the Land Development Regulations Sections 2.1, 4.2, 4.4, 4.5 and 4.6, to
 add definitions and add provisions for ADU's, Accessory Dwelling Units, and Tiny Homes for the City of
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Robert Angelo
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Robert Angelo
Planning and Zoning Tech.

Angelo, Robert

From:

LCR-Classifieds <classifieds@lakecityreporter.com>

Sent:

Monday, April 29, 2024 11:20 AM

To:

Angelo, Robert

Subject:

73990 73992 73991 RE: Non-Legal Ad for P&Z, BOA, and HPA for 05-14-2024

Attachments:

73991.pdf; 73992.pdf; 73990.pdf

Robert, all are scheduled to publish on May 2. Approval due by tomorrow please

P&Z: 3 col x 5.5 \$272.25 Historic: 3 col x 4.5 \$222.75

BOA: 3 col x 4 \$198

Thank you much,

Kymberlee Harrison 386-754-0401

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PH 386-754-0401

Why Local Newsprint Advertising?

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From: Angelo, Robert <AngeloR@lcfla.com>

Sent: Monday, April 29, 2024 8:57 AM

To: LCR-Classifieds <classifieds@lakecityreporter.com>

Subject: Non-Legal Ad for P&Z, BOA, and HPA for 05-14-2024

Kym

Please publish this ad in the body of the paper as a display ad in the May 2, 2024 paper.

Thank You Robert Angelo City of Lake City Growth Management

growthmanagement@lcfla.com

386-719-5820



PLEASE NOTE: Florida has a very broad public records law. Most written communications to or from City officials regarding City business are public records available to the public and media upon request. Your email communications may be subject to public disclosure.

NOTICE OF PUBLIC MEETING CITY OF LAKE CITY PLANNING AND ZONING BOARD

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Robert Angelo
Planning and Zoning Tech.



April 22, 2024

To Whom it May Concern

On May 14, 2024 the Planning and Zoning Board will be having a meeting at 5:30pm at 205 N. Marion. At this meeting we will be hearing a petition submitted by Randall Olaney, PE, as agent, for Concept Companies, owner, for a site plan review, SPR24-05, for parcel 08127-005, The site plan is to build a retail store-Dollar General located within the Commercial Intensive (CI) zoning district.

If you have any questions or concerns please call 386-752-2031 ext. 820 or email growthmanagement@lcfla.com.

Robert Angelo

Planning and Zoning Tech

City of Lake City

Columbia Co	unty Property Appraiser -	Sales Repo	ort			
Name	Address1	Address2	Address3	City	State	ZIF
FRENVEY, INC	P O BOX 2095		LAKE CITY	FL	32056	
ST JOHNS LLC	13820 W NEWBERRY RD STE 100	,	NEWBERRY	FL	32669	
VANN SAMUEL P SR TRUST ETAL	131 W DUVAL STREET	•	LAKE CITY	FL	32055	
NFD DEVELOPERS LLC	P O BOX 2166	•	LAKE CITY	FL	32056	
GARR KENNETH	229 SW MARVIN BURNETT RD	•	LAKE CITY	FL	32024	
BAKER CHESTER	47 BURNT SWAMPP RD	2*	EAST KINGSTON	NH	03827	
JAA INVESTMENT PROPERTIES, LLC	312 SW PILOTS WAY	25	LAKE CITY	FL	32024	
LAKE CITY CHURCH OF CHRIST INC	656 SW STATE RD 47	3	LAKE CITY	FL	32025	
KAMPMEYER ERVIN L LIVING TRUST	681 SW ST RD 47	3	LAKE CITY	FL	32025	
MARTIN CELIA S AS TRUSTEE	CELIA S MARTIN REV TRUST	973 SW STATE RD 47	LAKE CITY	FL	32025	
HAYDEN DONALD B	733 SW SR 47	₩	LAKE CITY	FL	32025	
POLMERSKI LAVONNA B	423 NW CLUBVIEW CR	2	LAKE CITY	FL	32055	
CANCER CENTERS OF NORTH FLORIDA LLC	PO BOX 80610	Si contraction de la contracti	INDIANAPOLIS	IN	46280	
CIVITAN REGIONAL BLOOD CENTER INC	D/B/A LIFESOUTH COMMUNITY BLOOD CENTERS, INC	4039 NEWBERRY RD	GAINESVILLE	FL	32607	
LAKE CITY, COLUMBIA COUNTY CHAMBER OF COMMERCE, INC	875 SW SR 47	×	LAKE CITY	FL	32025	

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FRENVEY, INC PO BOX:2095 LAKE CITY, FL 32056

CHW 11801 Research Dr Alachua, FL. 32615



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13820 W NEWBERRY RD STE 100 ST JOHNS LLC **NEWBERRY, FL 32669**

contests neturn necelpt		PLACE STICKER AT TOP OF ENVELOPE TO THE HIGHT OF THE RETURN ADDRESS, FOLD AT NOTTED LINE
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C. Date of Delivery	A. Signature X B. Received by (Printed Name)	 Complete items 1, 2, and 3. Print your name and address on the reverse so that we can return the card to you. Attach this card to the back of the mailpiece, or on the front if space permits.
ELIVERY	COMPLETE THIS SECTION ON DELIVERY	SENDER: COMPLETE THIS SECTION

Print your name and address on the reverse so that we can return the card to you. Complete items 1, 2, and 3. ST JOHNS LLC 1. Article Addressed to: Attach this card to the back of the maliplece or on the front if space permits. 13820 W NEWBERRY RD STE 100 NEWBERRY, FL 32669 PS Form 3811, July 2020 PSN 7530-02-000-9053 2. Acticle Number (Transfer from service label)
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☐ Signature Confirmation
Restricted Delivery C. Date of Delivery ☐ Agent
☐ Addresses



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NFD DEVELOPERS LLC LAKE CITY, FL 32056 PO BOX 2166

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Complete items 1, 2, and 3. Print your name and address on the reverse	A. Signature	☐ Agent ☐ Addressee
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MAIN DELENTAGE

299 SW MARVIN BURNETT RD LAKE CITY, FL 32024 **GARR KENNETH**

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EAST KINGSTON, NH 03827

BAKER CHESTER 47 BURNT SWAMPP RD

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Complete items 1, 2, and 3. Print your name and address on the reverse so that we can return the card to you. Attach this card to the back of the malipiece, or on the front if space permits. A Signature Attach this card to the back of the malipiece, or on the front if space permits. A Received by (Printed Name) C. Date of Delivery Paddress below: I Addressee B. Received by (Printed Name) C. Date of Delivery address below: I Yes GARR KENNETH 299 SW MARVIN BURNETT RD LAKE CITY, FL 32024 A Service Type Adult Signature Restricted Delivery Carditied Mail Restricted Delivery Confect on Delivery Restricted Delivery Confect on Delivery Confidence Name Priority Mail Expresse® Registered Mail Restricted Delivery Confect on Delivery Confidence Name Confidence Type Confect on Delivery Confidence Name Priority Mail Expresse® Registered Mail Restricted Delivery Confidence Name Confidence Type Confidenc

PS Form 3811, July 2020 PSN 7530-02-000-8053 **BAKER CHESTER** SENDER: COMPLETE THIS SECTION 2. Article Number (Transfer from service label) **EAST KINGSTON, NH 03827 47 BURNT SWAMPP RD** Attach this card to the back of the malipiece, or on the front if space permits. 1. Article Addressed to: Print your name and address on the reverse Complete items 1, 2, and 3. 55TE T26T T000 DEEE 2202 so that we can return the card to you. 9590 9402 8047 2349 1484 62 3. Service Type Adult Signature Cartified Malte Cortified Malte Collect on Delivery Collect on Delivery Collect on Delivery Restricted Delivery Insured Mail Restricted Delivery (over \$500) × COMPLETE THIS SECTION ON DELIVERY D. is delivery address different from item 1? Yes if YES, enter delivery address below: No B. Received by (Printed Name) A. Signature ☐ Priority Mail Express® ☐ Registered Mail Testricted ☐ Registered Mail Restricted ☐ Registered Mail Restricted Delivery ☐ Signature Confirmation™ ☐ Signature Confirmation y Restricted Delivery Domestic Return Receipt C. Date of Delivery ☐ Agent ☐ Addressee



312 SW PILOTS WAY JAA INVESTMENT PROPERTIES, LLC LAKE CITY, FL 32024



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656 SW STATE RD 47 LAKE CITY CHURCH OF CHRIST INC LAKE CITY, FL 32025

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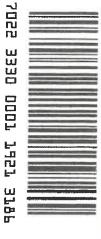
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681 SW ST RD 47 KAMPMEYER ERVIN L LIVING TRUST



LAKE CITY, FL 32025 -



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973 SW STATE RD 47 LAKE CITY, FL 32025 CELIA S MARVIN REV TRUST MARTIN CELIA S AS TRUSTEE

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LAKE CITY, FL 32025 CHAMBER OF COMMERCE, INC 875 SW SR 47 LAKE CITY, COLUMBIA COUNTY

CERTIFIED MAIL



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CANCER CENTERS OF NORTH FLORIDA INDIANAPOLIS, IN 46280 PO BOX 80610

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CIVITAN REGIONAL BLOOD CENTER INC D/B/A LIFESOUTH COMMUNITY BLOOD CENTERS, INC 4039 NEWBËRRY RD

GAINESVILLE, FL 32607

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		D/B/A LIFESOUTH COMMUNITY BLOOD
		CIVITAN REGIONAL BLOOD CENTER INC
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File Attachments for Item:

iv. ***POSTPONED***SPR23-10, Petition submitted by Carol Chadwick (agent) for Affiliated Property Management (owner), for a Site Plan Review for Aspire Dental Addition, in the Commercial Intensive Zoning District, and located on Parcel 07604-102, which is regulated by the Land Development Regulations section 4.13.

Project Summary

Project Name: Aspire Dental Addition

Project Number: SPR23-10

Parcel Number: 07604-102

Project Notes

Project type: Site Plan Review

• Future land use is: Commercial

Zoning designation is: Commercial Intensive

• Proposed use of the property: 699 square feet addition

- Land is conducive for use: A dental office is a conducive use, per the LDR section 4.13.2 and 4.12.2.5. However, per section 4.12.11.5, a dental office is required to have one (1) parking space for every 150 square feet of floor area. With the addition, the building would have 3,977 square feet of floor area. This would require 27 parking spaces. The site has eight paved parking spots and an undefined number of parking spots in a gravel area.
- See staff review for notes from directors and city staff for their comments.

Project Summary

Project SPR23-10 is for a site plan review and has been reviewed by city staff. Application is sufficient for review. After review of the petition the city staff has determined that the petition is not consistent with the land development regulations due to the fact of not having the required amount of parking. At this time the City has no other concerns.



DEPARTMENT OF GROWTH MANAGEMENT

205 North Marion Avenue Lake City, Florida 32055 Telephone: (386) 719-5750

growthmanagement@lcfla.com

REVIEW REPORT TO PLANNING AND ZONING, BOARD OF ADJUSTMENT AND HISTORICAL COMMITTEES' BY STAFF FOR SITE PLAN REVIEW, SPECIAL EXCEPTIONS, VARIANCES, COMPREHENSIVE PLAN AMENDMENTS/ ZONING AND CERTIFICATE OF APPROPRIATENESS

Date:
Request Type: Site Plan Review (SPR) Special Exception (SE) Variances (V)
Comprehensive Plan Amendment/Zoning (CPA/Z) Certificate of Appropriateness (COA) Project Number:
Project Name: Aspire Dental Addition
Project Address: 1788 SW Barnett Way, Lake City, FL
Project Parcel Number: 05-4S-17-07604-102
Owner Name: Affiliated Property Management
Owner Address: 14506 NW 11th Place, Newberry, FL 32669
Owner Contact Information: Telephone Number: 386-752-2836 Email: aspiredentallc@gmail.com Owner Agent Name: Carol Chadwick
Owner Agent Address: 1208 SW Fairfax Glen, Lake City, FL 32025
Owner Agent Address:

The City of Lake City staff has reviewed the application and documents provided for the above request and have determined the following.

Building Department: Reviewed by: ______Date: _____ Comments: Planning and Zoning: Reviewed by: As Date: 4/24/23 Comments: <u>Per section 4.12.11.5 pf the LDR, a dental office is required</u> to have one parking space for every 150 square feet of floor spaces. The site is required to have 27 space based on proposed site plan. Business License: Reviewed by: Marshall Sova (Apr 18, 2023 12:02 EDT) Date: Comments: Code Enforcement: Reviewed by: Marshall Sova (Apr 18, 2023 12:02 EDT) Date: Comments: Permitting: Reviewed by: _______Date: ______ Comments: _____

Growth Management - Building Department, Planning and Zoning, Code Enforcement, Permitting

Utilities – Water, Sewer, Gas, Water Distribution/Collections, Customer Service

Water Department: Reviewed by: Michael Osborn Jr. Methael Osborn Jr. M	Date:
Comments:	
Sewer Department: Reviewed by: Cody Pridgeon (Apr 18, 2023 13:03 EDT) Comments: See attached email from Cody	_Date:
Gas Department: Reviewed by: Steve Brown (Apr 24, 2023 08:15 EDT) Comments:	
Water Distribution/Collection: Reviewed by: Brian Scott (Apr 24, 2023 07:42 EDT) Comments:	
Customer Service: Reviewed by:Comments:	

Public Safety – Public Works, Fire Department, Police Department

Public Works: Reviewed by: Steve Brown (Apr 24, 2023 08:15 EDT)	Date:	-
Comments:		
Fire Department: Reviewed by:	Date:	
Comments:		
Police Department: Reviewed by:	Date:	-
Comments:		

Please provide separate pages for comments that will not fit in provided spaces and please label the pages for your department and for the project.

Angelo, Robert

From:

Pridgeon, Cody

Sent:

Tuesday, April 18, 2023 1:08 PM

To:

Angelo, Robert

Cc:

Pelham, Shasta; Johnson, Demetrius

Subject:

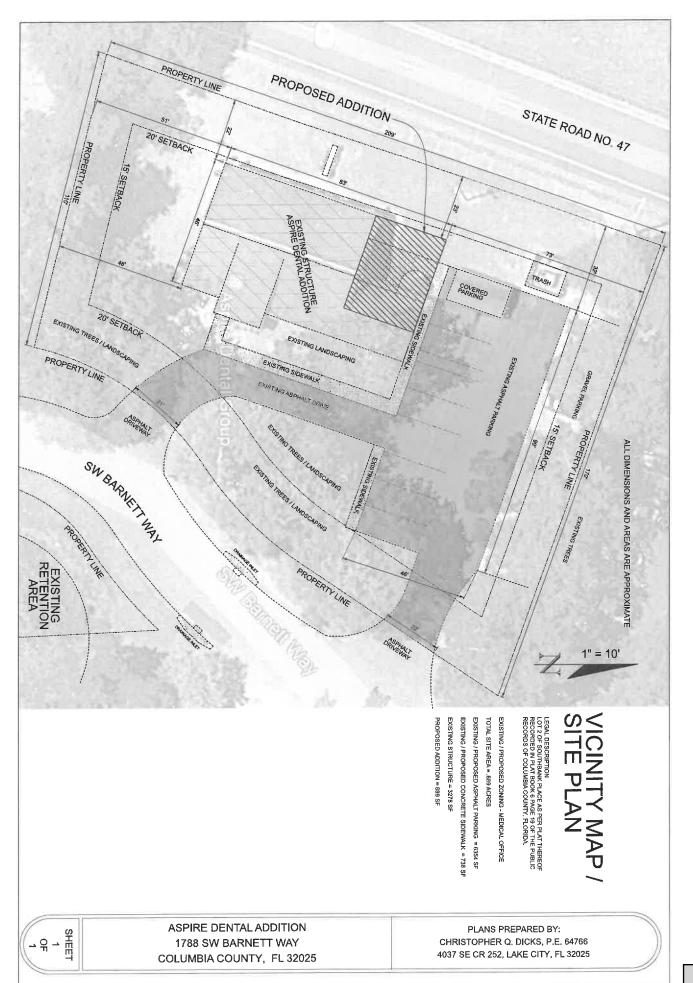
Aspire Dental

Robert the adobe sign thin that was sent to me earlier wouldn't allow me to put in comments so I'm sending you this email. Any dental offices need to comply with City Ordinance No. 2020-2149. This requires them to have a dental amalgam separator on their waste stream prior to entering the Citys collection system.

Cody Pridgeon City of Lake City Wastewater Director

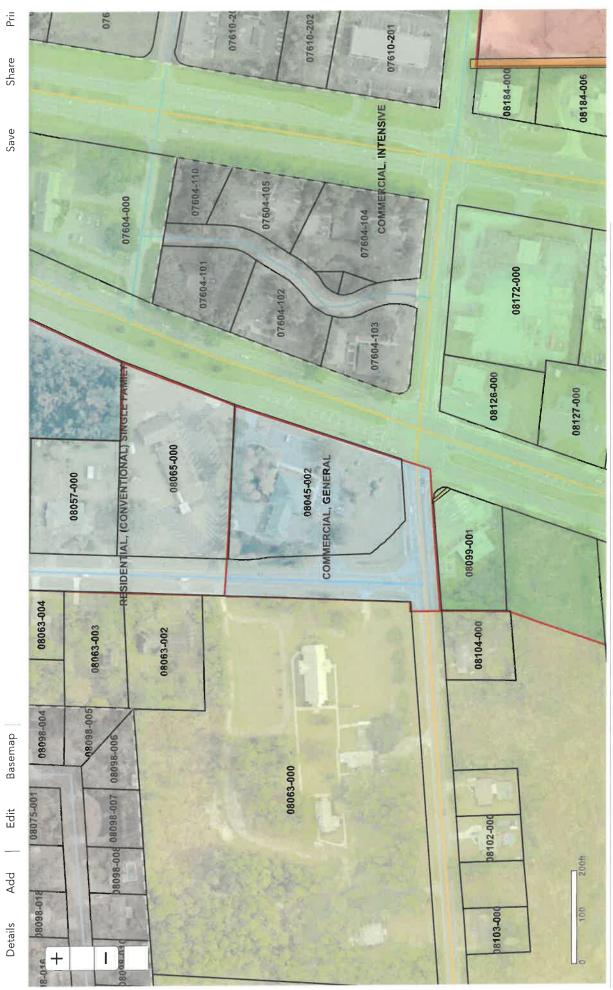
Office: (386)758-5455 Cell: (352)210-3086



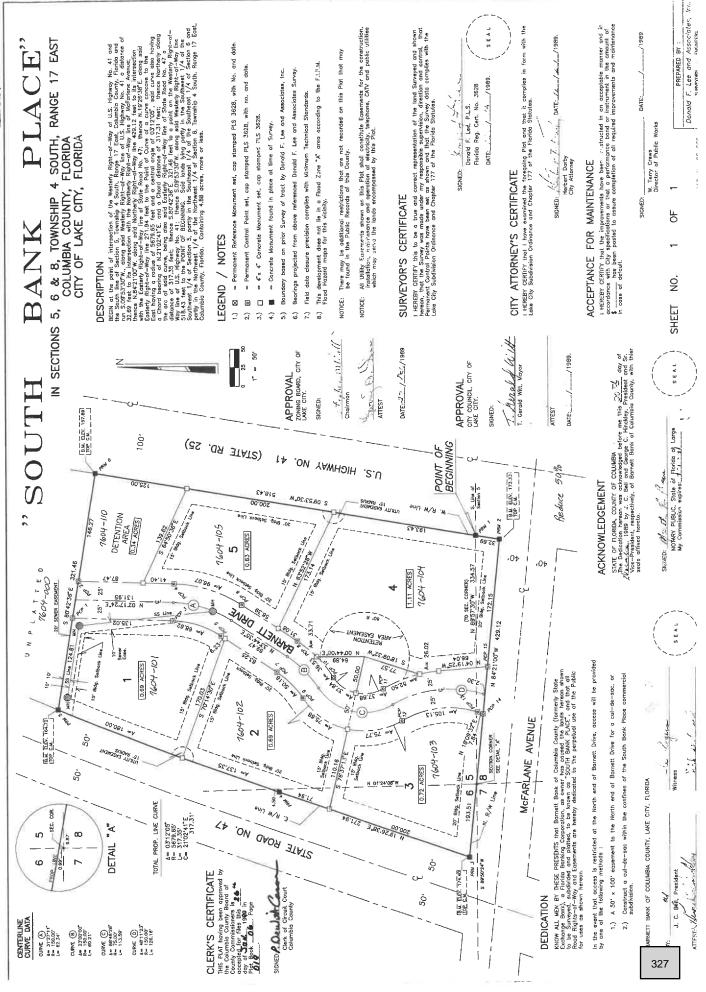


Home ∨ *P&Z/HPA

Updates are being made weekly. If you have any issues please email gis@lcfla.com



Updates are being made weekly. If you have any issues please email gis@lcfla.com





A.

GROWTH MANAGEMENT

205 North Marion Ave. Lake City, FL 32055 Telephone: (386)719-5750

E-Mail:

growthmanagement@lcfla.com

FOR PLANN	ING USE ONLY	
	# <u>SPR2-3-10</u>	
Application	Fee \$200.00	
Receipt No	2023-00042843	
Filing Date_		_
Completene	ss Date	

Site Plan Application

A.	PRO	JECT INFORMATION
	1.	Project Name: Aspire Deutal Addition
	2.	Address of Subject Property: 1788 SW Barnett Way, Lake Lity
	3.	Parcel ID Number(s): 05-65-17-07404-102
	4.	Future Land Use Map Designation: Commercial
	5.	Zoning Designation: CI
	6.	Acreage: 0.689
	7.	Existing Use of Property: Duntal office
	8.	Proposed use of Property: Dental office
	9.	Time of Davidonment (Check All That Apply)
	٠.	Increase of floor area to an existing structure: Total increase of square footage <u>199</u>
		() New construction: Total square footage
		Relocation of an existing structure: To square footage
		() Nelocution of the original of
B.	ADD	LICANT INFORMATION
D.		And Chatas Agent
	1.	Name of Applicant(s): Carp! Cnadwick, PE Title: Livil Engreer
	۷.	Company name (if applicable):
		Mailing Address: 1208 Sw Pair Lox Glun
		City: Lake C1 by State: FL Zip: 32025
		Telephone: (307) Laso 1772 Fax: () Email: Copenyor amail. lom
		PLEASE NOTE: Florida has a very broad public records law. Most written communications to
		or from government officials regarding government business is subject to public records
		requests. Your e-mail address and communications may be subject to public disclosure.
	3	If the applicant is agent for the property owner*.
	J.	Property Owner Name (title holder): Affiliated Property Munagement
		Mailing Address 1450 6 NW 11th Dlace
		City: Newberch State: FL Zip: 32469
		Telephone: (38% 75% 383 Eax:) Email: aspire dental legman
		PLEASE NOTE: Florida has a very broad public records law. Most written communications to
		or from government officials regarding government business is subject to public records
		requests. Your e-mail address and communications may be subject to public disclosure.
		*Must provide an executed Property Owner Affidavit Form authorizing the agent to act on
		hebalf of the property owner.

C. ADDITIONAL INFORMATION

1.	Is there any additional contract for the sale of, or options to purchase, the subject property?
	If yes, list the names of all parties involved:
	If yes, is the contract/option contingent or absolute: □ Contingent □ Absolute
2.	Has a previous application been made on all or part of the subject property? □Yes ★No
	Future Land Use Map Amendment: Yes No
	Future Land Use Map Amendment Application No.
	Site Specific Amendment to the Official Zoning Atlas (Rezoning): □YesNo
	Site Specific Amendment to the Official Zoning Atlas (Rezoning) Application No.
	Variance:□YesNo
	Variance Application No
	Special Exception:
	Special Exception Application No.

D. ATTACHMENT/SUBMITTAL REQUIREMENTS

- ✓ 1. Vicinity Map Indicating general location of the site, abutting streets, existing utilities, complete legal description of the property in question, and adjacent land use.
- 2. Site Plan Including, but not limited to the following:
 - a. Name, location, owner, and designer of the proposed development.
 - b. Present zoning for subject site.
 - Location of the site in relation to surrounding properties, including the means of ingress and egress to such properties and any screening or buffers on such properties.
 - d. Date, north arrow, and graphic scale not less than one inch equal to 50 feet.
 - e. Area and dimensions of site (Survey).
 - f. Location of all property lines, existing right-of-way approaches, sidewalks, curbs, and gutters.
 - g. $\,$ Access to utilities and points of utility hook-up.
 - h. Location and dimensions of all existing and proposed parking areas and loading areas.
 - Location, size, and design of proposed landscaped areas (including existing trees and required landscaped buffer areas).
 - j. Location and size of any lakes, ponds, canals, or other waters and waterways.
 - k. Structures and major features fully dimensioned including setbacks, distances between structures, floor area, width of driveways, parking spaces, property or lot lines, and percent of property covered by structures.
 - l. Location of trash receptacles.
 - m. For multiple-family, hotel, motel, and mobile home park site plans:
 - i. Tabulation of gross acreage.
 - ii. Tabulation of density.
 - iii. Number of dwelling units proposed.
 - iv. Location and percent of total open space and recreation areas.
 - v. Percent of lot covered by buildings.

City of Lake City - Growth Management Department 205 North Marion Ave, Lake City, FL 32055 ◆ (386) 719-5750

- vi. Floor area of dwelling units.
- vii. Number of proposed parking spaces.
- viii. Street layout.
- ix. Layout of mobile home stands (for mobile home parks only).
- 3. Stormwater Management Plan—Including the following:
 - a. Existing contours at one foot intervals based on U.S. Coast and Geodetic Datum.
 - b. Proposed finished elevation of each building site and first floor level.
 - c. Existing and proposed stormwater management facilities with size and grades.
 - d. Proposed orderly disposal of surface water runoff.
 - e. Centerline elevations along adjacent streets.
 - f. Water management district surface water management permit.
- ✓4. Fire Department Access and Water Supply Plan: The Fire Department Access and Water Supply Plan must demonstrate compliance with Chapter 18 of the Florida Fire Prevention Code, be located on a separate signed and sealed plan sheet, and must be prepared by a professional fire engineer licensed in the State of Florida. The Fire Department Access and Water Supply Plan must contain fire flow calculations in accordance with the Guide for Determination of Required Fire Flow, latest edition, as published by the Insurance Service Office ("ISO") and/or Chapter 18, Section 18.4 of the Florida Fire Prevention Code, whichever is greater.
- Concurrency Impact Analysis: Concurrency Impact Analysis of impacts to public facilities. For commercial and industrial developments, an analysis of the impacts to Transportation, Potable Water, Sanitary Sewer, and Solid Waste impacts are required.
- 6. Comprehensive Plan Consistency Analysis: An analysis of the application's consistency with the Comprehensive Plan (analysis must identify specific Goals, Objectives, and Policies of the Comprehensive Plan and detail how the application complies with said Goals, Objectives, and Policies).
- Legal Description with Tax Parcel Number (In Word Format).
- ✓8. Proof of Ownership (i.e. deed).
- Agent Authorization Form (signed and notarized).
- 10. Proof of Payment of Taxes (can be obtained online via the Columbia County Tax Collector's Office).
- 11. Fee. The application fee for a Site and Development Plan Application is \$200.00. No application shall be accepted or processed until the required application fee has been paid.

NOTICE TO APPLICANT

All eleven (11) attachments are required for a complete application. Once an application is submitted and paid for, a completeness review will be done to ensure all the requirements for a complete application have been met. If there are any deficiencies, the applicant will be notified in writing. If an application is deemed to be incomplete, it may cause a delay in the scheduling of the application before the Planning & Zoning Board.

A total of ten (10) copies of proposed site plan application and all support materials must be submitted along with a PDF copy on a CD. See City of Lake City submittal guidelines for additional submittal requirements.

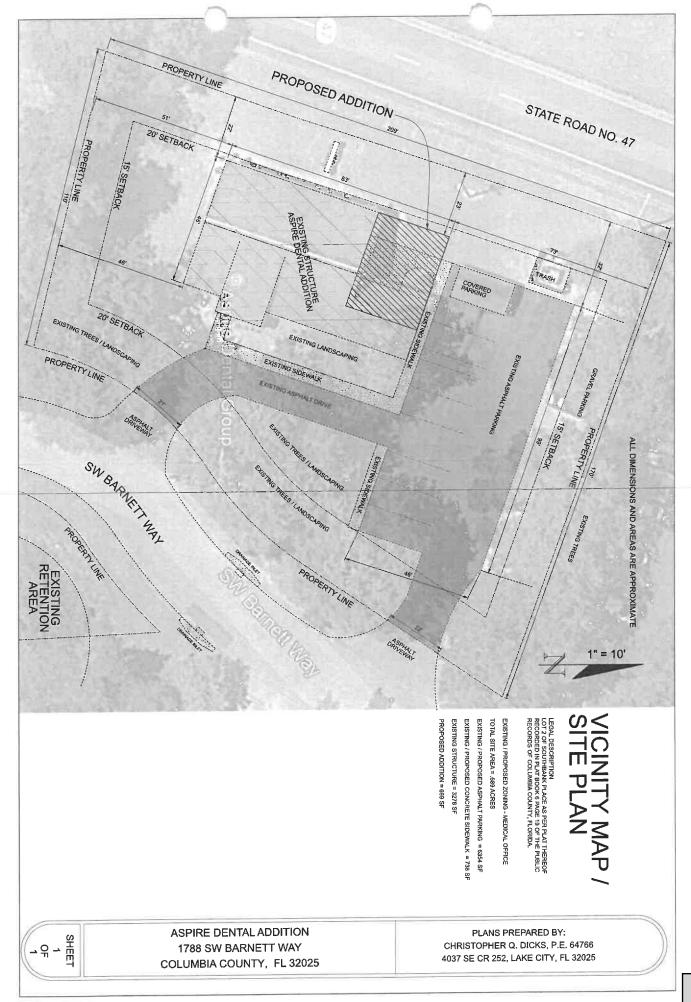
THE APPLICANT ACKNOWLEDGES THAT THE APPLICANT OR AGENT MUST BE PRESENT AT THE PUBLIC HEARING BEFORETHE PLANNING AND ZONING BOARD, AS ADOPTED IN THE BOARD RULES AND PROCEDURES, OTHERWISE THE REQUEST MAY BE CONTINUED TO A FUTURE HEARING DATE.

I hereby certify that all of the above statements and statements contained in any documents or plans submitted herewith are true and accurate to the best of my knowledge and belief.

(m) Drive	Digitally signed by Carol Chadwick
Applicant/Agent Name (Type or Print)	DN: c=US,
	O=Florida,
	dnQualifier=A014
Applicant/Agent Signature	* 10D0000017EB 6 Dte
	924CE0005954C, cn=Carol
Applicant/Agent Name (Type or Print)	Chadwick
	Date: 2023.03.31
Applicant/Agent Signature	12:30:53 -04'00' _{Date}
STATE OF FLORIDA COUNTY OF	
The foregoing instrument was acknowledged before	e me thisday of, 20, by (name of person acknowledging).
	Signature of Notary
(NOTARY SEAL or STAMP)	Printed Name of Notary
Personally Known OR Produced Identification Type of Identification Produced	_

City of Lake City - Growth Management Department 205 North Marion Ave, Lake City, FL 32055 ◆ (386) 719-5750





CAROLCHADWICK, P.E.

Civil Engineer
1208 S.W. Fairfax Glen
Lake City, FL 32025
307.680.1772
ccpewyo@gmail.com
www.carolchadwickpe.com

March 31, 2023

re: Aspire Dental Addition Drainage Memo

Per ERP-4-88-00236, each lot is permitted for 40% impervious area. Please refer to site plan for impervious area summary.

Lot size is 0.689 acres or 30013 s.f. Total impervious surface with this addition will be 11069 s.f. Total allowed impervious surfaces is 12005 s.f.

The proposed design and construction of this site shall not cause adverse impacts to:

- Existing surface water storage
- Conveyance capabilities
- Water quantity
- Flooding conditions
- Minimum flows and levels established by the State of Florida
- Water quality

Please contact me at 307.680.1772 if you have any questions.

Respectfully,



Digitally signed by Carol Chadwick DN: c=US, o=Florida, dnQualifier=A01410D0000017EB6 D924CE0005954C, cn=Carol Chadwick Date: 2023.03.31 12:30:30 -04'00'

Carol Chadwick, P.E.

CC Job #FL23 I 05

CAROL CHADWICK, P.E.

Civil Engineer

1208 S.W. Fairfax Glen
Lake City, FL 32025

307.680.1772

ccpewyo@gmail.com

www.carolchadwickpe.com

March 31, 2023

re: Aspire Dental Addition Fire Flow Report

The additional of 699 s.f. of building area will not require additional fire flow from the existing hydrant. Hydrant is located on SW Barnett Way on the northeast corner of the site.

Please contact me at 307.680.1772 if you have any questions.

Respectfully,



Digitally signed by Carol Chadwick DN: c=US, o=Florida, dnQualifier=A01410D0000017EB6 D924CE0005954C, cn=Carol Chadwick

Carol Chadwick, P.E.

Date: 2023.03.31 12:30:19 -04'00'

CAROL CHADWICK, P.E.

Civil Engineer

1208 S.W. Fairfax Glen

Lake City, FL 32025

307.680.1772

ccpewyo@gmail.com

www.carolchadwickpe.com

March 31, 2023

re: Aspire Dental Addition Concurrency Impact Analysis

The site is an existing dental office. The business currently utilizes public sewer and water systems.

Criteria for analyses:

- Trip generation was calculated per the ITE Trip Generation Manual, 9th edition, ITE code 720
- Potable Water Analysis per Chapter 64E-6.008 Florida Administrative Code, Table 1
- Sanitary Sewer Analysis Chapter 64E-6.008 Florida Administrative Code, Table 1
- Environmental Engineering: Tampa Typical Solid Waste Generation Rates

Summary of analyses:

- Trip generation: 43.82 ADT \$ 5.93 Peak PM trips
- Potable Water: 695 gallons per day
- Potable Water: 695 gallons per day
- Solid Waste: 2.98 c.y. per week

See attached Concurrency Worksheet.

Please contact me at 307.680.1772 if you have any questions.

Respectfully,



Digitally signed by Carol Chadwick DN: c=US, o=Florida, dnQualifier=A01410D0000017EB6D 924CE0005954C, cn=Carol Chadwick Date: 2023.03.31 12:30:08 -04'00'

Carol Chadwick, P.E.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

CC Job #FL23105

REVISED CONCURRENCY WORKSHEET

Trip Generation Analysis

ITE Code	ITE Use	ADT Multiplier	PM Peak Multiplier	KSF	Total ADT	Total PM Peak
720	Medical Dental Office	11.01	1.49	3.98	43.82	5.93
*Per emplo	yee					

Potable Water Analysis

Ch. 64E-6.008, F.A.C. Use	Ch. 64E-6.008, F.A.C. Gallons Per Day (GPD)	Ch. 64E-6.008, F.A.C. Multiplier*	Total (Gallons Per Day)
Medical Dental Office	250 + 15	250 + 13	695.00

^{*} Multiplier is based upon Ch. 64E.6008, Florida Administrative Code and can very from square footage, number of employees, number of seats, or etc. See Ch. 64E-6.008, F.A.C. to determine multiplier. (2 PRACTITIONERS & 13 EMPLOYEES)

Sanitary Sewer Analysis

Ch. 64E-6.008, F.A.C. Use	Ch. 64E-6.008, F.A.C. Gallons Per Day (GPD)	Ch. 64E-6.008, F.A.C. Multiplier*	Total (Gallons Per Day)
Medical Dental Office	250 + 15	250 + 13	695.00

^{*} Multiplier is based upon Ch. 64E.6008, Florida Administrative Code and can very from square footage, number of employees, number of seats, or etc. See Ch. 64E-6.008, F.A.C. to determine multiplier. (2 PRACTITIONERS & 13 EMPLOYEES)

Solid Waste Analysis

Use	Tons Per 100 s.f.	S.F.	Total (c.y. per week)
Medical Office	1.50	3977.00	2.98

CAROL CHADWICK, P.E.

Civil Engineer

1208 S.W. Fairfax Glen
Lake City, FL 32025
307.680.1772
ccpewyo@gmail.com
www.carolchadwickpe.com

March 31, 2023

re: Aspire Dental Addition Comprehensive Plan Consistency Analysis

The Aspire Dental Addition proposed site plan consistent with Lake City's Comprehensive Plan.

Future Land Use Element

GOAL I - IN RECOGNITION OF THE IMPORTANCE OF CONSERVING THE NATURAL RESOURCES AND ENHANCING THE QUALITY OF LIFE, THE CITY SHALL DIRECT DEVELOPMENT TO THOSE AREAS WHICH HAVE IN PLACE, OR HAVE AGREEMENTS TO PROVIDE, THE LAND AND WATER RESOURCES, FISCAL ABILITIES AND SERVICE CAPACITY TO ACCOMMODATE GROWTH IN AN ENVIRONMENTALLY ACCEPTABLE MANNER.

• Objective I.I The City shall continue to direct future population growth and associated urban development to urban development areas as established within this comprehensive plan.

Consistency: The subject property is an addition to an existing dental office.

• Policy 1.1.1 The City shall limit the location of higher density residential and high intensity commercial and industrial uses to areas adjacent to arterial or collector roads where public facilities are available to support such higher density or intensity. In addition, the City shall enable private subregional centralized potable water and sanitary sewer systems to connect to public regional facilities, in accordance with the objective and policies for the urban and rural areas within this future land use element of the comprehensive plan.

Consistency: The subject property is an addition to an existing dental office.

 Policy I. I. 2 The City's future land use plan map shall allocate amounts and mixes of land uses for residential, commercial, industrial, public and recreation to meet the needs of the existing and projected future populations and to locate urban land uses in a manner where public facilities may be provided to serve such urban land uses. Urban land uses shall be herein defined as residential, commercial and industrial land use categories.

Consistency: The commercial site is zoned Cl.

 Policy I.I.3 The City's future land use plan map shall base the designation of residential, commercial and industrial lands depicted on the future land use plan map upon acreage which can be reasonable expected to develop by the year 2023.

Consistency: The subject property is an addition to an existing dental office.

 Policy 1.1.4 The City shall continue to maintain standards for the coordination and siting of proposed urban development near agricultural or forested areas, or environmentally sensitive

CAROL CHADWICK, P.E. Page 2

areas (including but not limited to wetlands and floodplain areas) to avoid adverse impact upon existing land uses.

Consistency: The proposed use of the subject property is consistent with commercial properties and will not have any adverse environmental impacts on the existing land uses.

 Policy I.1.5 The City shall continue to regulate and govern future urban development within designated urban development areas in conformance with the land topography and soil conditions, and within an area which is or will be served by public facilities and services.

Consistency: No impacts to adjacent land topography or soil conditions will result due to a zooming or land use change of the subject property.

• Policy I. I. G The City's land development regulations shall be based on and be consistent with the following land use classifications and corresponding standards for densities and intensities within the designated urban development areas of the City. For the purpose of this policy and comprehensive plan, the phrase "other similar uses compatible with" shall mean land uses that can co-exist in relative proximity to other uses in a stable fashion over time such that no other uses within the same land use classification are negatively impacted directly or indirectly by the use.

Consistency: The proposed commercial development is compatible with other similar uses in the area and can co-exist without negative impacts to other uses in relative proximity to the development over time.

Please contact me at 307.680.1772 if you have any questions.

Respectfully,



Digitally signed by Carol Chadwick DN: c=US, o=Florida, dnQualifier=A01410D0000017EB6 D924CE0005954C, cn=Carol Chadwick

Carol Chadwick, P.E.

Date: 2023.03.31 12:29:45 -04'00'

PARCEL: 05-4S-17-07604-102

DESCRIPTION:

LOT 2 OF SOUTHBANK PLACE AS PER PLAT THEREOF RECORDED IN PLAT BOOK 6 PAGE 19 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA

Columbia County Property Appraiser

Jeff Hampton

Parcel: 3 05-48-17-07604-102 (28480) 🕞

Owner & Pi	roperty Info		Result: 1 of 0		
Owner	AFFILIATED PROPERTY MANAGE 14506 NW 11TH PLACE NEWBERRY, FL 32669	SEMENT LLC			
Site	1788 SW BARNETT WAY, LAKE CITY				
Description*	LOT 2 SOUTH BANK PLACE. 768-52 WD 1466-471,	21, 859-1397, 880-205	59, CD 884-2282		
Area	0.689 AC	S/T/R	05-4S-17		
Use Code**	PROFESS SVC/BLD (1900)	Tax District	1		

The <u>Description</u> above is not to be used as the Legal Description for this percel in any legal transaction.

"The <u>Use Code</u> is a Ft. Dept. of Revenue (DOR) code and is not maintained by the Property Appraiser's office.

Please contact your city or county Planning & Zoning office for specific zoning information.

Property & As	sessment Values			
2022 Certified Values		2023 Working Values		
Mkt Land	\$67,626	Mkt Land	\$67,626	
Ag Land	\$0	Ag Land	\$0	
Building	\$195,579	Building	\$190,220	
XFOB	\$8,551	XFOB	\$8,551	
Just	\$271,756	Just	\$266,397	
Class	\$0	Class	\$0	
Appraised	\$271,756	Appraised	\$266,397	
SOH Cap [?]	\$0	SOH Cap [7]	\$0	
Assessed	\$271,756	Assessed	\$266,397	
Exempt	\$0	Exempt	\$0	
Total Taxable	county:\$271,756 city:\$271,756 other:\$0 school:\$271,756	Total Taxable	county:\$266,397 city:\$266,397 ather:\$0 school:\$266,397	



Sales History						
Sale Date	Sale Price	Book/Page	Deed	V/I	Qualification (Codes)	RCode
4/19/2022	\$100	1486/0471	WD	1	U	11
5/18/1999	\$68,000	0880/2059	WD	V	U	01
5/28/1998	\$68,000	0859/1397	WD	V	Q	
10/27/1992	\$48,000	0768/0521	WD	V	U	04

ilding Characteristics					
Bldg Sketch	Description*	Year Bit	Base SF	Actual SF	Bldg Value
Sketch	OFFICE MED (5200)	1999	3136	3318	\$190,220

Extra Features &	Out Buildings (Codes)				
Code	Desc	Year Bit	Value	Units	Dims
0164	CONC BIN	1999	\$285.00	38.00	0 x 0
0260	PAVEMENT-ASPHALT	1999	\$6,175.00	6861.00	0 x 0
0166	CONC.PAVMT	1999	\$1,691.00	1127.00	0 x 0
0169	FENCE/WOOD	2012	\$100.00	1.00	0 x 0
0109	CARPORT F	2012	\$300.00	1.00	0 x 0

Land Br	eakdown				
Code	Desc	Units	Adjustments	Eff Rate	Land Value
1910	MEDIC OFF (MKT)	30,056,000 SF (0,689 AC)	1.0000/1.0000 1.0000/ /	\$2 /SF	\$67,626

Search Result: 1 of 0

© Columbia County Property Appraiser | Jeff Hampton | Lake City, Florida | 386-758-1083

by: GrizzlyLogic.com



Department of State / Division of Corporations / Search Records / Search by Entity Name /

Detail by Entity Name

Florida Limited Liability Company
AFFILIATED PROPERTY MANAGEMENT, LLC

Filing Information

Document Number

L16000074743

FEI/EIN Number

59-3529574

Date Filed

04/18/2016

Effective Date

08/19/1998

State

FL

Status

ACTIVE

Last Event

CONVERSION

Event Date Filed

04/18/2016

Event Effective Date

NONE

Principal Address

14506 N.W. 11TH PL.

NEWBERRY, FL 32669

Mailing Address

14506 N.W. 11TH PL.

NEWBERRY, FL 32669

Registered Agent Name & Address

HARVEY, FRANKIE J

14506 N.W. 11TH PL.

NEWBERRY, FL 32669

Name Changed: 04/28/2017

Authorized Person(s) Detail

Name & Address

Title manager

HARVEY, FRANKIE J

14506 N.W. 11TH PL.

NEWBERRY, FL 32669

Annual Reports

Report Year

Filed Date

2020

02/04/2020

2021	02/10/2021
2022	03/31/2022

Document Images

03/31/2022 - ANNUAL REPORT	View image in PDF format
02/10/2021 ANNUAL REPORT	View image in PDF format
02/04/2020 ANNUAL REPORT	View image in PDF format
05/10/2019 – ANNUAL REPORT	View image in PDF format
04/27/2018 - ANNUAL REPORT	View image in PDF format
04/28/2017 ANNUAL REPORT	View image in PDF format
04/18/2016 - Florida Limited Liability	View image in PDF format

da Department State Division of Corporations



GROWTH MANAGEMENT DEPARTMENT 205 North Marion Ave, Lake City, FL 32055

Phone: 386-719-5750

E-mail: growthmanagement@lcfla.com

AGENT AUTHORIZATION FORM

AGENTACTIONIZATION COM				
1. Robert Horry	(owner name), owner of property parcel			
number	(parcel number), do certify that			
the below referenced person(s) listed on this form is an officer of the corporation; or, partner as defined person(s) is/are authorized to sign, speak a relating to this parcel.	ined in Florida Statutes Chapter 468, and the			
Printed Name of Person Authorized	Signature of Authorized Person			
1. Gery JOHNSON	1. Lay Johnson			
2. Heren Tarr	2. A. Dan			
3. Carol Mudwick, PE	3.			
4.	4.			
5.	5.			
I, the owner, realize that I am responsible for all with, and I am fully responsible for compliance w Development Regulations pertaining to this parcel of the time the person(s) you have authorized officer(s), you must notify this department in writing authorization form, which will supersede all previous unauthorized persons to use your name and/or lie	is/are no longer agents, employee(s), or one of the changes and submit a new letter of the changes are changed as the changes			
	3.14.2023			
Owner Signature (Notarized)	Date			
NOTARY INFORMATION: STATE OF: FOR IM COUNTY OF:	COLUMBIA			
The above person, whose name is Robert personally appeared before me and is known by (type of I.D.) # 6/0.770.54.447-0 on Fupl	me or has produced identification			
NOTARY'S SIGNATURE	(Seal/Stamp)			
	CHARLENE N. PITMAN Notary Public - State of Florida Commission # HH 030304 My Comm. Expires Aug 9, 2024			

Columbia County Tax Collector

Tax Record

Last Update: 3/31/2023 11:02:59 AM EDT

Register for eBill

Ad Valorem Taxes and Non-Ad Valorem Assessments

The information contained herein does not constitute a title search and should not be relied on as such.

	Account Number		Tax Ty	/pe	Tax	Year
	R07604-102		REAL ES	STATE	2	022
AFFIL	ng Address LATED PROPERTY			y Address RNETT LAKE (CITY	
14506	NW 11TH PLACE		GEO Numi			
NEWBEI	RRY FL 32669		054S17-0	07604-102		
	Exempt Amount		Taxable	Value		
	See Below		See Be	low		
NO EXI	tion Detail	001	e Code		scrow Code	ı
<u>Legal</u>	Description (clic	k for full d	<u>lescriptio</u>	<u>n)</u>		4 0 0 0
05-4S-	-17 1900/1900.69 A	cres LOT 2 S	SOUTH BANK	PLACE. 768	-521, 859-	.1397,
880-20	059, CD 884-2282,	WD 1466-471,				
		Ad Valo	rem Taxes			
	hand have	Rate	Assessed	Exemption	Taxable	Taxes
raxing A	Authority		Value	Amount	Value	Levied
CITY OF L		4.9000	271,756	0	\$271,756	\$1,331.60 \$2,123.77
	COUNTY COMMISSIONERS	7.8150	271,756	0	\$271,756	\$2,123.77
	COUNTY SCHOOL BOARD		071 756	0	\$271,756	\$203.28
DISCRETION	MARY	0.7480	271,756 271,756	0	\$271,756	\$896.52
LOCAL		3.2990	271,756	0	\$271,756	\$407.63
CAPITAL O		1.5000	271,756	0	\$271,756	\$91.53
	RIVER WATER MGT DIST E HOSPITAL AUTHORITY	0.3368 0.0001	271,756	0	\$271,756	\$0.03
	Total Millage	18.5989	To	otal Taxes	\$	5,054.36
	N	on-Ad Valor	em Assess	ments		
Code						Amount
XLC	F CITY FIRE ASS	ESSMENT				\$519.27
			Tota	L Assessment	ts	\$519.27
				& Assessmen	te S	55,573.63
			Taxes	& Assessmen	-	75,515.05
			Taxes If Paid			ount Due

Date Paid	Transaction	Receipt	Item	Amount Paid
11/21/2022	PAYMENT	1501182.0001	2022	\$5,350.68

Prior Years Payment History

	Prior Year Taxes Due	
NO DELINQUENT TAXES		

File Attachments for Item:

v. SPR24-06, Petition submitted by Christopher A Gmuer, P.E.. (agent) for ERA Investments (owner), for a Site Plan Review for Lake City Hotels Phase 2, in the Commercial Intensive Zoning District, and located on parcel 02582-002, which is regulated by the Land Development Regulations section 4.13.



04/00/0004

DEPARTMENT OF GROWTH MANAGEMENT

205 North Marion Avenue Lake City, Florida 32055 Telephone: (386) 719-5750 growthmanagement@lcfla.com

REVIEW REPORT TO PLANNING AND ZONING, BOARD OF ADJUSTMENT AND HISTORICAL COMMITTEES' BY STAFF FOR SITE PLAN REVIEW, SPECIAL EXCEPTIONS, VARIANCES, COMPREHENSIVE PLAN AMENDMENTS/ ZONING AND CERTIFICATE OF APPROPRIATENESS

Date:
Request Type: Site Plan Review (SPR) Special Exception (SE) Variances (V)
Comprehensive Plan Amendment/Zoning (CPA/Z) Certificate of Appropriateness (COA)
Project Number: SPR 24-06
Project Name: Lake City Hotels Phase 2
Project Address: TBA
Project Parcel Number: 02582-002
Owner Name: ERA Investments, LLC
Owner Address: 162 NW Birdie PI, Lake City, FL 32055
Owner Contact Information: Telephone Number: 386-984-0732 Email: drpatel@primarycaremedic.com
Owner Agent Name: Christopher A. Gmuer, P.E.
Owner Agent Address: 2603 NW 13th Street, Box 314, Gainesville, FL
Owner Agent Contact Information: Telephone: 352-281-4928 Email: chrisg@gmuereng.com

The City of Lake City staff has reviewed the application and documents provided for the above request and have determined the following.

Growth Management – Building Department, Planning and Zoning, Code Enforcement, Permitting

uilding Department: Reviewed by:	Date: 4/29/2024
No comments at this time	
anning and Zoning: Reviewed by: Rolest Angelo FODIED33898E4BE	Date:
er section 4.12.2.11, a hotel is a permitted use.	
usiness License: Reviewed by: Markall Saa	Date:4/29/2024
ill need to apply for a occupational license before o	pening.
ode Enforcement: Reviewed by: Marked Soma	Date: 4/29/2024
o liens, codes or violations	
ermitting: Reviewed by:	Date: 4/30/2024
permits will be required	

Utilities – Water, Sewer, Gas, Water Distribution/Collections, Customer Service

Water Department: Reviewed by: Mich Oshorn	Date: 4/29/2024
No comments at this time	
DocuSigned by:	4 (20 (2024
Sewer Department: Reviewed by: Loty Indian	Date: 4/30/2024
none	
Gas Department: Reviewed by: Stw. brww.	Date:
No comment.	
Water Distribution/Collection: Reviewed by: علاما المحافقة المحاف	Date: 5/17/2024
no comments	
Docusigned by:	5/15/2024
Customer Service: Reviewed by Slasta Pullam	Date:
A tap application will need to be submitted in order sewer, and/or natural gas services. The Utility fees upon approval of the tap application.	to request water, will be calculated

Public Safety – Public Works, Fire Department, Police Department

Public Works: Reviewed by:	Stew Brown 8857DOCE8F2F4B5	Date: 4/30/2024
No comment.		
Fire Department: Reviewed b	y: Joshua Wilinger 6AA758BA8BA46E	Date: 4/30/2024
I have no issues		
Police Department: Reviewed	by:	Date:

NOTE: Please provide separate pages for comments that will not fit in provided spaces and please label the pages for your department and for the project.

State and County-FDOT, Suwannee River Water Management, School Board, Columbia County FDOT: Reviewed by:__ Date: Date: 5/6/2024 Suwannee River Water Management: Reviewed by: Garntt Spinar The applicant has applied for a permit with SRWMD. The permit number is ERP-023-209165-4. Date: 4/30/2024 School Board: Reviewed by: keith Hatcher No comments at this time. Date: 5/9/2024 County: Reviewed by: Usad Williams No issues were identified by this office at this time. This comment is provided by the County Engineer based only on the information contained in the application provided. This response does not constitute the engineer's professional opinion with respect to the project and does not constitute approval of any committee or board for Columbia County. Such opinions and approvals, if any, shall be as provided by County code or regulations.

NOTE: Please provide separate pages for comments that will not fit in provided spaces and please label the pages for your department and for the project.



GROWTH MANAGEMENT

205 North Marion Ave. Lake City, FL 32055 Telephone: (386)719-5750

E-Mail:

growthmanagement@lcfla.com

FOR PLANNING USE ONLY				
Application # SPR24-06				
Application Fee: \$200.00				
Receipt No. 2024-00049916				
Filing Date 4/24/24				
Completeness Date 4/29/24				

Site Plan Application

A.	PF	PROJECT INFORMATION			
	1.	Project Name: Lake City Hotels Phase 2			
	2.	Address of Subject Property: N/A			
	3. Parcel ID Number(s): 35-38-16-02582-002 4. Future Land Use Map Designation: Commercial				
	5.	Zoning Designation: CI			
	6.	Acreage: 3.971			
	7. Existing Use of Property: Vacant				
	8. Proposed use of Property: Hotels				
	9. Type of Development (Check All That Apply):				
	Increase of floor area to an existing structure. Total increase of area is				
	The state of the s				
	Relocation of an existing structure: Total square footage				
_					
B.	APPLICANT INFORMATION				
	1.	Applicant Status			
	2.	Name of Applicant(s): Christopher A. Gmuer, PE			
	Company name (if applicable): Gmuer Engineering, LLC				
	Mailing Address: 2603 NW 13th Street, Box 314				
		City; Gainesville State: FL Zip: 32609			
		retephone: (332) 201-4928 Fax: Fax: Fmail: chrisq@pmuereog.com			
PLEASE NOTE: Florida has a very broad public records law Most written and the second state of the second s					
		by continuent unitidis reparding government hyginage is and in the			
	3.	requests. Your e-mail address and communications may be subject to public disclosure. If the applicant is agent for the property owner*.			
15.784		Property Owner Name (title holder): ERA Investments, LLC			
		Mailing Address: 162 NW Birdie PI			
		City: Lake City State: FL Zip: 32055			
		Telephone: (386) 984-0732 Fax: Email: drpatel@primarycaremedic.com			
PLEASE NOTE: Florida has a very broad public records law. Most viritten assessment of the public records law.					
		or nome government unicials regarding government business is subject to and the			
		Provide an executed Property Owner Affidavit Form authorizing the			
		behalf of the property owner.			

C. ADDITIONAL INFORMATION

1.	Is there any additional contract for the sale of, or options to purchase, the subject property				
	If yes, list the names of all parties involved: N/A				
	If yes, is the contract/option contingent or absolute: ☐ Contingent ☐ Absolute				
2.	Has a previous application been made on all or part of the subject property? □Yes ■No				
	Future Land Use Map Amendment: Yes No				
	Future Land Use Map Amendment Application No				
	Site Specific Amendment to the Official Zoning Atlas (Rezoning): No				
Site Specific Amendment to the Official Zoning Atlas (Rezoning) Application No.					
	Variance: □Yes ■No				
	Variance Application No.				
	Special Exception:				
	Special Exception Application No.				

D. ATTACHMENT/SUBMITTAL REQUIREMENTS

- 1. Vicinity Map Indicating general location of the site, abutting streets, existing utilities, complete legal description of the property in question, and adjacent land use.
- 2. Site Plan Including, but not limited to the following:
 - a. Name, location, owner, and designer of the proposed development.
 - b. Present zoning for subject site.
 - c. Location of the site in relation to surrounding properties, including the means of ingress and egress to such properties and any screening or buffers on such properties.
 - d. Date, north arrow, and graphic scale not less than one inch equal to 50 feet.
 - e. Area and dimensions of site (Survey).
 - f. Location of all property lines, existing right-of-way approaches, sidewalks, curbs, and gutters.
 - g. Access to utilities and points of utility hook-up.
 - h. Location and dimensions of all existing and proposed parking areas and loading areas.
 - i. Location, size, and design of proposed landscaped areas (including existing trees and required landscaped buffer areas).
 - j. Location and size of any lakes, ponds, canals, or other waters and waterways.
 - k. Structures and major features fully dimensioned including setbacks, distances between structures, floor area, width of driveways, parking spaces, property or lot lines, and percent of property covered by structures.
 - l. Location of trash receptacles.
 - m. For multiple-family, hotel, motel, and mobile home park site plans:
 - i. Tabulation of gross acreage.
 - ii. Tabulation of density.
 - Number of dwelling units proposed. iii.
 - iv. Location and percent of total open space and recreation areas.
 - v. Percent of lot covered by buildings.

City of Lake City - Growth Management Department 205 North Marion Ave, Lake City, FL 32055 ♦ (386) 719-5750

- vi. Floor area of dwelling units.
- vii. Number of proposed parking spaces.
- viii. Street layout.
- ix. Layout of mobile home stands (for mobile home parks only).
- 3. Stormwater Management Plan—Including the following:
 - a. Existing contours at one foot intervals based on U.S. Coast and Geodetic Datum.
 - b. Proposed finished elevation of each building site and first floor level.
 - c. Existing and proposed stormwater management facilities with size and grades.
 - d. Proposed orderly disposal of surface water runoff.
 - e. Centerline elevations along adjacent streets.
 - f. Water management district surface water management permit.
- 4. Fire Department Access and Water Supply Plan: The Fire Department Access and Water Supply Plan must demonstrate compliance with Chapter 18 of the Florida Fire Prevention Code, be located on a separate signed and sealed plan sheet, and must be prepared by a professional fire engineer licensed in the State of Florida. The Fire Department Access and Water Supply Plan must contain fire flow calculations in accordance with the Guide for Determination of Required Fire Flow, latest edition, as published by the Insurance Service Office ("ISO") and/or Chapter 18, Section 18.4 of the Florida Fire Prevention Code, whichever is greater.
- Concurrency Impact Analysis: Concurrency Impact Analysis of impacts to public facilities. For commercial and industrial developments, an analysis of the impacts to Transportation, Potable Water, Sanitary Sewer, and Solid Waste impacts are required.
- 6. Comprehensive Plan Consistency Analysis: An analysis of the application's consistency with the Comprehensive Plan (analysis must identify specific Goals, Objectives, and Policies of the Comprehensive Plan and detail how the application complies with said Goals, Objectives, and Policies).
- 7. Legal Description with Tax Parcel Number (In Word Format).
- 8. Proof of Ownership (i.e. deed).
- 9. Agent Authorization Form (signed and notarized).
- 10. Proof of Payment of Taxes (can be obtained online via the Columbia County Tax Collector's Office).
- 11. Fee. The application fee for a Site and Development Plan Application is \$200.00. No application shall be accepted or processed until the required application fee has been paid.

NOTICE TO APPLICANT

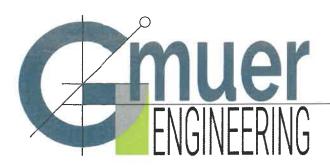
All eleven (11) attachments are required for a complete application. Once an application is submitted and paid for, a completeness review will be done to ensure all the requirements for a complete application have been met. If there are any deficiencies, the applicant will be notified in writing. If an application is deemed to be incomplete, it may cause a delay in the scheduling of the application before the Planning & Zoning Board.

A total of ten (10) copies of proposed site plan application and all support materials must be submitted along with a PDF copy on a CD. See City of Lake City submittal guidelines for additional submittal requirements.

THE APPLICANT ACKNOWLEDGES THAT THE APPLICANT OR AGENT MUST BE PRESENT AT THE PUBLIC HEARING BEFORETHE PLANNING AND ZONING BOARD, AS ADOPTED IN THE BOARD RULES AND PROCEDURES, OTHERWISE THE REQUEST MAY BE CONTINUED TO A FUTURE HEARING DATE.

I hereby certify that all of the above statements and statements contained in any documents or plans submitted herewith are true and accurate to the best of my knowledge and belief.

Janak Shukla, Manager, ERA Investments, LLC	
Applicant/Agent Name (Type or Print)	
JR. Shuhla.	11/16/23
Applicant/Agent Signature	Date
Applicant/Agent Name (Type or Print)	
Applicant/Agent Signature	Date
STATE OF FLORIDA COUNTY OF COLUMN DISTRICTORY	
The foregoing instrumen was a showledged before me this () MY COMMISSION EXPIRES 6-22-2027	day of Nov. 2023, by (name of person acknowledging).
(NOTARY SEALO STAMP) OF FLORIDA	Printed Name of Notary
Personally Known OR Produced identification Type of Identification Produced	
1 YPC OF INCIMITICATION I ROUGEOU	



2603 NW 13th St, Box 314 Gainesville, FL 32609 Ph. (352) 281-4928

gmuereng.com

April 25, 2024

Department of Growth Management, City of Lake City 205 North Marion Avenue, Lake City, Florida. 32055

Re: Lake City Hotels Phase 2

Dear Staff,

This package is submitted as a Site and Development Plan Application to Lake City. The project proposes the construction of a second hotel with associated parking and utility infrastructure at 3004 W US Hwy 90, Lake City, FL. The hotel will be internal to the site and will connect to the drive isles and utility mains constructed with Phase 1. Please see the list below of items included with this application.

Attachments:

- Fee in the amount of \$200.00
- · Site Plan Application
- Concurrency Impact Analysis
- · Comprehensive Plan Consistency Analysis
- Legal Description (word format)
- · Warranty Deed
- · Agent Authorization Form
- · Proof of Payment of Taxes
- · Stormwater Management Report
- Fire Department Access and Water Supply Plan
- SRWMD Permit
- · 2 Sets of Plans

Please let us know if you need any additional information for your review.

Sincerely, Gmuer Engineering, LLC Christopher A Gmuer, PE President

1 of 1

LAKE CITY HOTELS

PHASE 2

PROJECT INFORMATION PROJECT WARE LAKE CITY HOTHES. PHASE 2 al 2000 W UR 89 PARCELS 36-3614-0233-0402 OWNERS EAR MYSCHIERTS. LLC 1421 WAR BROWN BL LAKE CITY, FL 32086

CNL ENGINESY CHRISTOPHES A. GAMER, PH. GAMER, PH. 12TH ST BOX 314
GAMER BY STREET, PL. 12408

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SHEET INDEX C-000

SITE PLAN MAP

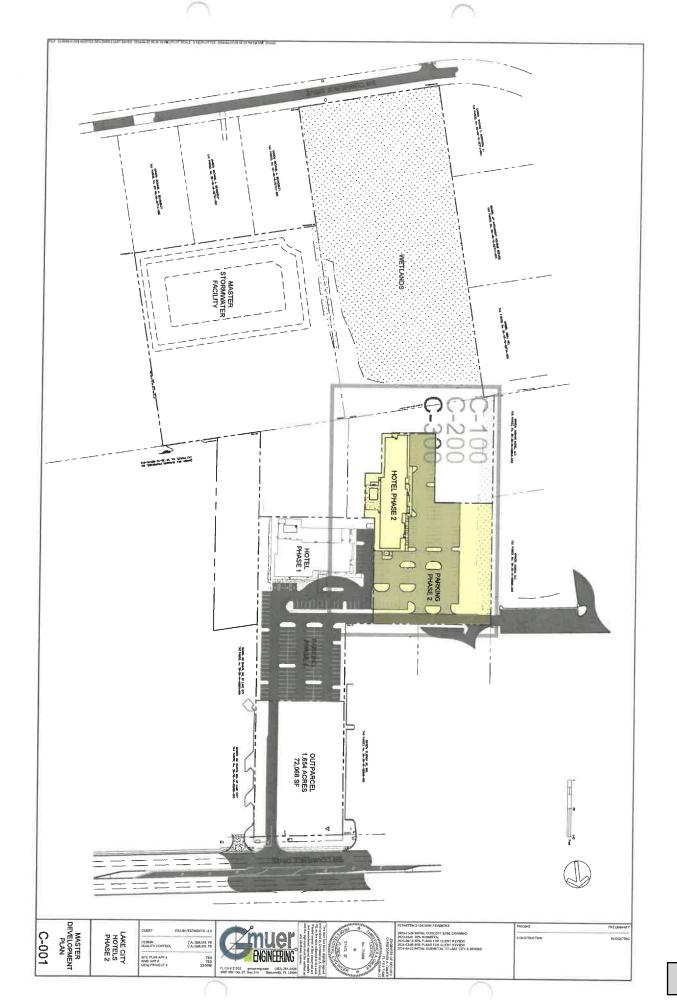
LAKE CITY HOTELS PHASE 2

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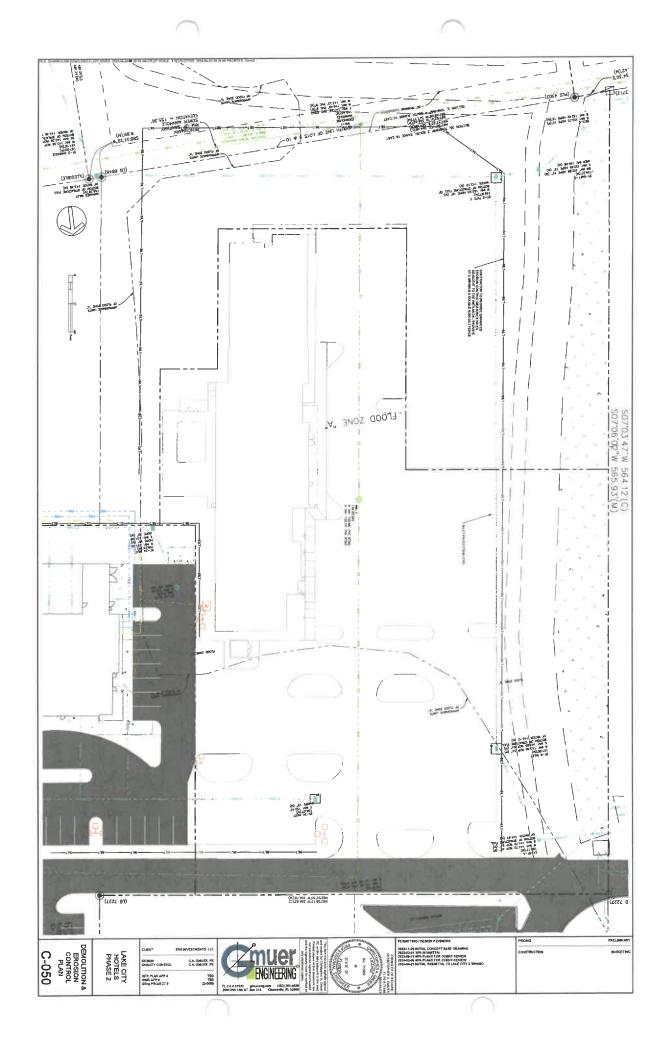
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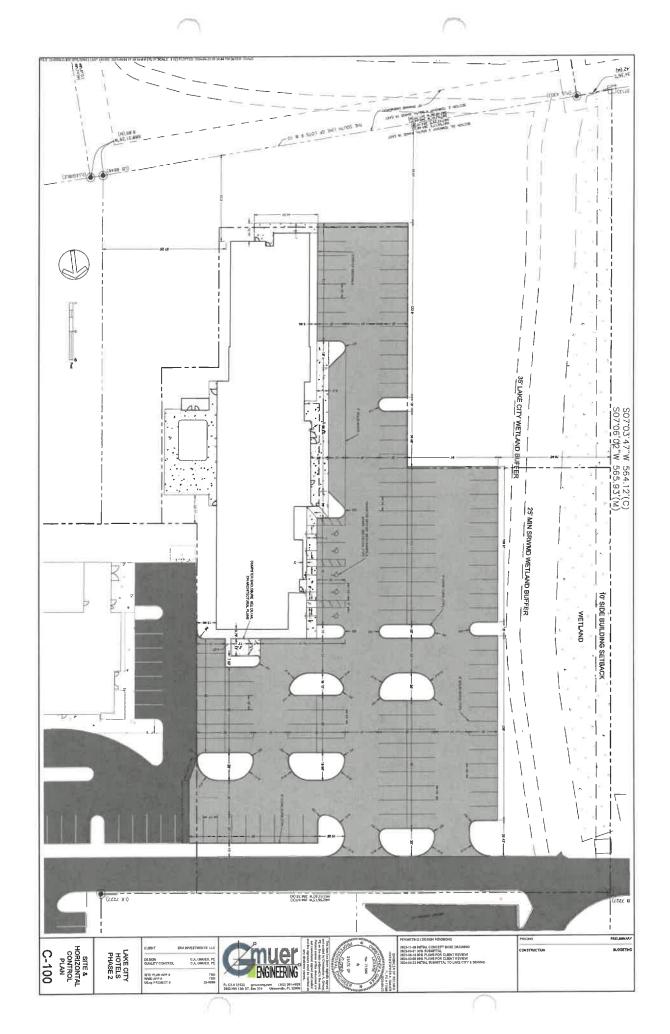
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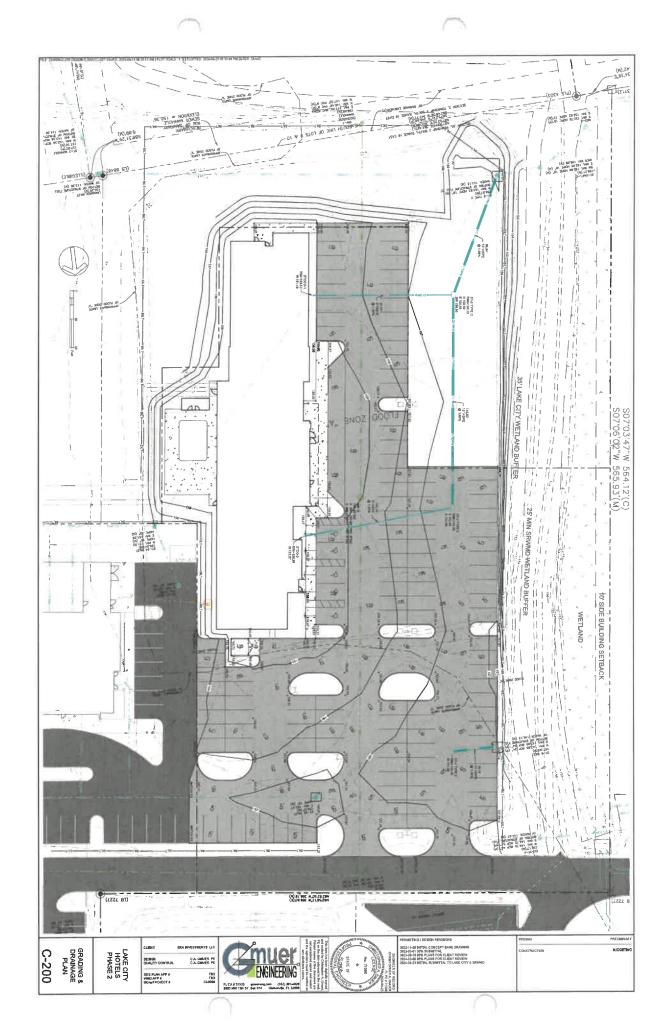


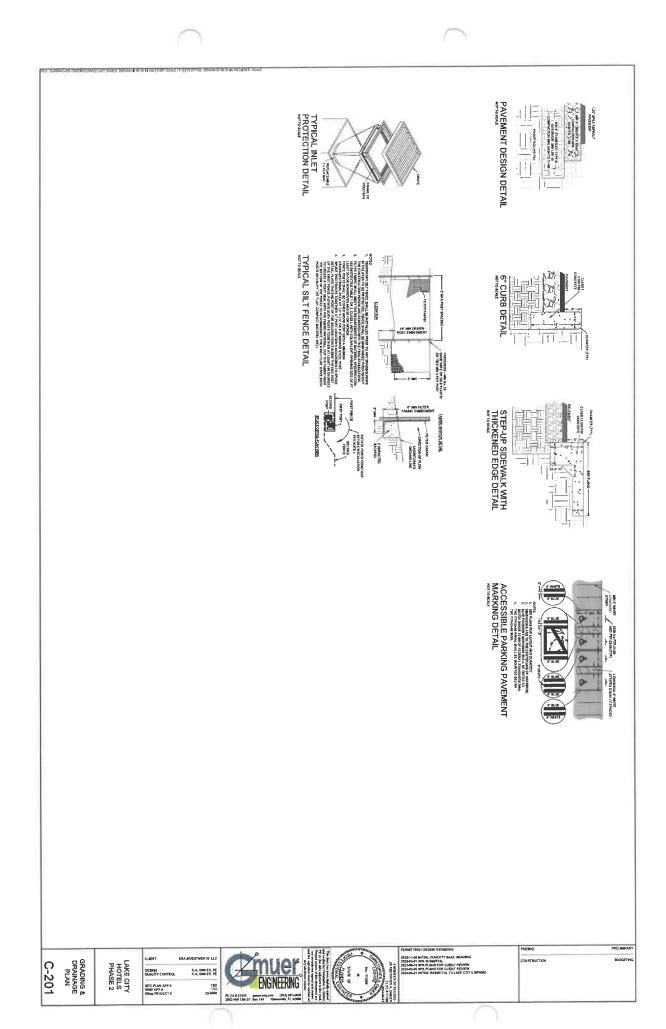


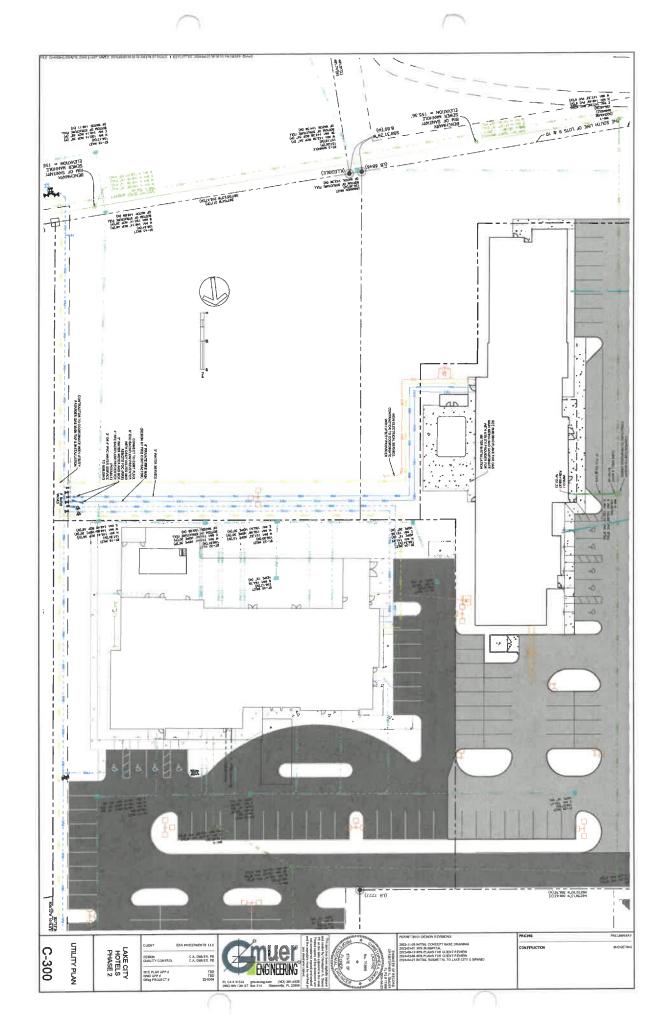
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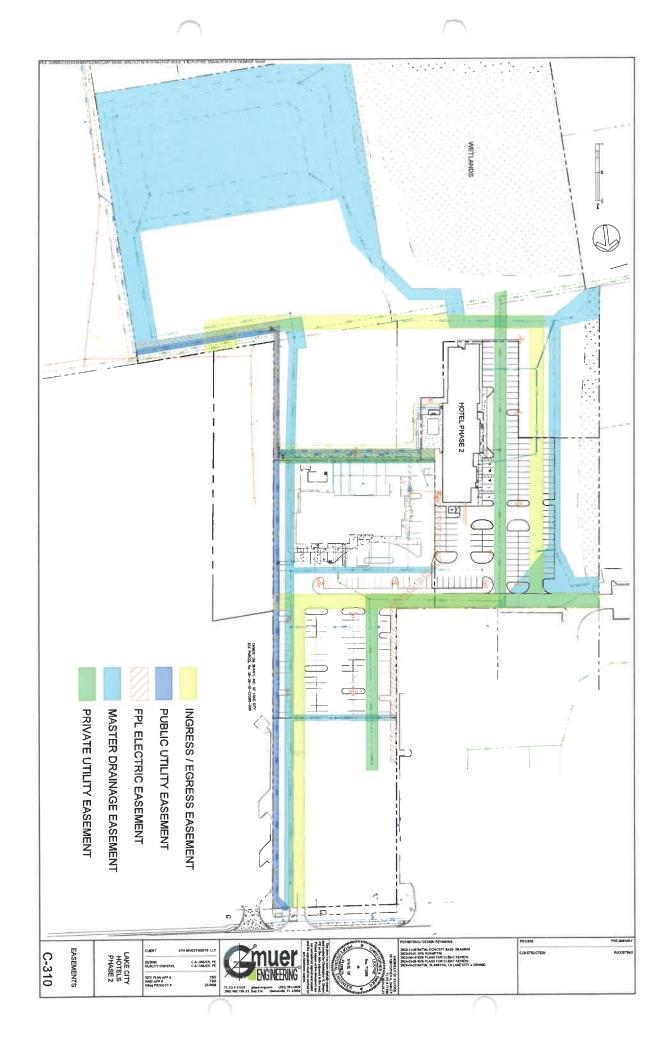


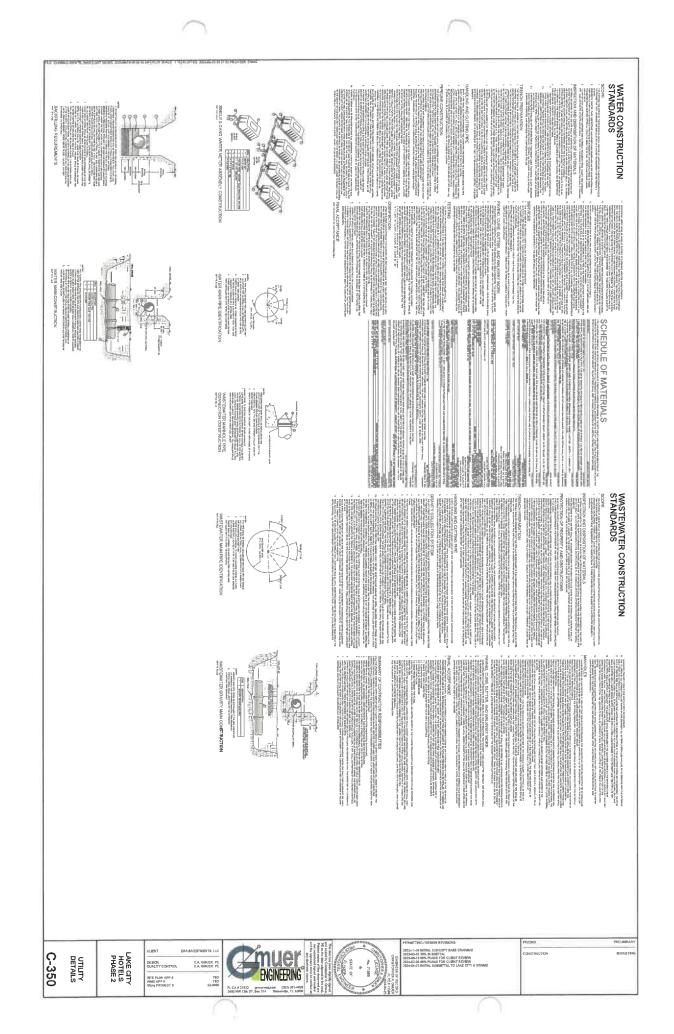


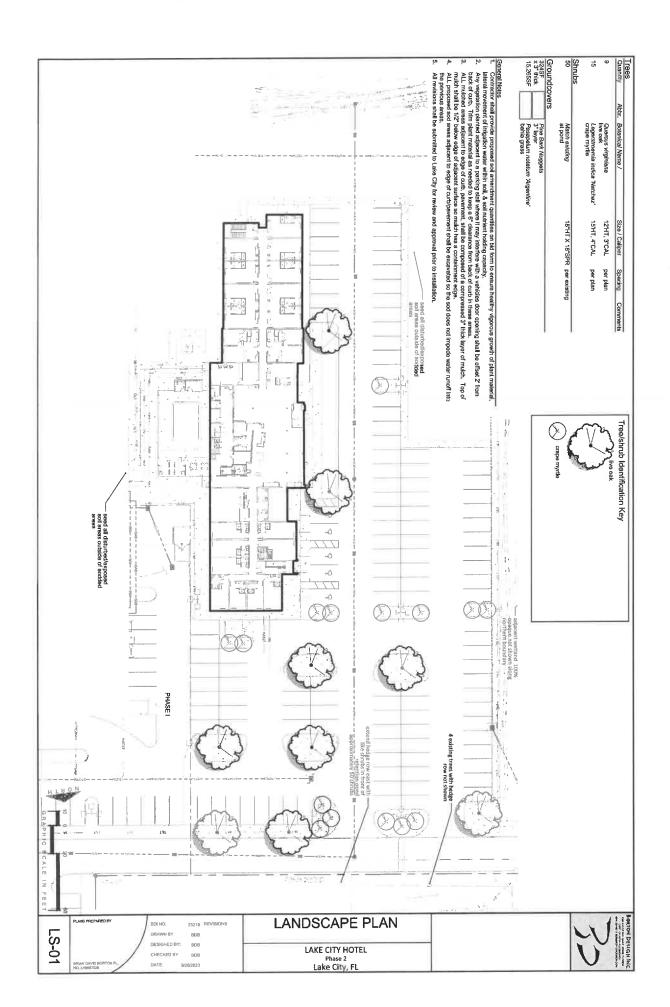


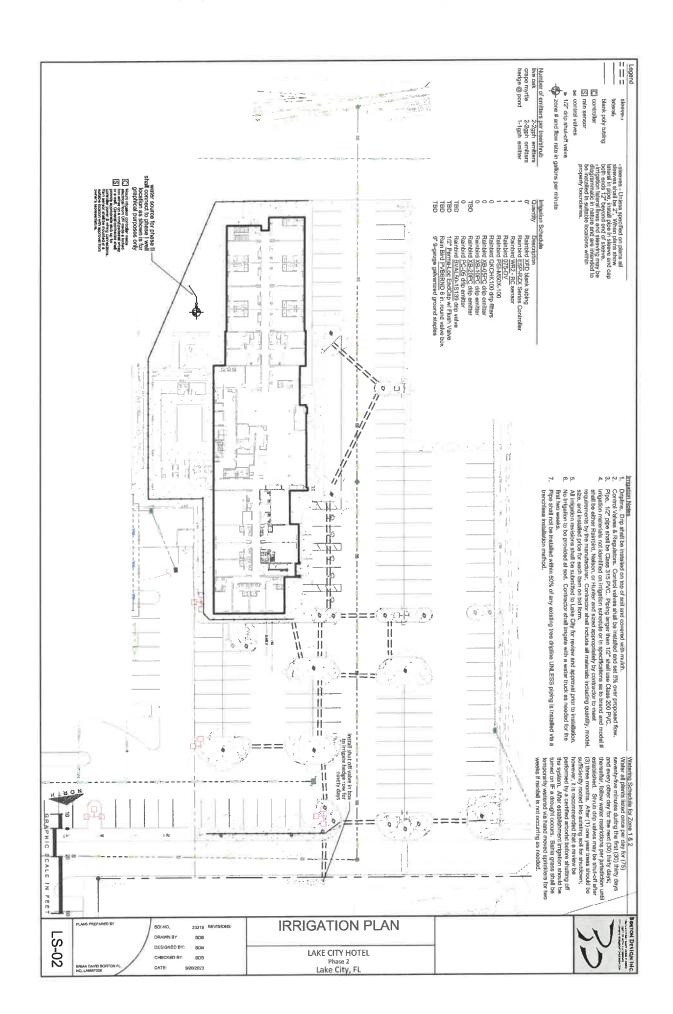


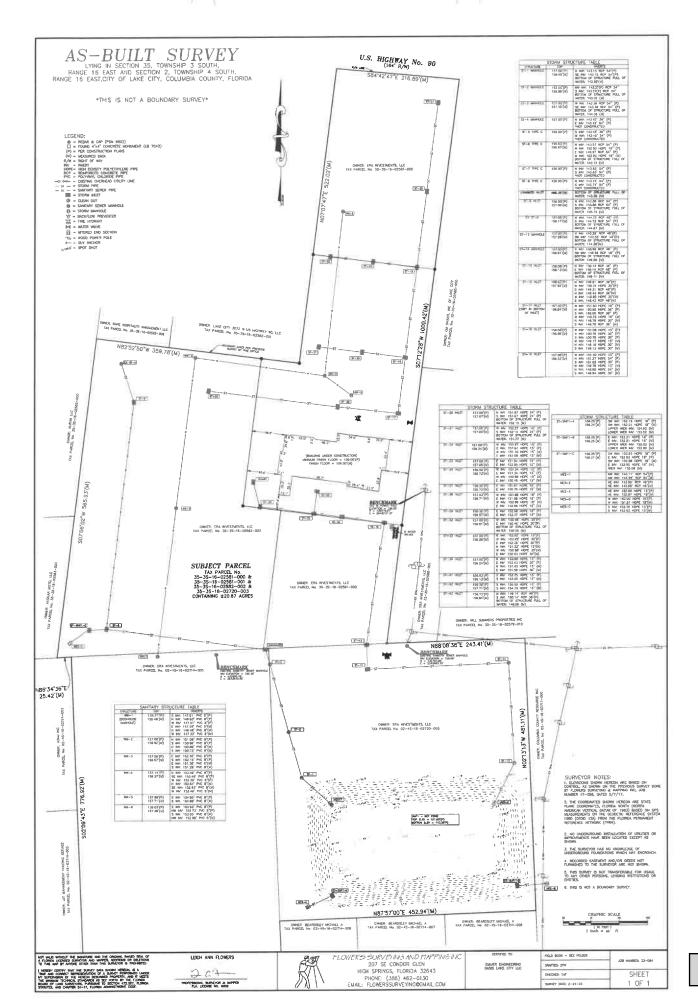


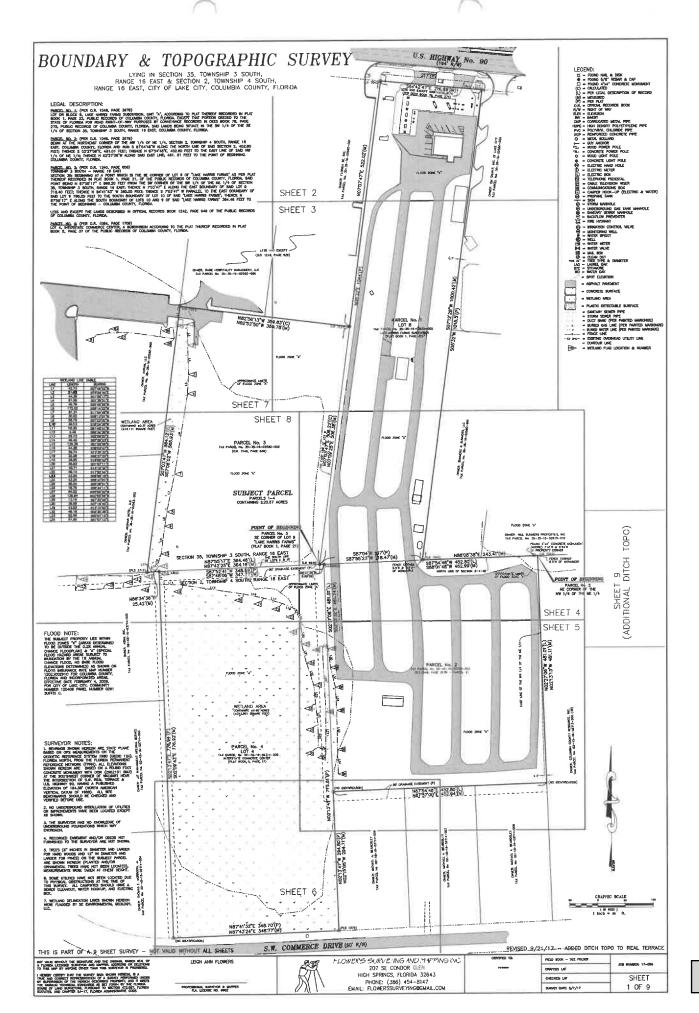


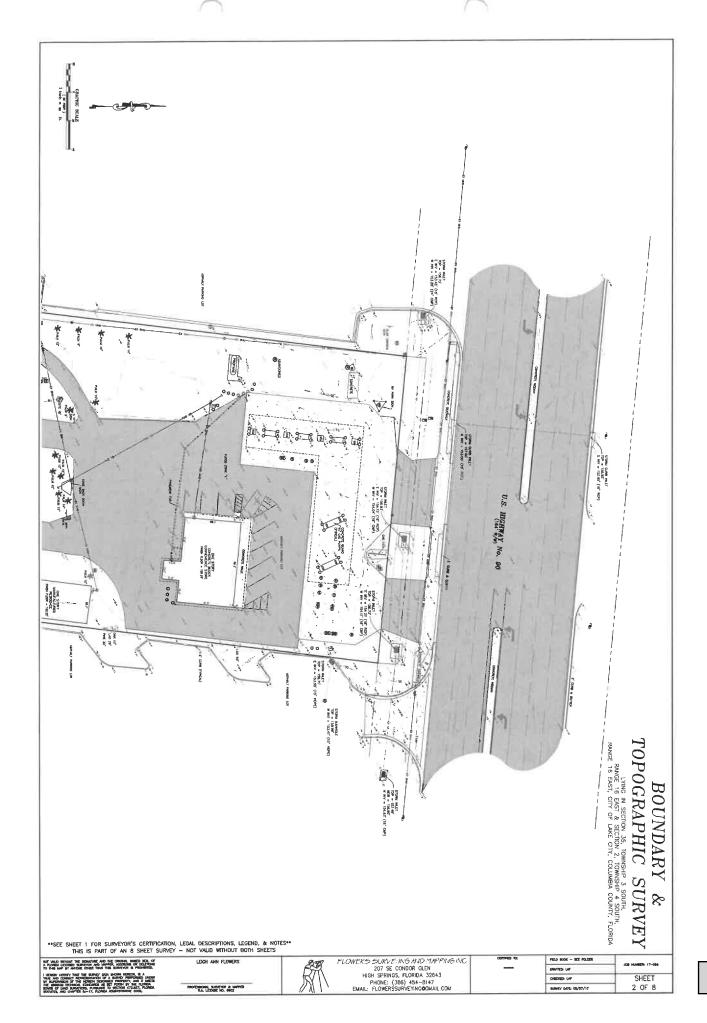


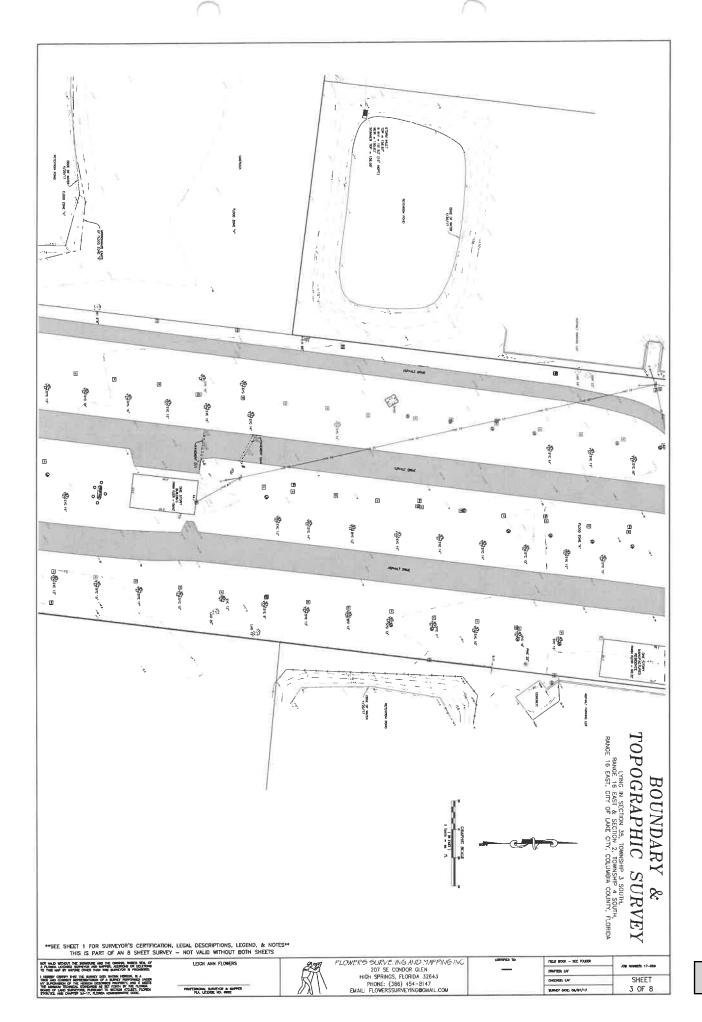


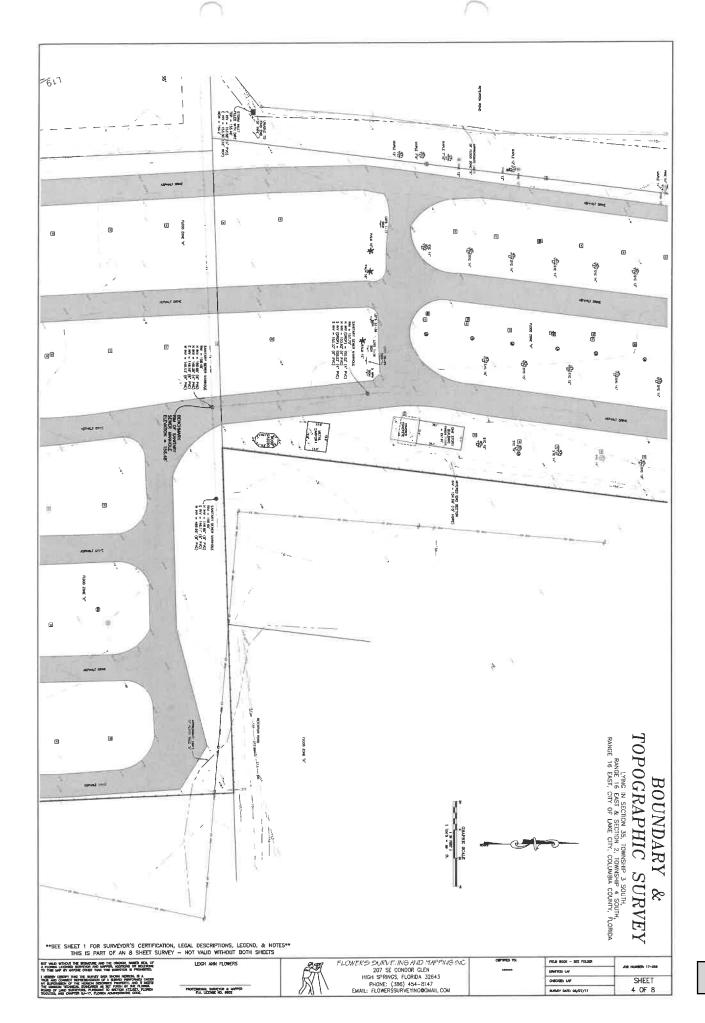


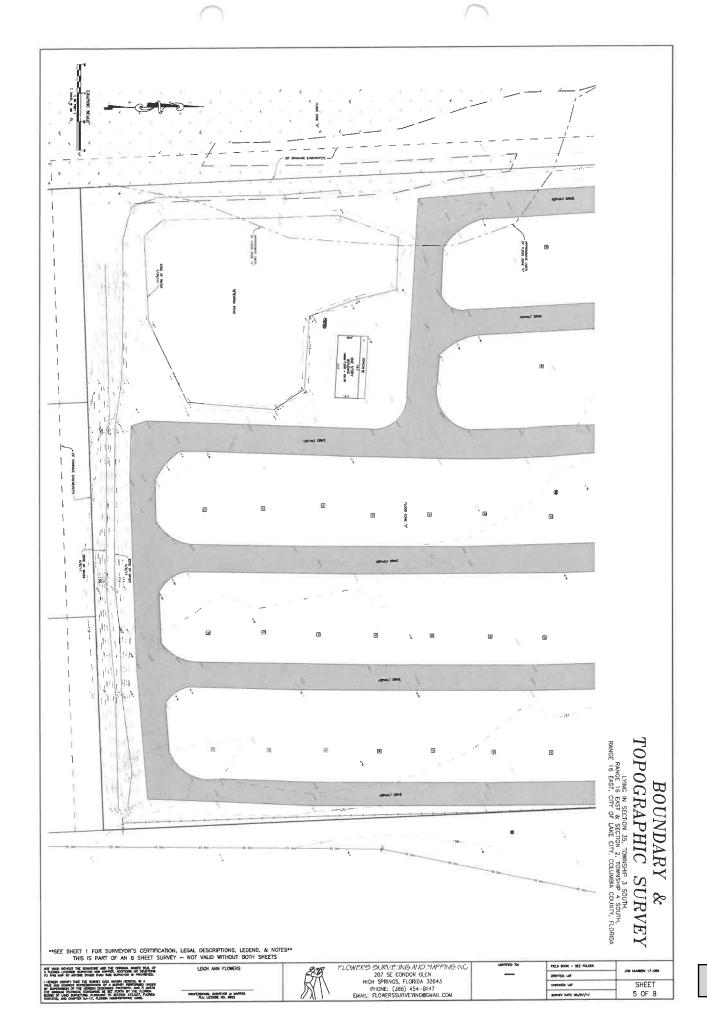


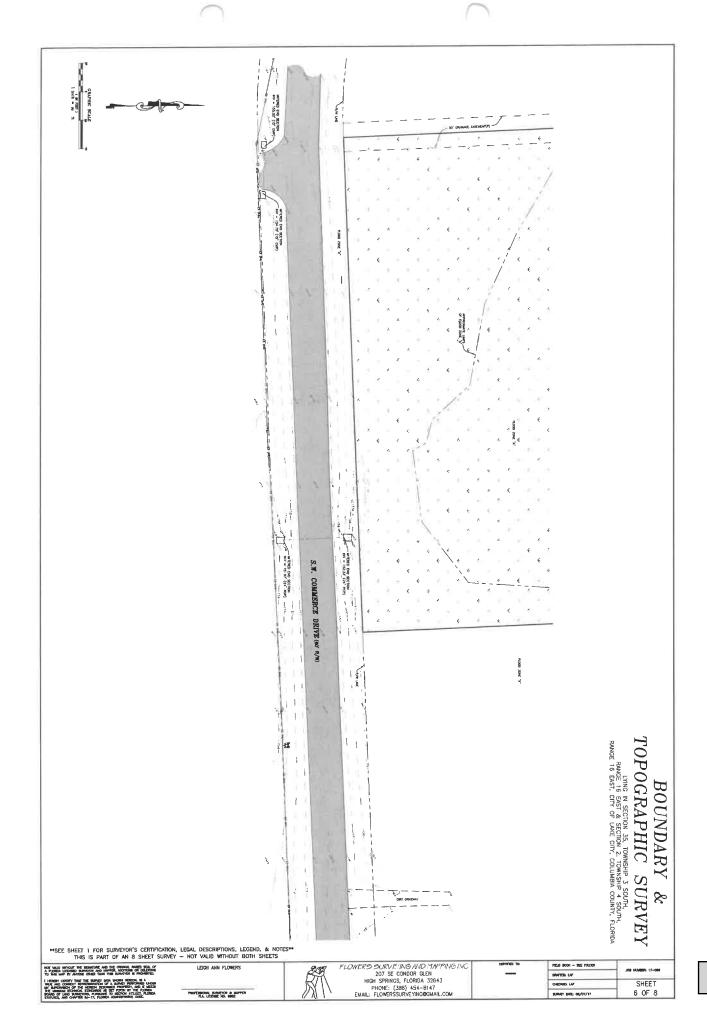


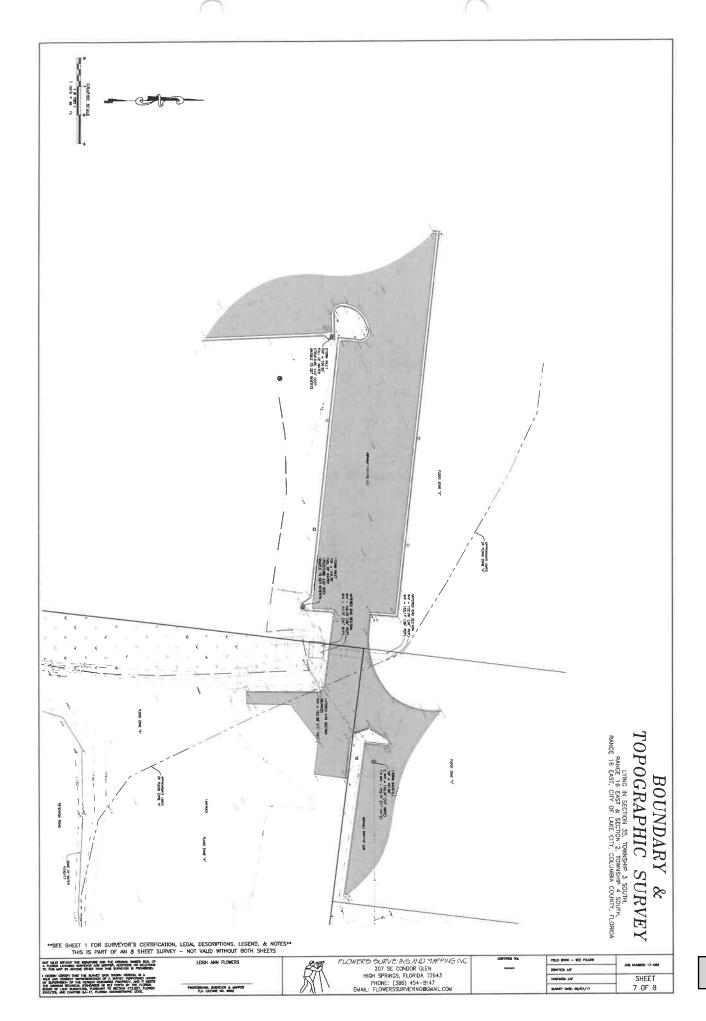


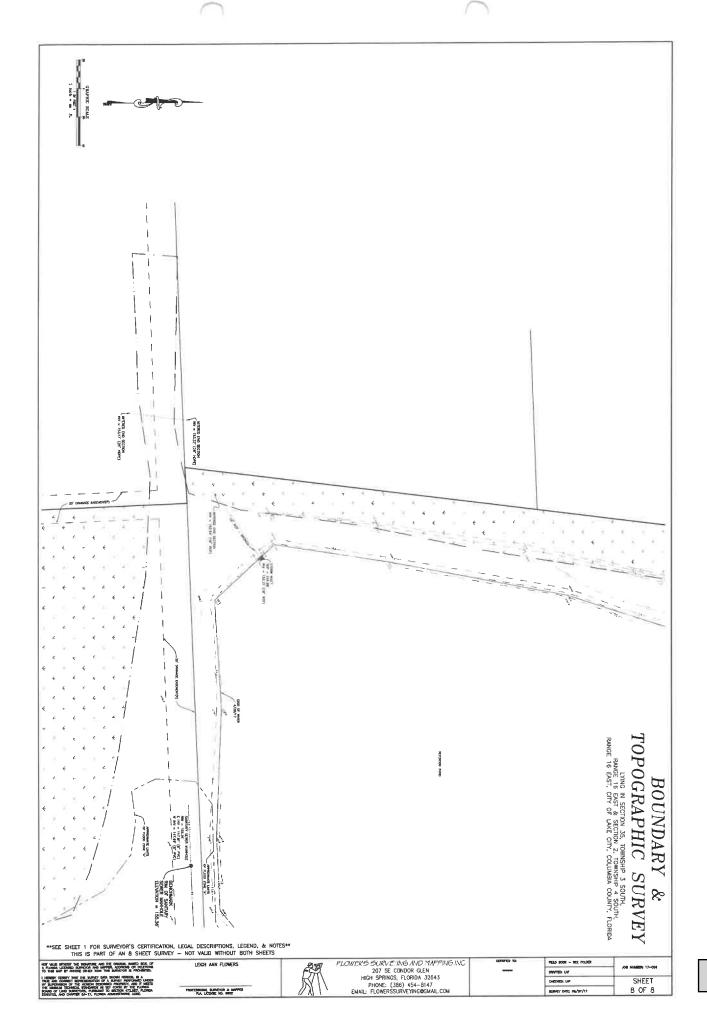


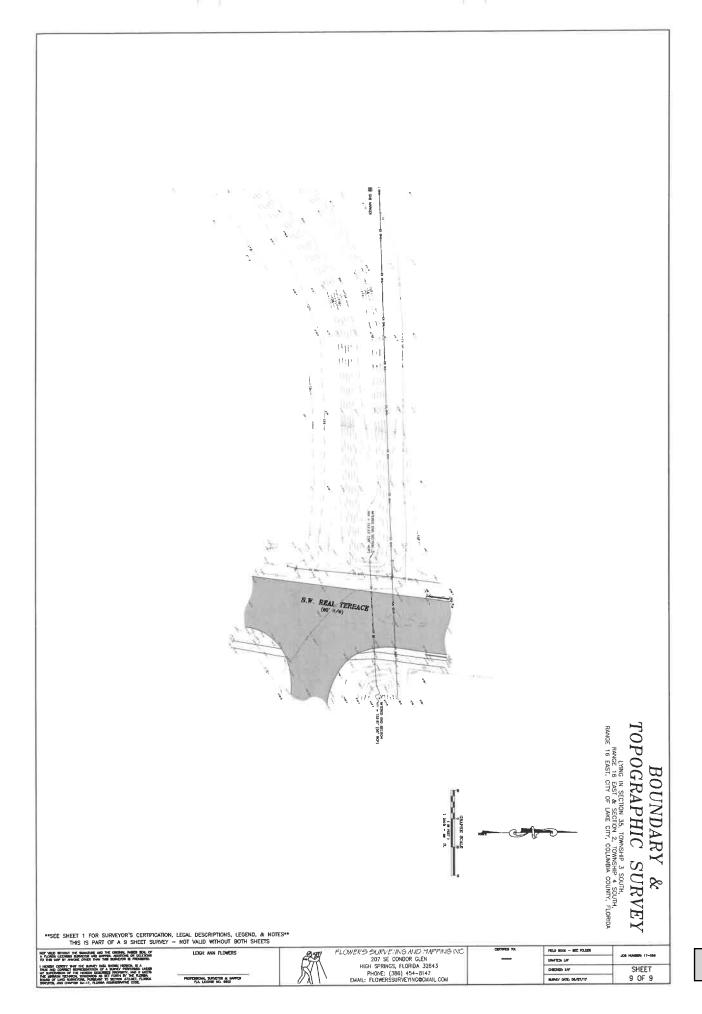


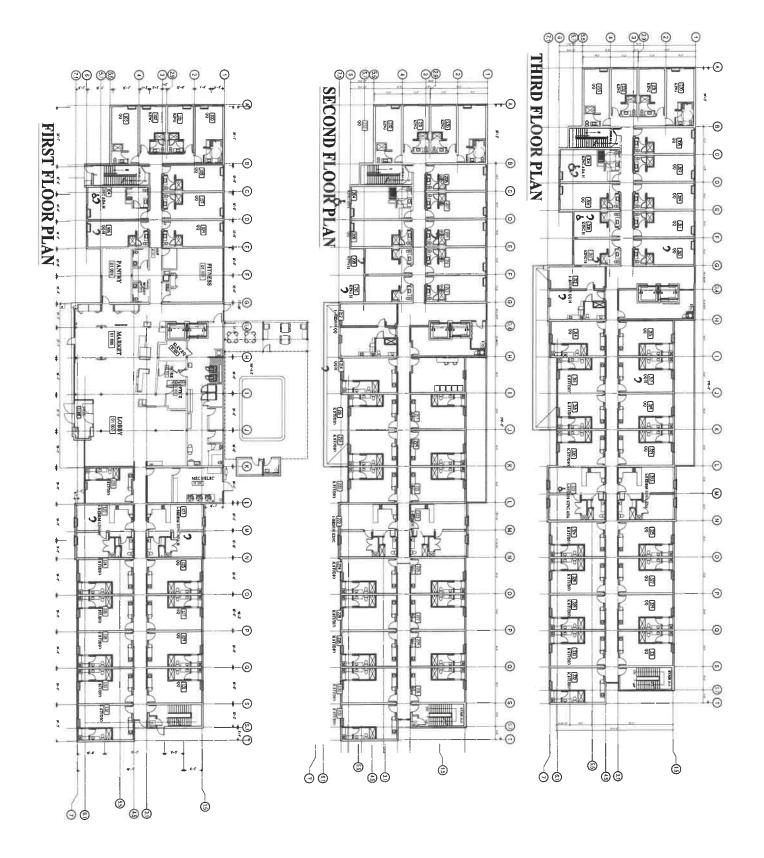












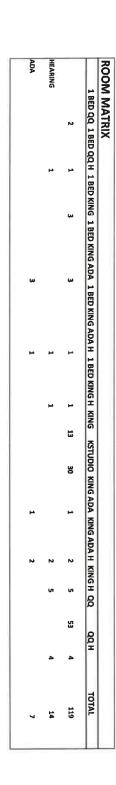


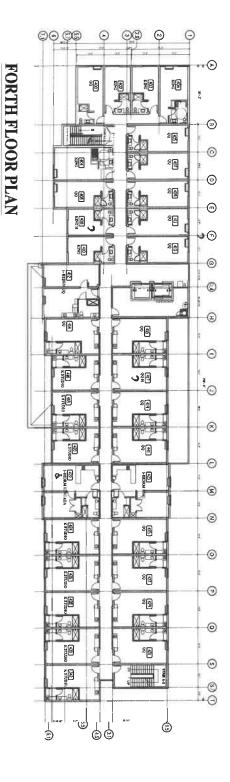
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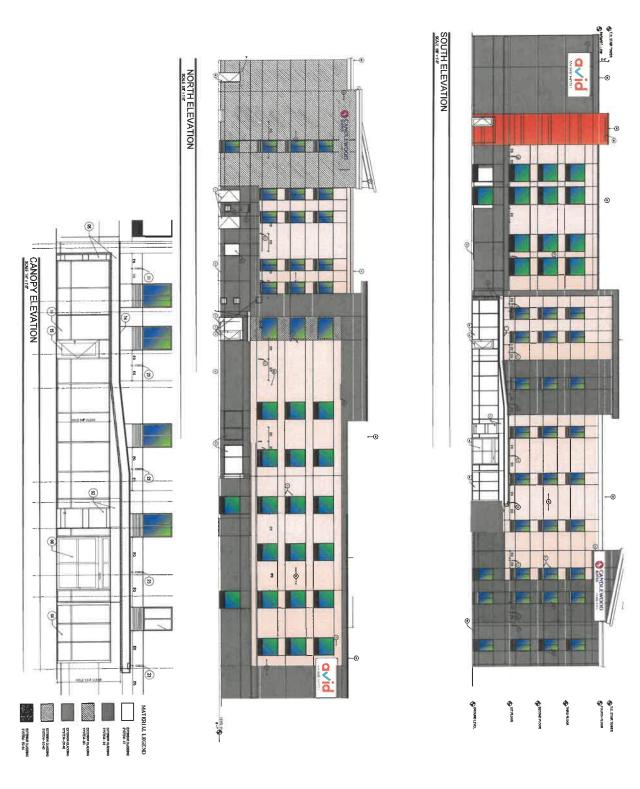
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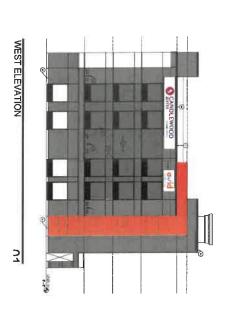


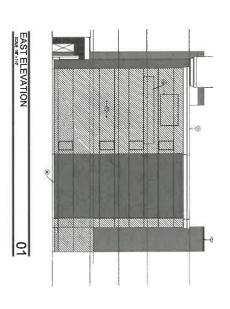


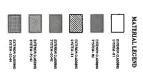




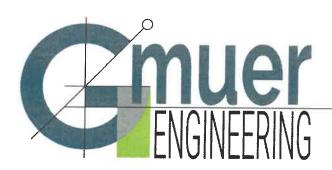












2603 NW 13th St, Box 314 Gainesville, FL 32609 Ph. (352) 281-4928

gmuereng.com

April 23, 2024

Suwannee River Water Management District

Re: Lake City Hotels Phase 2

This permit modification is submitted for the construction of an additional hotel within the drainage area of the master stormwater management system as permitted under SRWMD ERP 209165-3.

The original master plan was permitted under SRWMD ERP 209165-3 where the proposed wet stormwater facility was designed to provide water quality treatment and stormwater discharge attenuation for a master site plan that included runoff from two existing hotels, an existing restaurant, three new hotels, and two new restaurants, with associated driveway connections and parking. Under this master permit, the following quantities of impervious area were designed to be routed through the proposed wet stormwater management facility.

Permitted Impervious Areas under SRWMD ERP 209165-3				
Impervious (Existing) *	182,297 sf	4.1850 ac		
Impervious (Proposed)	335,589 sf	7.7041 ac		
Impervious (Future)	23,881 sf	0.5482 ac		
Total Permitted Impv.	541,767 sf	12.4373 ac		
* Existing Impervious Aera consisted of the Quality Inn,				
Ruby Tuesdays, and Holiday Inn to the west of the site				

Construction began on this permitted master system which included the complete wet stormwater management facility as designed and permitted, the complete master storm piping system that serves this master system. However, the scope of the proposed impervious area was reduced to those site improvements needed to serve the first hotel, Hotel A. This included the driveway connections to the western existing impervious areas, reconstructed driveway to US90, and the surface parking needed for Hotel A. This scope was referenced as Phase 1 in the associated plans and in the remainder of this report. An as-built has been submitted for this change. The resulting impervious area accounting following the Phase 1 construction into the master stormwater system are as follows.

Impervious Areas Accounting with Phase 1 As-Built					
Impervious (Existing) *	182,297 sf	4.1850 ac			
Impervious (Phase 1)	107,983 sf	2.4789 ac			
Impervious (Future)	251,487 sf	5.7733 ac			
Total Permitted Impv.	541.767 sf	12,4373 ac			

1 of 2



9225 CR 49 • LīVE OAK, FLORIDA 32060 • TELEPHONE 386/362-1001 • 800/226-1066 • FAX 386/362-1056

January 4, 2018

Mahendra Patel AURUM, L.L.C. 162 NW Birdie Lane Lake City, FL 32056-0575

SUBJECT: Permit Number ERP-023-209165-3

Lake City Hotels

Dear Mahendra Patel:

Enclosed is your individual permit issued by the Suwannee River Water Management District on January 04, 2018. This permit is a legal document and should be kept with your other important documents. Permit issuance does not relieve you from the responsibility of obtaining any necessary permits from any federal, state, or local agencies for your project.

Noticing Your Permit:

For noticing instructions, please refer to the noticing materials in this package regarding closing the point of entry for someone to challenge the issuance of your permit. Please note that if a timely petition for administrative hearing is filed, your permit will become non-final and any activities that you choose to undertake pursuant to your permit will be at your own risk.

Compliance with Permit Conditions:

To submit your required permit compliance information, go to the District's website at https://permitting.sjrwmd.com/srepermitting/jsp/start.jsp. Click to sign-in to your existing account or to create a new account. Select the "Apply/Submit" tab, select "Submit Compliance Data", enter your permit number, and select "No Specific Date" for the Compliance Due Date Range. You will then be able to view all the compliance submittal requirements for your project. Select "the compliance item that you are ready to submit and then attach the appropriate information or form. The forms to comply with your permit conditions are available at floridaswater.com/permitting under the section "Handbooks, forms, fees, final orders". Click on forms to view all permit compliance forms, then scroll to the ERP application forms section and select the applicable compliance forms. Alternatively, if you have difficulty finding forms or need copies of the appropriate forms, please contact the Resource Management Division at (386) 362-1001.

Transferring Your Permit:

Your permit requires you to notify the District in writing within 30 days of any change in ownership or control of the project or activity covered by the permit, or within 30 days of any change in ownership or control of the real property on which the permitted project or activity is located or occurs. You will need to provide the District with the information specified in rule 62-330.340, Florida Administrative Code (F.A.C.). Generally, this will require you to complete and submit Form 62-330.340(1), "Request to Transfer Permit".

Please note that a permittee is liable for compliance with the permit before the permit is transferred. The District, therefore, recommends that you request a permit transfer in advance in

accordance with the applicable rules. You are encouraged to contact District staff for assistance with this process.

Thank you and please let us know if you have additional questions. For general questions contact us at (386) 362-1001.

Sincerely,

Hugh Thomas
Executive Director

Enclosures: Permit

cc: District Permit File



9225 CR 49 • LIVE OAK, FLORIDA 32060 • TELEPHONE 386/362-1001 • 800/226-1066 • FAX 386/362-1056

ERP Individual Permit

PERMITTEE:

Mahendra Patel AURUM, L.L.C. 162 NW Birdie Lane Lake City, FL 32056-0575 PERMIT NUMBER: ERP-023-209165-3 DATE ISSUED: January 04, 2018 DATE EXPIRES: January 04, 2023

COUNTY: Columbia

TRS: S35 T3S R16E, S2 T4S R16E

PROJECT: Lake City Hotels

Upon completion, the approved entity to which operation and maintenance maybe transferred pursuant to rule 62-330.310 and 62-330.340 or 40B-4.1130, Florida Administrative Code (F.A.C) shall be:

Mahendra Patel AURUM, L.L.C. 162 NW Birdie Lane Lake City, FL 32056-0575

Based on the information provided to the Suwannee River Water Management District (District), the above mentioned project has met the conditions of issuance as found in subsection 62-330.301, subsections 62-330.407 through 62-330.635, or subsection 40B-4.3030, F.A.C. The permit is hereby in effect for the activity description below:

Previous permit was for the construction and operation of a surface water management system serving 6.10 acres of impervious surface on a total project area of 10.10 acres. This modification consists of relocating the master stromwater facility and the addition of 8.25 acres of impervious on a total project area of 17.61 acres. When constructed the project shall be in accordance with the application package submitted by Mahendra Patel, of Aurum, LLC., as an authorized agent for the Columbia County Board of County Commissioners, and the signed and sealed plans certified on October 31, 2017, by Christopher Gmuer, P.E., of Gmuer Engineering, LLC.

As the permittee and/or operation and maintenance entity, it is your responsibility to ensure that adverse off-site impacts do not occur either during or after the construction. Any additional construction or alterations not authorized by this permit may result in flood control or water quality problems both on and off site and will be a violation of District rule.

You and any other substantially affected persons are entitled to request an administrative hearing or mediation. Please refer to the enclosed notice of rights.

- All activities shall be implemented following the plans, specifications and performance criteria approved by this permit. Any deviations must be authorized in a permit modification in accordance with Rule 62-330.315, F.A.C. Any deviations that are not so authorized may subject the permittee to enforcement action and revocation of the permit under Chapter 373, F.S.
- 2. A complete copy of this permit shall be kept at the work site of the permitted activity during the construction phase, and shall be available for review at the work site upon request by the District staff. The permittee shall require the contractor to review the complete permit prior to beginning construction.
- 3. Activities shall be conducted in a manner that does not cause or contribute to violations of state water quality standards. Performance-based erosion and sediment control best management practices shall be installed immediately prior to, and be maintained during and after construction as needed, to prevent adverse impacts to the water resources and adjacent lands. Such practices shall be in accordance with the State of Florida Erosion and Sediment Control Designer and Reviewer Manual (Florida Department of Environmental Protection and Florida Department of Transportation June 2007), and the Florida Stormwater Erosion and Sedimentation Control Inspector's Manual (Florida Department of Environmental Protection, Nonpoint Source Management Section, Tallahassee, Florida, July 2008), which are both incorporated by reference in subparagraph 62-330.050(9)(b)5, F.A.C., unless a project-specific erosion and sediment control plan is approved or other water quality control measures are required as part of the permit.
- 4. At least 48 hours prior to beginning the authorized activities, the permittee shall submit to the District a fully executed Form 62-330.350(1), "Construction Commencement Notice,"[10-1-13], incorporated by reference herein (http://www.flrules.org/Gateway/reference.asp?No=Ref-02505), indicating the expected start and completion dates. A copy of this form may be obtained from the District, as described in subsection 62-330.010(5), F.A.C. If available, an District website that fulfills this notification requirement may be used in lieu of the form.
- 5. Unless the permit is transferred under Rule 62-330.340, F.A.C., or transferred to an operating entity under Rule 62-330.310, F.A.C., the permittee is liable to comply with the plans, terms and conditions of the permit for the life of the project or activity.
- 6. Within 30 days after completing construction of the entire project, or any independent portion of the project, the permittee shall provide the following to the Agency, as applicable:
 - For an individual, private single-family residential dwelling unit, duplex, triplex, or quadruplex — "Construction Completion and Inspection Certification for Activities Associated With a Private Single-Family Dwelling Unit" [Form 62-330.310(3)]; or
 - 2. For all other activities "As-Built Certification and Request for Conversion to Operational Phase" [Form 62-330.310(1)].

- 3. If available, an Agency website that fulfills this certification requirement may be used in lieu of the form.
- 7. If the final operation and maintenance entity is a third party:
 - 1. Prior to sales of any lot or unit served by the activity and within one year of permit issuance, or within 30 days of as-built certification, whichever comes first, the permittee shall submit, as applicable, a copy of the operation and maintenance documents (see sections 12.3 thru 12.3.3 of Volume I) as filed with the Department of State, Division of Corporations and a copy of any easement, plat, or deed restriction needed to operate or maintain the project, as recorded with the Clerk of the Court in the County in which the activity is located.
 - 2. Within 30 days of submittal of the as-built certification, the permittee shall submit "Request for Transfer of Environmental Resource Permit to the Perpetual Operation Entity" [Form 62-330.310(2)] to transfer the permit to the operation and maintenance entity, along with the documentation requested in the form. If available, an Agency website that fulfills this transfer requirement may be used in lieu of the form.
- 8. The permittee shall notify the District in writing of changes required by any other regulatory District that require changes to the permitted activity, and any required modification of this permit must be obtained prior to implementing the changes.
- 9. This permit does not:
 - 1. Convey to the permittee any property rights or privileges, or any other rights or privileges other than those specified herein or in Chapter 62-330, F.A.C.;
 - 2. Convey to the permittee or create in the permittee any interest in real property;
 - 3. Relieve the permittee from the need to obtain and comply with any other required federal, state, and local authorization, law, rule, or ordinance; or
 - 4. Authorize any entrance upon or work on property that is not owned, held in easement, or controlled by the permittee.
- 10. Prior to conducting any activities on state-owned submerged lands or other lands of the state, title to which is vested in the Board of Trustees of the Internal Improvement Trust Fund, the permittee must receive all necessary approvals and authorizations under Chapters 253 and 258, F.S. Written authorization that requires formal execution by the Board of Trustees of the Internal Improvement Trust Fund shall not be considered received until it has been fully executed.
- 11. The permittee shall hold and save the District harmless from any and all damages, claims, or liabilities that may arise by reason of the construction, alteration, operation, maintenance, removal, abandonment or use of any project authorized by the permit.
- 12. The permittee shall notify the District in writing:
 - Immediately if any previously submitted information is discovered to be inaccurate; and
 - 2. Within 30 days of any conveyance or division of ownership or control of the property or the system, other than conveyance via a long-term lease, and the new owner shall request transfer of the permit in accordance with Rule 62-330.340, F.A.C.

This does not apply to the sale of lots or units in residential or commercial subdivisions or condominiums where the stormwater management system has been completed and converted to the operation phase.

- 13. Upon reasonable notice to the permittee, District staff with proper identification shall have permission to enter, inspect, sample and test the project or activities to ensure conformity with the plans and specifications authorized in the permit.
- 14. If any prehistoric or historic artifacts, such as pottery or ceramics, stone tools or metal implements, dugout canoes, or any other physical remains that could be associated with Native American cultures, or early colonial or American settlement are encountered at any time within the project site area, work involving subsurface disturbance in the immediate vicinity of such discoveries shall cease. The permittee or other designee shall contact the Florida Department of State, Division of Historical Resources, Compliance and Review Section, at (850) 245-6333 or (800) 847-7278, as well as the appropriate permitting agency office. Such subsurface work shall not resume without verbal or written authorization from the Division of Historical Resources. If unmarked human remains are encountered, all work shall stop immediately and notification shall be provided in accordance with Section 872.05, F.S.
- 15. Any delineation of the extent of a wetland or other surface water submitted as part of the permit application, including plans or other supporting documentation, shall not be considered binding unless a specific condition of this permit or a formal determination under Rule 62-330.201, F.A.C., provides otherwise.
- 16. The permittee shall provide routine maintenance of all components of the stormwater management system to remove trapped sediments and debris. Removed materials shall be disposed of in a landfill or other uplands in a manner that does not require a permit under Chapter 62-330, F.A.C., or cause violations of state water quality standards.
- 17. This permit is issued based on the applicant's submitted information that reasonably demonstrates that adverse water resource-related impacts will not be caused by the completed permit activity. If any adverse impacts result, the District will require the permittee to eliminate the cause, obtain any necessary permit modification, and take any necessary corrective actions to resolve the adverse impacts.
- 18. A Recorded Notice of Environmental Resource Permit may be recorded in the county public records in accordance with Rule 62-330.090(7), F.A.C. Such notice is not an encumbrance upon the property.

WITHIN 30 DAYS AFTER COMPLETION OF THE PROJECT, THE PERMITTEE SHALL NOTIFY THE DISTRICT, IN WRITING, THAT THE FACILITIES ARE COMPLETE.

AUTHORIZED BY: Suwannee River Water Management District

By:

Hugh Thomas
Executive Director

NOTICE OF RIGHTS

- 1. A person whose substantial interests are or may be determined has the right to request an administrative hearing by filing a written petition with the Suwannee River Water Management District (District), or may choose to pursue mediation as an alternative remedy under Section 120.569 and 120.573, Florida Statutes, (F.S.), before the deadline for filing a petition. Choosing mediation will not adversely affect the right to a hearing if mediation does not result in a settlement. The procedures for pursuing mediation are set forth in Sections 120.569 and 120.57 F.S. Pursuant to Rule 28-106.111, Florida Administrative Code, (F.A.C.), the petition must be filed at the office of the District Clerk at District Headquarters, 9225 C.R. 49, Live Oak, Florida 32060 within twenty-one (21) days of receipt of written notice of the decision or within twenty-one (21) days of newspaper publication of the notice of District decision (for those persons to whom the District does not mail actual notice). A petition must comply with Chapter 28-106, F.A.C.
- 2. If the Governing Board takes action which substantially differs from the notice of District decision to grant or deny the pe1mit application, a person whose substantial interests are or may be determined has the right to request an administrative hearing or may choose to pursue mediation as an alternative remedy as described above. Pursuant to Rule 28-106.111, F.A.C., the petition must be filed at the office of the District Clerk at District Headquarters, 9225 C.R. 49, Live Oak, Florida 32060 within twenty-one (21) days of receipt of written notice of the decision or within twenty-one (21) days of newspaper publication of the notice of District decision (for those persons to whom the District does not mail actual notice). Such a petition must comply with Chapter 28-106, F.A.C.
- 3. A substantially interested person has the right to a formal administrative hearing pursuant to Section 120.569 and 120.57(1), F.S., where there is a dispute between the District and the party regarding an issue of material fact. A petition for formal hearing must comply with the requirements set forth in Rule 28-106.201, F.A.C.
- 4. A substantially interested person has the right to an informal hearing pursuant to Section 120.569 and 120.57(2), F.S., where no material facts are in dispute. A petition for an informal hearing must comply with the requirements set forth in Rule 28-106.301, F.A.C.
- 5. A petition for an administrative hearing is deemed filed upon receipt of the petition by the Office of the District Clerk at the District Headquarters in Live Oak, Florida.
- 6. Failure to file a petition for an administrative hearing within the requisite time frame shall constitute a waiver of the right to an administrative hearing pursuant to Rule 28-106.111, F.A.C.
- 7. The right to an administrative hearing and the relevant procedures to be followed is governed by Chapter 120, Florida Statutes, and Chapter 28-106, F.A.C.
- 8. Pursuant to Section 120.68, F.S., a person who is adversely affected by final District action may seek review of the action in the District Court of Appeal by filing a notice of appeal pursuant to the Florida Rules of Appellate Procedure, within 30 days of the rendering of the final District action.
- 9. A party to the proceeding before the District who claims that a District order is inconsistent with the provisions and purposes of Chapter 3 73, F. S., may seek review of the order pursuant to Section 373.114, F.S., by the Florida Land and Water Adjudicatory Commission, by filing a request for review with the Commission and serving a copy of the Department of Environmental Protection and any person named in the order within 20 days of adoption of a rule or the rendering of the District order.
- 10. For appeals to the District Courts of Appeal, a District action is considered rendered after it is signed on behalf of the District, and is filed by the District Clerk.

11. Failure to observe the relevant time frames for filing a petition for judicial review, or for Commission review, will result in waiver of the right to review.

CERTIFICATE OF SERVICE

I hereby certify that a copy of the foregoing Notice of Rights has been sent to:

Mahendra Patel AURUM, L.L.C. 162 NW Birdie Lane Lake City, FL 32056-0575 (386) 752-2209

This January 04, 2018

Deputy Clerk

Suwannee River Water Management District

9225 C.R. 49

Live Oak, Florida 32060

386.362.1001 or 800.226.1066 (Florida only)

cc: File Number: ERP-023-209165-3

NOTICING INFORMATION

Dear Permittee:

Please be advised that the Suwannee River Water Management District (District) has not published a notice in the newspaper advising the public that it has issued a permit for this project.

Newspaper publication, using the District's form, notifies members of the public of their right to challenge the issuance of the permit. If proper notice is given by newspaper publication, then there is a 21-day time limit to file a petition challenging the issuance of the permit.

To close the point of entry for filing a petition, you may publish (at your own expense) a onetime notice of the District's decision in a newspaper of general circulation within the affected area as defined in Section 50.011 of the Florida Statutes. If you do not publish a newspaper notice, the time to challenge the issuance of your permit will not expire.

A copy of the notice and a partial list of newspapers of general circulation are attached for your convenience. However, you are not limited to those listed newspapers. If you choose to close the point of entry and the notice is published, the newspaper will return to you an affidavit as proof of publication. In accordance with 40B-1.1010(4), F.A.C., a copy of the affidavit shall be provided to the District within 14 days of publication. A scanned copy of the affidavit may be forwarded to Tilda Musgrove by email at *tjm@srwmd.org* (preferred method) or send the original affidavit of publication to:

Tilda Musgrove Resource Management 9225 CR 49 Live Oak, FL 32060

If you have any questions, please contact me at 386.362.1001. Sincerely,

Tilda Musgrove

Business Resource Specialist Resource Management

Tilda Musgreve

NOTICE OF AGENCY ACTION TAKEN BY THE SUWANNEE RIVER WATER MANAGEMENT DISTRICT

Notice is given that the following	g permit was issued on		\$
(Name and address of applican	t)		
permit#	The project is locate	d in	County, Section
, Township	South, Range	East.	The permit authorizes a surface
water management system on _	acres for		
			known as
	eceiving water body is		(40)
(F.A.C.), the petition must be file Management Business Resource 32060 or by e-mail to tjm@srwn notice of intended District decisi actual notice). A petition must of	a written petition with the napter 28-106 and Rule ed (received) either by the Specialist at District and.org, within twenty-or (for those persons to comply with Sections 12	e Suwar 40BB-1 delivery a Headqua ne (21) da o whom t 20.54(5)(nnee River Water Management 1010, Florida Administrative Code at the office of the Resource arters, 9225 CR 49, Live Oak FL ays of newspaper publication of the the District does not mail or email b)4. and 120.569(2)(c), Florida
(fax). Mediation pursuant to Sec			accept a petition sent by facsimile

A petition for an administrative hearing is deemed filed upon receipt of the complete petition by the District Clerk at the District Headquarters in Live Oak, FL during the District's regular business hours. The District's regular business hours are 8 a.m. – 5 p.m., excluding weekends and District holidays. Petitions received by the District Clerk after the District's regular business hours shall be deemed filed as of 8 a.m. on the next regular District business day.

The right to an administrative hearing and the relevant procedures to be followed are governed by Chapter 120, Florida Statutes, Chapter 28-106, Florida Administrative Code, and Rule 40B-1.1010, Florida Administrative Code. Because the administrative hearing process is designed to formulate final agency action, the filing of a petition means the District's final action may be different from the position taken by it in this notice. Failure to file a petition for an administrative hearing within the requisite time frame shall constitute a waiver of the right to an administrative hearing. (Rule 28-106.111, F.A.C.).

If you wish to do so, you may request the Notice of Rights for this permit by contacting the Business Resource Specialist in the Division of Resource Management (RM), 9225 CR 49, Live Oak,, FL 32060, or by phone at 386.362.1001.

NEWSPAPER ADVERTISING

ALACHUA

Gainesville Sun Legal Advertising PO Box 14747 Gainesville, FL 32614

352.372.4222

BRADFORD

Bradford County Telegraph, Legal Advertising P. O. Drawer A Starke, FL 32901 904-964-6305/ fax 904-964-8628

COLUMBIA

Lake City Reporter 180 E Duval Street Lake City, FL 32055 386.754.0401

DIXIE

Dixie County Advocate 174 County Road 351 Cross City, FL 32628 352.498.3312

GILCHRIST

Gilchrist County Journal 207 N Main St Trenton, FL 32693 352.463.7135

HAMILTON

Jasper News 521 Demorest Street SE Live Oak, FL 32064 386.362.1734

JEFFERSON

Monticello News PO Drawer 772 Madison, FL 32344 850.997.3568

LAFAYETTE

Mayo Free Press 521 Demorest Street SE Live Oak, FL 32064 386,362,1734

LEVY

Levy County Journal PO Box 159 Bronson, FL 32621 352.486.2312

MADISON

Madison Carrier PO Drawer 772 Madison, FL 32344 850.973.4141

SUWANNEE

Suwannee Democrat 521 Demorest Street SE Live Oak, FL 32064 386.364.1734

TAYLOR

Taco Times PO Box 888 Perry, FL 32348 850.584.5513

UNION

Union County Times 125 E Main Street Lake Butler, FL 32054 386.496.2261



9225 CR 49 • LIVE OAK, FLORIDA 32060 • TELEPHONE 386/362-1001 • 800/226-1066 • FAX 386/362-1056

January 4, 2018

Janak Shukla ERA INVESTMENTS LLC 3010 W Us Highway 90 Lake City, FL 32055-7711

SUBJECT: Permit Number ERP-023-209165-3

Lake City Hotels

Dear Janak Shukla:

Enclosed is your individual permit issued by the Suwannee River Water Management District on January 04, 2018. This permit is a legal document and should be kept with your other important documents. Permit issuance does not relieve you from the responsibility of obtaining any necessary permits from any federal, state, or local agencies for your project.

Noticing Your Permit:

For noticing instructions, please refer to the noticing materials in this package regarding closing the point of entry for someone to challenge the issuance of your permit. Please note that if a timely petition for administrative hearing is filed, your permit will become non-final and any activities that you choose to undertake pursuant to your permit will be at your own risk,

Compliance with Permit Conditions:

To submit your required permit compliance information, go to the District's website at https://permitting.sjrwmd.com/srepermitting/jsp/start.jsp. Click to sign-in to your existing account or to create a new account. Select the "Apply/Submit" tab, select "Submit Compliance Data", enter your permit number, and select "No Specific Date" for the Compliance Due Date Range. You will then be able to view all the compliance submittal requirements for your project. Select "the compliance item that you are ready to submit and then attach the appropriate information or form. The forms to comply with your permit conditions are available at floridaswater.com/permitting under the section "Handbooks, forms, fees, final orders". Click on forms to view all permit compliance forms, then scroll to the ERP application forms section and select the applicable compliance forms. Alternatively, if you have difficulty finding forms or need copies of the appropriate forms, please contact the Resource Management Division at (386) 362-1001.

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Please note that a permittee is liable for compliance with the permit before the permit is transferred. The District, therefore, recommends that you request a permit transfer in advance in

Perry, Florida

accordance with the applicable rules. You are encouraged to contact District staff for assistance with this process.

Thank you and please let us know if you have additional questions. For general questions contact us at (386) 362-1001.

Sincerely,

Hugh Thomas Executive Director

Enclosures: Permit

cc: District Permit File

ERP Individual Permit

PERMITTEE:

Janak Shukla ERA INVESTMENTS LLC 3010 W Us Highway 90 Lake City, FL 32055-7711 PERMIT NUMBER: ERP-023-209165-3 DATE ISSUED: January 04, 2018

DATE EXPIRES: January 04, 2023

COUNTY: Columbia

TRS: S35 T3S R16E, S2 T4S R16E

PROJECT: Lake City Hotels

Upon completion, the approved entity to which operation and maintenance maybe transferred pursuant to rule 62-330.310 and 62-330.340 or 40B-4.1130, Florida Administrative Code (F.A.C) shall be:

Mahendra Patel AURUM, L.L.C. 162 NW Birdie Lane Lake City, FL 32056-0575

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As the permittee and/or operation and maintenance entity, it is your responsibility to ensure that adverse off-site impacts do not occur either during or after the construction. Any additional construction or alterations not authorized by this permit may result in flood control or water quality problems both on and off site and will be a violation of District rule.

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 - For an individual, private single-family residential dwelling unit, duplex, triplex, or quadruplex — "Construction Completion and Inspection Certification for Activities Associated With a Private Single-Family Dwelling Unit" [Form 62-330.310(3)]; or
 - For all other activities "As-Built Certification and Request for Conversion to Operational Phase" [Form 62-330.310(1)].

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 - 1. Prior to sales of any lot or unit served by the activity and within one year of permit issuance, or within 30 days of as-built certification, whichever comes first, the permittee shall submit, as applicable, a copy of the operation and maintenance documents (see sections 12.3 thru 12.3.3 of Volume I) as filed with the Department of State, Division of Corporations and a copy of any easement, plat, or deed restriction needed to operate or maintain the project, as recorded with the Clerk of the Court in the County in which the activity is located.
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- 9. This permit does not:
 - Convey to the permittee any property rights or privileges, or any other rights or privileges other than those specified herein or in Chapter 62-330, F.A.C.;
 - 2. Convey to the permittee or create in the permittee any interest in real property;
 - 3. Relieve the permittee from the need to obtain and comply with any other required federal, state, and local authorization, law, rule, or ordinance; or
 - 4. Authorize any entrance upon or work on property that is not owned, held in easement, or controlled by the permittee.
- 10. Prior to conducting any activities on state-owned submerged lands or other lands of the state, title to which is vested in the Board of Trustees of the Internal Improvement Trust Fund, the permittee must receive all necessary approvals and authorizations under Chapters 253 and 258, F.S. Written authorization that requires formal execution by the Board of Trustees of the Internal Improvement Trust Fund shall not be considered received until it has been fully executed.
- 11. The permittee shall hold and save the District harmless from any and all damages, claims, or liabilities that may arise by reason of the construction, alteration, operation, maintenance, removal, abandonment or use of any project authorized by the permit.
- 12. The permittee shall notify the District in writing:
 - Immediately if any previously submitted information is discovered to be inaccurate; and
 - 2. Within 30 days of any conveyance or division of ownership or control of the property or the system, other than conveyance via a long-term lease, and the new owner shall request transfer of the permit in accordance with Rule 62-330.340, F.A.C.

This does not apply to the sale of lots or units in residential or commercial subdivisions or condominiums where the stormwater management system has been completed and converted to the operation phase.

- 13. Upon reasonable notice to the permittee, District staff with proper identification shall have permission to enter, inspect, sample and test the project or activities to ensure conformity with the plans and specifications authorized in the permit.
- 14. If any prehistoric or historic artifacts, such as pottery or ceramics, stone tools or metal implements, dugout canoes, or any other physical remains that could be associated with Native American cultures, or early colonial or American settlement are encountered at any time within the project site area, work involving subsurface disturbance in the immediate vicinity of such discoveries shall cease. The permittee or other designee shall contact the Florida Department of State, Division of Historical Resources, Compliance and Review Section, at (850) 245-6333 or (800) 847-7278, as well as the appropriate permitting agency office. Such subsurface work shall not resume without verbal or written authorization from the Division of Historical Resources. If unmarked human remains are encountered, all work shall stop immediately and notification shall be provided in accordance with Section 872.05, F.S.
- 15. Any delineation of the extent of a wetland or other surface water submitted as part of the permit application, including plans or other supporting documentation, shall not be considered binding unless a specific condition of this permit or a formal determination under Rule 62-330.201, F.A.C., provides otherwise.
- 16. The permittee shall provide routine maintenance of all components of the stormwater management system to remove trapped sediments and debris. Removed materials shall be disposed of in a landfill or other uplands in a manner that does not require a permit under Chapter 62-330, F.A.C., or cause violations of state water quality standards.
- 17. This permit is issued based on the applicant's submitted information that reasonably demonstrates that adverse water resource-related impacts will not be caused by the completed permit activity. If any adverse impacts result, the District will require the permittee to eliminate the cause, obtain any necessary permit modification, and take any necessary corrective actions to resolve the adverse impacts.
- 18. A Recorded Notice of Environmental Resource Permit may be recorded in the county public records in accordance with Rule 62-330.090(7), F.A.C. Such notice is not an encumbrance upon the property.

WITHIN 30 DAYS AFTER COMPLETION OF THE PROJECT, THE PERMITTEE SHALL NOTIFY THE DISTRICT, IN WRITING, THAT THE FACILITIES ARE COMPLETE.

AUTHORIZED BY: Suwannee River Water Management District

By:

Hugh Thomas Executive Director

NOTICE OF RIGHTS

- 1. A person whose substantial interests are or may be determined has the right to request an administrative hearing by filing a written petition with the Suwannee River Water Management District (District), or may choose to pursue mediation as an alternative remedy under Section 120.569 and 120.573, Florida Statutes, (F.S.), before the deadline for filing a petition. Choosing mediation will not adversely affect the right to a hearing if mediation does not result in a settlement. The procedures for pursuing mediation are set forth in Sections 120.569 and 120.57 F.S. Pursuant to Rule 28-106.111, Florida Administrative Code, (F.A.C.), the petition must be filed at the office of the District Clerk at District Headquarters, 9225 C.R. 49, Live Oak, Florida 32060 within twenty-one (21) days of receipt of written notice of the decision or within twenty-one (21) days of newspaper publication of the notice of District decision (for those persons to whom the District does not mail actual notice). A petition must comply with Chapter 28-106, F.A.C.
- 2. If the Governing Board takes action which substantially differs from the notice of District decision to grant or deny the pe1mit application, a person whose substantial interests are or may be determined has the right to request an administrative hearing or may choose to pursue mediation as an alternative remedy as described above. Pursuant to Rule 28-106.111, F.A.C., the petition must be filed at the office of the District Clerk at District Headquarters, 9225 C.R. 49, Live Oak, Florida 32060 within twenty-one (21) days of receipt of written notice of the decision or within twenty-one (21) days of newspaper publication of the notice of District decision (for those persons to whom the District does not mail actual notice). Such a petition must comply with Chapter 28-106, F.A.C.
- 3. A substantially interested person has the right to a formal administrative hearing pursuant to Section 120.569 and 120.57(1), F.S., where there is a dispute between the District and the party regarding an issue of material fact. A petition for formal hearing must comply with the requirements set forth in Rule 28-106.201, F.A.C.
- 4. A substantially interested person has the right to an informal hearing pursuant to Section 120.569 and 120.57(2), F.S., where no material facts are in dispute. A petition for an informal hearing must comply with the requirements set forth in Rule 28-106.301, F.A.C.
- 5. A petition for an administrative hearing is deemed filed upon receipt of the petition by the Office of the District Clerk at the District Headquarters in Live Oak, Florida.
- 6. Failure to file a petition for an administrative hearing within the requisite time frame shall constitute a waiver of the right to an administrative hearing pursuant to Rule 28-106.111, F.A.C.
- 7. The right to an administrative hearing and the relevant procedures to be followed is governed by Chapter 120, Florida Statutes, and Chapter 28-106, F.A.C.
- 8. Pursuant to Section 120.68, F.S., a person who is adversely affected by final District action may seek review of the action in the District Court of Appeal by filing a notice of appeal pursuant to the Florida Rules of Appellate Procedure, within 30 days of the rendering of the final District action.
- 9. A party to the proceeding before the District who claims that a District order is inconsistent with the provisions and purposes of Chapter 3 73, F. S., may seek review of the order pursuant to Section 373.114, F.S., by the Florida Land and Water Adjudicatory Commission, by filing a request for review with the Commission and serving a copy of the Department of Environmental Protection and any person named in the order within 20 days of adoption of a rule or the rendering of the District order.
- 10. For appeals to the District Courts of Appeal, a District action is considered rendered after it is signed on behalf of the District, and is filed by the District Clerk.

11. Failure to observe the relevant time frames for filing a petition for judicial review, or for Commission review, will result in waiver of the right to review.

CERTIFICATE OF SERVICE

I hereby certify that a copy of the foregoing Notice of Rights has been sent to:

Janak Shukla ERA INVESTMENTS LLC 3010 W Us Highway 90 Lake City, FL 32055-7711

This January 04, 2018

Deputy Clerk

Suwannee River Water Management District

9225 C.R. 49

Live Oak, Florida 32060

386.362.1001 or 800.226.1066 (Florida only)

cc: File Number: ERP-023-209165-3

NOTICING INFORMATION

Dear Permittee:

Please be advised that the Suwannee River Water Management District (District) has not published a notice in the newspaper advising the public that it has issued a permit for this project.

Newspaper publication, using the District's form, notifies members of the public of their right to challenge the issuance of the permit. If proper notice is given by newspaper publication, then there is a 21-day time limit to file a petition challenging the issuance of the permit.

To close the point of entry for filing a petition, you may publish (at your own expense) a onetime notice of the District's decision in a newspaper of general circulation within the affected area as defined in Section 50.011 of the Florida Statutes. If you do not publish a newspaper notice, the time to challenge the issuance of your permit will not expire.

A copy of the notice and a partial list of newspapers of general circulation are attached for your convenience. However, you are not limited to those listed newspapers. If you choose to close the point of entry and the notice is published, the newspaper will return to you an affidavit as proof of publication. In accordance with 40B-1.1010(4), F.A.C., a copy of the affidavit shall be provided to the District within 14 days of publication. A scanned copy of the affidavit may be forwarded to Tilda Musgrove by email at *tjm@srwmd.org* (preferred method) or send the original affidavit of publication to:

Tilda Musgrove Resource Management 9225 CR 49 Live Oak, FL 32060

If you have any questions, please contact me at 386.362.1001. Sincerely,

Tilda Musgrove

Business Resource Specialist

Tilda Musgreve

Resource Management

NOTICE OF AGENCY ACTION TAKEN BY THE SUWANNEE RIVER WATER MANAGEMENT DISTRICT

Notice is given that the followin	g permit was issued on		.5
(Name and address of applican	it)		·
permit#	. The project is located	d in	County, Section
, Township	South, Range		The permit authorizes a surface
water management system on	acres for		p and a sum and a sum as a
			known as
The I	receiving water body is		
(F.A.C.), the petition must be file Management Business Resource 32060 or by e-mail to tjm@srwn	a written petition with the napter 28-106 and Rule ed (received) either by c ce Specialist at District I nd.org, within twenty-on	e Suwan 40BB-1 delivery a Headqua le (21) da	nee River Water Management 1010, Florida Administrative Code at the office of the Resource rters, 9225 CR 49, Live Oak FL
actual notice). A petition must o	comply with Sections 12 106, F.A.C. The District	0.54(5)(l will not a	accept a petition sent by facsimile

A petition for an administrative hearing is deemed filed upon receipt of the complete petition by the District Clerk at the District Headquarters in Live Oak, FL during the District's regular business hours. The District's regular business hours are 8 a.m. – 5 p.m., excluding weekends and District holidays. Petitions received by the District Clerk after the District's regular business hours shall be deemed filed as of 8 a.m. on the next regular District business day.

The right to an administrative hearing and the relevant procedures to be followed are governed by Chapter 120, Florida Statutes, Chapter 28-106, Florida Administrative Code, and Rule 40B-1.1010, Florida Administrative Code. Because the administrative hearing process is designed to formulate final agency action, the filing of a petition means the District's final action may be different from the position taken by it in this notice. Failure to file a petition for an administrative hearing within the requisite time frame shall constitute a waiver of the right to an administrative hearing. (Rule 28-106.111, F.A.C.).

If you wish to do so, you may request the Notice of Rights for this permit by contacting the Business Resource Specialist in the Division of Resource Management (RM), 9225 CR 49, Live Oak,, FL 32060, or by phone at 386.362.1001.

NEWSPAPER ADVERTISING

ALACHUA

Gainesville Sun Legal Advertising PO Box 14747 Gainesville, FL 32614 352,372,4222

BRADFORD

Bradford County Telegraph, Legal Advertising P. O. Drawer A Starke, FL 32901 904-964-6305/ fax 904-964-8628

COLUMBIA

Lake City Reporter 180 E Duval Street Lake City, FL 32055 386.754.0401

DIXIE

Dixie County Advocate 174 County Road 351 Cross City, FL 32628 352.498.3312

GILCHRIST

Gilchrist County Journal 207 N Main St Trenton, FL 32693 352.463.7135

HAMILTON

Jasper News 521 Demorest Street SE Live Oak, FL 32064 386.362.1734

JEFFERSON

Monticello News PO Drawer 772 Madison, FL 32344 850.997.3568

LAFAYETTE

Mayo Free Press 521 Demorest Street SE Live Oak, FL 32064 386.362.1734

LEVY

Levy County Journal PO Box 159 Bronson, FL 32621 352.486.2312

MADISON

Madison Carrier PO Drawer 772 Madison, FL 32344 850.973.4141

SUWANNEE

Suwannee Democrat 521 Demorest Street SE Live Oak, FL 32064 386.364.1734

TAYLOR

Taco Times PO Box 888 Perry, FL 32348 850.584.5513

UNION

Union County Times 125 E Main Street Lake Butler, FL 32054 386.496.2261 This application is for Phase 2 of the Lake City Hotels project and intends to modify the original master permit with an updated accounting of the impervious associated with Phase 1 and Phase 2. This scope of the Phase 2 project consists of Hotel B and the associated parking lot with drive isle connections to the drive isles of Phase 1. No change to the construction limits, flow patterns, control structures, etc. are proposed with this phase of the project. Please see the site plans for Phase 2 for additional details. The resulting impervious area accounting for the master system following Phase 2 construction will be as follows.

Impervious Areas Accour	nting with Propo	sed Phase 2		
Impervious (Existing) *	182,297 sf	4.1850 ac		
Impervious (Phase 1)	107,983 sf	2.4789 ac		
Impervious (Phase 2)	74,902 sf	1.7195 ac		
Impervious (Future)	176,585 sf	4.0538 ac		
Total Permitted Impv.	541,767 sf	12.4373 ac		
* Existing Impervious Aera consisted of the Quality Inn,				
Ruby Tuesdays, and Holiday Inn to the west of the site				

Please see the following exhibits attached to this stormwater report. Site Plans for the proposed Phase 2 construction have also been submitted for review. Historical details concerning the master permit can be found under SRWMD ERP # 209165-3 as well as previous permit sequences.

Attached Exhibits:

- Master Plan Post-Development Drainage Map permitted under SRWMD ERP # 209165-3
- Site Plan for Phase 1 Hotel A reflecting the current conditions of the site
- Proposed Site Plan for Phase 2 Hotel B reflecting the next phase of the development

Please let us know if you need any additional information for your review.

Sincerely,

Gmuer Engineering, LLC

Christopher A Gmuer, PE

President

Christopher

A Gmuer

2024.04.23

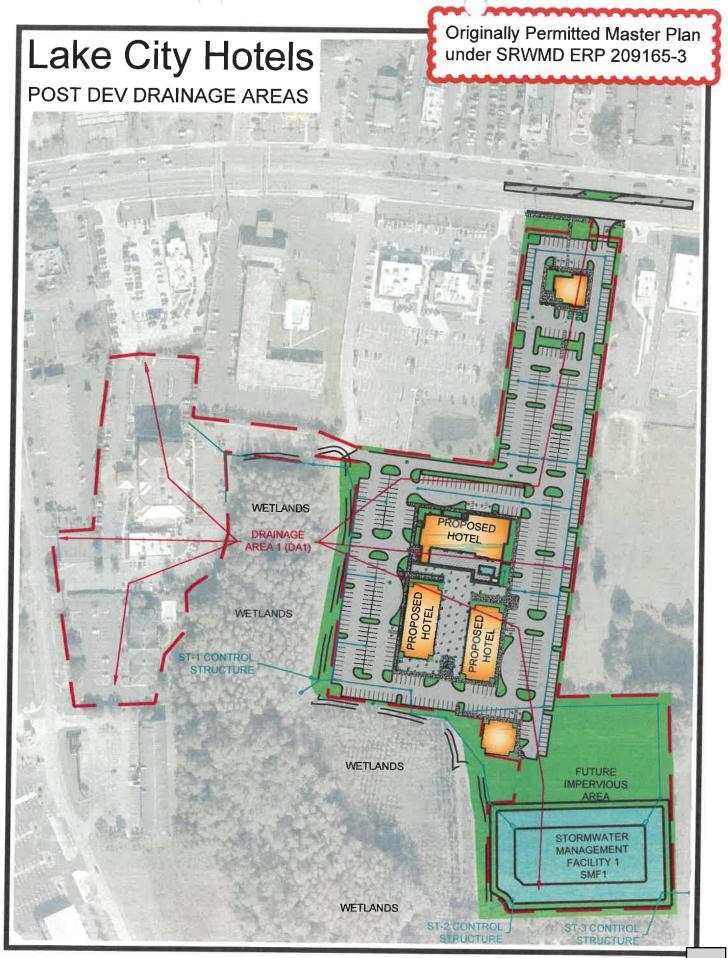
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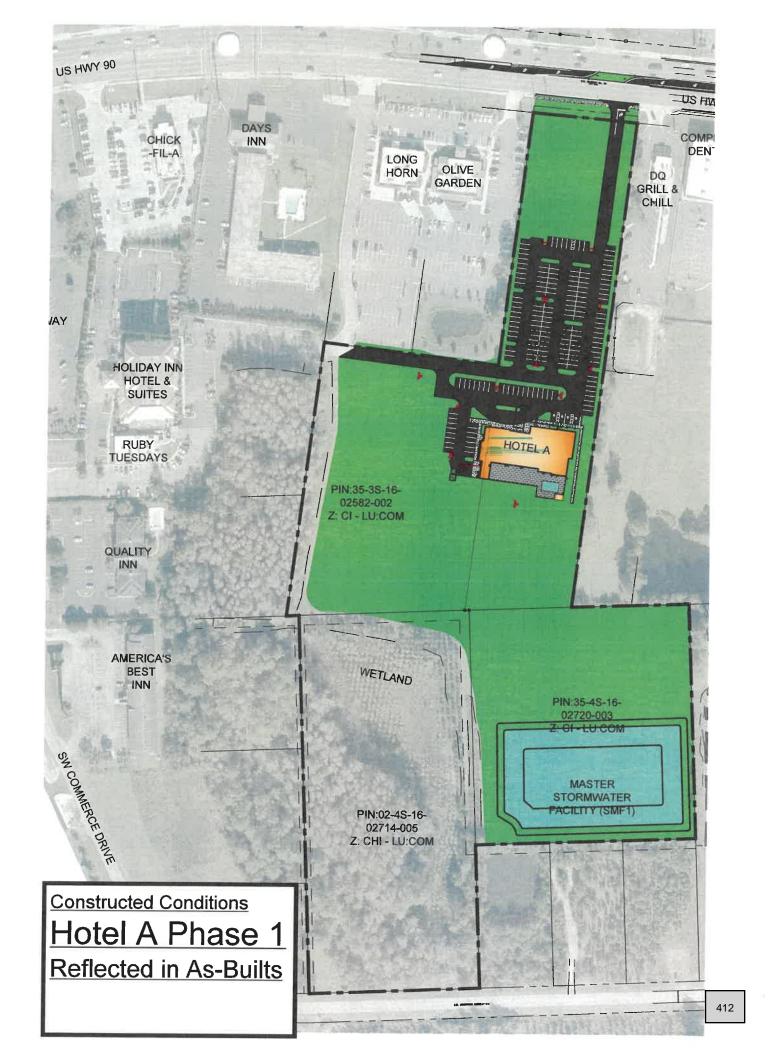
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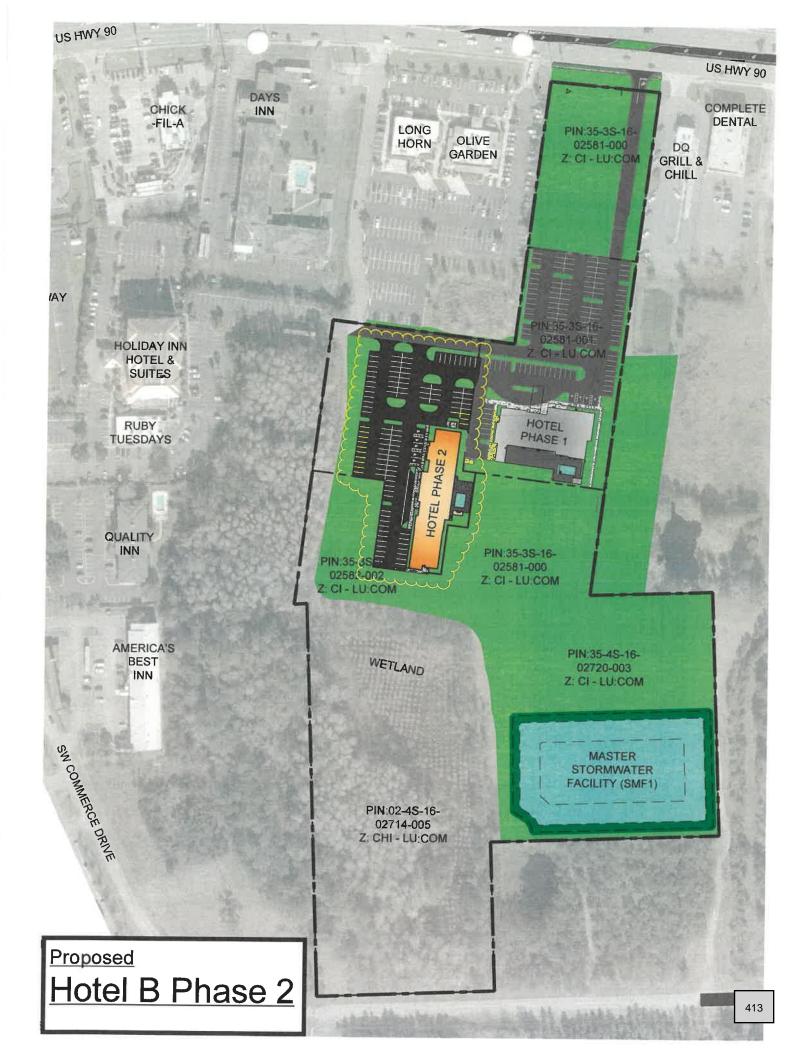
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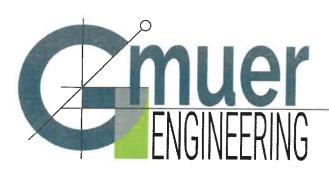
This item has been electronically signed and sealed by Christopher A. Gmuer, PE. using a SHA authentication code

Printed copies of this document are not considered signed and sealed and the SHA authentication code must be verified on any electronic copies









2603 NW 13th St, Box 314 Gainesville, FL 32609 Ph. (352) 281-4928

gmuereng.com

Re: Lake City Hotels - Phase 2

Consistency with NFPA 1 Fire Code, Florida Edition, Chap 18

Fire Apparatus Access to the site is being provided via "parking lot lanes" as allowed in 18.2.3.1.2

Fire Apparatus Access to within 50ft of the front and other doors is met as required by 18.2.3.2.1

Parking lots surround the Hotel and are considered access roads per 18.2.3.2.1.2

The parking lot allows access to all parts of the first floor within 150ft per 18.2.3.2.2

The hotel will be protected by an automatic sprinkler system.

The parking lot has multiple access points meeting the multiple access points requirement of 18.2.3.3

All dimensional, surface, turning, grade, etc requirements of 18.2.3.5 for access roads have been met.

A looped water main through the site was constructed as part of the Phase 1 plan to comply with 18.3.1

Fire Flow Requirements for Buildings per NFPA 1 Section 18.4

Building Area = 16,105 SF Footprint X 4-Stories = 64,420 SF GFA, Proposed Use = Hotel

Type of Construction: Either Type II(111), III(211), IV(2HH), or V(111) per NFPA 220

Fire Flow Area: 64,420 SF

Minimum Fire Flow and Duration: 4,500 GPM for 4 Hours (per 18.4.5.2.1)

Qualifies for Approved Automatic Sprinkler System Reduction: Yes, 75% Reduction to 1,125 GPM

Final Minimum Fire Flow and Duration: 1,125 GPM for 2 Hours is the minimum allowed by 18.4.5.3.2 & 4

ISO Fire Flow Calculations are not applicable to sprinklered buildings.

Christopher A Gmuer 2024.04.23

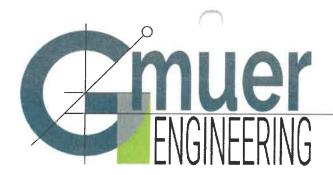
Christopher A Gmuer, PE FL PE 71599

23:06:34

No. 71599

This item has been electronically signed and se Christopher A. Gmuer, PE. using a SHA authentication code

Printed copies of this document are not considered signed and sealed and the SHA authentication code must be verified on any electronic copies



2603 NW 13th St, Box 314 Gainesville, FL 32609 Ph. (352) 281-4928

gmuereng.com

Lake City Hotels Phase 2 Concurrency Impact Analysis April 23, 2024

The proposed project adds a second hotel to the site with 119 rooms.

Transportation

Description	Use	Variable	AADT	AM Peak	PM Peak	
Hotel	ITE#310	119 Rooms	951	55	70	

Potable Water

Average Daily Flow (ADF): 119 Rooms x 100 GPD = 11,900 GPD

Peak Flow: 11,900 GPD x 2 Peaking Factor / 16 Hour Operating Period x 1 hour / 60 min = 25 GPM

Minimum 2" Water Meter with 4" RPZ BF Preventer

Sanitary Sewer

Average Daily Flow (ADF): 119 Rooms x 100 GPD = 11,900 GPD

Peak Flow: 11,900 GPD x 2 Peaking Factor / 16 Hour Operating Period x 1 hour / 60 min = 25 GPM

Solid Waste

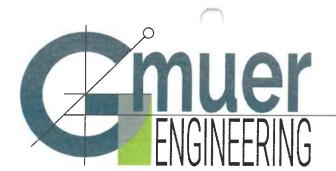
2 lbs/ room / day x 119 room x 365 days = 43.5 tons per year

Please let us know if you need any additional information for your review.

Sincerely,

Gmuer Engineering, LLC

Christopher A Gmuer, PE, President



2603 NW 13th St, Box 314 Gainesville, FL 32609 Ph. (352) 281-4928

gmuereng.com

Lake City Hotels Phase 2 - April 23, 2024 Consistency with the Comprehensive Plan

The proposed project adds a second hotel to the site with 119 rooms. The underlying land use for the property is Commercial and the zoning designation is CI, Commercial Intensive. The proposed hotel use is a permitted principal use and structure of the CG zoning district and per 14.13.2 is also a permitted principal use and structure. No special exception required.

The Comprehensive Plan language is provided and followed with the consistency statement in bold.

I FUTURE LAND USE ELEMENT

GOAL I - IN RECOGNITION OF THE IMPORTANCE OF ENHANCING THE QUALITY OF LIFE IN THE CITY, DIRECT DEVELOPMENT TO THOSE AREAS WHICH HAVE IN PLACE, OR HAVE AGREEMENTS TO PROVIDE, SERVICE CAPACITY TO ACCOMMODATE GROWTH IN AN ENVIRONMENTALLY ACCEPTABLE MANNER.

The property is zoned for the proposed use and infrastructure to support the proposed development was constructed in the first phase of the project to support this and all future phases.

OBJECTIVE I.1 The City Concurrency Management System shall make available or schedule for availability the public facilities for future growth and urban development as development occurs in order to provide for urban densities and intensities within the City.

The infrastructure to support the proposed development was constructed in the first phase of the project.

Policy I.1.1 The location of higher density residential, high intensity commercial and heavy industrial uses shall be directed to areas adjacent to arterial or collector roads, identified on the Future Traffic Circulation Map, where public facilities are available to support such higher density or intensity.

The proposed development is located in direct proximity to US90 adjacent to other hotels. An outparcel is being retained along the direct frontage of US90 for a restaurant or other use that would benefit from more direct access.

Policy I.1.2 The land development regulations of the City shall be based on and be consistent with the following land use classifications and corresponding standards for densities and intensities and shall establish the following floor area ratio(s) to be applied to each classification of land use:

COMMERCIAL

Lands classified as commercial use consist of areas used for the sale, rental, and distribution of products or performance of services, as well as public, charter and private elementary, middle and high schools. In addition, off-

site signs, churches and other houses of worship, private clubs and lodges, residential dwelling units, which existed within this category on the date of adoption of this objective, and other similar uses compatible with commercial uses may be approved as special exceptions and be subject to an intensity of less than or equal to 0.25 floor area ratio except within the (CG) Commercial, General, (CI) Commercial, Intensive, (C-CBD) Commercial-Central Business District and (CHI) Commercial, Highway Interchange districts being subject to an intensity of less than or equal to 1.0 floor area ratio.

The site is located within the (CI) Commercial, Intensive zoning district and meets the max FAR.

OBJECTIVE I.3 The City shall require that all proposed development be approved only where the public facilities meet or exceed the adopted level of service standard.

The infrastructure to support the proposed development was constructed in the first phase of the project and was designed to support all future phases.

Policy I.3.1 The City shall limit the issuance of development orders and permits to areas where the adopted level of service standards for the provision of public facilities found within the Comprehensive Plan are maintained. This provision also includes areas where development orders were issued prior to the adoption of the Comprehensive Plan.

The infrastructure to support the proposed development was constructed in the first phase of the project and was designed to support all future phases.

II TRANSPORTATION ELEMENT

GOAL II - PROVIDE FOR A TRANSPORTATION SYSTEM WHICH SERVES EXISTING AND FUTURE LAND USES.

Policy II.1.2. The City shall control the number and frequency of connections and access points of driveways and roads to arterials and collectors by requiring access points for state roads to be in conformance with Chapter 14-96 and 14-97, Florida Administrative Code, and the following requirements for non-state roads:

The initial phase of this project master planned and provided a traffic study that implemented the shared driveway connection to US90 and all of the shared internal drive isles interconnections which allow interconnectivity between all the adjacent properties. This includes access to SW Commerce Dr to the west through the Ruby Tuesday and Burger King parking lots, north through the Burger King and Chick-fil-a and LongHorn and Olive Garden parking lots.

Policy II.1.3. The City shall continue to require development to provide safe and convenient on-site traffic flow, which includes the provision for vehicle parking.

This proposed phase continues to follow the master plan outlined in the previous application.

Please let us know if you need any additional information for your review.

Gmuer Engineering, LLC

Christopher A Gmuer, PE

President

LEGAL DESCRIPTION:

PARCEL NO. 1; (PER O.R. 1048, PAGE 2678)

LOT OR BLOCK 8, LAKE HARRIS FARMS SUBDIVISION, UNIT "A". ACCORDING TO PLAT THEREOF RECORDED IN PLAT BOOK 1, PAGE 22, PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA, EXCEPT THAT PORTION DEEDED TO THE STATE OF FLORIDA FOR ROAD RIGHT-OF-WAY PURPOSES BY CONVEYANCE RECORDED IN DEED BOOK 78, PAGE 275, PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA. SAID LANDS BEING SITUATE IN THE SW 1/4 OF THE SE 1/4 OF SECTION 35, TOWNSHIP 3 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA.

PARCEL NO. 2: (PER O.R. 1048, PAGE 2678)

BEGIN AT THE NORTHEAST CORNER OF THE NW 1/4 OF NE 1/4, SECTION 2, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA AND RUN S 87'54'48"W ALONG THE NORTH LINE OF SAID SECTION 2, 452.80 FEET; THENCE S 02'27'06"E, 481.01 FEET; THENCE N 87'54'48"E, 452.80 FEET TO THE EAST LINE OF SAID NW 1/4 OF NE 1/4; THENCE N 02'27'06"W ALONG SAID EAST LINE, 481. 01 FEET TO THE POINT OF BEGINNING. COLUMBIA COUNTY, FLORIDA.

PARCEL NO. 3: (PER O.R. 1240, PAGE 658) TOWNSHIP 3 SOUTH - RANGE 16 EAST

SECTION 35: BEGINNING AT A POINT WHICH IS THE SE CORNER OF LOT 9 OF "LAKE HARRIS FARMS" AS PER PLAT THEREOF RECORDED IN PLAT BOOK 1, PAGE 21, OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA, SAID POINT BEING N 87'55'17" E 868.20 FEET FROM THE SW CORNER OF THE SW 1/4 OF THE NE 1/4 OF SECTION 35, TOWNSHIP 3 SOUTH, RANGE 16 EAST; THENCE N 7'03'47" E ALONG THE EAST BOUNDARY OF SAID LOT 9
718.40 FEET; THENCE N 84'41"43" W 360.00 FEET; THENCE S 7'03'47" W PARALLEL TO THE EAST BOUNDARY OF SAID LOT 9 765.25 FEET TO THE SOUTH BOUNDARY OF LOT 10 OF SAID "LAKE HARRIS FARMS"; THENCE N 87'55'17" E ALONG THE SOUTH BOUNDARY OF LOTS 10 AND 9 OF SAID "LAKE HARRIS FARMS" 364.46 FEET TO THE POINT OF BEGINNING - COLUMBIA COUNTY, FLORIDA.

LESS AND EXCEPT THE LANDS DESCRIBED IN OFFICIAL RECORDS BOOK 1242, PAGE 949 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

PARCEL NO. 4: (PER O.R. 1084, PAGE 1709)

LOT 4, INTERSTATE COMMERCE CENTER, A SUBDIMISION ACCORDING TO THE PLAT THEREOF RECORDED IN PLAT BOOK 5, PAGE 37 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

P.DeWitt Cason Clerk of Courts, Co....ibia County, Florida Doc Deed: 0.70

This Instrument Prepared By: RALPH R. DEAS, ESQUIRE 227 SE Hernando Avenue Lake City, Florida 32025 Telephone: (386) 754-0771

The Preparer of this Instrument has prepared NO Title Examination nor has the Preparer issued any Title Insurance or furnished any opinion regarding title, existence of liens, quantity of lands included, or the location of boundaries. The names, addresses, tax identification numbers and legal description are furnished by a party to this Instrument.

Inst: 201812015461 Date: 07/25/2018 Time: 3:41PM Page 1 of 3 B: 1365 P: 1047, P.DeWitt Cason, Clerk of Court Columbia, County, By: BD Deputy ClerkDoc Stamp-Deed: 0.70

CORRECTIVE SPECIAL WARRANTY DEED

THIS WARRANTY DEED, made and executed this 12th day of 12th day of 2018, by and between ANILKUMAR D. PATEL and HEMA PATEL (also known as HEMLATTA PATEL), husband and wife, 162 NW Birdie Place, Lake City, FL 32055, Grantor(s), and ERA INVESTMENTS, LLC, a Florida limited liability company, 3010 U.S. Highway 90 West, Lake City, Florida 32055, Grantee,

WITNESSETH:

That, for and in consideration of the sum of Ten and no/100 Dollars (\$10.00) and other valuable considerations in hand paid by the Grantee to the Grantor(s), the receipt and sufficiency whereof are hereby acknowledged, the Grantor(s) has granted, bargained and sold to the Grantee, and the Grantee's assigns forever, the following-described real property, situate, lying and being in Columbia County, Florida:

See Exhibit A, attached hereto and incorporated herein by reference.

N.B.: The purpose of this Corrective Deed is to correct the description of the property conveyed by that deed recorded at Official Records Book 1334, page 1565-1567, public records of Columbia County, Florida, which failed to set forth the parcel excluded from the deed as a "less-out," a parcel previously conveyed to a stranger to the said deed.

SUBJECT TO restrictions, easements and outstanding mineral rights of record, if any, and taxes after December 31, 2016.

TOGETHER with all the tenements, hereditaments and appurtenances thereunto belonging or in anywise appertaining.

TO HAVE AND TO HOLD the same in fee simple forever.

And the Grantor(s) does hereby warrant the title to said land, and will defend the same against the lawful claims of all parties claiming by, through, or under the said Grantor.

IN WITNESS WHEREOF, the Grantor(s) has signed and sealed these presents the day and year first above written.

Inst. Number: 201812015461 Boo' 1365 Page: 1048 Page 2 of 3 Date: 7/25 18 Time: 3:41 PM P.DeWitt Cason Clerk of Courts, County, Florida Doc Deed: 0.70

Signed, sealed and delivered In Presence of:	
Signature of Witness	ANILKUMAR D. PATEL (SEAL)
Printed/Typed Name of Witness	
Signature of Witness	HEMA PATEL (SEAL) (also known as HEMLATTA PATEL)
MITHILESH AMIN Printed/Typed Name of Witness	
STATE OF FLORIDA COUNTY OF MARION	<u></u>
personally known OR known as HEMLATTA PATEL), to me	cknowledged before me by ANILKUMAR D. PATEL, to me n after production of nd who DID NOT take an oath, and by HEMA PATEL (also personally known OR known after production of as identification, and who DID NOT take an
	Simethand Pand
(Seal if any)	Signature of Notary
	Printed/Typed Name of Notary Notary Public, State of Florida at Large Serial No. if any: Commission Expires:



EXHIBIT "A"

TOWNSHIP 3 SOUTH - RANGE 16 EAST

SECTION 35: Beginning at a point which is the Southeast corner of Lot 9 of "Lake Harris Farms" as per plat thereof recorded in Plat Book 1, Page 21 of the public records of Columbia County, Florida, said point being N 87 degrees 55 minutes 17 seconds E 868.20 feet from the Southwest corner of the SW ¼ of the NE ¼ of Section 35, Township 3 South, Range 16 East; thence N 7 degrees 03 minutes 47 seconds E along the East boundary of said Lot 9 718.40 feet; thence N 84 degrees 41 minutes 43 seconds W 360.00 feet; thence S 7 degrees 03 minutes 47 seconds W parallel to the East boundary of said Lot 9 765.25 feet to the South boundary of Lot 10 of said "Lake Harris Farms"; thence N 87 degrees 55 minutes 17 seconds E along the South boundary of Lots 10 and 9 of said "Lake Harris Farms" 364.46 feet to the POINT OF BEGINNING. Columbia County, Florida.

LESS AND EXCEPT that portion of the above-described real property conveyed by Grantors to FLORIDA SE, INC., recorded in Official Records Book 1242, pages 925, 929, and 933, public records of Columbia County, Florida, more particularly described as follows: Commence at the Southeast corner of Lot 9 of "Lake Harris Farms" as per plat thereof recorded in Plat Book 1, Page 21 of the public records of Columbia County, Florida,; thence with the East line of said Lot 9 N 8 degrees 23 minutes 37 seconds E, a distance of 506.22 feet to the POINT OF BEGINNING; thence continue with said East line, N 8 degrees 23 minutes 37 seconds E, a distance of 212.18 feet to the Southeast corner of premises described in Official Records Book 706, page 199; thence with the South line thereof, N 83 degrees 21 minutes 53 seconds W, a distance of 360.00 feet to the West line of said premises described in Official Records Book 1199, page 515; thence with said West line, S 08 degrees 23 minutes 37 seconds W, a distance of 201.13 feet; thence S 81 degrees 36 minutes 23 E, a distance of 359.83 feet to the POINT OF BEGINNING.

TOGETHER WITH a non-exclusive perpetual easement for ingress and egress over and across the West 30 feet of the following-described property:

TOWNSHIP 3 SOUTH - RANGE 16 EAST

SECTION 35: Beginning at a point which is the Southeast corner of Lot 9 of "Lake Harris Farms" as per plat thereof recorded in Plat Book 1, Page 21 of the public records of Columbia County, Florida, said point being N 87 degrees 55 minutes 17 seconds E 868.20 feet from the Southwest corner of the SW ¼ of the NE ¼ of Section 35, Township 3 South, Range 16 East; thence N 7 degrees 03 minutes 47 seconds E along the East boundary of said Lot 9 718.40 feet for the POINT OF BEGINNING; thence continue N 7 degrees 03 minutes 47 seconds E along said East boundary 310.00 feet to the South right-of-way line of State Road No. 10 (U.S. Highway No. 90), said point being 87.00 feet from and at right angle to the survey center line of said State Road; thence N 84 degrees 41 minutes 43 seconds W along said South right-of-way line 312.51 feet to the point of curvature of a right-of-way line curve being concave Southwesterly and having a radius of 3276.57 feet; thence Northwesterly along said South right-of-way line curve, a chord bearing and distance of N 85 degrees 06 minutes 35 seconds W 47.40 feet; thence S 7 degrees 03 minutes 47 seconds W parallel to the East boundary of said Lot 9 309.66 feet; thence S 84 degrees 41 minutes 43 seconds E 360.00 feet to the POINT OF BEGINNING. Columbia County, Florida.

N.B.: Grantor hereby states that the subject property is not the homestead of Grantor or of any member of Grantor's family. Neither the Grantor nor any member of Grantor's family lives or resides on the subject property or on any land adjacent thereto.



GROWTH MANAGEMENT DEPARTMENT 205 North Marion Ave, Lake City, FL 32055 Phone: 386-719-5750

E-mail: growthmanagement@lcfla.com

AGENT AUTHORIZATION FORM

, ERA Investments, LLC	(owner name), owner of property parcel
number 35-35-16-02582-002	(parcel number), do certify that
the below referenced person(s) listed on this for is an officer of the corporation; or, partner as de said person(s) is/are authorized to sign, speak a relating to this parcel.	rm is/are contracted/hired by me, the owner, or,
Printed Name of Person Authorized	Signature of Authorized Person
1. Christopher A. Gmuer, PE	1. OFFATO-
2.	2.
3.	3.
4.	4.
5.	5.
I, the owner, realize that I am responsible for all a with, and I am fully responsible for compliance we Development Regulations pertaining to this parcell at any time the person(s) you have authorized officer(s), you must notify this department in writing authorization form, which will supersede all previous unauthorized persons to use your name and/or like the	is/are no longer agents, employee(s), or no of the changes and submit a new letter of ous lists. Failure to do so may allow cense number to obtain permits.
The above person, whose name is personally appeared before me and is known by retype of LD.) OTARY'S SIGNATURE	ne or has produced identification
	MY COMMISSION EXPIRES 6-22-2027

Last Update: 4/24/2024 1:55:31 AM EDT

Tax Record

11/30/2023

PAYMENT



Details

Tax Record

» Print View Legal Desc. Tax Payment Payment History Print Tax Bill NEW! Change of Address

Searches

Account Number **GEO** Number

Owner Name

Property Address Mailing Address

Site Functions

Tax Search

Local Business Tax Contact Us County Login Home

Register for eBill

Ad Valorem Taxes and Non-Ad Valorem Assessments

The information contained herein does not constitute a title search and should not be relied on as such.

Account Number	Тах Туре		Tax Year
R02582-002	REAL ESTATE		2023
Mailing Address ERA INVESTMENTS LLC 162 NW BIRDIE PL	Property Addre	ss	
LAKE CITY FL 32055	GEO Number 353S16-02582-0	02	
Exempt Amount	Taxable Value	THE REAL PROPERTY.	Profiles Analysis and San
See Below	See Below		
Exemption Detail NO EXEMPTIONS	Millage Code	Escrow	Code
Legal Description (click for	or full description)		
35-3S-16 1001/10013.97 Acre	es BEG SE COR OF LOT 9 LA	KE HARRIS	FARMS, RUN
NE 718.40 FT, W 360 FT, S	765.25 FT, E 364.46 FT TO	POB. BEIN	G PART OF

& 10 LAKE HARRIS FARMS UNIT A. EX 1.75 AC DESC ORB 1242-933 & EX .42 AC DESC IN PART OF WD 1406-321(LYING See Tax Roll For Extra Legal

Taxing Authority	Rate	Assessed Value	Exemption	Taxable Value	Taxes
CITY OF LAKE CITY	4.9000	24,213	0	\$24,213	\$118.64
BOARD OF COUNTY COMMISSIONERS	7.8150	24,213	0	\$24,213	\$189.22
COLUMBIA COUNTY SCHOOL BOARD				+ /	4103,22
DISCRETIONARY	0.7480	24,213	0	\$24,213	\$18.11
LOCAL	3.2170	24,213	0	\$24,213	\$77.89
CAPITAL OUTLAY	1.5000	24,213	0	\$24,213	\$36.32
SUWANNEE RIVER WATER MGT DIST	0.3113	24,213	0	\$24,213	\$7.54
LAKE SHORE HOSPITAL AUTHORITY	0.0001	24,213	0	\$24,213	\$0.00
Total Millage	18.4914	To	otal Taxes		\$447.72

Non-Ad Valorem Assessments			
Code	Levying Authority	Amount	
XLCF	CITY FIRE ASSESSMENT	\$61.26	

If Paid By	Amount Due
Taxes & Assessments	\$508.98
Total Assessments	\$61.26

2023

				\$0.00
Date Paid	Transaction	Receipt	Item	Amount Daid

2101381.0003

Prior Years Payment History

\$488.62

	Prior Year Taxes Due	
NO DELINQUENT TAXES		



Department of State / Division of Corporations / Search Records / Search by Entity Name /

Detail by Entity Name

Florida Limited Liability Company ERA INVESTMENTS, LLC

Filing Information

Document Number

L05000041345

FEI/EIN Number

26-0119061

Date Filed

04/21/2005

State

FL

Status

ACTIVE

Last Event

REINSTATEMENT

Event Date Filed

09/27/2017

Principal Address

162 NW BIRDIE PLACE LAKE CITY, FL 32055

Changed: 05/01/2019

Mailing Address

162 NW BIRDIE PLACE LAKE CITY, FL 32055

Changed: 05/01/2019

Registered Agent Name & Address

SHUKLA, JANAK

321 SW RIDGE VIEW PLACE

LAKE CITY, FL 32024

Name Changed: 09/27/2017

Address Changed: 05/01/2019

Authorized Person(s) Detail

Name & Address

Title MGR

SHUKLA, JANAK R

3004,US HWY 90 WEST LAKECITY, FL 32055

Annual Reports

Report Year	Filed Date
2021	03/31/2021
2022	04/25/2022
2023	03/29/2023

Document Images

03/29/2023 – ANNUAL REPORT	View image in PDF format
04/25/2022 ANNUAL REPORT	View image in PDF format
03/31/2021 ANNUAL REPORT	View image in PDF format
04/30/2020 ANNUAL REPORT	View image in PDF format
05/01/2019 - ANNUAL REPORT	View image in PDF format
03/28/2018 ANNUAL REPORT	View image in PDF format
09/27/2017 - REINSTATEMENT	View image in PDF format
02/04/2016 ANNUAL REPORT	View image in PDF format
02/21/2015 ANNUAL REPORT	View image in PDF format
04/19/2014 - ANNUAL REPORT	View image in PDF format
03/21/2013 ANNUAL REPORT	View image in PDF format
02/27/2012 - ANNUAL REPORT	View image in PDF format
02/21/2011 ANNUAL REPORT	View image in PDF format
04/05/2010 - ANNUAL REPORT	View image in PDF format
03/26/2009 - ANNUAL REPORT	View image in PDF format
04/29/2008 - ANNUAL REPORT	View image in PDF format
02/18/2007 - ANNUAL REPORT	View image in PDF format
02/16/2006 - ANNUAL REPORT	View image in PDF format
04/21/2005 - Florida Limited Liabilites	View image in PDF format

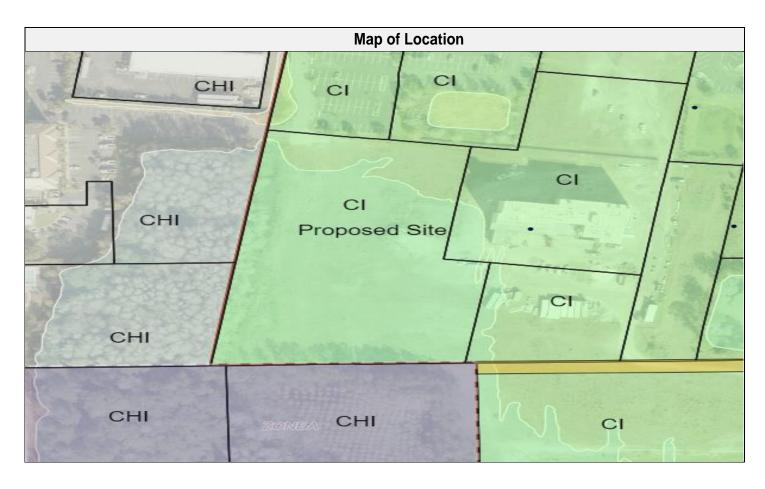
Florida Department of State, Division of Corporations

LAKE CITY GROWTH MANAGEMENT STAFF ANALYSIS REPORT

	Project Information
Project Name and Case No.	Lake City Hotels Phase 2 site plan review SPR24-06
Applicant	Christopher Gmuer, PE
Owner	ERA Investments, LLC
Requested Action	Site plan review for a, hotel, on parcel 02582-002
Hearing Date	06-11-2024
Staff Analysis/Determination	Sufficient for Review
Prepared By	Robert Angelo

Subject Property Information		
Size	+/- 3.971 Acres	
Location	Located behind Olive Garden	
Parcel Number	02582-002	
Future Land Use	Commercial	
Proposed Future Land Use	Commercial	
Current Zoning District	Commercial Intensive	
Proposed Zoning	Commercial Intensive	
Flood Zone-BFE	Flood Zone X and A Base Flood Elevation-N/A	

Land Use Table				
Direction	Future Land Use	Zoning	Existing Use	Comments
N	Commercial	CI	Restaurant	
E	Commercial	CI	Hotel	
S	Commercial	CHI	Vacant	
W	Commercial	CHI	Hotel and Restaurant	





Summary of Request			
Applicant has petitioned for a site plan review for the above parcels to build a hotel. This is phase 2.			



NOTICE LAND USE ACION

A PUBLIC HEARING IS SCHEDULED TO CONCIDER A REQUEST FOR:

SPR 24-06, a petition by Christopher A. Gmuer, P.E., as agent, to request a Site Plan Review approval be granted as provided for in Section 4.13 of the Land Development Regulations, to get approval on site plan for Lake City Hotels Phase 2 for a property located in the Commercial Intensive zoning district, in accordance with the submittal of the petition dated April 26, 2024, to be located on;

PARCEL 35-3s-16-02582-002

TOWNSHIP 3 SOUTH - RANGE 16 EAST

SECTION 35: BEGINNING AT A POINT WHICH IS THE SE CORNER OF LOT 9 OF "LAKE HARRIS FARMS" AS PER PLAT THEREOF RECORDED IN PLAT BOOK 1, PAGE 21, OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA, SAID POINT BEING N 87°55'17" E 868.20 FEET FROM THE SW CORNER OF THE SW 1/4 OF THE NE 1/4 OF SECTION 35, TOWNSHIP 3 SOUTH, RANGE 16 EAST; THENCE N 7°03'47" E ALONG THE EAST BOUNDARY OF SAID LOT 9 245.14 FEET; THENCE N 82°47"32" W 67.51 FEET; THENCE N 07°07"10" E 261.13 FEET; THENCE N 82°56"13" W 292.10 FEET; THENCE S 7°03'47" W PARALLEL TO THE EAST BOUNDARY OF SAID LOT 9 564.12 FEET TO THE SOUTH BOUNDARY OF LOT 10 OF SAID "LAKE HARRIS FARMS"; THENCE N 87°55'17" E ALONG THE SOUTH BOUNDARY OF LOTS 10 AND 9 OF SAID "LAKE HARRIS FARMS" 364.46 FEET TO THE POINT OF BEGINNING - COLUMBIA COUNTY, FLORIDA.

WHEN;	June 11, 2024 at 5:30pm or as soon after.
WHERE:	City Council Meeting Room, Second Floor, City Hall, located at 205 North Marion Avenue, Lake City, Florida. Members of the public may also view the meeting on our YouTube channel at: https://www.youtube.com/c/CityofLakeCity .

Copies of the site plan application are available for public inspection by contacting the Growth Management office at growthmanagement@lcfla.com or by calling 386-719-5820.

At the aforementioned public hearing, all interested parties may be heard with respect to the amendment.

FOR MORE INFORMAITON CONTACT ROBERT ANGELO PLANNING AND ZONING TECHNICIAN AT 386-719-5820

Angelo, Robert

From:

LCR-Classifieds <classifieds@lakecityreporter.com>

Sent:

Tuesday, May 28, 2024 11:00 AM

To:

Angelo, Robert

Subject:

RE: 78430 78431 78432 RE: Non-Legal Ad for P&Z, BOA, and HPA for 06-11-2024

Confirmed!

Thank you much,

Kymberlee Harrison 386-754-0401

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1086 SW Main Blvd. Ste 103, Lake City, FL 32055

PH 386-754-0401

Why Local Newsprint Advertising?

1 Newspaper readers are ENGAGED

2 Newspapers are viewed as TRUSTWORTHY

From: Angelo, Robert <AngeloR@lcfla.com> Sent: Tuesday, May 28, 2024 10:59 AM

To: LCR-Classifieds <classifieds@lakecityreporter.com>

Subject: RE: 78430 78431 78432 RE: Non-Legal Ad for P&Z, BOA, and HPA for 06-11-2024

Looks Good

Thank You
Robert Angelo
City of Lake City
Growth Management
growthmanagement@lcfla.com

200 710 5020

386-719-5820



PLEASE NOTE: Florida has a very broad public records law. Most written communications to or from City officials regarding City business are public records available to the public and media upon request. Your email communications may be subject to public disclosure.

From: LCR-Classifieds < classifieds@lakecityreporter.com >

Sent: Tuesday, May 28, 2024 10:37 AM

To: Angelo, Robert < AngeloR@lcfla.com >

Subject: 78430 78431 78432 RE: Non-Legal Ad for P&Z, BOA, and HPA for 06-11-2024

Here you go! P&Z 2x8 247.50 Historic: 2x6.25 206.25

BOA: 2x6 198.00

Thank you much,

Kymberlee Harrison 386-754-0401

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1086 SW Main Blvd. Ste 103, Lake City, FL 32055

PH 386-754-0401

Why Local Newsprint Advertising?

1 Newspaper readers are ENGAGED

2 Newspapers are viewed as TRUSTWORTHY

From: Angelo, Robert < Angelo R@lcfla.com >

Sent: Friday, May 24, 2024 3:49 PM

To: LCR-Classifieds <<u>classifieds@lakecityreporter.com</u>>

Subject: Non-Legal Ad for P&Z, BOA, and HPA for 06-11-2024

Kym

Please publish this ad in the body of the paper as a display ad in the May 30, 2024 paper.

Thank You Robert Angelo City of Lake City Growth Management growthmanagement@lcfla.com 386-719-5820



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OTICE OF PUBLIC MEETING CITY OF LAKE CITY PLANNING AND ZONING BOARD

THIS SERVES AS PUBLIC NOTICE the Planning and Zoning Board will hold a meeting on Tuesday, June 11, 2024 at 5:30 PM or as soon after.

Agenda items-

- 1. SPR 24-05, Petition submitted by Randall Olney, P.E., (agent) for Concept Companies, (owner), for a Site Plan Review for Dollar General, in a Commercial Intensive zoning district, and located on parcel 08127-005, which is regulated by the Land Development Regulations Section 4.13.
- 2. SPR 24-06, Petition submitted by Christopher A. Gmuer, P.E., (agent) for ERA Investments, LLC, (owner), for a Site Plan Review for Lake City Hotels Phase 2, in a Commercial Intensive zoning district, and located on parcel 02582-002, which is regulated by the Land Development Regulations Section 4.13.
- 3. SPR 23-10, Petition submitted by Carol Chadwick, P.E., (agent) for Affiliated Property Management, (owner), for a Site Plan Review for Aspire Dental Addition, in a Residential Office zoning district, and located on parcel 07604-102, which is regulated by the Land Development Regulations Section 4.10.

Meeting Location: City Council Chambers located on the 2nd Floor of City Hall at 205 North Marion Avenue, Lake City, FL 32055.

Members of the public may also view the meeting on our YouTube channel at:

https://www.youtube.com/c/CityofLakeCity

Pursuant to 286.0105, Florida Statutes, the City hereby advises the public if a person decides to appeal any decision made by the City with respect to any matter considered at its meetings or hearings, he or she will need a record of the proceedings, and that, for such purpose, he or she may need to ensure that a verbatim record of the proceedings is made, which record includes the testimony and evidence upon which the appeal is to be based.

SPECIAL REQUIREMENTS: Pursuant to 286.26, Florida Statutes, persons needing special accommodations to participate in this meeting should contact the City Manager's Office at (386) 719-5768.

Robert Angelo Planning and Zoning Tech.

CITY OF LAKE CITY CUSTOMER SEKVICE BUILDING 173 NW HILLSBORO STREET LAKE CITY, FL 32055

NOTICE OF PUBLIC MEETING CITY OF LAKE CITY PLANNING AND ZONING BOARD

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SPECIAL REQUIREMENTS: Pursuant to 286.26, Florida Statutes, persons needing special accommodations to participate in this meeting should contact the City Manager's Office at (386) 719-5768.

Robert Angelo Planning and Zoning Tech



May 13, 2024

To Whom it May Concern,

On June 10, 2024 the Planning and Zoning Board will be having a meeting at 5:30pm at 205 N. Marion. At this meeting we will be hearing a petition submitted by Christopher a. Gmuer, PE, as agent for ERA Investments, LLC, owner, for a site plan review, SPR 24-06, on parcel 02582-002. The site plan review is to build a hotel located within the Commercial Intensive (CI) zoning district.

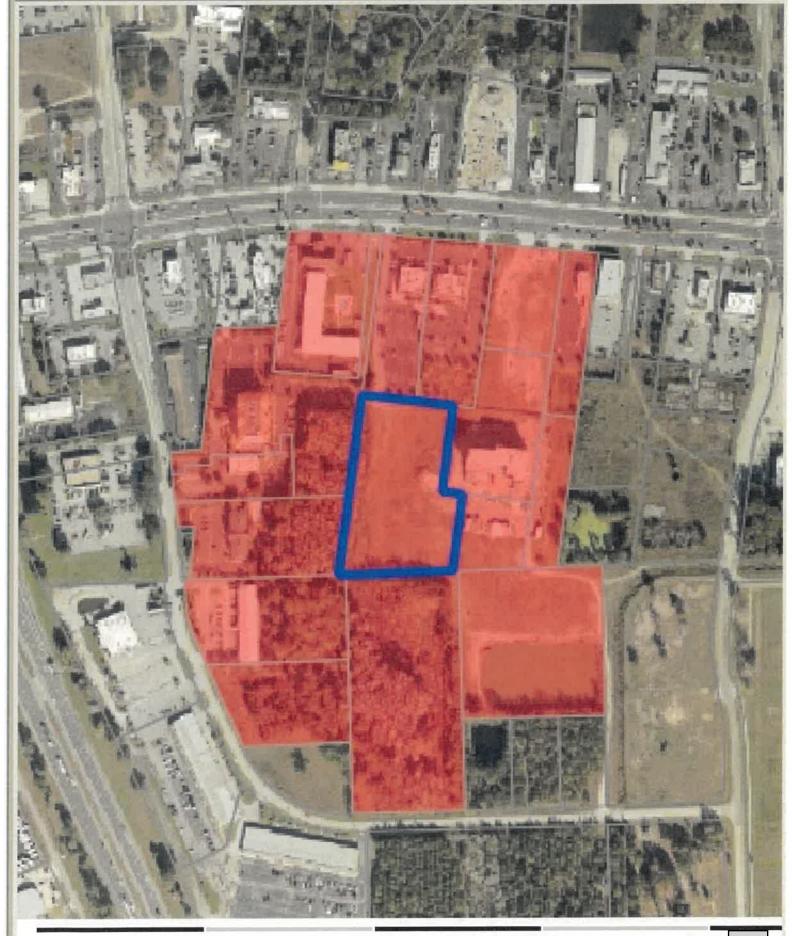
If you have any questions or concerns please call 386-752-2023 ext. 820 or email me at growthmanagement@lcfla.com.

Robert Angelo

Planning and Zoning City of Lake City

Columbia County Property Appraiser - Sales Report								
Name	Address1	Address2	Address3	City	State	ZIP		
JIV MANAGEMENT HOLDING SERVICES, LLC	369 TOCCOA PL	P.C.	JONESBORO	GA	30236			
ERA INVESTMENTS LLC	162 NW BIRDIE PL	-93	LAKE CITY	FL	32055			
ERA INVESTMENTS LLC	162 NW BIRDIE PL		LAKE CITY	FL	32055			
ERA INVESTMENTS LLC	162 NW BIRDIE PL		LAKE CITY	FL	32055			
ERA INVESTMENTS LLC	162 NW BIRDIE PL	0.00	LAKE CITY	FL.	32055			
NURUM LLC	162 NW BIRDIE PL	290	LAKE CITY	FL	32055			
STORE MASTER FUNDING XIII LLC	PO BOX 4069	646	MARYVILLE	TN	37802			
AKSHAR MOTEL LLC	285 SW COMMERCE BLVD	242	LAKE CITY	FL	32025			
ISMJ INC	339 SW COMMERCE DR	240	LAKE CITY	FL	32025			
OM SHANTI INC OF LAKE CITY	303 BEVERLY ST SE	59	LIVE OAK	FL	32064			
DASIS LAKE CITY LLC	162 NW BIRDIE PL	Var.	LAKE CITY	FL	32055			
ERA INVESTMENTS LLC	162 NW BIRDJE PL	925	LAKE CITY	FL	32055			
AKE CITY 3072 W US HIGHWAY 90, LLC	250 JOHN KNOX RD	SUITE 6	TALLAHASSEE	FL.	32303			
RARE HOSPITALITY MANAGMENT LLC	100 DARDEN CENTER DRIVE	6	ORLANDO	FL	32837			
R S MOTEL CORP	3696 W US HWY 90		LAKE CITY	FL	32055			

GIS Buffer



530 1060 1590

2120 437

Certified Mail Receipts for June 10th P&Z Meeting Site Plan Hotels Phase 2



