PLAN COMMISSION

City of Kaukauna **Council Chambers** Municipal Services Building 144 W. Second Street, Kaukauna

Thursday, May 22, 2025 at 4:00 PM

AGENDA

In-Person in Common Council Chambers, City of Kaukauna

- 1. Roll Call.
- Approval of Minutes.
 <u>a.</u> Approve Minutes from May 8, 2025
- 3. Old Business.
- 4. New Business.
 - a. Plan Commission Resource Memo
 - b. Site Plan Review-Haen Elementary School
 - c. Site Plan Review- 1801 Progress Way (G & G Machine Inc)
 - d. Site Plan Review- 101 Kelso Rd
 - e. Site Plan Review- Change to site plan at 1800 Crooks
- 5. Other Business.
- 6. Adjourn.

NOTICES

IF REQUESTED THREE (3) DAYS PRIOR TO THE MEETING, A SIGN LANGUAGE INTERPRETER WILL BE MADE AVAILABLE AT NO CHARGE.





Thursday, May 8, 2025 at 4:00 PM

MINUTES

In-Person

Mayor Penterman called the meeting to order at 4:00 p.m.

1. Roll Call

Members Present: Brett Jensen, Giovanna Feller, John Neumeier, John Moore, Michael Avanzi, Pennie Thiele, Mayor Tony Penterman

Absent: Ken Schoenike

Other(s) Present: Planning and Community Development Director Dave Kittel, Associate Planner Adrienne Nelson, Times Villager Reporter Brian Roebke

Moore made a motion to excuse the absent member. Seconded by Neumeier. Motion passed unanimously.

2. Approval of Minutes

a. Approve Minutes from April 17, 2025

Moore made a motion to approve the minutes from April 17, 2025. Seconded by Feller. Motion passed unanimously.

3. Old Business

a. Sign Ordinance Update

Director Kittel re-introduced the discussion on updating and reorganizing the city's sign ordinance. In March, direction was given to staff by Plan Commission to move forward with efforts to clean up and organize the sign ordinance with the goal of making the ordinance easier to understand and locate within the municipal code. Currently, regulations related to specific types of signage are found in 17.03, the definition section. Staff is suggesting moving signage regulations out of section 17.03 and moving additional sign and billboard information out of section 17.32 in order to create a sign code specific chapter within the zoning code called section 17.33.

Kittel explained that the next change suggested by staff is to update the language and format for signage in each individual zoning district. The first district being looked at by staff is the CCD (Commercial Core District). The draft format uses existing regulations but removes unnecessary verbiage with the goal of providing clarity on what is allowed for each type of signage. Staff is also proposing removing the CCD's limitation on one sign per wall of a building fronting a street or public way and instead allowing for multiple signs, with the caveat that signs are limited to 15% of the wall area. This is helpful in the case of buildings that have two storefronts, and it allows each business located in a building to have their own sign, provided that the total amount of signage does not exceed 15% of the wall area.

Kittel added that another suggestion by staff is to allow properties in the CCD that meet the requirements of the CHD (Commercial Highway District) to apply for signage in compliance with CHD signage requirements with approval of the Plan Commission. Kittel showed on a map the properties in the CCD that meet the size requirements of the CHD. He explained that, although there are several properties that meet the size requirements for CHD, only a few meet the setback requirements. Because of this, only a handful of properties could realistically take advantage of this opportunity: Kwik Trip, Wellness 360, McDonald's, and two apartment complexes. Plan Commission would ultimately have final say on whether signage is approved in these instances. At this time, staff is seeking feedback on how to proceed.

Moore asked if houses within the CCD would be able to apply for signage.

Kittel explained that houses would not be able to apply for signage because of their residential status, but that additional language could be added to clarify this.

Moore asked about buildings within the CCD where businesses are located on the first floor and rentals are located on the second floor. Would those rentals be able to display signs, perhaps in their windows?

Kittel stated that rentals would not be able to. These restrictions would apply strictly to commercial users.

Moore requested that this detail be further clarified in the ordinance. He also asked how the ordinance section would be relabeled.

Kittel explained that parts of section 17.03 and 17.32 would be moved to a new section in 17 called section 17.33.

Moore asked if this would be coming back before the Plan Commission before approval.

Kittel answered that it would be, and that it will need to go before the Legislative Committee and Common Council as well before final approval.

No motion made.

4. New Business

a. Rezoning Request – Parcel 322111500

Director Kittel introduced the proposed rezoning request for parcel 322111500. The proposal is to rezone this parcel from IND (Industrial) to CHD (Commercial Highway District). Upon review of the city's comprehensive plan, staff discovered that this parcel was always planned to be zoned CHD. At the time that this property was annexed into the City of Kaukauna, it was part of a larger parcel that was brought in as IND, and that zoning was never changed. Someone is now interested in purchasing the parcel and they are requesting that it be rezoned to CHD to allow for development outside of industrial use.

Moore asked what the different requirements between IND and CHD are regarding development.

Kittel explained that the change in zoning would allow for different uses of the property, such as for restaurants, health services, or financial services. Despite the size of the parcel, it is not conducive to industrial development because of the existing wetlands. CHD structures can be built on smaller parcels and therefore would be more conducive to commercial highway development. Light manufacturing is still allowed in the CHD as a special exception.

Neumeier made a motion to approve the rezoning of parcel 322111500 from IND to CHD and to recommend the same to the Common Council. Seconded by Moore. Motion passed unanimously.

b. Certified Survey Map Review – Parcel 322111500

Director Kittel introduced the proposed certified survey map for parcel 322111500. This CSM would divide the parcel into four lots, with one those lots being designated as an out lot. This will help with the future development of the land.

Moore asked if the lots created from this division, specifically lots 2 and 3, would be large enough for development. He also inquired further about the purpose of the out lot.

Kittel stated that lots 2 and 3 would be large enough for development. The out lot will allow for one right-of-way acquisition for the WisDOT improvement project. This right-of-way acquisition would be handled separately from the current CSM request.

Moore inquired as to why WisDOT selected this lot. They usually select an area closer to the intersection.

Kittel stated that the out lot is located in the lowest section of the parcel, and it will be the easiest one for the WisDOT to work with.

Moore made a motion to approval the certified survey map creating four lots for the Van Epern Family Trust. Seconded by Thiele. Motion passed unanimously.

c. Park Donation – Location Change Request Ben Bay Bench Associate Planner Nelson introduced a request by Benjamin Bay to change the location of his donated bench from the Locks Trail to Quarry Point Park. His bench has been ordered but has not yet been installed. The Public Works Department has already confirmed that a bench could be installed at Quarry Point Park.

Moore made a motion to approve the change in location from the Locks Trail to Quarry Point Park for Benjamin Bay, as the bench has not yet been installed, and direct staff to work with the donor to finalize the location of the bench. Seconded by Neumeier. Motion passed unanimously.

d. Site Plan Review - 1900 Tower Drive

Director Kittel introduced the site plan for the creation of a 30,000 square foot warehouse for Quick Transport for their property located at 1900 Tower Drive. The warehouse will be located at the rear of the property. Quick Transport is also proposing adding a third driveway onto Tower Drive. This would improve truck traffic and allow for better maneuvering. The proposed driveway is located far enough away from the existing driveway to not be a cause for concern. Staff has reviewed the plan and confirmed that it is in compliance with all covenants and meets all requirements set forth in city ordinance. The site plan has been approved by the Industrial Park Commission.

Jensen made a motion to approve the site plan as presented with the condition that, prior to issuance of building permits, Stormwater and Erosion Control permits must be obtained from the Engineering Department. Seconded by Feller. Motion passed unanimously.

e. 14.07 Ordinance Amendment

Associate Planner Nelson introduced the proposed ordinance amendment for section 14.07 of the municipal code. This amendment would require surveys for new buildings and additions or as deemed necessary by staff, which would help prevent future issues with permitting for items such as sheds or fences.

Director Kittel added that the vast majority of contractors who apply for permits through the city already submit surveys, but there are some that do not. This amendment would save staff hours of time as well as help ensure compliance with wetland restrictions.

Mayor Penterman asked for clarity on what new builds this amendment would apply to. Would it apply for constructing something simple, such as a shed?

Kittel clarified that this amendment is meant for new buildings and additions. Additional language can be added to further clarify this.

Neumeier stated that he liked the concept. He asked what would happen if plans changed over the course of the project. Would contractors be required to submit an updated survey?

Jensen stated that staff would need an updated survey. Staff already require updated documents if changes are made.

Moore made a motion to recommend that staff pursue this ordinance change. Seconded by Neumeier. Motion passed unanimously.

f. 17.51 Ordinance Amendment

Director Kittel introduced the proposed ordinance amendment for section 17.51 of the municipal code. This amendment would bring the city's municipal code into compliance with a change in state law that states that a zoning amendment only requires approval by a simple majority of a quorum of the members-elect. This amendment also changes the fee requirement from \$25.00 to "as set by the City Council by resolution from time to time".

Moore asked if all city fees are currently set by resolution.

Kittel explained that some fees are, particularly the newer fees. Staff eventually want to have all fees set by resolution and located in an index for reference. This would make them easier to find as well as to adjust when necessary.

Thiele made a motion to recommend approval of the updates to 17.51 to the Legislative Committee. Seconded by Moore. Motion passed unanimously.

g. Park Donation Updates

Associate Planner Nelson introduced the proposed update to the park donation policy guidelines. This update would increase the price for bench donations from \$1,200.00 to \$1,500.00 as well as add additional donation options. Language and layout of the policy guidelines would also be updated.

Director Kittel added that prices for the benches that the city orders have increased, not to mention prices for the plaque, the concrete pad, and staff time. The intent is to review costs annually to ensure that the city is able to breakeven.

Moore suggested that prices be set annually by the Planning and Community Development Department with approval from the Common Council in order to streamline the process and avoid giving out estimates. Staff should decide whether the plaque is considered in the overall price. Mayor Penterman stated that he appreciated the wider variety of donation options, but had questions as to who would maintain some of the items listed, such as the planters.

Kittel explained that, for now, the city planters fall to the Planning and Community Development Department to take care of. Staff, however, is working to formulate a new program for planter maintenance.

Moore made a motion to increase the park bench donation fee to \$1,500.00 and to recommend to Common Council that the Planning and Community Development Department be given permission to establish on an annual basis the donation prices, to include the product, plaque, and installation. Seconded by Neumeier. Motion passed unanimously.

h. Temporary Construction Easement – Wolfinger Apartments Director Kittel introduced the requested temporary construction easement for Wolfinger Apartments in Combined Locks. They will need to utilize part of a city pond by Jonen Park to do some grading.

Thiele asked if this would in any way damage the current pond.

Neumeier stated that the pond would only receive drainage from what falls on the backyards and patios of the planned development. Water from the gutters is directed away from this drainage area.

Moore asked if the city would be able to handle the additional water.

Neumeier explained that the city handles more water from this area currently than what will be handled after the development is complete.

Thiele made a motion to approve the temporary construction easement for Wolfinger Apartments. Seconded by Feller. Motion passed unanimously.

i. Park Donation Application Review – Penterman Bench Associate Planner Nelson introduced the donation request from Tony Penterman for a bench to be installed at the Kaukauna Municipal Pool in memory of Jeff Wittman. The bench would be in the City of Kaukauna's standard bench style and would include a plaque.

Thiele made a motion to approve the park bench donation for Tony Penterman in memory of Jeff Wittman with the condition that staff work with the donor to finalize the location of the bench. Seconded by Moore. Mayor Penterman abstained. Motion passed unanimously.

- 5. Other Business
 - a. None
- 6. Adjourn

Moore made a motion to adjourn the meeting. Seconded by Neumeier. Motion passed unanimously. The meeting adjourned at 5:02 p.m.





MEMO

PLANNING AND COMMUNITY DEVELOPMENT

То:	Plan Commission
From:	Dave Kittel, Director of Planning and Community Development
Date:	5/14/2025
Re:	Resource Guide

This memo is meant to provide Plan Commission Members with the basic documents and information to assist you in your role as a member of the Plan Commission. Most of the items can be found online and will have direct link from this memo. If there are resources or sources that are not electronic, the memo will provide instruction on how to access the information most easily.

Roles and Responsibilities:

The Plan Commission, an advisory body to the local governing body, plays a crucial role in guiding community development by preparing and administering a comprehensive plan, managing land use codes, and making recommendations on land use issues among other responsibilities. For the City of Kaukauna chapter 1.31 of the municipal code lays out the format, organization and matters referred to the Plan Commission

https://kaukauna.municipalcodeonline.com/book?type=ordinances#name=1.31_Cit y_Plan_Commission

Wisconsin <u>state statue 62.23</u> also provides information on the powers of a plan commission.

City of Kaukauna Resources:

• City Website – Links to all ordinances, resolutions, agendas, minutes, and data on the village can be found at our home page - https://kaukauna.gov/. For quick access to meeting minutes and agendas please use this link <a href="https://kaukauna-type:/https://kaukauna-type://kauka

<u>wi.municodemeetings.com/</u>. As you navigate the site, should you have suggestions for additional information or better presentation of information, please do not hesitate to contact the city staff.

• Code of Ordinances – The Municipal Code as it is currently codified can be found here:

https://kaukauna.municipalcodeonline.com/book?type=ordinances#name=Chapter_ 1_GENERAL_PROVISIONS

o Special attention should be paid to <u>Chapter 17</u>, this is chapter covers zoning and it details out each district, allowed uses, setbacks, etc...

o For Performance Standards for Industrial Development see Ch 17.38, and additional requirements such as landscaping are in Ch 17.52.

• Comprehensive Plan - In general terms, a comprehensive plan is long range planning document that guides the growth pattern of the City. City officials rely on the comprehensive plan to help them make decisions on how the physical development of the City should occur. The current plan can be found here: https://kaukauna.gov/wp-content/uploads/2016/06/full-plan-final.pdf

• Strategic Plan- – In 2023, the City's Common Council and staff worked to draft the City's first ever Strategic Plan. The City of Kaukauna 2024-2026 Strategic Plan is meant to serve as a road map for where the City is strategically headed over the next 3 years. The executive primer and other information can be found here: https://kaukauna.gov/wp-content/uploads/2024/10/Strategic-Plan-version-for-web.pdf

Other Resources:

• Plan Commission Handbook- The Plan Commission Handbook is designed to assist the plan commission, local government officials and citizens in understanding the roles and responsibilities of the plan commission and related decision-making standards. PDF download available at: https://www.uwsp.edu/cnr-ap/clue/Pages/publications-resources/PlanCommission

• Center for Land Use Education- The Center for Land Use Education (CLUE) is a joint venture of the College of Natural Resources at the University of Wisconsin - Stevens Point and the University of Wisconsin - Madison Division of Extension. It is a focal point for land use planning and management education. https://www.uwsp.edu/cnr-ap/clue/Pages/default.aspx • Zoning Ordinances Basic information: https://www.uwsp.edu/cnrap/clue/Documents/Zoning/Zoning_Ordinances.pdf

• A short 10-minute video on Variances this provides a great overview of the process and laws governing Variances. <u>https://www.youtube.com/watch?v=r_HcyUAlmuo</u>

• A webinar on the Role of the Plan Commission from the Center for Land Use Education: <u>https://www.youtube.com/watch?v=x9TUhnpcCYY</u>

Conclusion: This is not meant to be an exclusive list of resources for your position. Should you have questions or recommendations, please contact myself or other staff as appropriate.

Sincerely,

David hill

Dave Kittel Director of Planning & Community Development <u>Dkittel@Kaukauna.gov</u> 920.766.6370

UKAUNA



MEMO

PLANNING AND COMMUNITY DEVELOPMENT

To:	Plan Commission
From:	David Kittel Director of Planning and Community Development
Date:	5/16/2025
Re:	Site Plan Review – Haen Elementary School

Kaukauna Area School District is finalizing an addition on to Haen Elementary School. This addition is approximately 83,600 square feet and will add a gym and classrooms to the existing school. Attache to this report are the site plans for review.

Site Plan Review:

Site/Architectural: 17.32 (10) Supplementary District Regulations & applicable zoning

All set back requirements are met and all applicable ordinances are being complied with to include zoning requirements.

Landscape: 17.52 Landscaping Requirements

There are multiple trees and shrubs located on the property. There are no plans to remove these during the construction process. Additional trees are being planted along High Street and this portion will then be brought into compliance with the landscaping requirements. The Haen street side is not currently being worked on and no additional landscaping is being added at this time and no changes are proposed to the boundary areas between the school and residential properties.

Lighting:

The submitted photometric plan shows no direct lighting will be shining off the property.

Stormwater: 22 Stormwater Management

The developer will work with Engineering Department to complete Erosion Control and Stormwater Management permitting.

Ingress/Egress:

No concerns noted at this time

Public Safety:

No concerns noted at this time.

Façade: 17.53 Façade Standards

The proposed site elevations meet façade requirements.

Staff Recommendation:

Staff recommend approval of the development with the following conditions:

- Prior to issuance of building permits, must obtain Stormwater and Erosion Control permits from the Engineering Department
- If possible, additional landscaping should be added along Haen and the property lines between the school and residential properties.





SITE PLAN REVIEW APPLICATION

PROPERTY OWNER	APPLICANT (IF DIFFERENT PARTY THAN OWNER)		
Name:	Name:		
Mailing Address:	Mailing Address:		
101 Oak St, Kaukauna, WI, 54130			
Phone: 920-759-6104	Phone:		
Email: mcdanielc@kaukaunasd.org	Email:		
Describe the Proposed Project in Detail:			
Proposed addition & site improve	ements at Victor Haen Elementary School		
Propety Parcel (#):			
321058600			
Site Address/Location:			
1130 Haen Dr., Kaukauna, WI,	54130		
Current Zoning and Use:			
Institutional (IT), current site of Victor Haen Elementary School			
Proposed Zoning and Use:			
No Change			
Existing Gross Floor Area of Building:	Proposed Gross Floor Area of Building:		
51,800	83,600		
Existing Building Height:	Proposed Building Height:		
25'-0"	29'-4"		
Existing Number of Off-Street Parking Spaces:	Proposed Number of Off-Street Parking Spaces:		
83	83		
Existing Impervious Surface Coverage	Proposed Impervious Surface Coverage Percentage:		
ACO/	32%		
2070			

I certify that the attached drawings are, to the best of my knowledge, complete and drawn in accordance with all City of Kaukauna codes.

Owner/Agent Signature:

Owner/Agent Name (printed): Chris McDaniel

CITY OF KAUKAUNA

920.766.6300 kaukauna.gov



INDEX MAP



DESIGN DRAWINGS BENCHMARK:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM. BENCHMARK #1 BURY BOLT ON HYDRANT, LOCATED ON THE EAST SIDE OF THE INTERSECTION OF HAEN DRIVE AND WALTER STREET;

ELEVATION = 720.80BENCHMARK #2 NEW FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF HAEN DRIVE AND BOWERS LANE; ELEVATION = 711.28

DESIGN DRAWINGS UTILITY DISCLAIMER:

THE LOCATIONS, SIZES, AND TYPES OF UNDERGROUND PUBLIC ANE PRIVATE UTILITIES OR SUBSTRUCTURES SHOW HEREON WERE OBTAINED FROM VISUAL INSPECTION, FIELD MEASUREMENTS, AND/OR AS-BUILT PLANS. SANITARY SEWER AND STORM SEWER PIPE SIZES, INVERTS, DIRECTION, AND LOCATIONS BETWEEN MANHOLES ARE SUPPLEMENTED BY AS-BUILT PLANS AND/OR ESTIMATED BASED ON FIELD OBSERVATIONS. PRIOR TO CONSTRUCTION IN THE VICINITY OF ANY UTILITIES SHOWN HEREON, IT IS RECOMMENDED THAT THE LOCATIONS, DEPTHS, AND SIZES BE FIELD VERIFIED. THE LOCATIONS SHOWN HEREON ARE ONLY APPROXIMATE, WITH POSSIBILITY THAT ADDITIONAL UTILITY LINES NOT DISCOVERED, OR MARKED, DURING THE SEARCH OF RECORDS AND THE FIELD SURVEY MAY EXIST. ANY CONTRACTOR USING THE INFORMATION SHOWN HEREON IS HEREBY FOREWARNED THAT ANY EXCAVATION UPON THIS SITE MAY RESULT IN THE DISCOVERY OF ADDITIONAL UNDERGROUND UTILITIES NOT SHOWN HEREON. IN GENERAL, UNDERGROUND UTILITY LOCATIONS ARE SHOWN FROM UTILITY MARKINGS, BY OTHERS, AND/OR AS-BUILT PLANS, PROVIDED BY OTHERS. POINT OF BEGINNING MAKES NO WARRANTY OF ANY KIND, EXPRESS OR IMPLIED, WITH RESPECT TO THE EXISTING UTILITIES SHOWN HEREON, AND BELIEVES THAT THE INFORMATION CONTAINED HEREIN IS RELIABLE AND GENERALLY ACCURATE FOR THE PURPOSE INTENDED.

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KASD - VICTOR HAEN ELEMENTARY SCHOOL ADDITION & RENOVATION

KAUKAUNA AREA SCHOOL DISTRICT



CIVIL SHEET INDEX:

:000	TITLE SHEET
S 1.1	TOPOGRAPHIC SURVEY MAP
:100	DEMOLITION PLAN
200	LAYOUT PLAN - KEYNOTES
201	LAYOUT PLAN - DIMENSIONS
300	GRADING PLAN
2400	EROSION CONTROL PLAN
500	UTILITY PLAN
600-602	DETAILS
100	LANDSCAPE PLAN





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- 1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK. 2. ALL DEMOLITION MATERIALS SHALL BE REMOVED FROM THE SITE AND DISPOSED OF IN A LEGAL MANNER EXCEPT FOR THOSE ITEMS NOTED TO BE SALVAGED, WHICH SHOULD BE TURNED OVER TO THE OWNER. 3. INSTALL AND MAINTAIN ALL REQUIRED EROSION CONTROL MEASURES FOR PERIMETER
- PROTECTION PRIOR TO THE START OF DEMOLITION/CONSTRUCTION, IN ACCORDANCE WITH THE LOCAL AND STATE GOVERNING AUTHORITIES. 4. BIDDERS SHALL VISIT THE SITE AND REVIEW EXISTING CONDITIONS PRIOR TO THE BID DATE
- 5. ANY EXISTING PUBLIC & PRIVATE UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED, AND/OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING CONTRACTOR.
- 6. COORDINATE ALL PRIVATE & PUBLIC UTILITY REMOVAL, DISCONNECTION, &/OR RECONNECTION WITH RESPECTIVE UTILITY COMPANIES PRIOR TO BEGINNING WORK. 7. COORDINATE UTILITY SERVICE DISCONNECTIONS/OUTAGES WITH OWNER AND ANY IMPACTED
- NEIGHBORS. MINIMIZE DISRUPTIONS TO THE MAXIMUM EXTENT PRACTICAL. 8. ANY EXISTING UTILITIES NOT EXPRESSLY LABELED FOR DEMOLITION/REMOVAL ON THIS DOCUMENT SHALL BE LEFT IN PLACE AND IN THEIR CURRENT STATE OF OPERATION. CONTACT ENGINEER WHEREVER CLARIFICATION IS NEEDED.
- 9. STRIP TOPSOIL WITHIN THE PROJECT LIMITS IN ACCORDANCE WITH THE PROJECT MANUAL SPECIFICATIONS. 10. IF STRIPPED TOPSOIL IS STOCKPILED ON SITE, SILT FENCE SHALL BE INSTALLED AROUND
- THE BASE OF THE STOCKPILE TO PREVENT SEDIMENT TRANSPORT. 11. PRIOR TO PERFORMING WORK WITHIN PUBLIC RIGHT OF WAYS, NOTIFY AND COORDINATE WORK WITH THE LOCAL MUNICIPALITY.
- 12. MAINTAIN TRAFFIC CIRCULATION TO ALL NEIGHBORING PROPERTIES SHOWN ON THIS DOCUMENT. COORDINATE ALL WORK WITH NEIGHBORING PROPERTY OWNERS.

--- KEYNOTES:

1.	SAWCUT EXISTING CONCRETE CURBING	16.	MAINTAIN EXISTING LIGHT POLE
2.	REMOVE EXISTING CONCRETE CURBING	17.	MAINTAIN EXISTING U/G ELEC LINE
3.	MAINTAIN EXISTING CONCRETE CURBING		ELECTRICAL DESIGNER)
4.	MAINTAIN EXISTING CONCRETE STOOP	18.	REMOVE EXISTING STORM SEWER MH
5.	SAWCUT EXISTING BITUMINOUS PAVEMENT	19.	REMOVE EXISTING STORM SEWER PIPE
6.	SAWCUT EXISTING CONCRETE WALK	20.	MAINTAIN EXISTING STORM SEWER MH
7.	MAINTAIN EXISTING CONCRETE WALK	21.	MAINTAIN EXISTING STORM SEWER PIPE
8.	REMOVE EXISTING PLAYGROUND CURB	22.	REMOVE EXISTING WATER LINE
9.	REMOVE EXISTING PLAYGROUND EQUIPMENT	23.	MAINTAIN EXISTING WATER LINE (COORDINATE W/ BLDG PLUMBER)
10.	REMOVE EXISTING BASKETBALL GOAL	24.	MAINTAIN EXISTING SANITARY SEWER M
11.	REMOVE & SALVAGE EXISTING SIGN	25	MAINTAIN FYISTING SANITARY SEWER P
12.	REMOVE EXISTING CHAIN LINK FENCE	20.	
13.	REMOVE EXISTING BENCH	26.	MAINTAIN EXISTING F/O LINE
1 4		27.	CLEAR & GRUB EXISTING TREE
14.	REMUVE EXISTING LIGHT PULE	28.	MAINTAIN EXISTING TREE
15.	REMOVE EXISTING U/G ELEC LINE		

DEMOLITION HATCH PATTERNS:

BITUMINOUS REMOVAL (INCL. BASE)	
CONCRETE REMOVAL (INCL. BASE)	
INFIELD REMOVAL	
PLAYGROUND SURFACE REMOVAL (INCL. BASE MATERIAL)	
LANDSCAPING/WOODCHIP REMOVAL	

0' 30' 60' 1" = 30'



LINE W/ BLDG

WER PIPE WER MH WER PIPE

SEWER MH SEWER PIPE







- CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
 GRADE, LINE, AND LEVEL TO BE REVIEWED IN THE FIELD BY THE CONSTRUCTION MANAGER.
 ALL REQUIRED EROSION CONTROL MEASURES SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH LOCAL MUNICIPAL AND DEPARTMENT OF NATURAL RESOURCES
- REGULATIONS.
 4. SEE SHEET C400 FOR ALL REQUIRED EROSION CONTROL ELEMENTS.
 5. ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED AND OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING
- CONTRACTOR.
 6. VERIFY THE LOCATION OF ALL EXISTING UTILITIES PRIOR TO THE START OF DEMOLITION/CONSTRUCTION.
 7. BIDDERS SHALL VISIT THE SITE AND REVIEW EXISTING CONDITIONS PRIOR TO THE BID DATE.
- BIDDERS SHALL VISIT THE SITE AND REVIEW EXISTING CONDITIONS PRIOR TO THE BID I
 PRIOR TO STARTING WORK, VERIFY WITH THE LOCAL AUTHORITIES THAT ALL REQUIRED PERMITS HAVE BEEN ACQUIRED.
- 9. COORDINATE CONSTRUCTION IN THE RIGHT OF WAY WITH THE LOCAL AUTHORITIES. 10. PROVIDE PROPER BARRICADES, SIGNS, AND TRAFFIC CONTROL TO MAINTAIN THRU TRAFFIC ALONG ADJACENT STREETS IN ACCORDANCE WITH LOCAL MUNICIPAL REQUIREMENTS.
- SIDEWALK JOINTS SHALL BE INSTALLED AS INDICATED OR AS APPROVED BY THE CONSTRUCTION MANAGER.
 ALL NEW CONCRETE PAVEMENT AND CURB ON ADJACENT STREET SHALL BE TIED IN WITH
- #5 DOWEL BAR, MIN. 24" LONG, AT 18" O/C.
 13. ALL GENERAL LANDSCAPE AREAS SHALL BE SEEDED, FERTILIZED, AND CRIMP HAY MULCHED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

KEYNOTES:

- CONCRETE SIDEWALK
 CONCRETE STOOP
- (SEE ARCHITECTURAL PLANS)
- 3. 30" CONCRETE CURB & GUTTER
- DOWEL CURBING TO EXISTING WITH 2-#5 AT 18"
 O/C. EMBEDDED 6" TYP.
- 5. 24" DEPRESSED CURB
- 6. CURB TAPER/TRANSITION AREA
- 7. HANDICAP RAMP
- 8. 4' CHAIN LINK FENCE (INCL. FENCEGUARD MOW STRIP IN GRASS AREAS)
- 9. 20' CHAIN LINK BACKSTOP
- 10. 6' CHAIN LINK FENCE (INCL. FENCEGUARD MOWSTRIP IN GRASS AREAS)
- 11. 6'x24' CHAIN LINK DOUBLE GATE
- 12. SYNTHETIC TURF IN COURTYARD AREA
- 13. 4" OF PLAYGROUND FILL MATERIAL W/ POURED-IN-PLACE PLAYGROUND SURFACE BY PLAYGROUND VENDOR. SEPARATION FABRIC & UNDERDRAIN BY GENERAL CONTRACTOR. (GC TO EXCAVATE TO PROPOSED SUBGRADE DEPTHS)
- 14. PLAYGROUND EQUIPMENT BY PLAYGROUND VENDOR (AS APPROVED BY OWNER)
- 15. 6"x12" CONCRETE CURB AROUND PLAYGROUND
- 16. HOME PLATE (BASE PADS & PITCHING RUBBER BY OWNER)
- 17. BASKETBALL GOAL (KEEPER GOALS MODEL: CONTENDERINAC OR APPROVED EQUAL)
- 18. 2" BASKETBALL LAYOUT STRIPING
- 19. RAISE OR REPLACE EXISTING LIGHT POLE BASE
- 20. ACO K200 SERIES TRENCH DRAIN

- 1 (601) (2) (601)

PAVEMENT HATCH PATTERNS:

PROPOSED STANDARD ASPHALT PAVEMENT REPAIR & REPLACE PAVEMENT/CURB (IN KIND) FOLLOWING UTILITY INSTALLATION (SEE UTILITY PLAN)

PROPOSED STANDARD CONCRETE PAVEMENT

PROPOSED REINFORCED CONCRETE PAVEMENT

SYNTHETIC TURF IN COURTYARD AREA

POURED-IN-PLACE PLAYGROUND SURFACE









- 1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK. 2. GRADE, LINE, AND LEVEL TO BE REVIEWED IN THE FIELD BY THE CONSTRUCTION MANAGER.
- 3. ALL REQUIRED EROSION CONTROL MEASURES SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH LOCAL MUNICIPAL AND DEPARTMENT OF NATURAL RESOURCES REGULATIONS.
- 4. SEE SHEET C400 FOR ALL REQUIRED EROSION CONTROL ELEMENTS. 5. ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED AND OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING
- CONTRACTOR.
- DEMOLITION/CONSTRUCTION. 7. BIDDERS SHALL VISIT THE SITE AND REVIEW EXISTING CONDITIONS PRIOR TO THE BID DATE.
- 8. PRIOR TO STARTING WORK, VERIFY WITH THE LOCAL AUTHORITIES THAT ALL REQUIRED PERMITS HAVE BEEN ACQUIRED.
- 9. COORDINATE CONSTRUCTION IN THE RIGHT OF WAY WITH THE LOCAL AUTHORITIES. 10. PROVIDE PROPER BARRICADES, SIGNS, AND TRAFFIC CONTROL TO MAINTAIN THRU TRAFFIC
- ALONG ADJACENT STREETS IN ACCORDANCE WITH LOCAL MUNICIPAL REQUIREMENTS. 11. SIDEWALK JOINTS SHALL BE INSTALLED AS INDICATED OR AS APPROVED BY THE CONSTRUCTION MANAGER.
- 12. ALL NEW CONCRETE PAVEMENT AND CURB ON ADJACENT STREET SHALL BE TIED IN WITH #5 DOWEL BAR, MIN. 24" LONG, AT 18" O/C.
- 13. ÄLL GENERAL LANDSCAPE AREAS SHALL BE SEEDED, FERTILIZED, AND CRIMP HAY MULCHED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

\otimes **KEYNOTES:**

1. SEE SHEET C200 FOR LAYOUT PLAN KEYNOTES.

PAVEMENT HATCH PATTERNS:

PROPOSED STANDARD ASPHALT PAVEMENT	1 C600
REPAIR & REPLACE PAVEMENT/CURB (IN KIND) FOLLOWING UTILITY INSTALLATION (SEE UTILITY PLAN)	
PROPOSED STANDARD CONCRETE PAVEMENT	
PROPOSED REINFORCED CONCRETE PAVEMENT	- 3 C600
SYNTHETIC TURF IN COURTYARD AREA	
POURED—IN—PLACE PLAYGROUND SURFACE	

0' 30' 60' 1" = 30'





- CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
 THE PROPOSED SITE PLAN FINISH FLOOR ELEVATION OF 721.45 EQUALS THE PROPOSED BUILDING ARCHITECTURAL FINISH FLOOR ELEVATION OF 100.00'.
 GRADE, LINE, AND LEVEL SHALL BE REVIEWED IN THE FIELD BY THE CONSTRUCTION
- MANAGER.
 4. INSTALL AND MAINTAIN ALL REQUIRED EROSION CONTROL MEASURES IN ACCORDANCE WITH LOCAL MUNICIPAL AND DEPARTMENT OF NATURAL RESOURCES REGULATIONS.
 5. 6" OF TOPSOIL SHALL BE PROVIDED IN ALL GENERAL LAWN AREAS AND 12" SHALL BE DEPARTMENT OF NATURAL RESOURCES REGULATIONS.
- PROVIDED IN ALL PLANTING BED AREAS.
 6. SEE SHEET C400 FOR ALL REQUIRED EROSION CONTROL ELEMENTS.
 7. ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED,
- RELOCATED, AND/OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING CONTRACTOR.
 8. COORDINATE ALL EARTHWORK ACTIVITIES WITH THE RESPECTIVE TRADES RESPONSIBLE FOR THE INSTALLATION OF GAS, CABLE, TELEPHONE AND ELECTRICAL (INCLUDING MAIN SERVICE,
- SITE LIGHTING, CONDUITS AND SIGNAGE).
 9. EXCESS TOPSOIL SHALL BE REMOVED FROM SITE, UNLESS OTHERWISE DIRECTED BY THE OWNER. COORDINATE WITH OWNER FOR LOCATION OF STOCKPILE IF THE OWNER CHOOSES TO SALVAGE EXCESS TOPSOIL FOR FUTURE USE. SILT FENCE SHALL BE PLACED AROUND STOCKPILE.
- 10. ALL TESTING AND INSPECTION SHALL BE DONE IN ACCORDANCE WITH SPS 382.21. 11. THE LOCAL MUNICIPALITY SHALL BE CONTACTED PRIOR TO ANY EXCAVATION IN THE PUBLIC
- RIGHT-OF-WAY. 12. THE CONTRACTOR SHALL HAVE A TRAFFIC CONTROL PLAN APPROVED PRIOR TO WORK COMMENCING.
- THE LOCAL MUNICIPALITY SHALL OPERATE ALL EXISTING WATER VALVES IF NEEDED.
 GRADES AT BUILDING EDGE SHALL BE 6" BELOW FINISHED FLOOR ELEVATION EXCEPT AT DOOR WAY ENTRANCES OR UNLESS OTHERWISE NOTED.

GRADING LEGEND:

EXISTING CONTOUR	— — — 712 — — —
PROPOSED CONTOUR	712
PROPOSED SPOT ELEVATION	<u>(892.26)</u> -↔
PROPOSED RIM ELEVATION PROPOSED PIPE END INVERT ELEVATION	(RIM=893.56) -↔ (INV=893.56) -↔
PROPOSED TOP OF CURB ELEVATION	(<u>TC=893.56</u>) →
PROPOSED FLOW LINE ELEVATION	(FL=893.56)
PROPOSED MATCH ELEVATION (CONTRACTOR TO VERIFY)	<u>(892.05M</u>) ↔
PROPOSED STORM SEWER MANHOLE	S
PROPOSED NYLOPLAST DRAIN BASIN OR YARD DRAIN	•
PROPOSED STORM SEWER ENDWALL W/ RIP RAP	()





- CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
 NOTIFY THE LOCAL MUNICIPALITY AT LEAST 2 WORKING DAYS PRIOR TO THE START OF SOIL
- DISTURBING ACTIVITIES. 3. INSTALL ALL TEMPORARY EROSION CONTROL ELEMENTS PRIOR TO THE START OF
- DEMOLITION/CONSTRUCTION. 4. ALL ACTIVITIES SHALL BE CONDUCTED IN A LOGICAL SEQUENCE TO MINIMIZE THE AMOUNT OF BARE SOIL EXPOSED AT ANY ONE TIME. MAINTAIN EXISTING VEGETATION AS LONG AS POSSIBLE.
- 5. CRUSHED ROCK DRIVES FOR SEDIMENT TRACKING UTILIZING 3" CRUSHED ROCK SHALL BE MAINTAINED AT ALL CONSTRUCTION ENTRANCES TO THE SITE. THE ROCK DRIVE SHALL BE A MINIMUM OF 12" THICK AND BE A MINIMUM OF 50 FEET IN LENGTH BY THE WIDTH OF THE DRIVEWAY.
- 6. OFFSITE SEDIMENT DEPOSITS RESULTING FROM STORMWATER RUNOFF SHALL BE CLEANED BY THE END OF THE NEXT WORKDAY. OFFSITE SEDIMENT DEPOSITS RESULTING FROM CONTRACTOR ACTIVITIES, INCLUDING SOIL TRACKING, SHALL BE CLEANED EACH WORKDAY. EXCESSIVE AMOUNTS OF SEDIMENT OR DEBRIS TRACKED ONTO ADJACENT STREETS SHALL BE CLEANED IMMEDIATELY. FINE SEDIMENT ACCUMULATIONS ON ADJACENT STREETS SHALL SWEPT MECHANICALLY OR MANUALLY AT LEAST WEEKLY AND BEFORE IMMINENT RAINFALL.
- 7. DISTURBED GROUND OUTSIDE OF THE EVERYDAY CONSTRUCTION AREAS, INCLUDING SOIL STOCKPILES, THAT ARE LEFT INACTIVE FOR MORE THAN 7 DAYS SHALL BE TEMPORARILY STABILIZED BY SEEDING/MULCHING OR OTHER APPROVED METHODS.
- WASTE MATERIAL THAT IS GENERATED ON THE CONSTRUCTION SITE SHALL BE PROPERLY DISPOSED OF AND NOT ALLOWED TO RUN INTO RECEIVING WATERS.
 EROSION CONTROL DEVICES DESTROYED AS A RESULT OF CONSTRUCTION ACTIVITIES SHALL BE REPAIRED BY THE END OF EACH WORK DAY.
- 10. INSPECT ALL EROSION CONTROL MEASURES AT LEAST ONCE A WEEK AND AFTER ANY RAINFALL OF 0.5" OR MORE. MAKE NEEDED REPAIRS AND DOCUMENT ALL ACTIVITIES AS PER THE REQUIREMENTS OF THE NOTICE OF INTENT SUBMITTED BY THE PROJECT CIVIL ENGINEER.
- 11. ALL TEMPORARY EROSION CONTROL ELEMENTS SHALL REMAIN IN PLACE UNTIL A SUFFICIENT GROWTH OF VEGETATION IS ESTABLISHED AND THEN BE REMOVED AS PART OF THE BASE BID.
- IF SEDIMENT LADEN WATER NEEDS TO BE REMOVED FROM THE SITE, FILTER BAGS OR SCREENING SHALL BE USED IN ACCORDANCE WITH WI DNR TECHNICAL STANDARD 1061 TO PREVENT SEDIMENT DISCHARGE TO THE MAXIMUM EXTENT PRACTICABLE.
 COORDINATE ALL EARTHWORK ACTIVITIES WITH THE RESPECTIVE TRADES RESPONSIBLE FOR
- THE INSTALLATION OF GAS, CABLE, TELEPHONE AND ELECTRICAL (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE).
- 14. IF BARE SOIL IS EXPOSED DURING THE WINTER MONTHS, STABILIZATION BY MULCHING OR ANIONIC POLYACRYLAMIDE SHALL OCCUR PRIOR TO SNOWFALL OR GROUND FREEZE.
 15. SILT FENCE SHALL BE INSTALLED AROUND THE TOPSOIL STOCKPILE.
- 16. THE CONTRACTOR SHALL PERFORM INSPECTIONS AND MONITORING OF EROSION CONTROL PRACTICES IN ACCORDANCE WITH THE WI DNR "CONSTRUCTION SITE INSPECTION REPORT" FORM 3400-187. THIS FORM CAN BE FOUND IN THE CONSTRUCTION SPECIFICATIONS.

EROSION CONTROL LEGEND:

EXISTING CONTOUR	<u> </u>	_
PROPOSED CONTOUR	888	_
PROPOSED SILT FENCE	<u> </u>	
PROPOSED INLET PROTECTION	\diamond	-
EROSION CONTROL BLANKET		-
ROCK CONSTRUCTION ENTRANCE		
TEMPORARY SLOPE INTERRUPTION	· · · · · · · · · · · · · · · · · · ·	-
CONCRETE WASHOUT AREA (INSTALL PER EPA STANDARDS)		-
STORM WATER OVERLAND FLOW DIRECTION		

EROSION CONTROL SEQUENCING:

- INSTALL PERIMETER EROSION CONTROL
 BEGIN DEMOLITION
- BEGIN ROUGH GRADING AND UTILITY INSTALLATION
 DURING GRADING ACTIVITIES EXISTING GRASS AND VEGETATION, TO BE REMOVED, SHALL REMAIN IN PLACE FOR AS LONG AS POSSIBLE, TO AVOID SEDIMENT TRANSPORT.
- 5. TEMPORARY STABILIZATION ACTIVITY SHALL COMMENCE WHEN LAND DISTURBING CONSTRUCTION ACTIVITIES HAVE TEMPORARILY CEASED AND WILL NOT RESUME FOR A PERIOD EXCEEDING 14 CALENDAR DAYS.
- 6. FINAL STABILIZATION ACTIVITY SHALL COMMENCE WHEN LAND DISTURBING ACTIVITIES CEASE AND FINAL GRADE HAS BEEN REACHED ON ANY PORTION OF THE SITE.
- 7. IF DISTURBED AREAS MUST BE LEFT OVER WINTER, AN ANIONIC POLYACRYLAMIDE SHALL BE APPLIED TO ALL DISTURBED AREAS PRIOR TO GROUND FREEZE. SEE SPECIFICATIONS FOR DETAILS.







- 1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
- GRADE, LINE, AND LEVEL SHALL BE REVIEWED IN THE FIELD BY THE CONSTRUCTION MANAGER.
 ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED AND OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING
- CONTRACTOR.
 4. REFER TO THE PROPOSED BUILDING MECHANICAL/PLUMBING PLANS TO VERIFY EXACT CONNECTION LOCATIONS AND SIZES OF PROPOSED SANITARY SEWER AND WATER LATERALS.
- 5. COORDINATE ALL UTILITY WORK WITH THE RESPECTIVE TRADES RESPONSIBLE FOR THE INSTALLATION OF GAS, CABLE, TELEPHONE AND ELECTRICAL (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE).
- COORDINATE UTILITY SERVICE DISCONNECTIONS/OUTAGES WITH OWNER AND ANY IMPACTED NEIGHBORS. MINIMIZE DISRUPTIONS TO THE MAXIMUM EXTENT PRACTICAL.
 COORDINATE ALL WORK WITHIN THE PUBLIC RIGHT OF WAY WITH THE LOCAL MUNICIPALITY.
- 8. ALL TESTING AND INSPECTION SHALL BE DONE IN ACCORDANCE WITH SPS 382.21.
 9. THE PROPOSED WATER LINE SHALL HAVE A MINIMUM COVER OF 7'-6" TO THE TOP OF PIPE FROM PROPOSED FINISHED GRADE. SEE SHEET C300 FOR PROPOSED FINISHED GRADE.
- 10. THE MUNICIPALITY SHALL BE CONTACTED PRIOR TO ANY EXCAVATION IN THE PUBLIC RIGHT-OF-WAY, AND PRIOR TO CONNECTING SANITARY SEWER AND WATER LATERALS TO THE PUBLIC MAINS.
- THE CONTRACTOR SHALL HAVE A TRAFFIC CONTROL PLAN APPROVED PRIOR TO WORK COMMENCING.
 THE MUNICIPALITY SHALL OPERATE ALL EXISTING WATER VALVES, IF NEEDED.
- 13. FIELD VERIFY INVERT ELEVATION OF THE SANITARY SEWER, STORM SEWER, &/OR WATER PUBLIC MAIN, AT THE LOCATION OF THE SERVICE LATERAL CONNECTIONS, PRIOR TO CONNECTING THE LATERALS TO THE PUBLIC MAIN.

UTILITY LEGEND:

PROPOSED STORM SEWER	ST
PROPOSED PERFORATED UNDERDRAIN	<u> </u>
PROPOSED SANITARY SEWER	SS
PROPOSED WATER MAIN	——— w ———
PROPOSED STORM SEWER MANHOLE	(ST) - (10) (C602)
PROPOSED NYLOPLAST DRAIN BASIN OR YARD DRAIN	$\Theta - ($
GATE VALVE	
PROPOSED STORM SEWER ENDWALL W/ RIP RAP	

STORM MANHOLE SCHEDULE:

STRUCTURE #	STRUCTURE DETAILS
ST #1	$\begin{array}{llllllllllllllllllllllllllllllllllll$
	12" NYLOPLAST DRAIN BASIN W/ PEDESTRIAN GRATE W/ 6" SUMP
	RIM = 711.19 INV (N) = 708.33 DEPTH = 3.36'
51 #2	12" NYLOPLAST DRAIN BASIN W/ PEDESTRIAN GRATE W/ 6" SUMP
ST #3	$\begin{array}{llllllllllllllllllllllllllllllllllll$
	12" NYLOPLAST BASIN W/ SOLID COVER W/ 6" SUMP
ST #4	$\begin{array}{llllllllllllllllllllllllllllllllllll$
	18" NYLOPLAST BASIN W/ SOLID COVER W/ 6" SUMP
ST #5	RIM = 713.00 INV (SW) = 710.41 INV (SE) = 710.41 INV (NE) = 708.22 DEPTH = 5.28'
	24" NYLOPLAST BASIN W/ STANDARD GRATE W/ 6" SUMP
ST #6	RIM = 711.98 $INV (SW) = 707.97$ $INV (SE) = 704.70$ $INV (NW) = 704.70$ $DEPTH = 7.28'$
	60" I.D. PRECAST STRUCTURE W/ NEENAH R-1772 CASTING W/ SOLID COVER











- 1. CONTACT DIGGER'S HOTLINE 5 WORKING DAYS PRIOR TO THE START OF DEMOLITION/CONSTRUCTION. 2. 6" OF TOPSOIL SHALL BE PROVIDED IN ALL GENERAL LANDSCAPE AREAS. LANDSCAPE CONTRACTOR SHALL VERIFY THAT
- SPECIFIED PLANTING SOIL DEPTH IS PRESENT PRIOR TO PLANTING. 3. SEED/FERTILIZE/CRIMP HAY MULCH ALL GENERAL LANDSCAPE AREAS DISTURBED DURING CONSTRUCTION EXCEPT ARES
- LABELED AS SYNTHETIC TURF. 4. ALL PLANT MATERIALS LISTED SHALL MEET THE STANDARDS OF THE AMERICAN NURSERY & LANDSCAPE ASSOCIATION FOR THE SIZES GIVEN.
- 5. ALL TREES SHALL BE STAKED WITH A MINIMUM OF THREE STAKES. 6. ALL TREES IN THE TURF AREA SHALL HAVE A 5' DIAMETER CIRCLE OF 4" DEPTH SHREDDED HARDWOOD BARK MULCH. 7. CURV-RITE LANDSCAPING EDGING (SERIES 3000 3/16" X 4", WITH MILL FINISH) OR APPROVED EQUAL SHALL BE PLACED
- AROUND ALL LANDSCAPE BEDS. 8. 3" DEPTH OF SHREDDED HARDWOOD BARK MULCH SHALL BE PLACED IN ALL LANDSCAPE PLANTING BEDS AS NOTED. PROVIDE A SAMPLE TO THE OWNER FOR REVIEW AND APPROVAL PRIOR TO INSTALLATION.
- 9. 20 YEAR WEED BARRIER FILTER FABRIC SHALL BE PLACED BENEATH ALL LANDSCAPE HARDWOOD BARK MULCH. 10. COORDINATE ALL LANDSCAPE WORK WITH GAS, ELECTRIC, (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE) CABLE AND TELEPHONE CONSTRUCTION AND RESPECTIVE TRADES FOR THE INSTALLATION OF SAID UTILITIES.

PLANTING SCHEDULE:

TREES SYMBOLS	BOTANICAL NAME	COMMON NAME	INSTALLATION S SIZE M
AR GD GT	ACER SACCHARUM 'BAILSTA' GYMNOCLADUS DIOICUS 'ESPRESSO' GLEDITSIA TRIANCANTHOS 'SKYCOLE'	FALL FIESTA MAPLE ESPRESSO KENTUCKY COFFEE TREE SKYLINE HONEYLOCUST	2"CAL. 60'1 2"CAL. 50' ⁻ 2"CAL. 50' ⁻
SHRUBS SYMBOLS	BOTANICAL NAME	COMMON NAME	INSTALLATION SIZE M
CE DI PF PO SJM	CEANOTHUS AMERICANUS DIERVILLA IONICERA POTENTILLA FRUTICOSA 'GOLD STAR' PHYSOCARPUS OPULIFOLIUS 'SMPOTW' SPIRAEA JAPONICA 'WALBUMA'	NEW JERSEY TEA DWARF BUSH HONEYSUCKLE GOLD STAR POTENTILLA TINY WINE NINEBARK MAGIC CARPET SPIREA	3 GAL. 3'1 3 GAL. 3'1 3 GAL. 2' 3 GAL. 3- 3 GAL. 18-24'
ORNAMENT SYMBOLS	TAL GRASS BOTANICAL NAME	COMMON NAME	INSTALLATION SIZE M
СА	ÇALAMAGROSTIS X ACUTIFLORA 'KARL FOERSTER'	KARL FOERSTER FEATHER REED GRASS	3 GAL
PERENNIAL SYMBOLS	_S BOTANICAL NAME	COMMON NAME	INSTALLATION SIZE M
HW	HOSTA 'DIAMOND LAKE'	DIAMOND LAKE HOSTA	3 GAL 2
NF	NEPETA FAASSENII	NEPETA	3 GAL 20

KEYNOTES:

1. LANDSCAPE EDGING – SEE GENERAL NOTES 2. LANDSCAPE HARDWOOD MULCH – SEE GENERAL NOTES

LEGEND:

PROPOSED LANDSCAPE BED

PROPOSED: 100 SHRUBS

LANDSCAPE REQUIREMENTS:

REQUIREMENT: ADD AT LEAST ONE CANOPY TREE, NOT LESS THAN 1.5" CALIPER, FOR EVERY 75 LINEAR FEET WITHIN THE FRONT YARD SETBACK ADJACENT TO THE STREET. APPROXIMATELY 423 L.F. OF STREET FRONTAGE WITHIN PROJECT AREA/ 75 L.F. = 6 TREES REQUIRED ------

PROPOSED: 6 CANOPY TREES (1.5" CALIPER) REQUIREMENT: FOR EVERY 5 L.F. OF BUILDING STREET FRONTAGE, (1) SHRUB SHALL BE PLACED ON SITE. APPPROXIMATELY 496 L.F. OF BUILDING STREET FRONTAGE 5 L.F= 99.2 SHRUBS REQUIRED





A2 / A101B

A1 A300



3

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FLOOR PLAN NOTES BEFORE BEGINNING WORK, VERIFY THE EXISTENCE AND LOCATION OF PLUMBING, MECHANICAL AND ELECTRICAL SYSTEMS AND OTHER CONSTRUCTION AFFECTING THE WORK. IF DISCREPANCIES ARE DISCOVERED, NOTIFY ARCHITECT PROMPTLY. ALL DIMENSION STRINGS ARE TO FINISH FACE OF WALL, UNLESS NOTED

- OTHERWISE VERIFY STRUCTURAL ELEMENTS, INCLUDING BUT NOT LIMITED TO POST TENSION CABLES OR OTHER REINFORCEMENT EMBEDDED IN FLOOR SLABS PRIOR TO CORING OR CUTTING. COORDINATE VERIFICATION METHOD WITH **<OWNER>** <owner Representative>. REFER TO SHEET A010 FOR REFERENCED PARTITION TYPES AND DETAILS. WALLS NOT TAGGED ON THESE PLANS SHALL BE PARTITION TYPE S6A-S12. PROVIDE MOISTURE- AND MOLD-RESISTANT GYPSUM BOARD AT THE FOLLOWING LOCATIONS: ENTIRE WALL DIRECTLY BEHIND OR ADJACENT TO ANY PLUMBING FIXTURE INCLUDING, BUT NOT LIMITED TO, WATER CLOSETS, SHOWERS, DRINKING FOUNTAINS, SINKS, LAVATORIES AND EYEWASHES. ALL WALLS OF AREAS INCLUDING, BUT NOT LIMITED TO, SINGLE OCCUPANT TOILET ROOMS, JANITOR CLOSETS, SWIMMING POOLS, THERAPY POOLS, COMMERCIAL KITCHENS, DISHWASHING, ETC. WHERE CERAMIC TILE OR SOLID SURFACE WALL CLADDING IS SCHEDULED, PROVIDE CEMENT BOARD BACKER BOARD IN LIEU OF GYPSUM BOARD. LOCATE ALL DOOR JAMBS 4" FROM ADJACENT PERPENDICULAR WALL, UNLESS NOTED OTHERWISE. REPAIR, PATCH AND CLEAN ALL EXISTING SURFACES SCHEDULED TO REMAIN THAT ARE AFFECTED BY DEMOLITION OR NEW CONSTRUCTION. MAKE APPEAR NEW, MATCHING ADJACENT CONSTRUCTION. PREPARE ALL SURFACES AS REQUIRED FOR SCHEDULED FINISHES. SKIM COAT AND PREPARE ALL DAMAGED OR UNFINISHED SURFACES.> . <PATCH ALL WALL BASE SCARS AT EXISTING WALL SURFACES. PREPARE SMOOTH TO RECEIVE NEW WALL BASE FOR UNIFORM APPEARANCE.> <PATCH WALLS TO MATCH ADJACENT SURFACES AT ALL ABANDONED OR REMOVED OUTLET AND SWITCH LOCATIONS. BLANK COVER PLATES ARE NOT ACCEPTABLE.> <PREPARE FLOOR LEVEL AND SMOOTH FOR NEW HARD SURFACE FINISHES. PATCH EXISTING PENETRATIONS AND ABNORMALITIES IN CONCRETE FLOOR.> <ALL AREAS DISTURBED BY DEMOLITION SHALL BE PATCHED AND PROPERLY PREPARED FOR NEW FINISH APPLICATIONS. WHERE ADJACENT FINISHES ARE EXISTING TO REMAIN, PATCH AND MATCH EXISTING FINISHES. CONSULT WITH THE OWNER TO IDENTIFY ATTIC STOCK AND/OR PREVIOUS MATERIAL SPECIFICATIONS.> PROVIDE BLOCKING IN WALLS AS REQUIRED FOR NEW MILLWORK, CASEWORK, DOOR BUMPERS, ACCESSORIES, EQUIPMENT, FIXTURES, AND **< OWNER'S>** <TENANT'S> WALL-MOUNTED EQUIPMENT, INCLUDING BUT NOT LIMITED TO,
- TELEVISIONS/MONITORS, MARKER BOARDS, TACK BOARDS AND SIGNAGE. OPEN EXISTING WALLS AS NEEDED TO INSTALL CONCEALED BLOCKING WHERE REQUIRED, AND PATCH WALLS TO MATCH EXISTING CONSTRUCTION. COORDINATE WITH **<OWNER> <TENANT>** ON LOCATIONS OF WALL-MOUNTED EQUIPMENT.
- M. PROVIDE 3/4" FIRE-RETARDANT TREATED PLYWOOD AS BACKING PANELS FOR TELEPHONE & ELECTRICAL EQUIPMENT AT <ROOM NAME AND NUMBER.> INSTALL AT 1'-0" AFF TO FINISHED CEILING. PAINT PLYWOOD TO MATCH ADJACENT WALL FINISH. COORDINATE FINAL SIZE AND LOCATION WITH **<TENANT> <OWNER.>**

KEYNOTES PER SHEET

1ST FLR PLAN OVERALL 1/16" = 1'-0" | A101 TRUE PLAN

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PROJECT INFORMATION





ISSUANCE AND REVISIONS

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DATE	DESCRIPTION
03/12/2025	100% SCHEMATIC DESIGN
05/01/2025	PLAN COMMISSION



1





M. PROVIDE 3/4" FIRE-RETARDANT TREATED PLYWOOD AS BACKING PANELS FOR TELEPHONE & ELECTRICAL EQUIPMENT AT <ROOM NAME AND NUMBER.> INSTALL AT 1'-0" AFF TO FINISHED CEILING. PAINT PLYWOOD TO MATCH ADJACENT WALL FINISH. COORDINATE FINAL SIZE AND LOCATION WITH **<TENANT> <OWNER.>**



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PROJECT INFORMATION



1130 HAEN DRIVE KAUKAUNA, WI 54130

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ISSUANCE AND REVISIONS

DATE	DESCRIPTION
03/12/2025	100% SCHEMATIC DESIGN
05/01/2025	PLAN COMMISSION





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KEYNOTES PER SHEET

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PROJECT INFORMATION





ISSUANCE AND REVISIONS

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DATE	DESCRIPTION
03/12/2025	100% SCHEMATIC DESIGN
05/01/2025	PLAN COMMISSION



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3

TRUE PLAN

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PROJECT INFORMATION

KASD - VICTOR HAEN ELEMENTARY SCHOOL ADDITION & RENOVATION

1130 HAEN DRIVE KAUKAUNA, WI 54130

ISSUANCE AND REVISIONS

DATE	DESCRIPTION
03/12/2025	100% SCHEMATIC DESIGN
05/01/2025	PLAN COMMISSION



PROGRESS DOCUMENTS NOT FOR CONSTRUCTION These documents reflect progress and intent and may be subject to change, including additional detail. These are not final construction documents and shall not be used for final bidding or construction-related purposes. PROJECT MANAGER RS PROJECT NUMBER 323425-01 1ST FLR PLAN -AREA B

A101B

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PROJECT INFORMATION





ISSUANCE AND REVISIONS

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03/12/2025	100% SCHEMATIC DESIGN
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EXISTING TO REMAIN, PATCH AND MATCH EXISTING FINISHES. CONSULT WITH

THE OWNER TO IDENTIFY ATTIC STOCK AND/OR PREVIOUS MATERIAL

M. PROVIDE 3/4" FIRE-RETARDANT TREATED PLYWOOD AS BACKING PANELS FOR TELEPHONE & ELECTRICAL EQUIPMENT AT **<ROOM NAME AND NUMBER**.> INSTALL AT 1'-0" AFF TO FINISHED CEILING. PAINT PLYWOOD TO MATCH ADJACENT WALL FINISH. COORDINATE FINAL SIZE AND LOCATION WITH **<TENANT> <OWNER**.>

KEYNOTES PER SHEET

0105RELOCATED EXISTING ROCK CLIMBING WALL1044-01SEMI-RECESSED FIRE EXTINGUISHER CABINET WITH EXTINGUISHER1166-01GYMNASIUM DIVIDER CURTAIN1266-01300 SEAT TELESCOPIC BLEACHERS



PROJECT INFORMATION





ISSUANCE AND REVISIONS

DESCRIPTION
100% SCHEMATIC DESIGN
PLAN COMMISSION





A2 / A103B







- A. ALL CONTRACTORS SHALL COORDINATE AND LOCATE ALL ROOF OPENINGS AND PENETRATIONS WITH STRUCTURAL, PLUMBING, MECHANICAL AND ELECTRICAL DRAWINGS.
- B. PROVIDE WATER TIGHT INTEGRITY AT ALL PENETRATIONS AND EQUIPMENT PER ROOFING MANUFACTURERS STANDARD DETAILS AND REQUIREMENT FOR WARRANTY AND CURRENT NRCA STANDARDS.
- PROVIDE 4'-0" X 4'-0" PANS AT ROOF DRAINS.
- PROVIDE TAPERED INSULATION SADDLES AND CRICKETS AT 1/4" PER FOOT AT ALL ROOF TOP EQUIPMENT AND PENETRATIONS TO ENSURE POSITIVE DRAINAGE.

ROOF PLAN LEGEND



0	





PROJECT INFORMATION KASD - VICTOR

HAEN ELEMENTARY SCHOOL ADDITION & RENOVATION

D 1130 HAEN DRIVE KAUKAUNA, WI 54130

ISSUANCE AND REVISIONS ____

DATEDESCRIPTION05/01/2025PLAN COMMISSION

KEY PLAN С В SHEET INFORMATION **PROGRESS DOCUMENTS** NOT FOR CONSTRUCTION These documents reflect progress and intent and may be subject to change, including additional detail. These are not final construction documents and shall not be used for final bidding or construction-related purposes. PROJECT MANAGER RS PROJECT NUMBER 323425-01 Α ROOF PLAN - AREA A103A

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PROJECT INFORMATION

KASD - VICTOR HAEN ELEMENTARY SCHOOL ADDITION & RENOVATION

D 1130 HAEN DRIVE KAUKAUNA, WI 54130

ISSUANCE AND REVISIONS

Β

DATEDESCRIPTION05/01/2025PLAN COMMISSION

KEY PLAN \mathbf{C} SHEET INFORMATION **PROGRESS DOCUMENTS** NOT FOR CONSTRUCTION These documents reflect progress and intent and may be subject to change, including additional detail. These are not final construction documents and shall not be used for final bidding or construction-related purposes. PROJECT MANAGER RS PROJECT NUMBER Α 323425-01 ROOF PLAN - AREA A103B

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PROJECT INFORMATION KASD - VICTOR

HAEN ELEMENTARY SCHOOL ADDITION & RENOVATION

D 1130 HAEN DRIVE KAUKAUNA, WI 54130

ISSUANCE AND REVISIONS

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DATEDESCRIPTION05/01/2025PLAN COMMISSION

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PROJECT INFORMATION

KASD - VICTOR HAEN ELEMENTARY SCHOOL ADDITION & RENOVATION

D 1130 HAEN DRIVE KAUKAUNA, WI 54130

ISSUANCE AND REVISIONS

С

В

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DATEDESCRIPTION05/01/2025PLAN COMMISSION

KEY PLAN С В SHEET INFORMATION PROGRESS DOCUMENTS NOT FOR CONSTRUCTION These documents reflect progress and intent and may be subject to change, including additional detail. These are not final construction documents and shall not be used for final bidding or construction-related purposes. PROJECT MANAGER RS PROJECT NUMBER 323425-01 ROOF PLAN - AREA A103D

A6 A311

1



ROOF PLAN NOTES

- A. ALL CONTRACTORS SHALL COORDINATE AND LOCATE ALL ROOF OPENINGS AND PENETRATIONS WITH STRUCTURAL, PLUMBING, MECHANICAL AND ELECTRICAL DRAWINGS.
- B. PROVIDE WATER TIGHT INTEGRITY AT ALL PENETRATIONS AND EQUIPMENT PER ROOFING MANUFACTURERS STANDARD DETAILS AND REQUIREMENT FOR WARRANTY AND CURRENT NRCA STANDARDS.
- C. PROVIDE 4'-0" X 4'-0" PANS AT ROOF DRAINS.
- PROVIDE TAPERED INSULATION SADDLES AND CRICKETS AT 1/4" PER FOOT AT ALL ROOF TOP EQUIPMENT AND PENETRATIONS TO ENSURE POSITIVE DRAINAGE.

ROOF PLAN LEGEND

RD	ROOF DRAIN		
OFD	OVERFLOW DRAIN		
DS	DOWNSPOUT		
←	- SLOPE DIRECTION OF ROOF & TAPERED INSULATION		
	CRICKETS AND SADDLES		
TAPERED INSULATION			
	NO PENETRATION ZONE THIS AREA		
	SEE SHEET A000 FOR ROOF ASSEMBLY INFORMATION		
	KEYNOTES PER SHEET		
0550-07	PREFABRICATED ALUMINUM ROOF LADDER		
B3010-01	SLOPED STRUCTURE ROOF SYSTEM, REFER TO A000		
B3010-02	SLOPED STRUCTURE ROOF SYSTEM, REFER TO A000		
B3010-03	FLAT STRUCTURE ROOF SYSTEM, REFER TO A000		
B3010-04	FLAT STRUCTURE ROOF SYSTEM, REFER TO A000		

7

A3 ROOF PLAN OVERALL 1/16" = 1'-0" | A103 TRUE PLAN 6



PROJECT INFORMATION





D

ISSUANCE AND REVISIONS

DATE	DESCRIPTION
03/12/2025	100% SCHEMATIC DESIGN
05/01/2025	PLAN COMMISSION

	KEY PLAN
В	SHEET INFORMATION
	PROGRESS DOCUMENTS NOT FOR CONSTRUCTION These documents reflect progress and intent and may be subject to change, including additional detail. These are not final construction documents and shall not be used for final bidding or construction-related purposes.
	PROJECT MANAGER RS
Α	PROJECT NUMBER 323425-01
	ROOF PLAN
	A103













PROJECT INFORMATION

F

KASD - VICTOR HAEN ELEMENTARY SCHOOL ADDITION & RENOVATION

D 1130 HAEN DRIVE KAUKAUNA, WI 54130

ISSUANCE AND REVISIONS

DATE	DESCRIPTION
03/12/2025	100% SCHEMATIC DESIGN
05/01/2025	PLAN COMMISSION

KEY PLAN







2

1



3D VIEW NORTH - GYM



3



3D VIEW SOUTHEAST - BUS LANE

3D VIEW SOUTHEAST - PLAY AREA

FOR REFERENCE ONLY TAG - TYP

6



PROJECT INFORMATION

F

D

С



1130 HAEN DRIVE KAUKAUNA, WI 54130

ISSUANCE AND REVISIONS

DATE	DESCRIPTION
03/12/2025	100% SCHEMATIC DESIGN
05/01/2025	PLAN COMMISSION

KEY PLAN



Α





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3D VIEW AERIAL - NORTH



3

4



3D VIEW AERIAL - SOUTH

3D VIEW NORTH

7

6



PROJECT INFORMATION

Ε



D 1130 HAEN DRIVE KAUKAUNA, WI 54130

ISSUANCE AND REVISIONS

DATE	DESCRIPTION
03/12/2025	100% SCHEMATIC DESIGN
05/01/2025	PLAN COMMISSION

KEY PLAN

С





3 4 5 6

 N
 ELECTRICAL SITE PLAN

 1" = 30'-0"
 E002

1

SITE PLAN NOTES

A. COORDINATE LOCATIONS OF ALL UNDERGROUND CONDUIT FROM OWNER.
B.
A.
B.
CACH LIGHT POLE SHALL INCLUDE THE HUBBELL WISCAPE IN-FIXTURE MODULE AS FOLLOWS: HUBBELL: WIR-RMS 277>



PROJECT INFORMATION

KASD - VICTOR HAEN ELEMENTARY SCHOOL ADDITION & RENOVATION

D 1130 HAEN DRIVE KAUKAUNA, WI 54130

ISSUANCE AND REVISIONS

DATEDESCRIPTION05/01/2025PLAN COMMISSION

KEY PLAN

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ElumTools General U	se Global I	lluminance Res	u
Calculation Points Name	Average	Maximum	
Addition Exterior Extended	6 fc	43 fc	
	•		-

SITE LIGHTING FIXTURE SCHEDULE							
TYPE	MOUNTING	DESCRIPTION	VOLTAGE	LAMP	WATTAGE	MANUFACTURER	CATALOG NUMB
C1	RECESSED	LED RECESSED CAN CANOPY LIGHT	MVOLT	LED	20.5	GOTHAM	IVO6 D 20LM 30K 80CRI 50D
W1	SURFACE	LED WALL PACK	MVOLT	LED	71.0	ACUITY	WDGE3 LED P3 70CRI R



PROJECT INFORMATION KASD - VICTOR HAEN ELEMENTARY SCHOOL ADDITION & RENOVATION

D 1130 HAEN DRIVE KAUKAUNA, WI 54130

ISSUANCE AND REVISIONS

DATEDESCRIPTION05/01/2025PLAN COMMISSION

KEY PLAN

SHEET INFORMATION

Α

PROGRESS DOCUMENTS NOT FOR CONSTRUCTION These documents reflect progress and intent and may be subject to change, including additional detail. These are not final construction documents and shall not be used for final bidding or construction-related purposes. PROJECT MANAGER ΡN PROJECT NUMBER 323425-01 ELECTRICAL SITE PHOTOMETRICS PLAN E003

Erosion Control/Storm Water Management Maintenance/Operation Plan

For:

Kaukauna Area School District Victor Haen Elementary Building Addition & Site Improvements

PREPARED BY:



Point of Beginning

4941 KIRSCHLING COURT STEVENS POINT, WI 54481 (715) 344-9999 ■ (715) 344-9922

Located in:

City of Kaukauna, Outagamie County, Wisconsin

> Dated: May 1, 2025

TABLE OF CONTENTS

Storm Water Management Plan Kaukauna Area School District Kaukauna, Wisconsin

1.0	BAC	KGROUND & GENERAL INFORMATION
	1.1	Introduction and Project Location1
	1.2	Project Description
	1.3	Project Requirements
	1.4	General Project Data1
2.0	EXIS	STING DRAINAGE CONDITIONS
	2.1	Existing Drainage Area
	2.2	Existing Drainage Calculation Summary
	2.3	Existing Off-Site Drainage
3.0	PRO	POSED DRAINAGE CONDITIONS
	3.1	Proposed Drainage Areas
	3.2	Post-Development Runoff Summary
	3.3	Proposed Detention/Retention Areas
4.0	POS	T-DEVELOPMENT PERFORMANCE STANDARDS
	4.1	Total Suspended Solids
	4.2	Infiltration
	4.3	Peak Discharge
	4.4	Summary
5.0	CON	ISTRUCTION SITE PERFORMANCE STANDARDS
	5.1	Erosion Control
	5.2	Construction Site Erosion Control Measures4
	5.3	Operation and Maintenance, Short-term
	5.4	Operation and Maintenance, Long-term
6.0	SUM	MARY
	6.1	General
Appe	ndices	
	А.	Proposed Layout Plan
	В.	Geotechnical Data
	C.	Existing Drainage Map and Calculations
	D .	Proposed Drainage Map and Calculations
	E.	Proposed Erosion Control Plan
	F.	State of Wisconsin Construction Site Inspection Report, Post Construction Long- Term Storm Water Management Checklist, and DNR Notice of Termination

EROSION CONTROL/ STORM WATER MANAGEMENT/MAINTENANCE/OPERATION PLAN for Kaukauna Area School District Kaukauna, WI

1.0 BACKGROUND & GENERAL INFORMATION

1.1 Introduction and Project Location

Point of Beginning, Inc. has been retained to perform storm water management calculations and prepare a storm water management plan per NR216.47 and NR151, for the proposed site improvements at Victor Haen Elementary School in Kaukauna, WI. The project is located in a Part of Lot 3 of Block 51 of the Assessor's Plat of the City of Kaukauna, located in Part of Private Claim 1, Section 14, Township 21 North, Range 18 East, City of Kaukauna, Outagamie County, Wisconsin.

1.2 Project Description

The proposed project consists of constructing a building addition, a new poured-in-place playground, and various site improvements. Storm sewer will be installed for storm water capture and rate control. (see Layout Plan in **Appendix A**).

1.3 **Project Requirements**

The project area includes approximately 3.77 acres. Since the project exceeds one acre, a Wisconsin Department of Natural Resources Notice of Intent application/permit (NOI-WPDES per WDNR) will be applied for.

The storm water management plan for this project is developed in accordance with municipal standards and the NOI-WPDES requirements and NR216.47/NR151.121 for redevelopment sites.

1.4 General Project Data

Soils

Based on the geo-technical data collected in 2021, the site is situated primarily on fill material. The geotechnical data containing soil hydrologic classes are attached in **Appendix B**.

Groundwater

Per the geotechnical report, ground water was encountered from 7.5' - 19'. It is not expected to impact construction.

Wetlands

The DNR's Surface Water Viewer online tool shows no possible wetland areas across the residential lots to the north. All site improvements have been kept a minimum of 75' from the potential wetland area, per the Surface Water Viewer.

Precipitation

The following precipitation rates from the NOAA Atlas 14 Point Precipitation Frequency Estimates: WI, have been utilized for storm water calculations:

P _{2,24}	=	2.45"
P _{10,24}	=	3.51"
$P_{100,24}$	=	5.50"

2.0 EXISTING DRAINAGE CONDITIONS

2.1 Existing Drainage Area

The existing drainage area includes sub-basin E1-E2, which includes the greater project area at large as well as a slim area along the south property line where a sidewalk is proposed. Runoff from E1 & E2 drains over the surface to on-site storm inlets connected to the City's storm sewer system. An existing drainage map can be found in **Appendix C**.

2.2 Existing Drainage Calculation Summary

Existing drainage calculations utilize TR-55 methodology and results for a 2, 10, and 100-year design storm are included. Where composite Times of Concentration are less than 0.1 hours (6.0 minutes), the TR-55 recommended minimum Time of Concentration of 0.1 hours (6.0 minutes) was used. Existing drainage calculations are provided in **Appendix C**.

2.3 Existing Off-Site Drainage

Existing surrounding storm water runoff draining directly onto the project area is negligible.

3.0 PROPOSED DRAINAGE CONDITIONS

3.1 Proposed Drainage Areas

The proposed drainage areas within the project limits are represented by sub-basin D1-D3. Subbasin D1 consists of the areas of the site which lie north and south of the proposed building addition. Sub-basin D2 consists of the slim area along the south property line, as in sub-basin E2. Runoff from D1 & D2 drains over the surface to on-site storm inlets connected to the City's storm sewer system. Sub-basin D3 consists of the building addition and the area on the northeast side of the addition. Runoff from D3 is comprised largely of the drainage from the building addition's interior storm sewer system. Runoff from D3 drains to the depressed area adjacent to the northeast side of the building (Pond 1P). This is a dry depression area intended for rate control during larger storm events. Pond 1P drains via storm sewer to the City's storm sewer system. A proposed drainage area map is provided in **Appendix D**.

3.2 Post-Development Runoff Summary

Proposed drainage calculations utilize TR-55 methodology and results for a 2, 10, and 100-year design storm have been attached. Where composite Times of Concentration are less than 0.1 hours (6.0 minutes), the TR-55 recommended minimum Time of Concentration of 0.1 hours (6.0 minutes) was used. A proposed drainage area map and calculations are provided in **Appendix D**.

3.3 **Proposed Detention Areas**

The depression area created on the northeast side of the building addition provides rate control and is modeled as Pond 1P. 1P is a dry basin with a 6" outlet pipe at an elevation of 710.50'. The top of the depression area is at an elevation of 713.00'. During the 100-year rainfall event, HydroCAD calculations indicate that the high water level reaches an elevation of 712.21', therefore no emergency overflow occurs.

See Section 4.3 Peak Discharge for runoff rates. See Appendix D for runoff calculations.

4.0 POST-DEVELOPMENT PERFORMANCE STANDARDS

4.1 Total Suspended Solids

With no parking or roadway areas included in the project, TSS removal is not required.

4.2 Infiltration

According to NR151.124(3)(b)(3), redevelopment post-construction sites are exempt from the infiltration requirements of NR151.

4.3 Peak Discharge

BMPs shall be employed to maintain or reduce the peak runoff discharge rates, to the maximum extent practicable, as compared to pre-development conditions.

The pre-development and post-development peak rates of discharge and volumes of discharge leaving the site runoff rates are summarized below. See **Appendix D** for HydroCAD modeling routing diagrams, summaries, and node listings.

	Pre-Development	Post-Development
	Site Drainage (1L)	Site Drainage (1L)
2-year 24-hour Peak Flow	4.81 cfs	3.79 cfs
10-year 24-hour Peak Flow	8.64 cfs	6.31 cfs
100-year 24-hour Peak Flow	16.51 cfs	11.36 cfs

Summary

The modeling of the site drainage demonstrates that discharge rates are reduced. As such, the requirements set by the Department of Natural Resources for total peak discharge, and infiltration are met by the proposed design.

The Storm Water Erosion Control Plan and the Storm Water Management Plan show that BMP engineering practices in hydrology planning and design have been considered and the resulting development will function as a positive addition to the community while sustaining environmental benefits in storm water management and quality.

5.0 CONSTRUCTION SITE PERFORMANCE STANDARDS

5.1 Erosion Control

The purpose of this control plan is to provide guidelines that comply with the state and local requirements, as well as to make recommendations regarding erosion control and storm water management. The construction of this development is a critical phase in terms of storm water management and runoff control. Construction site erosion control will help minimize the impact of development, enhance and protect local environment, and protect the surrounding project area by applying best management practices for erosion control at construction sites. This work shall be planned and executed in accordance with the Wisconsin Department of Natural Resources Storm Water Management Technical Standards and/or accepted local engineering practice. The owner/developer will be responsible for erosion control during the process of construction. Silt fence, site vegetation, and erosion mat will be utilized to keep sediment from leaving the construction site. See Appendix E.

5.2 Construction Site Erosion Control Measures

The following erosion control devices may be used on the project site at any time during the construction phases to ensure compliance with NR 216 and local erosion control requirements, as applicable.

a) Inlet Protection (WDNR 1060)

Inlet protection is a temporary barrier applied around storm drains. It is designed to prevent sediment from entering the storm sewer system. All fabrics used for inlet protection devices must be selected from the list of approved fabrics certified for inlet protection, Geotextile Fabric, Type FF in the current edition of the Wisconsin Department of Transportation Product Acceptability List (PAL).

b) Non-channel Erosion Mat (WDNR 1052)

The purpose of this practice is to protect the soil surface from the erosive effect of rainfall and prevent sheet erosion during the establishment of grass or other vegetation, and to reduce soil moisture loss due to evaporation. This practice applies to both Erosion Control Re-vegetative Mats (ECRM) and Turf-Reinforcement Mats (TRM).

- 1. CLASS I: A short-term duration (minimum of 6 months), light duty, organic mat with photodegradable plastic or biodegradable netting.
- a. Type A Use on erodible slopes 2.5:1 or flatter.
- b. Type B Double netted product for use on erodible slopes 2:1 or flatter.
- c) Interim Manufactured Perimeter Control and Slope Interruption Products (WDNR 1071)

The purpose of the installation of these products is to reduce uninterrupted slope length to slow the velocity of runoff so as to retain transported sediment from disturbed areas. This practice applies to Log-Type products.

d) Site Vegetation

Existing site vegetation outside of project limits shall be protected and maintained to the maximum extent practicable. Existing site vegetation within the project limits shall remain undisturbed until construction schedule warrants disturbance. For disturbed areas vegetation that resists erosion, maintains slow storm water velocities, and retains sediment from runoff shall be provided by the contractor. Temporary seeding may be required for disturbed areas that are subject to long periods of construction inactivity. Temporary vegetation is used when areas are disturbed and may remain unfinished long enough to allow vegetation to grow and assist with erosion control. Permanent vegetation is encouraged as soon as possible in the construction process.

e) Trackout Protection (WDNR 1057)

Stone tracking pads will be constructed at all entrances to the construction site to minimize sediment tracking onto existing streets. A minimum of one construction entrance is required for the project site. Tracking pads are temporary and will be removed or much of the aggregate will be removed before the site is completed.

f) Waste and Material Disposal

All waste and unused building materials (including garbage, debris, cleaning wastes, or other construction materials) shall be properly disposed of and not allowed to be carried by runoff into a receiving channel or inlet.

5.3 Operation and Maintenance, Short-term

The Owner of this project in Kaukauna, Outagamie County, Wisconsin, is directly responsible for implementation and maintenance of the construction site erosion control measures.

The Contractor shall conduct the following inspections:

- Weekly inspections of implemented erosion and sediment controls.
- Inspections of erosion and sediment controls within 24 hours after precipitation event 0.5 inches or greater which results in runoff during active construction periods.

The Contractor shall maintain weekly written reports of all inspections that include:

- The date, time, and exact place of the inspection.
- The name of the individual who performed the inspection.
- An assessment of the condition of erosion and sediment controls.
- A description of any erosion and sediment control implementation and maintenance performed.
- A description of the present phase of construction at the site.

Repairs shall be made immediately, as required, to maintain effectiveness, until permanent vegetation is established. All repairs to erosion control devices shall be documented. The attached Wisconsin Department of Natural Resources Construction Site Inspection Report (Form 3400-187) may be used for recording purposes. A copy of Form 3400-187 can be found in **Appendix F**.

5.4 Operation and Maintenance, Long-term

The Owner of this project in Kaukauna, Outagamie County, Wisconsin, is directly responsible for the operation, inspection, and maintenance of all storm water facilities located within the site, as described below.

• Area Storm Sewer:

Inspection: Accumulation of sediment and/or debris within storm sewer pipe, and/or outfall. Look for damage to pipe and outfall. Maintenance: Remove accumulated sediment and/or debris within the pipe and/or within or near inlets. Repair damaged to pipe and/or inlets. If the damage is un-repairable then the pipe and/or inlet shall be replaced.

The aforementioned inspection and maintenance schedule shall be performed after any rainfall event exceeding one inch of rainfall, and at a minimum semi-annually in early spring and fall. All inspections and maintenance shall be documented, and the OWNER shall keep all inspection and maintenance reporting/records onsite and available upon request of the City and/or Wisconsin Department of Natural Resources.

6.0 SUMMARY

6.1 General

The proposed development as outlined above meets all Wisconsin Department of Natural Resources storm water regulations pertaining to redevelopment.

For the temporary construction site scenario, sediment transport from this site to adjacent properties will be reduced by the erosion control devices and conservation practice standards.

This plan meets state storm water BMPs and provides an environmentally sound and practical solution for the future storm water runoff generated from the development of this site.

APPENDIX A

Proposed Layout Plan



GENERAL NOTES:

- CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
 GRADE, LINE, AND LEVEL TO BE REVIEWED IN THE FIELD BY THE CONSTRUCTION MANAGER.
 ALL REQUIRED EROSION CONTROL MEASURES SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH LOCAL MUNICIPAL AND DEPARTMENT OF NATURAL RESOURCES
- REGULATIONS.
 4. SEE SHEET C400 FOR ALL REQUIRED EROSION CONTROL ELEMENTS.
 5. ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED AND OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING
- CONTRACTOR.
 6. VERIFY THE LOCATION OF ALL EXISTING UTILITIES PRIOR TO THE START OF DEMOLITION/CONSTRUCTION.
 7. BIDDERS SHALL VISIT THE SITE AND REVIEW EXISTING CONDITIONS PRIOR TO THE BID DATE.
- BIDDERS SHALL VISIT THE SITE AND REVIEW EXISTING CONDITIONS PRIOR TO THE BID I
 PRIOR TO STARTING WORK, VERIFY WITH THE LOCAL AUTHORITIES THAT ALL REQUIRED PERMITS HAVE BEEN ACQUIRED.
- 9. COORDINATE CONSTRUCTION IN THE RIGHT OF WAY WITH THE LOCAL AUTHORITIES. 10. PROVIDE PROPER BARRICADES, SIGNS, AND TRAFFIC CONTROL TO MAINTAIN THRU TRAFFIC ALONG ADJACENT STREETS IN ACCORDANCE WITH LOCAL MUNICIPAL REQUIREMENTS.
- SIDEWALK JOINTS SHALL BE INSTALLED AS INDICATED OR AS APPROVED BY THE CONSTRUCTION MANAGER.
 ALL NEW CONCRETE PAVEMENT AND CURB ON ADJACENT STREET SHALL BE TIED IN WITH
- #5 DOWEL BAR, MIN. 24" LONG, AT 18" O/C.
 13. ALL GENERAL LANDSCAPE AREAS SHALL BE SEEDED, FERTILIZED, AND CRIMP HAY MULCHED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

KEYNOTES:

- CONCRETE SIDEWALK
 CONCRETE STOOP
- (SEE ARCHITECTURAL PLANS)
- 3. 30" CONCRETE CURB & GUTTER
- DOWEL CURBING TO EXISTING WITH 2-#5 AT 18"
 O/C. EMBEDDED 6" TYP.
- 5. 24" DEPRESSED CURB
- 6. CURB TAPER/TRANSITION AREA
- 7. HANDICAP RAMP
- 8. 4' CHAIN LINK FENCE (INCL. FENCEGUARD MOW STRIP IN GRASS AREAS)
- 9. 20' CHAIN LINK BACKSTOP
- 10. 6' CHAIN LINK FENCE (INCL. FENCEGUARD MOWSTRIP IN GRASS AREAS)
- 11. 6'x24' CHAIN LINK DOUBLE GATE
- 12. SYNTHETIC TURF IN COURTYARD AREA
- 13. 4" OF PLAYGROUND FILL MATERIAL W/ POURED-IN-PLACE PLAYGROUND SURFACE BY PLAYGROUND VENDOR. SEPARATION FABRIC & UNDERDRAIN BY GENERAL CONTRACTOR. (GC TO EXCAVATE TO PROPOSED SUBGRADE DEPTHS)
- 14. PLAYGROUND EQUIPMENT BY PLAYGROUND VENDOR (AS APPROVED BY OWNER)
- 15. 6"x12" CONCRETE CURB AROUND PLAYGROUND
- 16. HOME PLATE (BASE PADS & PITCHING RUBBER BY OWNER)
- 17. BASKETBALL GOAL (KEEPER GOALS MODEL: CONTENDERINAC OR APPROVED EQUAL)
- 18. 2" BASKETBALL LAYOUT STRIPING
- 19. RAISE OR REPLACE EXISTING LIGHT POLE BASE
- 20. ACO K200 SERIES TRENCH DRAIN

$\begin{array}{c} & 4 \\ & 600 \\ & 5 \\ & 600 \\ & 6$

- 1 (601) (2) (601)

PAVEMENT HATCH PATTERNS:

PROPOSED STANDARD ASPHALT PAVEMENT REPAIR & REPLACE PAVEMENT/CURB (IN KIND) FOLLOWING UTILITY INSTALLATION (SEE UTILITY PLAN)

PROPOSED STANDARD CONCRETE PAVEMENT

PROPOSED REINFORCED CONCRETE PAVEMENT

SYNTHETIC TURF IN COURTYARD AREA

POURED–IN–PLACE PLAYGROUND SURFACE







APPENDIX B

Geotechnical Data



2135 South 116th Street West Allis, WI 53227

April 11, 2025

Neil Henriksen Point of Beginning, Inc. 4941 Kirschling Court Stevens Point, WI 54481

Subject: Geotechnical Consulting Services - *revised* Victor Haen Elementary School Addition 1130 Haen Drive, Kaukauna, Wisconsin

Dear Mr. Henriksen:

GeoTest, Inc. (GeoTest) has prepared this geotechnical engineering report related to the above-referenced project. This report describes the subsurface exploration and laboratory testing programs and presents recommendations regarding civil and structural engineering design aspects of the project, as well as other construction considerations.

Project Description

Point of Beginning is assisting with the design of an addition to the existing school building located at 1130 Haen Drive in Kaukauna, Wisconsin. The location of the property is illustrated on Figure 1 in Appendix A.

The existing building is a single-story structure with a finished floor elevation of about 722 feet. A portion of the building has a lower-level floor elevation of about 716 feet due to a topographic change. The addition will consist of a single-story, slab-on-grade structure, that will have an assumed finished floor elevation of 722 feet. The development plan is illustrated on Figure 2 in Appendix A.

Structural loads are not expected to exceed 400 kips for columns and 6 kips per linear foot (klf) for walls. The floor slab will be subjected to a maximum load of 100 pounds per square foot (psf). To achieve the finished floor elevations for the addition, maximum fills of about 5 feet are anticipated.

The area of the proposed addition is currently grass and pavements. The ground surface slopes downward from the south to the north with an elevation of about 5 feet.

Scope of Work

Geotechnical Subsurface Exploration

The geotechnical exploration program consisted of twelve borings (B-1 through B-12) drilled to depths of 25 feet below the existing ground surface. The boring locations are identified on Figure 2 in Appendix A. The ground surface elevations at the boring locations were provided by Point of Beginning.





The borings were drilled using conventional hollow-stem augers. Soil samples were obtained at 2.5-foot intervals to a depth of 10 feet and 5-foot intervals thereafter. The soil samples were obtained by split-barrel sampling procedures, in general accordance with ASTM D1586. Representative portions of the samples were sealed in glass jars and returned to Point of Beginning for laboratory testing and classification.

Descriptive logs for each boring, which describe the methods of advancement, sample types, sample depths, and observations regarding soil and groundwater conditions, were prepared at the time of the field work. These logs were utilized by Point of Beginning as an aid to prepare the final logs included in Appendix B.

Water level information, if encountered, was noted during drilling. All drilling and sampling procedures are described in Appendix C.

Laboratory Testing

Point of Beginning examined and visually classified each sample, based on texture and plasticity, in accordance with the Unified Soil Classification System (USCS). Like soils were grouped into strata that are described on the soil logs. The notes included on the logs and chart describing this system of classification is included in Appendix B.

The laboratory testing program consisted of the following:

- Water content testing on all samples.
- Calibrated hand penetrometer (Q_p) testing on all fine grained (clay and silt).
- Atterberg Limits testing on two samples.

The laboratory test results are presented on the final logs or reports included in Appendix B. All laboratory procedures are described in Appendix C.

Soil and Groundwater Conditions

The following narrative is a generalization of the subsurface conditions encountered at the borings. Soil conditions can vary in areas between the sampling locations. For a more-detailed description of the subsurface conditions encountered at each boring location, please refer to the attached logs in Appendix B.

<u>General Soil Conditions</u> - The general soil profile at the boring locations, below asphalt pavement, concrete, or topsoil, consisted of clay and sand fill materials to depths of about 1.5 to 7 feet at six borings and stratified layers of native clay, silt, and sand. Organic peat was encountered at one boring (B-2) to a depth of about 3.5 feet.

The fills were likely related to past grading activities at the site when the school was constructed. The native soil layers varied in their coarse sand, silt and gravel content.



Item 4.b.

<u>*Fill Materials*</u> – One clay fill sample exhibited stiff consistency, with a Q_p value of 3,500 pounds per square foot (psf). Three additional clay fill samples were disturbed and could not be tested.

The relative density of three sand fill samples was loose to medium dense, with N-values of 3, 9, and 10. These values equate to strengths that range from about 1,000 psf to 2,500 psf.

Typically, moisture contents are considered high if they are above 20% in fine-grained soils and 15% in coarse-grained (sand and gravel) soils. The moisture content in the four clay fill samples ranged from 18.5% to 21.4%. Three samples (75%) exceeded 20%. The moisture content in the three sand fill samples ranged from 12.9% to 17.1%. Two samples (67%) exceeded 15%.

<u>Native Fine-Grained Soils</u> – Forty-two native clay and silt samples exhibited stiff to hard consistencies, with Q_p values ranging from 2,000 psf to greater than 9,000 psf. Using 9,000 psf as the maximum value, the average was 4,321 psf. Twenty-two samples (52%) were less than 4,000 psf (stiff) and four samples (10%) exceeded 8,000 psf (hard). Sixteen additional samples were disturbed and could not be tested.

The moisture content in fifty-eight native clay samples ranged from 12.2% to 35.2%. Twenty-eight samples (48%) exceeded 20%.

<u>Native Coarse-Grained Soils</u> – The relative density of sixteen native sand samples was medium dense, with N-values that ranged from 13 to 27. The average was 17. These values equate to strengths that range from about 3,000 psf to 6,000 psf.

The moisture content in the native sand samples ranged from 13.7% to 25.8%. Fourteen samples (88%) exceeded 15%.

Groundwater Conditions

Free groundwater and perched water (wet and saturated soils) was encountered at all borings during drilling. Based on the moisture contents and visual classifications of the samples, water existed at depths that ranges from 1 to 8 feet (elevations of 710 to 718 feet).

Fluctuations in the groundwater table elevation should be expected with variations in precipitation, evapotranspiration, surface runoff, etc. Also, shallow perched groundwater conditions should be expected where relatively permeable granular soils are underlain by relatively impermeable cohesive soils, especially following precipitation events.

Analysis and Recommendations

There are eight primary issues that should be considered when planning this project.



2135 South 116th Street West Allis, WI 53227

- Undocumented (absence of quality control testing) fill soils exist on the property. Typically, fills are a concern for structural support because they could have been placed inconsistently and not sufficiently compacted, potentially causing excessive total and/or differential settlements for foundations. The field data indicates the fills are variable in their strength characteristics, and therefore, not considered suitable for support of structural elements.
- The bearing soils at the assumed footing elevations will be variable, including native clay and sand that exhibited variable strengths, undocumented fills, and engineered fills. Variable-strength bearing soils can cause differential settlements.
- Clean sandy (small quantities of fines) soils were encountered that will pose excavating challenges, especially related to trench stability.
- Clay soils were present on the property, which are sensitive to construction activity, and actions to stabilize the subgrade during construction should be planned.
- Organic peat was encountered at one boring location (B-2). These soils, which could also exist in other areas, are not suitable for the support of structures or engineered fill. They should be removed where needed.
- Shallow soils (less than 5 feet) with high moisture content were present at all boring locations, which will cause significant grading and excavating challenges, and actions to stabilize the soils during construction should be planned.
- Shallow perched water and groundwater exists that should require dewatering planning.
- Care must be taken to ensure the integrity of the existing foundations are maintained.

Foundation Support

Foundation design includes evaluating for both bearing capacity and settlement; one or the other controls the design. Conventional foundation systems are typically designed using an allowable bearing capacity based on maximum total and differential settlements of 1 inch and ³/₄ inch, respectively. The addition can be supported by conventional spread footings. However, due to the presence of undocumented fills and organic soils, some soil improvement is expected.

The estimated footing pad elevations are 718 for exterior footings and 720 for interior footings. The following table identifies the depths/elevations where suitable soils are anticipated.

Boring	Surface	*Suitable Soil	*Suitable Soil
No.	Elevation (ft)	Depth (ft)	Elevation (ft)
B-1	720.77	0.5	720
B-2	714.33	3.5	711
B-3	715.68	3.5	712
B-4	717.41	0.5	717
B-5	716.05	3.5	712.5
B-6	717.05	1.0	716



Boring No.	Surface Elevation (ft)	*Suitable Soil Depth (ft)	*Suitable Soil Elevation (ft)
B-7	718.26	1.0	717
B-8	721.37	1.5	720
B-9	720.17	7.0	713
B-10	720.16	3.5	716.5
B-11	717.84	0.5	717.5
B-12	717.99	4.0	714

*Rounded to the nearest $\frac{1}{2}$ -foot.

Given the shallow depths of the unsuitable materials, over-excavation is considered the most-viable method. The bearing soils will consist of variable strength soils (1,000 psf to greater than 9,000 psf) or engineered fills placed to raise the existing grades or replace unsuitable soils. Because the bearing soils will vary, it is recommended that a conservative allowable bearing capacity be used by the structural engineer to design the footings to minimize differential settlements.

The foundation should be designed using an allowable bearing capacity value of 3,000 psf, with the understanding that some soil improvement will still be required. If a higher bearing capacity value is desired, additional soil improvement (e.g., over-excavation) under portions of the building would be required.

Traditionally, perimeter footings and interior footings in unheated areas should bear at a depth of at least 48 inches below the final exterior grade to provide adequate frost protection. If desired, exterior footings can bear at shallower depths by following ASCE 32-01 (American Society of Civil Engineers, Design and Construction of Frost-Protected Shallow Foundations, 2001). Interior footings not subject to frost can bear directly beneath the floor slab.

Seismic Design

The soil conditions present at a site are utilized in determining the Seismic Design Category (SDC) for structures. Part of selecting the SDC is determining the Site Class for the soils, which categorizes common soil conditions into broad classes, where typical ground motion attenuation and amplification effects are assigned. Site Class is determined based on the average properties of the soil within 100 feet of the ground surface. Geotechnical engineers use a variety of parameters to characterize the engineering properties of these soils, including general soil classifications (e.g., hard rock, soft clay, etc.), N-values, and laboratory testing.

Site Class A includes hard rock that is typically found only in the eastern United States. The types of rock typically found in the western states include various volcanic deposits, sandstones, shales, and granites that commonly have the characteristic appropriate to either Site Class B or C. Sites with very dense sands and gravels or very stiff to hard clay deposits also may qualify as Site Class C. Sites with relatively stiff cohesive or medium



Item 4.b.

dense non-cohesive soils, including mixtures of clays, silts, and sands, are categorized as Site Class D. Site Class D is the most common site class throughout the United States. Sites along rivers or other waterways underlain by deep soft clay deposits are categorized as Site Class E. Sites where soils are subject to liquefaction or other ground instabilities are categorized as Site Class F and site-specific analyses are required.

Based on the types of soils present at the boring locations at this property, and their apparent engineering properties, Site Class D is assigned to the site, as defined in the International Building Code (2015) Section 1613.

Floor Slab Support

Most of the existing soils (except for the organic soils) are suitable for support of concrete floor slabs. However, due to the presence of variable soils, floor slab area should be proof-rolled and soft areas removed or improved prior to the placement of base course materials. An average subgrade modulus value of 125 pounds per cubic inch (pci) is appropriate.

Engineered Fill, Wall, and Utility Trench Backfill

All engineered fill, wall, and utility trench backfill should consist of inorganic materials, free of debris, not exceed 3 inches in size, and should be placed in 8 to 10-inch loose lifts compacted to a minimum of 95 percent of the maximum dry density (Modified Proctor). The fill should be moisture conditioned to be within 3± percent of the optimum moisture content.

The on-site soils can be reused as engineered fill, assuming they do not include deleterious materials (organic soils, wet soils, etc.). However, due to the moisture sensitive nature of clay and silt soils, their use could pose construction challenges regarding achieving the required compaction requirements. The grading contractor may choose to use coarse-grained soil that can be more easily compacted and would be less sensitive to moisture levels.

Construction Considerations

All loose, wet, organic, disturbed, or otherwise unsuitable surface soils should be stripped from structural and engineered fill areas prior to any construction activities. The exposed subgrade soils and all engineered fills should be observed, tested, and documented by a representative of the geotechnical engineer. Large structural areas, such as the building footprint, pavement, and engineered fill zones, should be proof-rolled to identify lowstrength or disturbed areas that need to be removed or improved.

Footing excavations and all structural subgrade soils should be evaluated to confirm the bearing materials are consistent with those identified in this report and anticipated by the structural engineer. If unanticipated conditions are encountered, the geotechnical and structural engineers should be notified immediately. All footing pads must bear upon suitable native soils or engineered fill soils that have been confirmed in the field by a



2135 South 116th Street West Allis, WI 53227

representative of the geotechnical engineer. Where unsuitable bearing soils, such as fill, organic, disturbed, wet, frozen, or low-strength (less than the design bearing capacity) soils are encountered, the excavation should be extended to competent bearing soil. If extended, the footing pads can be constructed at the base of the excavations, or the excavations can be backfilled with clean, crushed stone or lean concrete.

Most soils on-site will be sensitive to disturbances from construction activity due to their clay and silt content and high moisture content. Construction activities can cause significant reductions in soil strength and support capabilities. In addition, moisture sensitive soils that are or become wet will likely impact grading and compaction schedules. Care should be taken during construction to protect these soils from moisture or disturbance from equipment. Placing a working subbase layer of 3-inch crushed stone or utilizing a cement stabilization program in areas subjected to construction traffic could be beneficial and reduce the need to strip disturbed soils.

The structural design should also consider the proximity of new footings in relation to existing footing. Imparting new loads to existing footings could induce additional settlement and new footing excavations could affect the structural integrity of existing footings.

It is likely that excavations will encounter shallow groundwater or perched water. Filtered sump pumps and drawing water from sump pits should be adequate to remove water that collects in excavations. Sump pits should be lined with a geotextile and filled with open-graded, free- draining aggregate.

Surface water should not be allowed to collect in excavations or on prepared subgrades during or after construction. Areas should be sloped to facilitate removal of collected surface runoff. Positive site drainage should be provided to reduce infiltration of surface water around the perimeter of structures and within pavement areas.

Excavation walls may need to be sloped or braced for stability and safety reasons. The Owner and Contractor should be aware of, and become familiar with, applicable local, state, and federal safety regulations, including current OSHA Excavation and Trench Safety Standards. Construction-site safety generally is the responsibility of the Contractor, who should also be responsible for the means, methods, and sequencing of construction operations.

The Contractor should be aware that slope height, slope inclination, or excavation depths should in no case exceed those specified in local, state, or federal safety regulations, (e.g., OSHA Health and Safety Standards for Excavations, 29 CFR Part 1926), or successor regulations. The shallow soils encountered in the borings are Type A, B, and C when applying the OSHA regulations. Such regulations are strictly enforced, and if they are not followed, the Owner, Contractor, and/or earthwork Subcontractor(s) could be liable for substantial penalties.



Item 4.b.

General Qualifications

The services provided by GeoTest were performed with the degree of skill and care typically performed by other members of the geotechnical engineering profession, practicing in this locale, at this time. No other warranty, expressed or implied, is given.

We appreciate the opportunity to provide geotechnical engineering services. If you have any questions, or require any further assistance, please feel free to contact us.

Sincerely,

1 ml

Michael D. Frede, P.E. Technical Director/Senior Engineer



Appendix A

- Figure 1 Project Location Diagram
- Figure 2 Boring Location Diagram



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Appendix B

- General Notes
- Boring Logs o B-1 through B-12
- Unified Soil Classification System (USCS)
- Atterberg Limits Test Reports
 - B-3:23.5-25'
 - o B-12:3.5-5'





Drilling and Sampling Abbreviations:

AD AS BS DD FT GP GS HA HS	Solid-Stem Auger Auger Sample Bulk Sample Diamond Core Drilling Fish Tail Geoprobe Giddings Sampler Hand-Auger Drilling Hollow-Stem Auger	OS PMT RD SS SS ST VS WOH WS Blows per foot of a 140-poun sampler, except where otherw	Osterberg Sampler, 3-inch-O.D. Shelby Tube Pressuremeter Test (In Situ) Rotary Drilling Split-Spoon Sampler, 1.375-inch-I.D., 2-inch-O.D. (Unless otherwise noted) Shelby Tube Sampler, 2-inch-O.D. (Unless otherwise noted) Vane Shear Weight of Hammer Wash Sample d hammer falling 30 inches on a 2-inch-O.D. split-spoon ise noted.
		sampler, except where otherw	ise noted.

Water Level Measurement Abbreviations:

AAR	After Auger Removal	BCR	Before Casing Removal	WS	While Sampling
AB	After Boring	DCI	Dry Cave In		
ACR	After Casing Removal	WCI	Wet Cave In		
BAR	Before Auger Removal	WD	While Drilling		
BCI	Before Casing Installation	WL	Water Level		

Water levels indicated on the boring logs are the levels measured in the boring at the times indicated. In relatively pervious soils, the observed water levels are considered a reliable indicator of groundwater positions. In relatively impervious soils, the accurate determination of groundwater elevations may not be possible, even after several days of observations. In this case, other indicators of groundwater position, such as sealed observation wells or piezometers, may be required.

Gradation Description and Terminology:

Coarse-grained granular soils have more than 50% of their dry weight retained on a #200 sieve (0.074 mm); they include boulders, cobbles, gravel, sand, and combinations thereof. Fine-grained soils have less than 50% of their dry weight retained on a #200 sieve. Fine-grained granular soils are non-cohesive, and include silt; fine-grained cohesive soils include silty clay, and clay.

Major Component of Sample	Size Range	Description of Components Present in Sample	Percent of Dry Weight
Boulders	Over 8" (200 mm)	Trace	<5
Cobbles	8" to 3" (200 to 75 mm)	Few	5 - 10
Gravel	3" to #4 sieve (75 to 4.76 mm)	Little	15 - 25
Sand	#4 to #200 sieve (4.76 to 0.074 mm)	Somo	30 - 45
Silt	Passing #200 sieve (0.074 to 0.005 mm)	Some	36 - 48
Clay	Smaller than 0.005 mm		

Consistency of Cohesive Soils

Relative Density of Granular Soils

Unconfined Compressive Strength, Qu. tsf	Consistency	N, Blows per 12 inches	Relative Density
<0.25	Very Soft	0 - 3	Very Loose
0.25 - 0.49	Soft	4 - 9	Loose
0.50 - 0.99	Medium Stiff	10 - 29	Medium Dense
1.00 - 1.99	Stiff	30 - 49	Dense
2.00 - 3.99	Very Stiff	50 - 80	Very Dense
>4.00	Hard	>80	Extremely Dense

SOIL I	BORI	NG LOG					ъ		Item 4.b.
Boring	By:	Point of Beginning Inc.		Auger:		3.25"HSA			
Project:Victor Haen Elementary School AdditionLocation:See MapRig:Mobile B57 ATV						Drillers: Date:		DC/TH 3/26/25	
RIG:		Mobile B57 ATV Classification/Description	#	Sample	N	Rec	Elev	ation:	720.77 Notes
(ft.)		Classification Description	π	Depth (ft.)	1	(in.)	111	(tsf)	notes
1	-	Concrete 5"	1	1 - 2.5	10	17"	М	4.5	MC 17.4%
2	-	Brown F-M Sand W/ Little Silt and Little Gravel USCS - SP							
3	-	Brown Silty Sandy Clay w/ Little Gravel USCS - CL	2	3.5 - 5	10	0"			
4	-								
5	-								
6	-	6.0'	3	6 - 7.5	16	12"	W	1.75	MC 19.3%
7	-	USCS - ML							
8	-				0		~		
9	-	9.0 Prown Silty Sandy Clay w/ Trace Graval	4	8.5 - 10	9	13"	S	1.5	MC 19.9%
10	-	Brown Sitty Sandy Clay w/ Trace Graver							
11	-								
12	-								
13	-	12 51	5	12.5 15	ſ	111	G	1.0	MC 20 50/
14	-	Brown Silty Sandy Clay w/ Little Gravel	5	13.5 - 15	0	11"	2	1.0	MC 20.5%
15	-	0505-01							
16	-								
17	-								
18	-	18.5'	6	18.5 - 20	21	10"	s		MC 20.9%
19	-	Gray Sandy Silt w/ Trace Gravel USCS - ML	Ŭ	10.5 20	21	10	5		1110 20.970
20	-	Water @ 19.0'							
21	-								
22	-	23.5'							
23	-	Brown Silty Clay w/ Trace Gravel USCS - CL		22.5.25	4	10"		1.0	
24	-	E.O.B. 25' Water at 19.0' at Completion Backfilled with Pontonite China	7	23.5 - 25	4	18"	S	1.0	MC 21.5%
25	-	Backfilled with Bentonite Chips							
Point o	of Beg	inning Inc.						POB#	25.2022

SOIL	BORI	NG LOG					ъ		Item 4.b.
Boring	By:	Point of Beginning Inc.						ing: jer:	3.25"HSA
Projec Locatio Rig:	t: on:	Victor Haen Elementary School Addition See Map Mobile B57 ATV					Dril Date	e: lers: e: ation:	DC/TH 3/25/25 714 33
Depth		Classification/Description	#	Sample	N	Rec	M	Qp (taf)	Notes
(11.)	-	Dark Brown Silty Sandy Clay Topsoil	1	0 - 2	2	18"	S	(181)	MC 34.3%
1 2	- -	Dark Brown Silty Clayey Peat USCS - PT							
3	-								
4	- -	3.5' Brown Silty Sandy Clay w/ Trace Gravel	2	3.5 - 5	10	16"	S	2.75	MC 20.7%
5	-	0505-01							
6	-		3	6 - 7.5	11	15"	S	2.75	MC 22.1%
7	-								
8	-	0.51		0.5.10	6	1.61	G	2.25	
9	-	8.5' Brown/Red Sily Clay w/ Little Sand and Trace Grave USCS - CL	4 	8.5 - 10	6	16"	8	2.25	MC 21.0%
10	-	Water @ 9.0'							
11	-								
12	-								
13	-		5	12.5 15	Λ	10"	G	1.5	MC 21 20/
14	-		3	13.3 - 13	4	18	3	1.5	MC 21.2%
15	-								
16	-								
17	-								
18	-			10.5 00		1.0#	G	1.5	
19	-		0	18.5 - 20	4	18"	2	1.5	MC 24.1%
20	-								
21	-								
22	- -								
23	-								
24	-	E.O.B. 25' Water at 9.0' at Completion	7	23.5 - 25	2	18"	S	1.0	MC 21.8%
25	-	Backfilled with Bentonite Chips							
Point o	of Beg	inning Inc.				ı		POB#	25.2022

SOIL I	BORI	NG LOG					ъ			ltem 4.b.
Boring	By:	Point of Beginning Inc.				Auger:			5"HSA	
Project Locatio Rig:	t: on:	Victor Haen Elementary School Addition See Map Mobile B57 ATV					Page Dril Date Elev	e: lers: e: vation:	D 3/	1 of 1 C/TH 25/25 715.68
Depth (ft)		Classification/Description	#	Sample Depth (ft)	Ν	Rec (in)	Μ	Qp (tsf)	Nc	otes
1	-	Dark Brown Silty Sandy Clay Topsoil	1	0 - 2	4	18"	S	(151)	MC 2	21.4%
2	- - -	(Fill) USCS - CL								
3 4	- - -	3.5' Brown Silty Sandy Clay w/ Trace Gravel	2	3.5 - 5	16	16"	М		MC 1	6.5%
5	-	USCS - CL								
6	-	Wester @ 7.5!	3	6 - 7.5	25	16'	W		MC 1	7.9%
8	- -	w ater (<i>w</i> /.5'								
9	-	9.0 Brown/Red Silty Clay w/ Little Sand and Trace Grave	4	8.5 - 10	24	15"	Μ	4.5	MC 1	2.2%
10	-	USCS - CL								
11 12	-									
13	- -		5	125 15	o	15"	G	1 75	MC	0.10/
14	-		3	15.5 - 15	8	15	2	1./3	MC	19.1%
15 16	-									
17	-									
18	- -		6	18.5 - 20	9	18"	S	1.75	MC 2	21.9%
19	-									
20	-									
21	-									
22	-									
23 24		E.O.B. 25' Water at 7.5' at Completion	7	23.5 - 25	4	18"	S	1.25	MC 2	23.1%
25 Point o	- of Beg	Backfilled with Bentonite Chips						POB#	25.202	2

SOIL	BORI	NG LOG					ъ		Item 4.b.
Boring	g By:	Point of Beginning Inc.		Bor Aug Pag	er:	3.25"HSA			
Projec Locati	t: on:	Victor Haen Elementary School Addition See Map					Dril Date	lers:	DC/TH 3/26/25
Rig:		Mobile B57 ATV Classification/Description	#	Sampla	N	Dec	Elev	vation:	717.41 Notes
(ft.)		Classification Description	π	Depth (ft.)	1	(in.)	101	(tsf)	notes
1		Dark Brown Silty Sandy Clay Topsoil	1	0 - 2	6	18"	S	2.5	MC 20.4%
2	-	Brown Silty Sandy Clay w/ Trace Gravel USCS - CL							
3	-								
5	-		2	3.5 - 5	19	12"	М		MC 16.2%
4	-	4.51							
5	-	Brown Mostly Fine Silty Sand w/ Trace Gravel USCS - SP-SM							
6	-		3	6 - 7.5	16	10"	S		MC 18.2%
7	-	6.5' Brown F-M Sand w/ Little Silt and Trace Gravel USCS - SP							
8	-								
9	-	8.5' Brown Silty Sandy Clay w/ Trace Gravel	4	8.5 - 10	16	12"	М	3.75	MC 14.9%
10	-	0505-02							
11	-								
12	-	Water @ 12.0'							
13	-		5	12 5 15	14	15"	ç	2 75	MC 22 29/
14	-		5	15.5 - 15	14	15	٥	2.75	WIC 25.276
15	-								
16	-								
17	-								
1/	-								
18	-		6	18 5 20	Q	12"	c	2.0	MC 21 70/
19	-			10.3 - 20	0	13	6	2.0	1910 21./70
20	-								
21	-								
22	-								
23	-								
	-	E.O.B. 25'	7	23.5 - 25	8	16"	W	1.5	MC 18.3%
24	-	water at 12.0' at Completion Backfilled with Bentonite Chips							
25	-	1							25 2022
Point	or Red	inning inc.						rob#	25.2022
SOIL	BORI	NG LOG					ъ		Item 4.b.
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Boring	By:	Point of Beginning Inc.					Aug	er:	3.25"HSA
Projec Locatio	t: on:	Victor Haen Elementary School Addition See Map		Page Dril Date	e: lers:	DC/TH 3/26/25			
Rig:		Mobile B57 ATV					Elev	ation:	716.05
Depth (ft.)		Classification/Description	#	Sample Depth (ft.)	Ν	Rec (in.)	М	Qp (tsf)	Notes
1		Asphalt 7.5"	1	1 - 2.5	8	16"	S		MC 20.5%
2	-	Brown F-M Sand and Gravel w/ Little Silt (CABC)							
3	-	Dark Brown Silty Sandy Clay w/ Little Gravel	2	255	12	15"	м		MC 16 8%
4	-	USCS - CL 3 5'	2	5.5 - 5	15	15	IVI		WIC 10.870
5	-	Brown Silty Sandy Clay w/ Trace Gravel USCS - CL							
6	-	4.5' Brown Sandy Silt w/ Trace Gravel	3	6 - 7.5	17	13"	S		MC 21.0%
7	-	ÚSCS - ML							
8	-		4	8.5 - 10	18	14"	W		MC 16.3%
9	-								
10	-								
11	-								
12	-								
13	-	13.5' Brown Silty Clay w/ Little Sand and Trace Gravel	5	13.5 - 15	6	16"	S	1.25	MC 20.6%
15	-	USCS - CL							
16	-								
17									
18	-								
19	-		6	18.5 - 20	6	14"	S	1.25	MC 28.9%
20	-								
21	-								
22	-								
23	-	EOP 25	7	22 5 25	1	16"	q	15	MC 20 20/
24	-	Water at 14.0' at Completion Backfilled with Bentonite Chins	/	23.3 - 23	4	10	6	1.3	IVIC 20.370
25 Point o	- of Beg	inning Inc.						POB#	25.2022

SOIL I	BORI	NG LOG					р.		Item 4.b.
Boring	By:	Point of Beginning Inc.		Bor Aug Pag	er:	3.25"HSA			
Project Locatio Rig:	t: on:	Victor Haen Elementary School Addition See Map Mobile B57 ATV		Drillers: Date: Elevation:		DC/TH 3/26/25 717.05			
Depth		Classification/Description	#	Sample	N	Rec	M	Qp	Notes
(ft.)		Dork Prown Silty Sondy Clay Tongoil	1	Depth(ft.)	0	(in.)	м	(tsf)	MC 15 80/
1	-		1	0-2	0	10	IVI		MC 13.870
2	-	Brown/Red Silty Sandy Clay w/ Trace Gravel USCS - CL							
3	-								
5	-	Brown Sandy Silt w/ Trace Gravel	2	3.5 - 5	15	14"	М		MC 17.7%
4	-	ÚSCS - ML							
5	-								
5	-								
6	-	(5)	3	6 - 7.5	19	14"	S		MC 17.5%
7	-	Brown Mostly Fine Sand w/ Some Silt							
,	-	and Trace Gravel							
8	-	USCS - SP-SM	4	0 5 10	10	1.61		4.5	
9	-	Brown/Red Silty Sandy Clay w/ Trace Gravel	4	8.5 - 10	12	16"	М	4.5	MC 16.2%
10	-	0363-62							
11	-								
11	-								
12	-								
13	-								
15	-	13.5'	5	13.5 - 15	17	11"	S		MC 18.1%
14	-	Gray Mostly Fine Silty Sand w/ Trace Gravel							
15	-	USCS - SP-SM Water @ 14 0'							
10	-								
16	-								
17	-								
	-								
18	-	Brown Silty Clay w/ Some Sand and Trace Gravel	6	18 5 20	6	18"	w	15	MC 18 20/
19	-	USCS - CL	0	18.3 - 20	0	10	vv	1.5	IVIC 10.270
20	-								
21	-								
22	-								
	-	Red/Brown Clay w/ Little Silt and Trace Gravel							
23	-	USCS - CH	_	22.5.25	C	1.61	C	2.0	
24	-	E.U.B. 25' Water at 14 0' at Completion	1	23.5 - 25	8	16"	S	3.0	MC 35.2%
21	-	Backfilled with Bentonite Chips							
25 Point o	- of Beg	inning Inc.						POB#	25.2022

SOIL I	BORI	NG LOG					D		Item 4.b.
Boring	By:	Point of Beginning Inc.					Bor Aug Bog	3.25"HSA	
Project: Location:		Victor Haen Elementary School Addition See Map		Dril Date	lers:	DC/TH 3/27/25			
Rig:		Mobile B57 ATV	Dee	Elev	ation:	718.26			
(ft.)		Classification/Description	Ŧ	Sample Depth (ft.)	IN	(in.)	IVI	Qp (tsf)	Notes
(100)	-	Dark Brown Silty Sandy Clay Topsoil	1	0-2	7	16"	W	(101)	MC 19.7%
1	-	12"							
2	-	Brown/Red Silty Sandy Clay w/ Trace Gravel							
2	-	0505-01							
3	-								
	-	3.5'	2	3.5 - 5	11	15"	Μ	1.75	MC 14.5%
4	-	Brown Silty Clay w/ Some Sand and Trace Gravel							
5	-	USCS - CL							
5	-								
6	-		3	6 - 7.5	12	12"	Μ		MC 17.0%
7	-								
/	-								
8	_								
Ũ	-	8.5'	4	8.5 - 10	22	15"	S		MC 25.2%
9	-	Brown F-M Sand w/ Some Silt and Trace Gravel							
10	-	USCS - SP-SM Water @ 10.0							
10	-	water @ 10.0							
11	-								
	-								
12	-								
13	-								
15	_		5	13.5 - 15	14	18"	S		MC 23.1%
14	-		_			_			_
1.5	-								
15	-								
16	_								
	-								
17	-								
18	-								
10	-	18.5'	6	18.5 - 20	4	18"	S	1.25	MC 24.1%
19	-	Brown Silty Clay w/ Little Sand and Trace Gravel		•	-	Ĩ			
	-	USCS - CL							
20	-								
21	-								
	-								
22	-								
22	-								
23	-	E.O.B. 25'	7	23.5 - 25	5	18"	s	1.5	MC 20.6%
24	-	Water at 10.0' at Completion	Ĺ	20	·			1.0	1.10 20.070
	-	Backfilled with Bentonite Chips							
25 B cint	-	inning Inc							25 2022
roint o	и веб	mming inc.						rub#	23.2022

SOIL I	BORI	NG LOG					ъ		Item 4.b.
Boring	By:	Point of Beginning Inc.	Auger:			3.25"HSA			
Project Locatio	t: on:	Victor Haen Elementary School Addition See Map		Page Dril Date	e: lers: e:	DC/TH 3/26/25			
Depth		Classification/Description	#	Sample	N	Rec	M	Qp	Notes
(ft.)		Congrete		Depth (ft.)		(in.)		(tsf)	
1	-	5"	1	1 - 2.5	3	16"	W		MC 17.1%
2	-	Brown F-M Sand w/ Little Silt and Trace Gravel (Fill)							
3	-	Brown Silty Sandy Clay w/ Trace Gravel			_				
4	-	USCS - CL	2	3.5 - 5	6	10"	Μ		MC 16.5%
	-								
5	-								
6	-	6.0' Dreamy/Ded Silty See dy Clear m/ Trease Crearel	3	6 - 7.5	6	12"	S	1.75	MC 20.6%
7	-	USCS - CL							
o	-								
0	-		4	8.5 - 10	16	11"	S		MC 18.4%
9	-	9.0' Brown F-M Silty Sand w/ Trace Gravel							
10	-	USCS - SP-SM							
11	-	Water @ 10.0'							
12	-								
12	-								
13	-	13 5'	5	135-15	15	14"	s		MC 15.7%
14	-	Brown Sandy Silt w/ Trace Gravel USCS - ML	5	15.5 - 15	15		5		WIC 15.770
15	-								
16	-								
17	-								
1/	-								
18	-	18 5'	6	185-20	17	16"	м	25	MC 15.6%
19	-	Brown Silty Clay w/ Little Sand and Trace Gravel USCS - CL		10.5 - 20	1/		141	2.3	1010 10.070
20	-								
21	-								
22	-								
22	-								
25	-	E.O.B. 25'	7	23.5 - 25	3	18"	S	1.25	MC 21.7%
24	-	Water at 10.0' at Completion							
25 Point of	- of Beg	inning Inc.						POB#	25.2022

SOIL	BORI	NG LOG					D		Item 4.b.
Boring	By:	Point of Beginning Inc.					Bor Aug	er:	3.25"HSA
Projec Locatio Rig:	t: on:	Victor Haen Elementary School Addition See Map Mobile B57 ATV		Dril Date	e: lers: e: ation:	DC/TH 3/27/25 720.17			
Depth		Classification/Description	#	Sample	N	Rec	M	Qp (tsf)	Notes
(11.)	-	Dark Brown Silty Sandy Clay Topsoil	1	0 - 2	10	18"	М	(181)	MC 16.1%
1 2	- - -	Brown/Red Silty Sandy Clay w/ Trace Gravel (Possible Fill)							
3	-	USCS - CL 1.5'							
4	- -	Brown F-M Silty Sand w/ Trace Gravel (Possible Fill) USCS - SP-SM	2	3.5 - 5	8	12"	W	1.75	MC 18.5%
5	-	3.5' Drown/Dod Silty Sondy Clay w/ Troop Croyal							
6	-	(Possible Fill) USCS - CL	3	6 - 7.5	16	14"	S		MC 17.9%
7	-	7.0' Brown F-M Silty Sand w/ Trace Gravel							
8	-	USČS - SP-SM	1	85 10	27	11"	ç		MC 16 804
9	-		т	0.5 - 10	21	11	5		WIC 10.870
10	-								
11	-								
12	-								
13	- -					1.0.11	a		
14	-	Water @ 14.0'	5	13.5 - 15	14	10"	S		MC 20.5%
15	-								
16	-								
17	-								
18	-	18 5'	6	18 5 - 20	10	16"	ç	2 25	MC 25 2%
19	-	Brown Silty Clay w/ Little Sand and Trace Gravel	0	10.3 - 20	10	10	3	2.23	IVIC 23.270
20	-	0303-01							
21	-								
22	-								
23	-								
24	-	E.O.B. 25' Water at 14.0' at Completion Backfilled with Pontonite China	7	23.5 - 25	6	15"	S	1.5	MC 26.7%
25	- -	Dackinicu with Bentonite Chips						Β ΩΡ#	25 2022
r omt (n Deg	mming me.						г UD#	23.2022

SOIL	BORI	NG LOG					р.		Item 4.b.
Boring	By:	Point of Beginning Inc.					Bori Aug	ing: er:	3.25"HSA
Projec Locatio Rig:	t: on:	Victor Haen Elementary School Addition See Map Mobile B57 ATV		Dril Date Elev	e: lers: e: vation:	DC/TH 3/26/25 720.16			
Depth (ft)		Classification/Description	#	Sample Depth (ft)	N	Rec (in)	M	Qp (tsf)	Notes
1	-	Dark Brown Silty Sandy Clay Topsoil	1	0 - 2	9	18"	Μ	(131)	MC 12.9%
2	-	Brown/Red Silty Sandy Clay w/ Trace Gravel (Possible Fill)							
3	-	1.5'							
4	- - -	Dark Brown Silty Sandy Clay w/ Trace Gravel (Possible Fill) USCS - SP-SM	2	3.5 - 5	13	13"	W	2.5	MC 17.0%
5	-	3.5' Brown/Red Silty Sandy Clay w/ Trace Gravel							
6	-	USCS - CL 6.0'	3	6 - 7.5	30	11"	S		MC 27.2%
7	-	Brown Sandy Silt w/ Trace Gravel USCS - ML							
8	-	8.5'	4	8.5 - 10	11	8"	S		MC 25.8%
9	-	Brown F-M Sand w/ Little Silt and Trace Gravel USCS - SP				_			
10	-	Water @ 9.0'							
11	-								
12	-								
13	-		5	12 5 15	15	10"	ç		MC 22 09/
14	-		5	15.5 - 15	15	10	3		WIC 22.070
15	-								
16	-								
17	-								
18	-								
10	-		6	18.5 - 20	13	13"	S		MC 23.1%
20	-								
21	-								
21	-								
22	-	23.5' Brown Silty Sandy Clay w/ Trace Gravel							
23	-	USCS - CL E O B 25'	7	23.5 - 25	8	18"	S	2.25	MC 194%
24	-	Water at 9.0' at Completion Backfilled with Bentonite Chips	,	25.5 - 25	0	10	2	2.23	MC 17.770
25 Point o	- of Beg	inning Inc.						POB#	25.2022

SOIL	BORI	NG LOG					D		Item 4.b.
Boring	By:	Point of Beginning Inc.		Born Aug	er:	3.25"HSA			
Projec Locatio	t: on:	Victor Haen Elementary School Addition See Map		Dril Date	lers:	DC/TH 3/27/25			
Rig:		Mobile B57 ATV					Elev	ation:	717.84
Depth (ft.)		Classification/Description	#	Sample Depth (ft.)	N	Rec (in.)	М	Qp (tsf)	Notes
1	-	Dark Brown Silty Sandy Clay Topsoil 7"	1	0 - 2	7	12"	S		MC 23.1%
2	-	Brown/Gray Silty Sandy Clay w/ Trace Gravel USCS - CL							
3	-								
4	- -	3.5' Brown F-M Silty Sand w/ Trace Gravel USCS - SP-SM	2	3.5 - 5	26	10"	W		MC 10.1%
5	-								
6	-		3	6 - 7.5	20	13"	S		MC 20.8%
7	-								
8	-		4	8.5 - 10	14	14"	S		MC 23 3%
9	-			0.0 10	11	1	2		1110 2010 / 0
10	-								
11	-								
12	-								
13	-	13.5'	5	13.5 15	13	13"	м	35	MC 14 3%
14	-	Brown Sandy Clay w/ Little Silt and Trace Gravel USCS - CL	5	15.5 - 15	15	15	101	5.5	WIC 14.370
15	-	Water @ 14.0'							
16	-								
17	-								
18	-	18.5'	6	18.5 - 20	26	18"	W	2.25	MC 16.3%
19	-	Brown F-M Silty Sand w/ Trace Gravel USCS - SP-SM	Ŭ						
20	-	19.5' Brown Silty Sandy Clay w/ Trace Gravel							
21	-	USCS - CL							
22	-								
23	-		7	22.5.25	0	1 4 11	5	2.25	MC 20 20/
24		E.O.B. 25' Water at 14.0' at Completion Backfilled with Bentonite Chips	/	23.5 - 25	9	14"	2	2.25	MC 20.2%
25 Point o	- of Beg	inning Inc.						POB#	25.2022

SOIL	BORI	NG LOG					D		Item 4.b.
Boring	By:	Point of Beginning Inc.					Bor Aug	3.25"HSA	
Projec Locatio	t: on:	Victor Haen Elementary School Addition See Map						lers: e:	DC/TH 3/27/25
RIG:		NIODILE B57 ATV Classification/Description	#	Sampla	N	Dee	Elev	ation:	/17.99 Notes
(ft.)		Classification/Description	#	Depth (ft.)	IN	(in)	11/1	(tsf)	INOLES
1	-	Dark Brown Silty Sandy Clay Topsoil (Fill)	1	0 - 2	7	18"	S	()	MC 21.2%
2	-								
3	-								
Δ	-	4 0'	2	3.5 - 5	12	16"	М	4.5	MC 13.6%
5	-	Brown Silty Sandy Clay w/Little Gravel USCS - CL							
6	-	6.0'	3	6 - 7.5	21	14"	w		MC 12.6%
7	-	Brown F-M Silty Clayey Sand w/ Trace Gravel USCS - CL							_
8	-								
9	-		4	8.5 - 10	23	15"	W		MC 15.5%
10	-								
11	-								
12	-								
13	-								
14	-	13.5' Brown/Red Silty Clay w/ Some Sand and Trace Grave	5	13.5 - 15	6	15"	W	2.0	MC 19.6%
15	-	USCS - CL Water @ 14.0'							
16	-								
17	-								
18	-								
19	-	19.0'	6	18.5 - 20	13	14"	S		MC 13.7%
20	-	Brown F-M Silty Sand w/ Trace Gravel USCS - SP-SM							
21	-								
22	-	23.5'							
23	-	Brown/Red Silty Clay w/ Some Sand and Trace Grave USCS - CL	 	22.5.25	7	1.7"	C	1.5	
24	-	E.O.B. 25' Water at 14.0' at Completion	/	23.5 - 25	1	16"	S	1.5	MC 24.0%
25	- -	BackTilled with Bentonite Chips							25 2022
Point o	oi Reã	inning inc.						rob#	25.2022



Ма	ajor Divis	ions	Group symbols	Typical Names	Laboratory classification criteria				
	tction is ize)	gravels no fines)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines	$\widehat{\underbrace{\mathbf{p}}_{0}} \qquad \underbrace{\mathbf{p}}_{\mathbf{p}} = \underbrace{\mathbf{D}_{0}}_{\mathbf{D}_{10}} \text{ greater than 4; } \mathbf{C}_{c} = \frac{(\mathbf{D}_{30})^{2}}{\mathbf{D}_{10} \times \mathbf{D}_{60}} \text{ between 1 and 3}$				
ieve size)	ivels f coarse fra o. 4 sieve s	Clean (Little or	GP	Poorly graded gravels, gravel-sand mixtures, little or no fines	Not meeting all gradation requirements for GW				
ained soils arger than No. 200 s	Gra than half of jer than No	s with fines reciable nt of fines)	GM d	Silty gravels, gravel-sand-silt mixtures	Atterberg limits below "A" Atterberg limits below "A" Above "A" line with P.I. Between 4 and 7 are borderline cases				
	(More 1 larç	Gravels (Appre amount	GC	Clayey gravels, gravel-sand-clay mixtures	Atterberg limits above "A" requiring use of dual symbols				
Coarse-gr naterial is l	action is size)	sands no fines)	sw	Well-graded sands, gravelly sands, little or no fines	$ \begin{array}{c} \begin{array}{c} \begin{array}{c} \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\$				
(More than half of m	nds f coarse fra lo. 4 sieve	Clean (Little or	SP	Poorly graded sands, gravelly sands, little or no fines	Note meeting all gradation requirements for SW				
	Sa than half o aller than N	s with fines preciable unt of fines)	SM d	Silty sand, sand-silt mixtures	Atterberg limits below "A" Limits plotting in hatched zone with P.I. less than 4 and 7 are borderline				
	enoM) sma	Sands v (Appr amount	SC	Clayey sands, sand-clay mixtures	Image: Section of the section of th				
size)	/s 1an 50)		ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity	60 For classification of fine-grained soils and fine fraction of coarse-				
200 sieve	ilts and cla	lts and cla		lts and clay limit less th		lts and clay limit less th		Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silt clays, lean clays	50 Atterburg Limits plotting in hatched area are borderline classifications requiring use of dual symbols
soils ¥r than No.	S	(Liquid	OL	Organic silts and organic silty clays of low plasticity	40 Equation of A-line: PI=0.73(LL-20)				
e-grained ; al is smalle	sk	than 50)	мн	Inorganic silts, micaceous or diatomaceous fine sandy or silty soil, elastic silts	S OH and MH				
Fin f of materia	ilts and cla	mit greater	СН	Inorganic clays of high plasticity, fat clays					
re than hal	Sil (Liquid lin		он	Organic clays of medium to high plasticity, organic silts	CL-ML ML and OL 0 10 20 30 40 50 60 70 80 90 100				
юW)	Highly	soils	Pt	Peat and other highly organic soils	Liquid Limit Plasticity Chart				

ltem 4.b.

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Appendix C

- Drilling Procedures
- Sampling Procedures
- Laboratory Procedures





Hand-Auger Drilling (HA)

A sampling device is driven into the soil to the desired sample depth by a sledge hammer. After extracting the sample, the hole is advanced by a hand auger until the next sampling depth is reached. The manual driving of the sampler, especially into cohesive soils, may result in some sample disturbance. However, there are some situations where this method is the only viable option.

Solid-Stem Auger Drilling (AD)

Continuous flight augers are turned and hydraulically advanced by a truck- or track-mounted unit to create a borehole. In solid-stem auger drilling, casing and drilling mud are not typically used to maintain an open borehole.

Hollow-Stem Auger Drilling (HS)

Continuous flight augers having open stems are used to advance the borehole. The open stem allows the sampling tool to be used without removing the augers from the borehole. Hollow-stem augers maintain an open borehole during the sampling operations. This sampling method is not appropriate for geotechnical investigation beneath the water table, especially in granular soils.

Rotary Drilling (RD)

Various cutting bits, in conjunction with circulating drilling fluid, are used to advance the borehole. Surface casing is used to maintain sidewall stability in the top several meters of the borehole, and to facilitate the circulation of the drilling fluid into the mud tank.

Diamond Core Drilling (DD)

A double-tube or triple-tube core barrel with a diamond bit cuts an annular space around a cylinder of rock or cemented material. When the coring has proceeded to the desired core run length, the core is broken off and the sample is retained by a core catcher just above the diamond bit. Samples recovered by this procedure are placed in sturdy core boxes in sequential order.



Auger Sampling (AS)

Soil samples are obtained as cuttings from the auger flights as they are lifted from the borehole. Auger samples provide a general indication of subsurface conditions; however, they do not provide undisturbed samples, nor do they provide samples from specific depths. Due to the possible loss of soil components, or the mixing of soil components from various elevations, auger samples may not be representative of in-situ soil conditions.

Split-Barrel Sampling (SS) - ASTM Standard D-1586-84

A 2-inch-O.D. split-barrel sampler is driven into the soil a distance of 18 inches by a 140pound hammer free-falling 30 inches. The first 6 inches of penetration is usually considered a seating drive. The Standard Penetration Resistance value is the number of blows of the hammer over the final 12 inches of driving. This value provides an indication of the in-place relative density of granular soils. The indication should be considered qualitative, since many variables such as drill crews, drill rigs, drilling procedures, and hammer-rod-sampler assemblies can significantly affect the Standard Penetration Resistance value. A representative portion of the soil sample is recovered from the split-barrel sampler, placed in a sample jar, and delivered to our laboratory for further examination and possible testing.

Shelby Tube Sampling Procedure (ST) - ASTM Standard D-1587-83

A 2- or 3-inch-diameter thin-walled seamless steel tube having a sharp cutting edge is hydraulically pushed into the soil to obtain a relatively undisturbed sample. This procedure is generally used for cohesive soils. The Shelby tubes are carefully handled to minimize sample disturbance, and delivered to a laboratory where the soil is extruded from the tube, examined, and tested.

American Society for Testing and Materials

ASTM 1586

Standard Method for Penetration Test and Split-Barrel Sampling of Soils¹

This standard is issued under the fixed designation D 1586; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of the last revision. A number in parentheses indicates the year of the last reapproval. A superscript epsilon (\in) indicates an editorial change since the last revision or reapproval.

This method has been approved for use by agencies of the Department of Defense and for listing in the DOD Index of Specifications and Standards.

1. Scope

1.1 This method describes the procedure, generally known as the Standard Penetration (SPT), for driving a splitbarrel sampler to obtain a representative soil sample and a measure of the resistance of the soil to penetration of the sampler.

1.2 This standard may involve hazardous materials, operations, and equipment. This standard does not purport to address all of the safety problems associated with its use. It is the responsibility of whoever uses this standard to consult and establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use. For a specific precautionary statement, see 5.4.1.

1.3 The values stated in inch-pound units are to be regarded as the standard.

2. Applicable Documents

2.1 ASTM Standards:

D2487 Test Method for Classification of Soils for Engineering Purposes²

D2488 Practice for Description and Identification of Soils (Visual-Manual Procedure)²

D4220 Practice for Preserving and Transporting Soil Samples²

3. Descriptions of Terms Specific to This Standard

3.1 anvil--that portion of the driveweight assembly while the hammer strikes and through which the hammer energy passes into the drill rods.

3.2 cathead-the rotating drum or windlass in the rope-cathead lift system around which the operator wraps a rope to lift and drop the hammer by successively tightening and loosening the rope turns around the drum

3.3 drill rods—rods used to transmit downward force and torque to the drill bit while drilling a borehole.

3.4 drive-weight assembly--a device consisting of the hammer, hammer fall guide, the anvil, any hammer drop system.

3.5 hammer-that portion of the drive-weight assembly consisting of the 140 ± 2 lb $(63.5 \pm 1 \text{ kg})$ impact weight which is successfully lifted and dropped to provide the energy that accomplishes the sampling and penetration.

3.6 hammer drop system--that portion of the drive-weight assembly by which the operator accomplishes the lifting and dropping of the hammer to produce the blow.

3.7 hammer fall guide--that part of the drive-weight assembly used to guide the fall of the hammer.

3.8 N-value—the blowcount representation of the penetration resistance of the soil. The N-value, reported in blows per foot, equals the sum of the number of blows required to drive the sampler over the depth interval of 6 to 18 in. (150 to 450 mm) (see 7.3).

3.9 ΔN -the number of blows obtained from each of the 6-in. (150-mm) intervals os sampler penetration (see 7.3).

3.10 number of rope turns-the total contact angle between the rope and the cathead at the beginning of the operator's rope slackening to drop the hammer; divided by 360° (see Fig. 1). 3.11 sampling rods--rods that connect the drive-weight assembly to the sampler. Drill rods are often used for this purpose.

3.12 SPT--abbreviation for Standard Penetration Test, a term by which engineers commonly refer to this method.

4. Significance and Use

4.1 This method provides a soil sample for identification purposes and for laboratory tests appropriate for soil obtained from a sampler that may produce large shear strain disturbance in the sample.

4.2 This method is used extensively in a great variety of geotechnical exploration projects. Many local correlations and widely published correlations which relate SPT blowcount, or N-value, and the engineering behavior of earthworks and foundation are available.

5. Apparatus

5.1 Drilling Equipment-Any drilling equipment that provides at the time of sampling a suitably clean open hole before insertion of the sampler and ensures that the penetration test is performed on undistributed soil shall be acceptable. The following pieces of equipment have proven to be suitable for advancing a borehole in some subsurface conditions.

⁴This method is under the jurisdiction of ASTM Committee D-18 on Soil and Rock and is the direct responsibility of subcommittee D18.02 on Sampling and Related Field Testing for Soil Investigations.

Current edition approved Sept. 11, 1984. Published November 1984. Originally published as D1586-58T. Last previous edition D1586-67 (1974).

²Annual Book of ASTM Standards, Vol 04.08.

5.1.1 Drag, Chopping and Fishtail Bits, less than 6.5 in. (162 mm) and greater than 2.2 in. (56 mm) in diameter may be used in conjunction with openhole rotary drilling or casing-advancement drilling methods. To avoid disturbance of the underlying soil, bottom discharge bits are not permitted; only side discharging bits are permitted.

5.1.2 Roller-Cone Bits, less than 6.5 in. (162 mm) and greater than 2.2 in. (56 mm) in diameter may be used in conjunction with open-hole rotary drilling or casing-advancement drilling methods if the drilling fluid discharge is deflected.

5.1.3 Hollow-Stem Continuous Flight Augers, with or without a center bit assembly, may be used to drill the boring. The inside diameter of the hollow-stem augers shall be less than 6.5 in. (162 mm) and greater that 2.2 in. (56 mm).

5.1.4 Solid, Continuous Flight, Bucket and Hand Augers, less than 6.5 in. (162 mm) and greater than 2.2 in. (56 mm) in diameter may be used if the soil on the side of the boring does not cave into the sampler or sampling rods during the sampling.

5.2 Sampling Rods--Flush-joint steel drill rods shall be used to connect the split-barrel sampler to the driveweight assembly. The sampling rod shall have a stiffness (moment of inertia) equal to or greater than that of a parallel wall "A" rod (a steel rod which has an outside diameter of 1 5/8 in. (41.2 mm) and an inside diameter of 1 1/8 in. (28.5 mm).

NOTE 1--Recent research and comparative testing indicates the type rod used, with stiffness ranging from "A" size rod to "N" size rod, will usually have a negligible effect on the N-values to depths of at least 100 ft. (30 m).

5.3 Split-Barrel Sampler--The sampler shall be constructed with the dimensions indicated in Fig. 2. The driving shoe shall be hardened steel and shall be replaced or repaired when it becomes dented or distorted. The use of liners to produce a constant inside diameter of 1 3/8 in. (35 mm) is permitted, but shall be noted on the penetration record if used. The use of a sampler retainer basket is permitted, and should also be noted on the penetration record if used.

NOTE 2--Both theory and available test datas suggest that N-values may increase 10 to 30% when liners are used.

5.4 Drive-Weight Assembly:

5.4.1 Hammer and Anvil--The hammer shall weigh 140 ± 2 lb $(63.5 \pm 1 \text{ kg})$ and shall be a solid rigid metallic mass. The hammer shall strike the anvil and make steel on steel contact when it is dropped. A hammer fall guide permitting a free fall shall be used. Hammers used with the cathead and rope method shall have an unimpeded overlift capacity of at least 4 in. (100 mm). For safety reasons, the use of hammer assembly with an internal anvil is encouraged.

NOTE 3--It is suggested that the hammer fall guide be permanently marked to enable the operator or inspector to judge the hammer drop height.

5.4.2 Hammer Drop System--Ropecathead, trip, semi-automatic, or automatic hammer drop systems may be used, providing the lifting apparatus will not cause penetration of the sampler while re-engaging and lifting the hammer.

5.5 Accessory Equipment-Accessories such as labeled, sample containers, data sheets, and groundwater level measuring devices shall be provided in accordance with the requirements of the project and other ASTM standards.

6. Drilling Procedure

6.1 The boring shall be advanced incrementally to permit intermittent or continuous sampling. Test intervals and locations are normally stipulated by the project engineer or geologist. Typically, the intervals selected are 5 ft. (1.5 m) or less in homogeneous strata with test and sampling locations at every change of strata.

6.2 Any drilling procedure that provides a suitably clean and stable hole before insertion of the sampler and assures that the penetration test is performed on essentially undisturbed soil shall be acceptable. Each of the following procedures have proven to be acceptable for some subsurface conditions. The subsurface conditions anticipated should be considered when selecting the drilling method to be used.

6.2.1 Open-hole rotary drilling method.

6.2.2 Continuous flight hollow-stem auger method.

6.2.3 Wash boring method.

6.2.4 Continuous flight soli method.

6.3 Several drilling methods produce unacceptable borings. The process of jetting through an open tube sampler and then sampling when the desired depth is reached shall not be permitted. The continuous flight solid auger method shall not be used for advancing the boring below a water table or below the upper confining bed of a confined noncohesive stratum that is under artesian pressure. Casing may not be advanced below the sampling elevation prior to sampling. Advancing a boring with bottom discharge bits is not permissible. It is not permissible to advance the boring for subsequent insertion of the sampler solely by means of previous sampling with the SPT sampler. . . .

6.4 The drilling fluid within the boring or hollow-stem augers shall be maintained at or above the in situ groundwater level at all times during drilling, removal of drill rods, and sampling.

7. Sampling and Testing Procedure

7.1 After the boring has been advanced to the desired sampling elevation and excessive cuttings have been removed, prepare for the test with the following sequence of operations.

7.1.1 Attach the split-barrel sampler to the sampling rods and lower into borehole. Do not allow the sampler to drop onto the soil to be sampled.

7.1.2 Position the hammer above and attach the anvil to the top of the sampling rods. This may be done before the sampling rods and sampler are lowered into the borehole.

7.1.3 Rest the dead weight of the sampler, rods, anvil, and drive weight on the bottom of the boring and apply a seating blow. If excessive cuttings are encountered at the bottom of the boring, remove the sampler and sampling rods from the boring and remove the cuttings.

7.1.4 Mark the drill rods in three successive 6-in. (0.15-m) increments so that the advance of the sampler under the impact of the hammer can be easily observed for each 6-in. (0.15-m) increment.

7.2 Drive the sampler with blows from the 140-lb (63.5-kg) hammer and count the number of blows applied in 7.2.1 A total of 50 blows have been applied during any one of the three 6-in. (0.15-m) increments described in 7.1.4.

7.2.2 A total of 100 blows have been applied.

7.2.3 There is no observed advance of the sampler during the application of 10 successive blows of the hammer.

7.2.4 The sampler is advanced the complete 18 in. (0.45 m) without the limiting blow counts occurring as described in 7.2.1, 7.2.2, or 7.2.3.

Record the number of blows 7.3 required to effect each 6 in. (0.15 m) of penetration or fraction thereof. The first 6 in. is considered to be a seating drive. The sum of the number of blows required for the second and third 6 in. of penetration is termed the "standard penetration resistance", or the "N-value". If the sampler is driven less than 18 in. (0.45 m), as permitted in 7.2.1, 7.2.2, or 7.2.3, the number of blows per each complete 6 in. (0.15-m) increment and per each partial increment shall be recorded on the boring log. For partial increments, the depth of penetration shall be reported to the nearest 1 in. (25 mm), in addition to the number of blows. If the sampler advances below the bottom of the boring under the static weight of the hammer, this information should be noted on the boring log.

7.4 The raising and dropping of the 140-lb (63.5-kg) hammer shall be accomplished using either the following two methods:

7.4.1 By using a trip, automatic, or semi-automatic hammer drop system which lifts the 140-lb (63.5 kg) hammer and allows it to drop 30 ± 1.0 in. (0.76 m ± 25 mm) unimpeded.

7.4.2 By using a cathead to pull a rope attached to the hammer. When the cathead and rope method is used the system and operation shall conform to the following:

7.4.2.1 The cathead shall be essentially free of rust, oil, or grease and have a diameter in the range of 6 to 10 in. (150 to 250 mm).

7.4.2.2 The cathead should be operated at a minimum speed of rotation of 100 RPM, or the approximate speed of rotation shall be reported on the boring log.

7.4.2.3 No more than 2 1/4 rope turns on the cathead may be used during the performance of the penetration test, as shown in Fig. 1.

NOTE 4--The operator should generally use either 1 3/4 of 2 1/4 rope turns, depending upon whether or not the rope comes off the top (1 3/4 turns) or the bottom (2 1/4 turns) of the cathead. It is generally known and accepted that 2 3/4or more rope turns considerably impedes the fall of the hammer and should not be used to perform the test. The cathead rope should be maintained in a relatively dry, clean, and unfrayed condition.

7.4.2.4 For each hammer blow, a 30in. (0.76 m) lift and drop shall be employed by the operator. The operation of pulling and throwing the rope shall be performed rhythmically without holding the rope at the top of the stroke.

7.5 Bring the sampler to the surface and open. Record the percent recovery or length of sample recovered. Describe the soil samples recovered as to composition, color, stratification, and condition, then place one or more representative portions of the sample into sealable moisture-proof containers (jars) without ramming or distorting any apparent stratification. Seal each container to prevent evaporation of soil moisture. Affix labels to the containers bearing job designation, boring number, sample depth, and the blow count per 6-in. (0.15 m) increment. Protect the samples against extreme temperature changes. If there is a soil change within the jar for each stratum and note its location in the sampler barrel.

8. Report

8.1 Drilling information shall be recorded in the filed and shall include the following:

8.1.1 Name and location of job,

8.1.2 Names of crew,

8.1.3 Type and make of drilling machine,

8.1.4 Weather conditions,

8.1.5 Date and time of start and finish of boring,

8.1.6 Boring number and location (station and coordinates, if available and applicable),

8.1.7 Surface evaluation, if applicable

8.1.8 Method of advancing and cleaning the boring,

8.1.9 Method of keeping boring open,

8.1.10 Depth of water surface and

drilling depth at time of a noted loss or drilling fluid, and time and date when reading or notation was made,

8.1.11 Location of strata changes,

8.1.12 Size of casing, depth of cased portion of boring,

8.1.13 Equipment and method of driving sampler,

8.1.14 Type of sampler and length and inside diameter of barrel (note use of liners),

8.1.15 Size, type and section length of the sampling rods, and

8.1.16 Remarks.

8.2 Data obtained for each sample shall be recorded in the field and shall include the following:

8.2.1 Sample depth and, if utilized, the sample number,

8.2.2 Description of soil,

8.2.3 Strata changes within sample,

8.2.4 Sampler penetration and recovery lengths, and

8.2.5 Number of blows per 6-in. (0.15 m) or partial increment.

9. Precision and Bias

9.1 Variations in N-values of 100% or more have been observed when using different standard penetration test apparatus and drillers for adjacent borings in the same soil formation. Current opinion, based on field experience, indicates that when using the same apparatus and driller N-values in the same soil can be reproduced with coefficient or variation of about 10%.

9.2 The use of faulty equipment, such as extremely massive or damaged anvil, a rusty cathead, a low speed cathead, an old, oily rope, or massive or poorly lubricated rope sheaves can significantly contribute to differences in Nvalues obtained between operator-drill rig systems.

9.3 The variability in N-values produced by different drill rigs and operators may be reduced by measuring the part of the hammer energy delivered into the drilling rods from the sampler and adjusting N on the basis of comparative energies. A method for energy measurement and N-value adjustment is currently under development.

Item 4.b.

ASTM Designation: D 1586



FIG. 1 Definitions of the number of rope turns and the angle for (a) counterclockwise rotation and (b) clockwise rotation of the cathead



 $E = 0.10 \pm 0.02 \text{ in. } (2.54 \pm 0.25 \text{ mm}) \\ F = 2.00 \pm 0.05 - 0.00 \text{ in. } (50.8 \pm 1.3 - 0.0 \text{ mm}) \\ G = 16.0^\circ \text{ to } 23.0^\circ$

The 1 1/2 in. (38 mm) inside diamter split barrel may be used with a 16-gage wall thickness split liner. The penetrating end of the drive shoe may be slightly rounded. Metal or plastic retainers may be used to retain soil samples.

FIG. 2 Split-Barrel Sampler

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This standard is subject to revision at any time by the responsible technical committee and must be reviewed every five years and if not revised, either reapproved or withdrawn. Your comments are invited either for revision of this standard or for additional standards and should be addressed to ASTM Headquarters. Your comments will receive careful consideration at a meeting of the responsible technical committee, which you may attend. If you feel that your comments have not received a fair hearing you should make your views known to the ASTM Committee on Standards, 1916 Race St., Philadelphia, PA 19103.



Water Content (W_c)

The water content of a soil is determined by weighing a moist soil sample, drying it in an oven for approximately 24 hours, and reweighing the sample to determine the moisture loss. The water content is the ratio of the weight of water in the soil to the weight of the dry soil. Water content is typically expressed as a percentage.

Calibrated Hand Penetrometer (Q_p)

In the calibrated hand penetrometer test, the unconfined compressive strength of a soil is estimated to a maximum value of 4.5 tons per square foot (tsf) by measuring the resistance of the soil sample to penetration by a spring-calibrated plunger. The hand penetrometer test device has been carefully calibrated by its manufacturer with the results of numerous unconfined compressive strength tests. This test provides a quick, simple, and low-cost testing procedure from which soil strength can be estimated.

Unconfined Compression Test (Q_u)

In the unconfined compression strength test, an undisturbed cylinder of soil is loaded axially until the soil fails to carry additional load, or until 20% strain has been reached, whichever occurs first. The undrained shear strength of a cohesive soil is usually considered to equal half of the unconfined compressive strength.

Dry Density (γ_d)

The dry density of a soil is the weight of dry soil in a unit volume. The soil's total unit weight is typically calculated by weighing a cylinder of soil, and dividing the weight by the cylinder's volume as calculated by measuring the cylinder's height and diameter at several locations. The soil's dry density is then determined by correcting the cylinder's weight to account for its water content measured as described above. Use of this value is often made when estimating the degree of compaction of a soil.

Classification of Samples

Soil samples are classified on the basis of their texture and plasticity in accordance with the Unified Soil Classification System (USCS). The two-letter designator in parentheses following each soil description on the boring logs represents the applicable unified classification. If the designator is capitalized, the classification has been confirmed by the appropriate index testing. If the designator is lower-case, the classification has been visually estimated.







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APPENDIX C

Existing Drainage Map and Calculations



EXISTING	DRAINAGE AREA LIMITS	
EXISTING	Tc FLOW PATH	

EXISTING DRAINAGE AREA LABEL





Item 4.b.

Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
2.773	78	Grass (E1, E2)
0.182	98	Gravel (E1)
0.070	98	PIP Playground (E1)
0.580	98	Paved (E1)
0.167	78	Wood Chip Playgrounds (E1)
3.772	82	TOTAL AREA

Victor Haen - EX	MS
Prepared by Point of Beginning, Inc.	
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SE 24-hr 4 2-Year Rainfall=2.45" Printed 5/1/2025 Page 3

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SCS TR-20 method, UH=SCS, Weighted-Q Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentE1: Site	Runoff Area=157,398 sf 23.04% Impervious Runoff Depth=1.09" Flow Length=315' Tc=11.4 min CN=WQ Runoff=4.68 cfs 0.329 af
SubcatchmentE2: SE Edg	Runoff Area=6,928 sf 0.00% Impervious Runoff Depth=0.76" Flow Length=197' Slope=0.0076 '/' Tc=6.0 min CN=78 Runoff=0.18 cfs 0.010 af
Link 1L: to City Storm Sev	ver Inflow=4.81 cfs 0.339 af Primary=4.81 cfs 0.339 af

Total Runoff Area = 3.772 ac Runoff Volume = 0.339 af Average Runoff Depth = 1.08" 77.94% Pervious = 2.940 ac 22.06% Impervious = 0.832 ac

MSE 24-hr 4 2-Year Rainfall=2.45" Printed 5/1/2025 LC Page 4

Summary for Subcatchment E1: Site

Runoff = 4.68 cfs @ 12.20 hrs, Volume= Routed to Link 1L : to City Storm Sewer 0.329 af, Depth= 1.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 4 2-Year Rainfall=2.45"

	A	rea (sf)	CN	Description		
*		25,267	98	Paved		
*		7,939	98	Gravel		
*		7,265	78	Wood Chip	Playground	ds
*		3,051	98	PIP Playgro	ound	
*	1	13,876	78	Grass		
	1	57,398		Weighted A	verage	
	1	21,141		76.96% Pe	rvious Area	L
		36,257		23.04% Im	pervious Ar	ea
	Тс	Length	Slope	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft)) (ft/sec)	(cfs)	
	9.8	55	0.0084	0.09		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.45"
	1.6	260	0.0315	5 2.66		Shallow Concentrated Flow,
						Grassed Waterway Kv= 15.0 fps
	11.4	315	Total			

Summary for Subcatchment E2: SE Edge

Runoff = 0.18 cfs @ 12.14 hrs, Volume= 0.010 af, Depth= 0.76" Routed to Link 1L : to City Storm Sewer

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 4 2-Year Rainfall=2.45"

	Α	rea (sf)	CN	Description					
*		6,928	78	Grass					
		6,928		100.00% P	ervious Are	a			
	Tc (min)	Length (feet)	Slope (ft/ft)	e Velocity) (ft/sec)	Capacity (cfs)	Description			
_	2.5	197	0.0076	6 1.31		Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps			
	2.5	197	Total,	Total, Increased to minimum Tc = 6.0 min					

Summary for Link 1L: to City Storm Sewer

Inflow Are	ea =	3.772 ac, 2	22.06% Impe	ervious,	Inflow Dep	oth = 1.	08" for 2-Y	'ear event
Inflow	=	4.81 cfs @	12.19 hrs,	Volume	= C).339 af		
Primary	=	4.81 cfs @	12.19 hrs,	Volume	= C).339 af,	Atten= 0%,	Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

MSE 24-hr 4 10-Year Rainfall=3.51" Printed 5/1/2025 LLC ______ Page 1

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SCS TR-20 method, UH=SCS, Weighted-Q Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentE1: SiteRunoff Area=157,398 sf 23.04% Impervious Runoff Depth=1.91"
Flow Length=315' Tc=11.4 min CN=WQ Runoff=8.38 cfs 0.576 afSubcatchmentE2: SE Edge
Flow Length=197' Slope=0.0076 '/' Tc=6.0 min CN=78 Runoff=0.38 cfs 0.020 afLink 1L: to City Storm SewerInflow=8.64 cfs 0.596 af
Primary=8.64 cfs 0.596 af

Total Runoff Area = 3.772 ac Runoff Volume = 0.596 af Average Runoff Depth = 1.90" 77.94% Pervious = 2.940 ac 22.06% Impervious = 0.832 ac

Victor Haen - EX	MSE 24-hr 4	100-Year Rainfall=5.50"
Prepared by Point of Beginning, Inc.		Printed 5/1/2025
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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SCS TR-20 method, UH=SCS, Weighted-Q Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentE1: Site	Runoff Area=157,398 sf 23.04% Impervious Runoff Depth=3.63" Flow Length=315' Tc=11.4 min CN=WQ Runoff=15.96 cfs 1.093 af	
SubcatchmentE2: SE Ed	Runoff Area=6,928 sf 0.00% Impervious Runoff Depth=3.14" Now Length=197' Slope=0.0076 '/' Tc=6.0 min CN=78 Runoff=0.78 cfs 0.042 af	
Link 1L: to City Storm Sev	er Inflow=16.51 cfs 1.135 af Primary=16.51 cfs 1.135 af	

Total Runoff Area = 3.772 ac Runoff Volume = 1.135 af Average Runoff Depth = 3.61" 77.94% Pervious = 2.940 ac 22.06% Impervious = 0.832 ac

APPENDIX D

Proposed Drainage Map and Calculations



PROPOSED	DRAINAGE AREA LIMITS	
PROPOSED	Tc FLOW PATH	
PROPOSED	DRAINAGE AREA LABEL	
 PROPOSED	PONDING AREA LABEL	

0' 30' 60' 1" = 30'





Printed 5/1/2025 Page 2

Rainfall Events Listing (selected events)
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Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	2-Year	MSE 24-hr	4	Default	24.00	1	2.45	2

Item 4.b.

Area Listing (all nodes)

Area	CN	Description
 (acres)		(subcatchment-numbers)
0.734	98	Building (D3)
1.552	78	Grass (D1)
0.701	74	Grass (D2, D3)
0.154	98	PIP Playground (D1)
0.426	98	Paved (D1)
0.094	98	Sidewalk (D2, D3)
0.110	39	Synth Turf Courtyard w/ underdrain (D1)
3.772	84	TOTAL AREA

Victor Haen - PR	MSE 24-hr 4 2-Year Rainfall=2.45"
Prepared by Point of Beginning, Inc.	Printed 5/1/2025
HydroCAD® 10.10-6a s/n 05316 © 2020 HydroCAD Software Solutions	LLC Page 4
Time span=0.00-48.00 hrs, dt=0.05 hrs,	, 961 points
Runoff by SCS TR-20 method, UH=SCS,	Weighted-Q
Reach routing by Stor-Ind+Trans method - Pond ro	uting by Stor-Ind method
SubcatchmentD1: SiteRunoff Area=97,684 sfFlow Length=264'Tc=10.7	25.89% Impervious Runoff Depth=1.10" min CN=WQ Runoff=2.97 cfs 0.205 af
SubcatchmentD2: SE Sidewalk Runoff Area=6,928 sf	32.79% Impervious Runoff Depth=1.12"
Flow Length=197' Slope=0.0760 '/' Tc=6.0	min CN=WQ Runoff=0.25 cfs 0.015 af
SubcatchmentD3: Bldg Area Runoff Area=59,714 sf	56.63% Impervious Runoff Depth=1.51"
Tc=6.0	min CN=WQ Runoff=2.85 cfs 0.172 af
Pond 1P: Depression StoragePeak Elev=711.49' St6.0" Round Culvert n=0.013 L=18.0	orage=2,092 cf Inflow=2.85 cfs 0.172 af ' S=0.0050 '/' Outflow=0.68 cfs 0.172 af
Link 1L: to City Storm Sewer	Inflow=3.79 cfs 0.392 af Primary=3.79 cfs 0.392 af

Total Runoff Area = 3.772 acRunoff Volume = 0.392 afAverage Runoff Depth = 1.25"62.65% Pervious = 2.363 ac37.35% Impervious = 1.409 ac

Summary for Subcatchment D1: Site

Runoff = 2.97 cfs @ 12.19 hrs, Volume= Routed to Link 1L : to City Storm Sewer 0.205 af, Depth= 1.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 4 2-Year Rainfall=2.45"

	A	rea (sf)	CN	Description						
*		18,571	98	98 Paved						
*		6,718	98	PIP Playgro	ound					
*		4,790	39	Synth Turf	Courtyard v	v/ underdrain				
*		67,605	78	Grass	-					
		97,684		Weighted A	verage					
		72,395		74.11% Pe	rvious Area					
		25,289		25.89% Imp	pervious Ar	ea				
	Тс	Length	Slope	e Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
	9.3	40	0.0130	0.07		Sheet Flow,				
						Grass: Dense n= 0.240 P2= 2.45"				
	1.4	224	0.0310	2.64		Shallow Concentrated Flow,				
						Grassed Waterway Kv= 15.0 fps				
	10.7	264	Total							

Summary for Subcatchment D2: SE Sidewalk

Runoff = 0.25 cfs @ 12.13 hrs, Volume= Routed to Link 1L : to City Storm Sewer 0.015 af, Depth= 1.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 4 2-Year Rainfall=2.45"

	Are	ea (sf)	CN	Description	l		
*		2,272	98	3 Sidewalk			
*		4,656	74	Grass			
		6,928	3 Weighted Average				
	4,656 67.21% Pervious Area						
		2,272		ea			
	Тс	Length	Slope	e Velocity	Capacity	Description	
<u>(m</u>	in)	(feet)	(ft/ft) (ft/sec)	(cfs)		
(0.8	197	0.0760) 4.14		Shallow Concentrated Flow,	
						Grassed Waterway Kv= 15.0 fps	
(0.8	197	Total,	tal, Increased to minimum Tc = 6.0 min			
Summary for Subcatchment D3: Bldg Area

Runoff = 2.85 cfs @ 12.13 hrs, Volume= Routed to Pond 1P : Depression Storage 0.172 af, Depth= 1.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 4 2-Year Rainfall=2.45"

	Area (sf)	CN	Description			
*	31,979	98	Building			
*	1,835	98	Sidewalk			
*	25,900	74	Grass			
	59,714		Weighted A	verage		
	25,900		43.37% Pe	rvious Area	a	
	33,814		56.63% Imp	pervious Ar	rea	
۲ mi)	Гс Length n) (feet)	Slop (ft/f	e Velocity t) (ft/sec)	Capacity (cfs)	Description	
6	.0				Direct Entry,	

Summary for Pond 1P: Depression Storage

Inflow Area	a =	1.371 ac, 5	6.63% Impe	ervious, Inflow I	Depth = 1	.51" fo	r 2-Ye	ar event
Inflow	=	2.85 cfs @	12.13 hrs,	Volume=	0.172 a	f		
Outflow	=	0.68 cfs @	12.40 hrs,	Volume=	0.172 at	f, Atten=	76%,	Lag= 16.0 min
Primary	=	0.68 cfs @	12.40 hrs,	Volume=	0.172 a	f		-
Routed	to Link 2	1L : to City S	torm Sewer					

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 711.49' @ 12.40 hrs Surf.Area= 5,161 sf Storage= 2,092 cf

Plug-Flow detention time= 24.6 min calculated for 0.172 af (100% of inflow) Center-of-Mass det. time= 24.5 min (800.4 - 775.9)

Volume	Inv	ert Avai	I.Storage	Storage Descript	ion		
#1	710.5	50'	17,258 cf	Custom Stage D	ata (Irregular) Lis	ted below (Recalc))
Elevatio (fee	on et)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
710.5 711.0 712.0 713.0	50 00 00 00	1 2,147 9,747 13,142	1.0 181.0 452.0 525.0	0 366 5,490 11,402	0 366 5,855 17,258	1 2,608 16,263 21,960	
Device	Routing	In	vert Outle	et Devices			
#1	#1 Primary 710.50' 6.0" Round Culvert L= 18.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 710.50' / 710.41' S= 0.0050 '/' Cc= 0.900 n= 0.013, Flow Area= 0.20 sf).900

Primary OutFlow Max=0.68 cfs @ 12.40 hrs HW=711.49' (Free Discharge) ☐ 1=Culvert (Barrel Controls 0.68 cfs @ 3.45 fps)

Summary for Link 1L: to City Storm Sewer

Inflow Area	a =	3.772 ac, 3	37.35% Impe	ervious,	Inflow De	epth = 1	1.25"	for 2-Y	ear even	t
Inflow	=	3.79 cfs @	12.19 hrs,	Volume	=	0.392 a	f			
Primary	=	3.79 cfs @	12.19 hrs,	Volume	=	0.392 a	f, Att	en= 0%,	Lag= 0.0) min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Victor Haen - PR	MSE 24-hr 4 10-Year Rainfall=3.51"
Prepared by Point of Beginning, Inc.	Printed 5/1/2025
<u>HydroCAD® 10.10-6a s/n 05316 © 2020 HydroCA</u>	D Software Solutions LLC Page 1
Time span=0.00-48	.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-2) method, UH=SCS, Weighted-Q
Reach routing by Stor-Ind+Trans	s method - Pond routing by Stor-Ind method
SubcatchmentD1: Site River Flow L	noff Area=97,684 sf 25.89% Impervious Runoff Depth=1.89" ength=264' Tc=10.7 min CN=WQ Runoff=5.22 cfs 0.353 af
SubcatchmentD2: SE Sidewalk F	Runoff Area=6,928 sf 32.79% Impervious Runoff Depth=1.91"
Flow Length=197' Slop	be=0.0760 '/' Tc=6.0 min CN=WQ Runoff=0.44 cfs 0.025 af
SubcatchmentD3: Bldg Area Ro	noff Area=59,714 sf 56.63% Impervious Runoff Depth=2.40" Tc=6.0 min CN=WQ Runoff=4.56 cfs 0.274 af
Pond 1P: Depression Storage	Peak Elev=711.77' Storage=3,869 cf Inflow=4.56 cfs 0.274 af
6.0" Round Cu	vert n=0.013 L=18.0' S=0.0050 '/' Outflow=0.83 cfs 0.274 af
Link 1L: to City Storm Sewer	Inflow=6.31 cfs 0.652 af Primary=6.31 cfs 0.652 af
Total Runoff Area = 3.772 ac	Runoff Volume = 0.652 af Average Runoff Depth = 2.08
62	65% Pervious = 2.363 ac 37.35% Impervious = 1.409 ac

Item 4.b.

Victor Haen - PR	MSE 24-hr 4 100-Year Rainfall=5.50"
Prepared by Point of Beginning, Inc.	Printed 5/1/2025
HydroCAD® 10.10-6a s/n 05316 © 2020 HydroCAD Software Solution	ions LLC Page 2
Time span=0.00-48.00 hrs, dt=0.05	hrs, 961 points
Runoff by SCS TR-20 method, UH=S	SCS, Weighted-Q
Reach routing by Stor-Ind+Trans method - Ponc	d routing by Stor-Ind method
SubcatchmentD1: SiteRunoff Area=97,684Flow Length=264'Tc=1	4 sf 25.89% Impervious Runoff Depth=3.55" 10.7 min CN=WQ Runoff=9.80 cfs 0.664 af
SubcatchmentD2: SE Sidewalk Runoff Area=6,928	8 sf 32.79% Impervious Runoff Depth=3.59"
Flow Length=197' Slope=0.0760 '/' Tc=	=6.0 min CN=WQ Runoff=0.82 cfs 0.048 af
SubcatchmentD3: Bldg Area Runoff Area=59,714	4 sf 56.63% Impervious Runoff Depth=4.18"
Tc=	=6.0 min CN=WQ Runoff=7.95 cfs 0.478 af
Pond 1P: Depression StoragePeak Elev=712.216.0" Round Culvertn=0.013L=1	1' Storage=7,978 cf Inflow=7.95 cfs 0.478 af 18.0' S=0.0050 '/' Outflow=1.01 cfs 0.478 af
Link 1L: to City Storm Sewer	Inflow=11.36 cfs 1.189 af Primary=11.36 cfs 1.189 af

Total Runoff Area = 3.772 acRunoff Volume = 1.189 afAverage Runoff Depth = 3.78"62.65% Pervious = 2.363 ac37.35% Impervious = 1.409 ac

APPENDIX E

Proposed Erosion Control Plan



GENERAL NOTES:

- CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
 NOTIFY THE LOCAL MUNICIPALITY AT LEAST 2 WORKING DAYS PRIOR TO THE START OF SOIL
- DISTURBING ACTIVITIES. 3. INSTALL ALL TEMPORARY EROSION CONTROL ELEMENTS PRIOR TO THE START OF
- DEMOLITION/CONSTRUCTION. 4. ALL ACTIVITIES SHALL BE CONDUCTED IN A LOGICAL SEQUENCE TO MINIMIZE THE AMOUNT OF BARE SOIL EXPOSED AT ANY ONE TIME. MAINTAIN EXISTING VEGETATION AS LONG AS POSSIBLE.
- 5. CRUSHED ROCK DRIVES FOR SEDIMENT TRACKING UTILIZING 3" CRUSHED ROCK SHALL BE MAINTAINED AT ALL CONSTRUCTION ENTRANCES TO THE SITE. THE ROCK DRIVE SHALL BE A MINIMUM OF 12" THICK AND BE A MINIMUM OF 50 FEET IN LENGTH BY THE WIDTH OF THE DRIVEWAY.
- 6. OFFSITE SEDIMENT DEPOSITS RESULTING FROM STORMWATER RUNOFF SHALL BE CLEANED BY THE END OF THE NEXT WORKDAY. OFFSITE SEDIMENT DEPOSITS RESULTING FROM CONTRACTOR ACTIVITIES, INCLUDING SOIL TRACKING, SHALL BE CLEANED EACH WORKDAY. EXCESSIVE AMOUNTS OF SEDIMENT OR DEBRIS TRACKED ONTO ADJACENT STREETS SHALL BE CLEANED IMMEDIATELY. FINE SEDIMENT ACCUMULATIONS ON ADJACENT STREETS SHALL SWEPT MECHANICALLY OR MANUALLY AT LEAST WEEKLY AND BEFORE IMMINENT RAINFALL.
- 7. DISTURBED GROUND OUTSIDE OF THE EVERYDAY CONSTRUCTION AREAS, INCLUDING SOIL STOCKPILES, THAT ARE LEFT INACTIVE FOR MORE THAN 7 DAYS SHALL BE TEMPORARILY STABILIZED BY SEEDING/MULCHING OR OTHER APPROVED METHODS.
- WASTE MATERIAL THAT IS GENERATED ON THE CONSTRUCTION SITE SHALL BE PROPERLY DISPOSED OF AND NOT ALLOWED TO RUN INTO RECEIVING WATERS.
 EROSION CONTROL DEVICES DESTROYED AS A RESULT OF CONSTRUCTION ACTIVITIES SHALL BE REPAIRED BY THE END OF EACH WORK DAY.
- 10. INSPECT ALL EROSION CONTROL MEASURES AT LEAST ONCE A WEEK AND AFTER ANY RAINFALL OF 0.5" OR MORE. MAKE NEEDED REPAIRS AND DOCUMENT ALL ACTIVITIES AS PER THE REQUIREMENTS OF THE NOTICE OF INTENT SUBMITTED BY THE PROJECT CIVIL ENGINEER.
- 11. ALL TEMPORARY EROSION CONTROL ELEMENTS SHALL REMAIN IN PLACE UNTIL A SUFFICIENT GROWTH OF VEGETATION IS ESTABLISHED AND THEN BE REMOVED AS PART OF THE BASE BID.
- IF SEDIMENT LADEN WATER NEEDS TO BE REMOVED FROM THE SITE, FILTER BAGS OR SCREENING SHALL BE USED IN ACCORDANCE WITH WI DNR TECHNICAL STANDARD 1061 TO PREVENT SEDIMENT DISCHARGE TO THE MAXIMUM EXTENT PRACTICABLE.
 COORDINATE ALL EARTHWORK ACTIVITIES WITH THE RESPECTIVE TRADES RESPONSIBLE FOR
- THE INSTALLATION OF GAS, CABLE, TELEPHONE AND ELECTRICAL (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE).
- 14. IF BARE SOIL IS EXPOSED DURING THE WINTER MONTHS, STABILIZATION BY MULCHING OR ANIONIC POLYACRYLAMIDE SHALL OCCUR PRIOR TO SNOWFALL OR GROUND FREEZE.
 15. SILT FENCE SHALL BE INSTALLED AROUND THE TOPSOIL STOCKPILE.
- 16. THE CONTRACTOR SHALL PERFORM INSPECTIONS AND MONITORING OF EROSION CONTROL PRACTICES IN ACCORDANCE WITH THE WI DNR "CONSTRUCTION SITE INSPECTION REPORT" FORM 3400-187. THIS FORM CAN BE FOUND IN THE CONSTRUCTION SPECIFICATIONS.

EROSION CONTROL LEGEND:

EXISTING CONTOUR	<u> </u>	_
PROPOSED CONTOUR	888	_
PROPOSED SILT FENCE	<u> </u>	
PROPOSED INLET PROTECTION	\diamond	-
EROSION CONTROL BLANKET		-
ROCK CONSTRUCTION ENTRANCE		
TEMPORARY SLOPE INTERRUPTION	· · · · · · · · · · · · · · · · · · ·	-
CONCRETE WASHOUT AREA (INSTALL PER EPA STANDARDS)		-
STORM WATER OVERLAND FLOW DIRECTION		

EROSION CONTROL SEQUENCING:

- INSTALL PERIMETER EROSION CONTROL
 BEGIN DEMOLITION
- BEGIN ROUGH GRADING AND UTILITY INSTALLATION
 DURING GRADING ACTIVITIES EXISTING GRASS AND VEGETATION, TO BE REMOVED, SHALL REMAIN IN PLACE FOR AS LONG AS POSSIBLE, TO AVOID SEDIMENT TRANSPORT.
- 5. TEMPORARY STABILIZATION ACTIVITY SHALL COMMENCE WHEN LAND DISTURBING CONSTRUCTION ACTIVITIES HAVE TEMPORARILY CEASED AND WILL NOT RESUME FOR A PERIOD EXCEEDING 14 CALENDAR DAYS.
- 6. FINAL STABILIZATION ACTIVITY SHALL COMMENCE WHEN LAND DISTURBING ACTIVITIES CEASE AND FINAL GRADE HAS BEEN REACHED ON ANY PORTION OF THE SITE.
- 7. IF DISTURBED AREAS MUST BE LEFT OVER WINTER, AN ANIONIC POLYACRYLAMIDE SHALL BE APPLIED TO ALL DISTURBED AREAS PRIOR TO GROUND FREEZE. SEE SPECIFICATIONS FOR DETAILS.





114

APPENDIX F

State of Wisconsin Construction Site Inspection Report, Post Construction Long-Term Storm Water Management Checklist, And Notice of DNR Notice of Termination State of Wisconsin Department of Natural Resources (DNR) PO Box 7921, Madison WI 53707-7921 dnr.wi.gov

Form 3400-187 (R 02/2025)

Page 1 of 2

Notice: This form was developed in accordance with s. NR 216.48 Wis. Adm. Code for WPDES permittees' convenience; however, use of this specific form is voluntary. Multiple copies of this form may be made to compile the inspection report. Inspections of the construction site and implemented erosion and sediment control best management practices (BMPs) must be performed weekly and within 24 hours after a rainfall event 0.5 inches or greater.

Construction Site Name and Location (Project, Municip	Site/Facility ID No. (FIN):	Site/Facility ID No. (FIN):			
Onsite Contact/Contractor:				Onsite Phone/Cell:	
Note: In accordance with s. NR 216.48 (4), Wis. Adm. C on site and made available upon request. Repair notification that repair or replacement is needed	ode, in or repl PLEAS	spection reports, along with e ace erosion and sediment con SE PRINT LEGIBLY.	rosion control and s trol best manageme	torm water management plans, are req ent practices within 24 hours of an insp	uired to be maintained ection or department
Date of inspection: Time Start End:	of insp	ection: () am () pm () am () pm	Type of inspection	: O Weekly O Precipitation Event	○ Other (specify)
Weather/Site Conditions: Ory Temp. °F Antecedent Variable Soil Moisture Wet O	Frozen Frozen Melting	n or snow covered (Thaw predicted in next week) g Snow/slush	 Describe current phase of construction: (k) Scheduled Final Stabilization Date for Universal Soil Loss Equation (USLE) ¹: 		
Last Rainfall Date: Incres Last Rainfall Date: Name(s) of individual(s) performing inspection:			Project on Schedu Inspector Phone/C	ule ² ? () Yes () No Cell:	
I certify that the information contained on this form	s an ac	ccurate assessment of site co	nditions at the time	of inspection: Date:	
Inspection Questions:	Yes	No (Identify Actions Re	quired):	Location/Comments:	Actions Completed by Date & Initials
1. Is the erosion control plan accessible to operators?		Provide onsite copy			
2 Is the permit certificate posted where visible?		Post certificate			
 Is the permit certaincate posted where visible. Is the current phase of construction on sequence with the site-specific erosion and sediment control plan, including installation/stabilization of ponds and ditches? 		Add sediment control Install missing ditch/pipe/pond Stabilize bare soil			
4. Are all erosion and sediment control BMPs shown on plan properly installed and in functional condition?		Repair Modify Install/Replace			
 Is inlet protection properly installed and functioning in all inlets likely to receive runoff from the site? 		☐ Clean ☐ Replace ☐ Install			
6. Is the air free of fugitive dust resulting from construction activity and bare soil exposure?		Apply water Apply dust control product			

¹ The Universal Soil Loss Equation (USLE) model and the Construction Site Soil Loss and Sediment Discharge Guidance are available at: http://dnr.wi.gov/topic/stormwater/standards/const_standards.html

² If the project is not an echedule than the soil loss summary for the project should be reviewed, and schedule, plan or practices modified accordingly

State of Wisconsin Department of Natural Resources dnr.wi.gov

'final stabilization'?

CONSTRUCTION SITE INSPECTION REPO Page 2 of 2

Form 3400-187 (R 02/2025)

Item 4.b.

Inspection Questions:	Yes	No (Identify Actions Required):	Location/Comments:	Actions Completed by Date & Initials
7 Is the public right of way curb line free of tracked soil and accumulation?		Install tracking pad Viden/lengthen pad Amend stone/Add geotextile Install wheel washing station Close entrance/exit Limit traffic across disturbed areas Sweep road and curb line		
 Are wetlands, lakes, streams, ditches, or storm sewer downstream of the site free of sedimentation and turbid water leaving the site?³ 	ers	Add sediment controls Modify operations Contact DNR to verify extent of cleanup required		
9. Is dewatering and/or vehicle and equipment washing being done in a manner that prevents erosion and sediment discharge?	•	Install treatment train Install energy dissipation Modify discharge location Modify intake to reduce sediment		
10. Are soil stockpiles existing for more than 7 days covered and stabilized?		Seed Install mat/mulch/polymer Cover with tarp/plastic sheeting		
11. Are downstream channels and other downhill areas protected from scour and erosion?		Install energy dissipation at outrail Install ditch checks Install slope interruption Install onsite detention		
 Are good housekeeping practices or treatment cont in place to prevent the discharge of chemicals, cement, trash, and other materials into wetlands, waterways, storm sewers, ditches, or drainage-way 	s? ⁴	 Properly dispose of trash Provide concrete washout station Contact DNR to verify extent of cleanup required 		
13. Is the plan reflective of current site operations and does it address all erosion and sediment control issues identified during the inspection?		Revise sequence Revise sediment control BMP Revise erosion control BMP Revise post-construction storm water BM	ЛР	
14. Are all areas where construction has temporarily ceased (and will not resume for more than 2 weeks temporarily stabilized?	;)	Topsoil & seed Install mat/mulch/polymer Cover with tarp/plastic sheeting		
15. Are all areas at final grade permanently vegetate or stabilized with other treatments?	ed 🗌	Topsoil & seed Install mat/mulch/polymer Sod Install stone base		
16. Have temporary sediment controls been remove areas of the site that meet the permit definition of 'final stabilization'?	əd in 🔲 əf	 Water to establish vegetation Repair or reseed areas Remove temporary practices 		

3 If sediment discharge enters a wetland or waterbody, the permittee should consult with DNR staff to determine if sediment cleanup and/or additional control measures are required.

⁴ The permittee shall notify the DNR immediately via the spills hotline at (800)943-0003 of any release or spill of a hazardous substance to the environment in accordance with s. 292.11, Wis. Stats., and ch. NR 706, Wis. Adm. Code. 117

Notice of Termination – Storm Water Discharges Assod With Land Disturbing Construction Activities General Permit

Form 3400-162 (R 12/14)

Page 1 of 2

Item 4.b.

This Notice of Termination (NOT) form is authorized by s. 283.37, Wis. Stats. Submittal of a completed NOT to the Department is mandatory for any landowner of a construction site regulated under 40 CFR Part 122, Chapter 283, Wis. Stats., and Chapter NR 216, Wis. Adm. Code. Failure to submit a completed NOT to the Department after the construction site undergoes final stabilization may result in forfeitures up to \$10,000 per day, pursuant to s. 283.92 (2), Wis. Stats. Personally identifiable information on this NOT may be used for other water quality program purposes.

Submission of this NOT constitutes notice that the landowner identified in Section I, no longer intends to be authorized by a general WPDES permit to discharge storm water associated with land disturbing construction activities from the construction site identified in Section III of this NOT.

All necessary information must be provided on this NOT. Failure to complete this NOT correctly may result in rejection of this NOT by the Department. Please read all instructions before completing. Please type or clearly print your answer to <u>all</u> questions

Section I: Landowner Information				
Business Name	Authorized Representative			
Mailing Address	City		State	ZIP Code
			WI	
E-mail	Phone Number (area code)	Alternate P	hone Nu	mber
Section II: Contractor Information				
Business Name	Contact Person			
Mailing Address	City		State	ZIP Code
			WI	
E-mail	Phone Number (area code) Alternate Phone Nun		mber	

Section III: Facility/Site Location Information

Site Name

Location Address/Description	WDNR Site Number					
City O Township O Village	9				County	
PLSS Information	Township	Range		Section	Quarter	Quarter-Quarter
	N		West			
Attach photos of the current site conditions				Date photos wer	e taken	~

Section IV: Certification

I certify under penalty of law that disturbed soils at the identified site have undergone final stabilization and temporary erosion and sediment control measures have been removed or that all storm water discharges associated with construction activity that are authorized by a general WPDES storm water discharge permit have otherwise been eliminated. I understand that by submitting this Notice of Termination, I am no longer authorized to discharge storm water associated with construction activity by the general WPDES permit, and that discharging pollutants in storm water associated with construction activity to waters of Wisconsin is unlawful where the discharge is not authorized by a WPDES permit.

NOTE: The person signing below must be a representative of the landowner as defined in s. NR 216.55 (4) Wis. Adm. Code. "Landowner" for purposes of this NOT is defined in s. NR 216.002 (13), Wis. Adm. Code. Failure to have this NOT properly signed will result in its rejection.

Signature of Landowner/Authorized Representative	Date Signed	
Printed Name of Landowner/Authorized Representative	Title	

Mail this completed NOT form to the appropriate Wisconsin Department of Natural Resources office in the region where the facility is located. See the instructions on page 2 of this form for regional office addresses.

Form 3400-162 (R 12/14)

Page 2 of 2

Instructions

Section I: Landowner Information

Provide the legal name of the person, firm, public organization, or any other entity that owns the construction site described in Section III of this application and holds or qualifies for an applicable general or individual constructions site storm water discharge permit. The mailing address and phone number given should be for the authorized representative.

Section II: Contractor Information

Provide the legal name of the person, firm, or any other entity that acted as the major contractor in charge or operating the construction site described in Section III of this application. The mailing address and phone number given should be for the contact person.

Section III: Construction Site Information

Enter the construction site's official or legal name and complete address, including county, city, state and zip code. Be sure to include the quarter-quarter, guarter, section, township and range (the nearest quarter section) of the site. If the site is on more than one quarter, enter the quarter that best describes the location of the site. Use additional space if needed to describe the site location. The WDNR Site Number can be found in the upper right corner of the original letter conferring coverage under the general permit from the WDNR.

Required: Attach photos of the current site conditions and provide the date the photos were taken.

Section IV: Certification

State Statutes provide for severe penalties for submitting false information on this NOT form. State regulations require this NOT to be signed as follows:

- 1. For a corporation, by a responsible corporate officer including president, secretary, treasurer, vice president, manager, or
 - a duly authorized representative having overall responsibility for the operation covered by this permit. For a unit of government, by a ranking elected official or other duly authorized representative.
- For a unit of government, by a ranking elected official or other duly authorized represent
 For a partnership, by a general partner; and for a sole proprietorship, by the proprietor.
- 4. For a limited liability company, by a manager.

Sign the form and print the name of the individual signing the NOT and date of signature. If the form was prepared by a consultant or someone other than an employee of the site landowner, provide the name and address of the preparer.

If you need additional information about the NOT for construction activities, please contact the Department at (608) 267-7694.

Mailing Address

Unless otherwise directed, mail this completed NOT Form to the WDNR office associated with the county of the site location:

		NORTHER	N REGION (NOR)	
Ashland Barron Bayfield Burnett	Douglas Florence Forest Iron	Langlade Lincoln Oneida Polk Price	Rusk Sawyer Taylor Vilas Washburn	WDNR Baldwin Service Center 890 Spruce Street Baldwin, WI 54002 715-684-2914 ext. 109
		NORTHEA	ST REGION (NER)	
Brown Calumet Door Fond du Lac	Green Lake Kewaunee Manitowoc Marinette	Marquette Menominee Oconto Oneida Reservation	Outagamie Shawano Waupaca Waushara Winnebago	WDNR Northeast Regional Headquarters 2984 Shawano Avenue Green Bay, WI 54313-6727 920-662-5100
		WEST CENT	RAL REGION (WCR)	
Adams Buffalo Chippewa Clark	Crawford Dunn Eau Claire Jackson Juneau	La Crosse Marathon Monroe Pepin Pierce	Portage St. Croix Trempealeau Vernon Wood	WDNR Baldwin Service Center 890 Spruce Street Baldwin, WI 54002 715-684-2914 ext. 109
		SOUTH CEN	TRAL REGION (SCR)	
Columbia Dane Dodge	Grant Green Iowa	Jefferson LaFayette Richland	Rock Sauk	WDNR South Central Regional Headquarters 3911 Fish Hatchery Road Fitchburg, WI 53711 608-275-3266
		SOUTHEA	ST REGION (SER)	
Kenosha Milwaukee	Ozaukee Racine	Sheboygan Walworth	Washington Waukesha	WDNR Waukesha Service Center 141 N.W. Barstow Street, Room 180 Waukesha, WI 53188 262-574-2100

Storm Water Management Practices Post Construction Long-Term Storm Water Management Checklist

Site Name:	Victor Haen Elementary – Building Addition & Site Improvements
Location:	Kaukauna, Outagamie County, Wisconsin
Responsible Party:	The Owner is of the property responsible for the post construction long-term storm water management upkeep. This checklist may be utilized when performing inspections after any rainfall event exceeding one inch of rainfall, and at a minimum semi-annually in early spring and fall.
Date of Inspection:	(mm/dd/yy)
Time of Inspection:	(start/end)
Type of Inspection:	(annual/quarterly/precipitation event)
Weather:	
Inspector's Name:	
<u>Component Inspected:</u>	Repairs Required: Comments:
General Site Area	
-Sediment Depos	its
-Trash Accumula	tion
-Plant Life	
-Surface Erosion	
-Mulch	
Site Vegetation	
Area Storm Sewer Pipe	
Grass and Plants througho	ut Site
-Bare Spots	
-Dead Plant Mate	rial
-Washouts	



EROSION CONTROL AND STORMWATER MANAGEMENT PERMIT APPLICATION

Applicant Information									
Applicant Name (Indiv., Org. or Entity) Kaukauna Area School Di	istrict Cł	uthorized Repres	sentative		Title Dir. of Operations & Finance				
Mailing Address 101 Oak St	C	^{City} Kaukauna			State WI Postal Code 5413				
E-mail Address mcdanielc@kaukaunasd.c	org T	elephone (includ 20-759-6104	e area code)		Fax (includ	e area code)			
Landowner Information (if different than Applicant)									
Name (Organization or Entity)	С	ontact Person			Title				
Mailing Address	С	ity			State	Postal Code			
E-mail Address	Т	elephone (includ	e area code)		Fax (includ	e area code)			
Other Contact Information (check one):	er / Con	sultant 🗌 Co	ntractor / Bui	Ider] Agent / Ot	her			
Name (Organization or Entity) Point of Beginning	g c	ontact Person G	ieno Ca	rlson	Telephone 715-344-999	(include area code) 39			
Mailing Address 4941 Kirschling Ct	С	^{ity} Stevens	Point		State WI	Postal Code 54481			
Project or Site Location									
^{Site Name (Project):} Victor Haen bldg Additio	on		Parcel Nur	^{nbers:} 32	210586	00			
Address / Location: 1130 Haen Dr., Kaukaur	na, V	VI, 54130	Plat / CSM	/ Lot No.	[:] Lt 3, Bk	51, Assessor's Plat			
Permit Type & Fees (check all that apply)									
Erosion Control < 1 acre or 43,560 sq.ft. Disturbed Are Erosion Control ≥ 1 acre or 43,560 sq.ft. Disturbed Are	ea (EC1 ea (EC2) Storm	water Manag water Manag	gement < gement ≥	20,000 sq.fi 20,000 sq.fi	t. Impervious Area (SM1) t. Impervious Area (SM2)			
Total Disturbed Area ^{3.772 acres} 164309		· · · · · · · · · · · · · · · · · · ·	saft x S	\$0 0002 /	saft (EC2)	= \$ 32.86			
New Impervious Area		sa.ft. x \$0.0025 / sa.ft. (SM2) = \$ 85.79							
Base	e Fee: \$	\$200 (EC1), \$250	0 (EC2), \$20	0 (SM1).	\$500 (SM2)	= \$ 750			
		<i>(µ i i i i i i i i i i</i>	Т	otal Appli	ication Fee	= \$ 868.65			
Duration of Land Disturbance ^{4.5 mos} 20			weeks x \$2	5 / week	(EC1, EC2)	= \$ 500			
Start Date 08/18/25		Ba	se Fee: \$25	0 (EC2).	\$500 (SM2)	= \$ 750			
End Date 12/30/25			Т	otal Insp	ection Fee	= \$ 1,250			
	DEDMIT	EEE (Applicati	on Eag + Ing	naction	$F_{\alpha\alpha} = \Phi$				
Certification & Permission			on ree + ins	pection	ree) = \$				
Certification: I hereby certify that I am the landowner of the property which is the subject of this Permit Application. I certify that the information contained in this form and attachments is true and accurate. I understand that failure to comply with any or all of the provisions of the ordinances and/or permit may result in notices, fines / forfeitures, stop work orders, permit revocation, and cease & desist orders. Permission: As landowner of the property, I hereby give the Director of Public Works or designee, permission to enter and inspect the property to evaluate this permit application, determine compliance with ordinances, and perform corrective actions after issuing proper notice to the landowner. Applicant Signature									
Landowner Signature (required)				Date Si	U5/U1/2U25				
Crown			Providence in the second second		05/01/2025				
LEAVE BLA	NK – F		L USE ON	LY					
Date Application Received:		Fee Received	\$		Receipt No:				
Construction Site ID / Permit No:		Date Issued:			Issued By:				

PLEASE CONTACT CITY OF KAUKAUNA ENGINEERING DEPARTMENT AT 920-766-6305 WITH QUESTIONS.

Storm Sewer Design Calculations - WI DSPS Area Method

PROJECT	Victor Haen Elementary - Site Improvements
LOCATION	Kaukauna

CALCULATIONS BY	GMC	
DATE	5/1/2025	
CHECKED BY	GMC	
DATE	5/1/2025	

Point of Beginning

Struc	tures		Areas (SF))		Des	ign Flo	w Rate (GPM)		Pipe Se	lections	Pipe Flow Characteristics					
from	to	Grass	Paved / Gravel	Roof	Grass	Paved / Gravel	Roof	Pipe Flow	Total	size or diameter, inches	Slope, %	Capacity, Qfull, gpm	VelocityFull, Vfull, fps	Q/Qfull	approx. depth of flow, in	V/Vfull	V, actual, fps
ST #1	Ex 10"	0	782	0	0	24	0	0	24	4	7.50	252	6.43	0.10	0.82	0.64	4.12
ST #2	Ex 15"	14436	6672	0	139	205	0	0	344	8	1.04	597	3.81	0.58	3.53	0.95	3.61
ST #3	ST #5									6	4.62	Ex. Bldg Flow Unknown					
ST #4	EW #1				0	0	0	0	1184	12	0.26	Flow from Bldg Plumbing Designe				signer	
6" INLET	ST #5				0	0	0	0	373	6	0.50		Engin	eered	for Rate	e Conti	rol



MEMO

PLANNING AND COMMUNITY DEVELOPMENT

To:	Industrial Park Commission
From:	David Kittel Director of Planning and Community Development
Date:	5/14/2025
Re:	Site Plan Review – 1801 Progress Way (G & G Machine)

G & G Machine is preparing to build a 8,455 square foot addition on to the existing building at 1801 Progress Way. The addition will match the existing façade and blend in to the building. The facility has ample parking currently and additional parking stalls will not be needed with the addition.

The site plan has been reviewed by the Industrial Pack Commission (IPC). The IPC is recommending approval of the site plan with the condition that prior to issuance of building permits, they must obtain Stormwater and Erosion Control permits from the Engineering Department.

Site Plan Review:

Site/Architectural: 17.32 (10) Supplementary District Regulations & applicable zoning

All set back requirements are met and all applicable ordinances are being complied with to include zoning requirements and .

Landscape: 17.52 Landscaping Requirements

There are multiple trees and shrubs located on the property. There are no plans to remove these during the construction process. Landscaping requirements are being met.

Lighting:

This addition is not adding any lighting.

Stormwater: 22 Stormwater Management

The developer will work with Engineering Department to complete Erosion Control and Stormwater Management permitting.

Ingress/Egress: No Concerns noted

Public Safety:

No concerns noted at this time.

Façade: 17.53 Façade Standards

The proposed site elevations meet façade requirements and will match the existing façade on the building.

Staff Recommendation:

Staff recommend approval of the development with the following conditions:

• Prior to issuance of building permits, must obtain Stormwater and Erosion Control permits from the Engineering Department





SITE PLAN REVIEW APPLICATION

PROPERTY OWNER	APPLICANT (IF DIFFERENT PARTY THAN OWNER)								
Name: Mark Stumpf	Name: Jordan Nolle								
Mailing Address:	Mailing Address:								
1801 Progress Way Kankann, WI	1400 Lombardi Ave #101s, Green Bay, WI 54304								
Phone: 920 766 9788	Phone: 920-883-1161								
Email: mark @ gg machinewi, com	Email: jnolle@ruekert-mielke.com								
PROPERTY INFORMATION									
Described the Proposed Project in Detail: An 8,455 SF building expansion with a small amount of concrete and asphalt replacement and a redirected storm sewer system.									
Property Parcel (#): 322098900									
Site Address/Location: 2801 Progress Way Kaukauna, WI	54130								
Current Zoning and Use: IND - Manufacturing and office space	e								
Proposed Zoning and Use: IND - Manufacturing and office space	e								
Existing Gross Floor Area of Building: 33,885	Proposed Gross Floor Area of Building: 42,340								
Existing Building Height: 32	Proposed Building Height: 28								
Existing Number of Off-Street Parking Spaces: Proposed Number of Off-Street Parking Spaces 55									
55	Existing Impervious Surface Coverage Percentage: Proposed Impervious Surface Coverage Percentage 26.4								

Owner/Agent Signature: ______

Owner/Agent Name (printed): Grant Frazz

920.766.6300 www.cityofkaukauna.com

SITE PLAN REVIEW PROCEDURE

The Plan Review process is required for all new commercial, industrial or multifamily buildings, and building expansions/additions or structures.

Early in the process, consult the Site Application Checklist (below) for a complete list of plan requirements and contact staff in the Planning and Community Development Department for initial direction and assistance. In addition, it is your responsibility to notify utility companies regarding your proposed development.

Completed Site Plans must be submitted 14 business days prior to the intended Plan Commission meeting. Those plans will be distributed amongst various City departments for an initial review. After review, questions, comments, and requested revisions will be returned to the applicant in advance of the Plan Commission meeting.

SITE PLAN CHECKLIST

- ✓ Completed Site Plan application
- Completed Erosion Control and Stormwater Management Permit application and necessary fees
- ✓ Calculations for sanitary sewer and water
- ✓ Calculations for storm sewer design
- ✓ Site Plan set to include:
 - Site Plan layout and streets, including designated fire lanes
 - Utilities, grading, and drainage plan
 - o Erosion control plan
 - o Landscape and lighting plan
 - Architectural elevation and construction details
 - Floor plan set
 - Any other plans or information deemed necessary by the Planning and Community Development

SITE PLAN SUBMISSION

- 1. Email to Lily Paul lpaul@kaukauna-wi.org
- 2. In-person drop off City of Kaukauna, Attn: Lily Paul, 144 W. 2nd Street, Kaukauna, WI 54130

Scheduling Plumbing Plan Review and Checklist for General Plumbing Plan Review

Summary Sheet

Sec	Section 1. GENERAL PLAN REVIEW APPLICATION INFORMATION													
Ele	Electronic plumbing plan reviews shall be submitted online at: https://esla.wi.gov/PortalCommunityLogin. Paper plan submittals are no longer													
acc A tr	eptec	a by the Department. A complete set of plans all aid for the application can be accessed at: htt	tos://d	isps.wi.gov/Documents/HowToSubmitforaP	lumbi	ngPlanReview.pdf								
1	Rui	Iding or Project Name: G&G Kaukauna	districted											
2														
3	Tvo	e of Project: Check all that are applicable.												
.	.,,,	New		Revision to a previously approved plan review										
	N	Addition/Alteration		Extension to a previously approved plan n	eview									
	×	Permission to start (Fill out Section 3)												
4.	4. Health Care and Health Care Related Facilities: Check all that are applicable to the type of building submitted. If not applicable, then proceed													
		Health care facility [SPS 381.01(116)]		Health care related facility [SPS 381.01(1	1/m)	Care Delated Excilities for more								
		See the Plumbing Plan Review Recap & Insp	ection	Note: the checklist provided above is use	d by t	he Department and delegated								
		municipalities conducting plan review and ins	pectic	on for these types of occupancies.										
5.	5. Types of Installation Components (Equipment Types): Check all that are applicable.													
	Link	to eSLA equipment definitions can be found b	y visi	Ing: eSLA Plumbing Equipment Delinitions	E I	Regulated Contaminant Water								
	Ц	Building Drain & Vent, Sanitary		Interior Containment Tank		Treatment – Other								
	Ц	Building Drain & Vent, Storm		Interior Containment Tails		Regulated Contaminant Water								
	Ц	Building Sewer, Sanitary	ш	Accomply Health Care		Treatment – Radium								
		Building Sewer, Storm		Interior Grease Intercentor	П	Sanitary Dump Station								
	Ш	Campground/Recreational Vehicle Faik		Interior Mixed Wastewater Treatment		Siphonic Roof Drain Engineered								
		Drainage System, Sanitary		Device		System								
	Ц	Campground/Recreational Vehicle Faik	п	Interior Non-Potable Water System		Sovent Engineered System								
	п	Compare Jystem, Storm		Interior Oil Interceptor		Storm Detention System								
		Water Supply System		Interior Potable Water Tank		Storm Subsurface Infiltration Plumbing								
		Car Wash Intercentor		Interior Wastewater Treatment Device		Water Distribution System*								
		Chemical Waste System		Manufactured Home Community Water		Water Reuse - Blackwater								
		Controlled Roof Drain Engineered System		Supply System		Water Reuse - Clearwater								
		Drainage System, Storm		Multipurpose Piping System		Water Reuse – Graywater								
		Exterior Containment Tank	×	Private Interceptor Main Sewer,		Water Reuse – Stormwater								
		Exterior Cross Connection Control		Sanitary*		Water Service*								
		Assembly. Health Care		Private Interceptor Main Sewer, Storm*		Water Treatment5 Chlorine								
	П	Exterior Grease Interceptor		Private Water Main*		Water Treatment – Chloramine								
	n	Exterior Mixed Wastewater Treatment		Provent Engineered System		Water Treatment – Chlorine Dioxide								
		Device		Pure Water System		Water Treatment – Silver/Copper								
		Exterior Non-Potable Water System		Regulated Contaminant Water		Water Treatment Thermal								
		Exterior Oil Interceptor		Treatment – Arsenic		Water Treatment – Ultrafiltration								
		Exterior Potable Water Tank		Regulated Contaminant Water		Water Treatment Ultraviolet System								
		Exterior Wastewater Treatment Device,		Treatment – Bacteria		Water Treatment – Ultrafiltration								
		Storm		Regulated Contaminant Water		Water Treatment – Ultraviolet System								
		Garage Catch Basin		Treatment – Nitrate		Altemate Vacuum Waste System								

* Permission to Start is acceptable for this plumbing equipment. See Section 3 for more information.

** See Section 4 for more information.

*** Note *** Interior Cross Connection Control Assembly, Non-Health Care and Exterior Cross Connection Control Assembly, Non-Health Care Devices and Assemblies are no longer included in plumbing plan review submittals. These Devices and Assemblies are required to be registered and tested and submitted to the Department per <u>SPS 382.22(8)</u>.

				-	
Section Plumb submit	on 2 bing itted	z. I pla in	PLUMBING PLAN SUBMITTAL DUCUMENTS n submittal documents have two categories. Subsection 1 is the plu the application. Check all that are applicable.	mbir	ing plan requirements. Subsection 2 is all other documents to be
1. F	۶LU	MB	ING PLAN REQUIREMENTS: Check all that are applicable.		
E	×	Pla	n Index		
{	×	Sit	e-Specific Plan		line showing what is
			Plan must show the locations, sizes, and slopes of all sanitary sewers, storm sewers (including the roof drain system), and water service oiping within the property lines.		Site grade run off plans and contour lines showing what is drained to the plumbing system
			GPM flow rates and maximum capacity are labeled next to each pipe size and slope. Include all pipe sizes and discharge rates.		 Geotechnical reports must not be included in the Site-Specific Plan.
[x	Flo	or Plan		a station of additions shall include existing loads.
			Plan must include complete plumbing floor plans for each floor, must show all sizes and locations of horizonal drains, water distribution lines, fixtures, and equipment to be installed.		 Remodeling of additions shall include charge terminations. See additional requirements on the <u>Water Reuse Checklist</u>, if applicable.
[x	lso	metric Diagrams		the storm area
	_		30°/60° isometric diagrams of the drain, vent, water distribution, and interior storm systems.		 Indicate water supply, orainage institle units, and commerce drainage with gpm loads with each change in pipe diameter.
l	×	Ro	of Plan	onda	tarv overflow drain systems per IBC 1611.3.
	_		Include elevations of parapets waits, sizes of soupport and of the		
l	×	Ge	All close must be accordly signed per SPS 382 20(4)(c)		 List fixture and appliance manufacturers and model numbers.
		8	Fictures appliances or equipment may need product approval.		 Complete sizing calculations for all grease interceptors.
		1	Fixtures, appliances, or equipment may need protect opposite		 Identify specific materials for installations as listed in SPS 384
		5	Provide product approval letters for each health care appliance -		 Plumbing specifications and other pertinent documents (can be submitted under Subsection 2)
1	5	St	numper and Clearwater Plumbing Systems Specific Requirement	s per	er SPS 382.36
1	لت		Calculations showing all systems upstream of detention are desig	ned,	d, at a minimum to pass the 10-year, 24-hour storm event.
			Calculations conforming to the requirements of SPS 382.36(5) inc features are designed to pass the design discharge flow from deter	lude Intio	led showing all plumbing systems downstream of detention ion and all additional flows.
		5	Volume calculations for the 2-year, 24-hour storm and the 100-ye	ar, 2	24-hour storm included showing not damage to property.
			An Operation and Maintenance Plan is included that contains all t	he re	required information outlined in SPS 382.36(13).
			Calculations showing 72-hour drain down time for dry detention s	/ster	ems for the design storm event per SPS 382.36(6)(g)1.
			Calculations showing maximum 6-inch stormwater depth for the c	esig	ign storm event on paved surfaces per SPS 382.30(0)(g)2.
		8	Calculations showing surface ponding will drain within 24 hours a	iter t	the design storm event per SPS 382.36(6)(g)3.
		St	ornwater and Clearwater Subsurface Infiltration Plumbing Systems	Re	equirements per SPS 382.365
			A site and soil evaluation must be included in accordance with the	e rec	equirements in SPS 385.40(3)(a) and 385.30 (1)(c).
			Soil Evaluation - Storm (SBD-10793) form signed by the CST/PS and are included with a signed site map. Form available at: https://www.sci.uk/action.com/sci.uk/action/sci.	S ha //ds	sps.wi.gov/Documents/Programs/Plumbing/SBD10793.edf
			Soil profile evaluations used to determine soil application rates sh	all t	be conducted using soil pits per SPS 363.20(2)(2).
			Soil profile evaluations used to determine or identify soil horizon groundwater or bedrock shall be conducted using either soil pits	dept or so	oths, soil color, soil texture, recoxmorphic feature colors of coparts soil borings per SPS 385.20(2)(c)2.
		2	Soil pits elevations reported on form SBD-10793 correspond with	the	e elevations shown on the stille operation right
			Calculations demonstrating groundwater mounding will not impact	t sy	ystem performance when the width of the system exceeded of the
		8	Calculations showing subsurface drainage system will drain dow down within 24 hours after a storm event.	n wit	ithin 72 hours after a storm event and schade policing was a same
		8	Details with section views of infiltration systems included showing	; ele	revalions of all childer components.
		8	Documentation showing the influent quality complies with the rec	uire	ements in Table 302.70-1101 substituee initiation and angulater
		8	Laboratory test results or other documentation included that dem plumbing system meets or will be treated to the minimum require	mer	strates that stormwater collected or site for disc in all of the storm and on an
2.	AD	DIT	IONAL SUBMITTAL REQUIREMENTS: Check all that are application	ulė.).
		С	omplete water calculations per SPS 382.40(7). Indicate the plan	bage	iete below for instructions and form.
	×	S	ubmit water calculations separately if not located on the plans	. LIF	
		h	ttps://dsps.wi.gov/Documents/Programs/Plumbing/SBD6479Instruc	non	ns.pui
		h	ttps://dsps.wi.gov/Documents/Programs/Plumbing/SBD6479.pdf		

lternate A	Approval at: https://dsps.my.salesforce.com/sfc/p/#000000	ULAZ3/a/0y000020(0)110/1/0/000000101
		Request is for the following specific plumbing equipment installations.
n enocific	ad within the Alternate Approval, a submittal of a complete	Building Sewer, Sanitary;
at of plan	s is required to utilize the permission to start.	Private Interceptor Main Sewer, Startity,
		Denvire Interceptor Main Sever, Storm,
cope of in	nstallations are limited to below grade only and a	Building Sewer, Storm,
aximum	of 18-inches above floor.	U water Service,
	a state data ha	D Private Water Main,
lumbing	equipment requested to the right must also be	
hecked i	In Section 1.	Water Distribution System
s the buil een revie	lding owner, I request to begin plumbing installations prior t ewed, and to remove or replace any non-code complying co	to plan review approval I agree to make any changes required after plans have onstruction and make revisions to plans on any changes. I will not permit any
stallation	to exceed 18 inches above the unexcavated libor.	
	Stand Aught	5-8-2020
	1 and share	Date
	Building Owner's Signature	
		G THE JAPMO WATER DEMAND CALCULATOR (WDC)
ection 4	Approval at	S NO. US WAS SPAY
itemate //dst	ps.my.salesforce.com/sfc/p/#t0000000LAz5/a/8y000004t1k	G/h62oQttBGrkNbyAB2wU1XneBnVcRwHSmw0_111ASP01
82.40(7) ater sup onpublic nd applia	outlined in the alternate approval. I understand this offerna oply and principal branches for one- and two-family dwelling multiple dwellings, as defined by s. SPS 381.01(155) and ances.	is as specified in s. SPS 320.02(1)(a), (ce), (cm), or (cs) Wis. Adm. Code and (162) Wis. Adm. Code, with water conserving plumbing fixtures, fixture fittings
he appli	cant acknowledges the following items:	the ten MIDC submittal
1.	Review and include a copy of the DSPS approval PP-03	1603529-PTOAA letter with the IAPMO WDC submittan
2.	Provide verbiage for a sign or posting with permanent tag specific IAPMO Water Demand Calculator Sizing system	gging at the building control valve and water heater control valve to identify the
а	Provide IAPMO WDC calculations for each piece of distri	ibution piping using the IAPMO WDC sizing method.
4	All piping sized using the IAPMO WDC alternative shall o	display bold, underlined and italicized GPM loads on the isometric plan sneets.
4. 5.	WSFUs shall not be combined with WDC GPM's (mains fixture, if adding to the WDC method distribution system.	or vertical risers); therefore, provide actual fixture GPMs loads for each non WD . Separate water distribution piping systems may use Wis Code SPS 382.40(7) eginning of any IAMPO WDC system method sizing piping.
6.	Water distribution piping ½" or ¾" in diameter serving plu available for uniform loss ("A" value) in Tables SPS 382.	umbing fixtures shall not have a load greater than those assessed per pressure 40 4-11 Wis. Adm. Code and tables for ASTM D1785 and ASTM F441 in the
	appendix.	5 (up for the IAPMO
7.	All fixtures and replacement fixtures shall be at or below Water Demand Calculator Sizing system. Provide fixture	e cut sheets with low flow & energy star certification with the IAPMO submittal.
8.	Water supply piping shall be sized and installed in strict Adm. Code and the alternate approval.	accordance with IAMPU Water Demand Carculator V. 212, 2007-200
		Date
	Applicant's Signature	
Section	5. ATTESTATION	
Applican	it acknowledges that the submittal is complete and accurate	e.
Applican business by the D	nt acknowledges that any additional application or submitta s days or the plan is subject to denial. Applicant further ack tepartment within fifteen (15) business days or the plan is s	I information requested must be received by the Department which we (c) knowledges that any additional plan review information requested must be receiv subject to denial.
Include	this form with the plan review application separately f	rom the plan documents.
menude	Jordan Nolle	5/9/2025
		Date

Page 3 of 3

Area Method Worksheet for Storm Sewer Design

G&G Kaukauna														Last Updated: 5/5/2025					
			Structures Design Information											Pipe Information					
Length	Upstream Elevation	Downstream Elevatior	Upstream Structure	Downstream Structure	Drainage Area	Design Storm	Roof Area	Pavement Area	Lawn Area	Direct Runoff	Other Runoff	Design Runoff	Storm Sewer Size	Storm Sewer Material	Slope of Storm Sewer (ft/ft)	Pipe Capacity Flowing Full			
ft					Acres	Yr	Acres	Acres	Acres	GPM	GPM	GPM	in.		ft/ft	GPM			
25.1	700.89	700.74	RD 1	RD 2	0.123	10	0.123	0.000	0.000	206.07	0.00	206.07	6	PVC	0.0060	230.53			
24.8	700.74	700.59	RD 2	RD 3	0.103	10	0.103	0.000	0.000	172.56	206.07	378.64	8	PVC	0.0060	496.47			
24.9	700.59	700.44	RD 3	RD 4	0.103	10	0.103	0.000	0.000	172.56	378.64	551.20	10	PVC	0.0060	900.16			
25.2	700.44	700.29	RD 4	RD 5	0.104	10	0.104	0.000	0.000	174.24	551.20	725.44	10	PVC	0.0060	900.16			
25.1	700.29	700.14	RD 5	RD 6	0.104	10	0.104	0.000	0.000	174.24	725.44	899.68	12	HDPE	0.0060	1463.76			
9.9	700.14	700.08	RD 6	TEE IN	0.114	10	0.114	0.000	0.000	190.99	899.68	1090.68	12	HDPE	0.0060	1463.76			
77.1	701.56	700.08	EXISTING TRENCH DRAIN	TEE IN	0.023	10	0.000	0.023	0.000	30.83	0.00	30.83	6	PVC	0.0192	412.70			
8.1	700.08	700.03	TEE IN	MH 1	0.131	10	0.131	0.000	0.000	219.48	1121.50	1340.98	12	HDPE	0.0060	1463.76			

Assumptions:

1) Peak flow of stormwater was calculated using the 'Area method' as described in **SPS 382.36(5)(a)1**.

2) When calculating stormwater peak flow, the drainage area to each pipe run was divided by the described divisors in SPS 382.36(5)(a)1. For roofs the divisor is 26 sf/gpm. For paved or graveled ground surfaces the divisor is 32.5 sf/gpm. For lawns, parks, and similar land surfaces the divisor is 104 sf/gpm.

5) Pipe Capacity flowing flow was calculated using Manning's Equation 6) Manning's n-value = 0.011 for PVC and HDPE pipe

Area Method Worksheet for Storm Sewer Design

24.46 117.83 348.96 174.72 564.08 373.09 381.88 122.79



EROSION CONTROL AND STORMWATER MANAGEMENT PERMIT APPLICATION

Applicant Name (Indiv., Org. or Entity) Jordan Nolle Authorized Representative Title Project Engineer Mailing Address 1400 Lombardi Ave #101s City Green Bay State WI Podeal Code 54304 E-mail Address in Olle@Unckert-mielke.com 320-883-1161 Fax (include area code) Fax (include area code) Landowner Information (f different than Applent) Contract Person Title Out Mailing Address Contract Person Title Out State Podal Code 54304 Mailing Address Contract Person Wark State Podal Code Podal Code Cher Contract Information (check one) Engineer / Consultant Contract Person Fax (include area code) Telephone (include area code) <th>Applicant Information</th> <th></th> <th></th> <th></th> <th></th>	Applicant Information					
Mailing Address 1400 Lombardi Ave #101s City Green Bay State With Postal Code 54304 E-mail Address inolle@ruekert-mielke.com Telephone (include area code) Exat (include area code) Name (Organization or Entity) KM Investments Contact Person Mailing Address Bol Progress Way Contact Person Mailing Address Bol Progress Way Contact Person Mailing Address Telephone (include area code) Postal Code Mailing Address Contact Person Fac (include area code) Mailing Address Contact Person Postal Code Mailing Address Contact Person Fac (include area code) Mailing Address Contact Person Person Mailing Address Mailer Constant Person Person Mailing Address Mailer Constant Person Person Mailing Address 1400 Lombardi Ave #101s Contact Person Cher Contact Information (include area code) Person Numbers: 322:098:000 Project of Site Location Person Numbers: 32:20:098:000 Site Name (Project): G&G Kaukauna Wit 54:100 Errorison Control <1 area or 43:560 sq.t. Diskubed Area (EC1)	Applicant Name (Indiv., Org. or Entity) Jordan Nolle	Authorized Representative		TitlePr	Title Project Engineer	
E-mail Address jnolle@ruekert-mielke.com Telephone (include ana code) g20-883-1161 Fax (include ana code) g20-883-1161 Landowner Information (if different than Applicant) Contact Person Mark Strong f Title Our	Mailing Address 1400 Lombardi Ave #101s	city Green	Bay	State W	Postal Code 54304	
Landowner Information (if different than Applicant) Contact Person Title Marke (Orgenization of Entity) R.M. Invictanuets Contact Person Mark State Postal Code Mailing Address ISO1 Rog yusts Way Telephone (include area code) Fax (include area code) Pax (include area code) Cher Contact Information (check one): Imagineer / Consultant Contact Person Jordan Nolle Telephone (include area code) Other Contact Information (check one): Imagineer / Consultant Contact Person Jordan Nolle Telephone (include area code) Other Contact Information (check one): Imagineer / Consultant Contact Person Jordan Nolle Telephone (include area code) Mailing Address 1400 Lombardi Ave #101s City Green Bay State Telephone (include area code) Project or Site Location State Person State Will Postal Code Address / Location: 280.08 KG Kaukauna, WI 54130 Plat / CSM / Lol No: Telephone (include area code) Image: Control < 1 are or 43,560 sq.t. Disturbed Area (EC1)	E-mail Address jnolle@ruekert-mielke.com	Telephone (includ 920-883-1161	le area code)	Fax (incl	ude area code)	
Name (Organization or Entity) Q.R.M. Invictionus Contact Person Mark Stung F The Outrow Mailing Address ISOL Projects Way Mark Stung F State City State WU Postal Code Fmail Address Telephone (include area code) Fax (include area code) Park (include area code) Other Contact Information (check one): Engineer / Consultant Engineer / Consultant Mare (Organization or Entity) Contact Person Jordan Nolle Agent / Other Project or Site Location Contact Person Jordan Nolle State WI Postal Code Site Name (Project): G&G Kaukauna Parcel Numbers: 3220989000 Address / Location State Address / Location State	Landowner Information (if different than Applicant)					
Mailing Address City State Postal Code E-mail Address Telephone (include area code) Fax (include area code) Fax (include area code) Other Contact Information (check one): Image: Contractor / Builder Agent / Other Name (Organization or Entify): Ruekert and Mielke Contact Person Jordan Nolle Telephone (include area code) Mailing Address 1400 Lombardi Ave #1101s City Green Bay State WI Postal Code Project or Site Location Site Nume (Project): G&G Kaukauna Parcel Numbers: 322098900 Postal Code Address / Location: 2801 Progress Way, Kaukauna, WI 54130 Plat/ CSM / Lot No:: Parcel Numbers: 322098900 Permit Type & Fees (check all that apply) Erosion Control > 1 acre or 43,560 sq.tt. Disturbed Area (EC1) Stornwater Management < 20,000 sq.tt. Impervious Area (SM2)	Name (Organization or Entity) REM Investments	Contact Person Mark	Stumpf	Title C	16m mg	
E-mail Address (Mark @ 3344/mixui. Conn Telephone (include area code) W0 76/4 9188 Fax (include area code) W0 76/4 9188 Fax (include area code) Part of the contract information (check one): Engineer / Consultant Contract Person Jordan Nolle Telephone (include area code) 20.883-1161 Mailing Address 1400 Lombardi Ave #101s Contact Person Ordan Nolle Telephone (include area code) 20.883-1161 Postal Code Mailing Address 1400 Lombardi Ave #101s City Green Bay State WI Parcel Numbers: 322098900 Postal Code Project or Site Location State Value (Project): C&&G Kaukauna, Address / Location: 2801 Progress Way, Kaukauna, WI 54130 Parcel Numbers: 322098900 Plat / CSM / Lot No: Commuter Management < 20,000 sq.ft. Impervious Area (SM1)	Mailing Address	City Kankau	*114	State WT	Postal Code	
Mark (@ 33wathicut, Con. Utb / CC 4 183 Other Contact Information (check one): Engineer / Consultant Contractor / Builder Agent / Other Name (Organization or Entity) Ruekert and Mielke Contact Person Jordan Nolle Stats WI Postal Code Mailing Address 1400 Lombardi Ave #101s City Green Bay Stats WI Postal Code Project or Site Location Engineer / Comat Person Site Name (Project): C8&G Kaukauna Parcel Numbers: 322098900 Address / Location: 2801 Progress Way, Kaukauna, WI 54130 Parcel Numbers: 322098900 Permit Type & Fees (check all that apply) Ension Control < 1 are or 43,560 sq.ft. Disturbed Area (EC1)	E-mail Address	Telephone (includ	le area code)	Fax (incl	ude area code)	
Other Contact Information (check one): Engineer / Consultant Contractor / Builder Telephone (include area code) Name (Organization or Entity) Ruekert and Mielke Contact Person Jordan Nolle Telephone (include area code) Mailing Address 1400 Lombardi Ave #101s City Green Bay State WI Postal Code Project or Site Location Base for Site Location: Parcel Numbers: 322098900 Plat / CSM / Lot No: Address / Location: 20.01 Progress Way, Kaukauna, WI 54130 Plat / CSM / Lot No: Comment of the state of 43,560 sq.ft. Disturbed Area (EC1) Stormwater Management < 20,000 sq.ft. Impervious Area (SM2)	Mark @ ggnachinwi.com	410 166	4188		011	
Name (Organization or Entity) Contact Parson Jordan Nolle Telephone (nuclear are code) Mailing Address 1400 Lombardi Ave #101s City Green Bay State Wil Postal Code Project or Site Location State Vill Postal Code Postal Code Site Name (Project): G&G Kaukauna Parcel Numbers: 322098900 Parcel Numbers: 322098900 Address / Location: 2801 Progress Way, Kaukauna, WI 54130 Plat / CSM / Lot Not: Parcel Numbers: 322098900 Permit Type & Fees (check all that apply) Erssion Control < 1 acre or 43,560 sq.ft. Disturbed Area (EC1)	Other Contact Information (check one): Engineer / C	Consultant Co	ontractor / Builder	Agent /	Other	
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Image: Erosion Control < 1 acre or 43,560 sq.ft. Disturbed Area (EC1)	Permit Type & Fees (check all that apply)					
Total Disturbed Area 10335 sq.ft. x \$0.0002 / sq.ft. (EC2) = \$2.07 New Impervious Area 9194 sq.ft. x \$0.0025 / sq.ft. (SM2) = \$22.98 Base Fee: \$200 (EC1), \$250 (EC2), \$200 (SM1), \$500 (SM2) = \$200 200 Total Application Fee = \$ Duration of Land Disturbance weeks x \$25 / week (EC1, EC2) = \$ Start Date Base Fee: \$250 (EC2), \$500 (SM2) = \$0 End Date Total Inspection Fee = \$ Certification & Permission Certification & Permission Certification: I hereby certify that I am the landowner of the property which is the subject of this Permit Application. I certify that the information contained in this form and attachments is true and accurate. I understand that failure to comply with any or all of the property to evaluate this permit application. I certify that the property to evaluate this permit application, determine compliance with ordinances, and perform corrective actions after issuing proper notice to the landowner. Applicant Signature Date Signed Jorddan Nolle Date Signed Leave BLANK – FOR MUNICIPAL USE ONLY Date Signed Leave BLANK – FOR MUNICIPAL USE ONLY Date Issued: Leave BLANK – FOR MUNICIPAL USE ONLY Date Signed St.	■ Erosion Control < 1 acre or 43,560 sq.ft. Disturbed Area (EC1)					
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	Construction Site ID / Permit No:	Date Issued:	Date Issued: Issued By:		d By:	

PLEASE CONTACT CITY OF KAUKAUNA ENGINEERING DEPARTMENT AT 920-766-6305 WITH QUESTIONS.

G&G MACHINE, INC. CITY OF KAUKANA OUTAGAMIE COUNTY WISCONSIN



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	C2.0
	C3.0
	C4.0
	C5.0
	C6.0
C7.	.0 - ####





RUEKERT/MIELKE TAKES NO RESPONSIBILITY FOR ANY UNDERGROUND STRUCTURES OR BURIED MATERIALS SUCH AS, BUT NOT LIMITED TO, FOUNDATIONS, WELLS, SEPTIC, HOLDING TANKS, UTILITIES, HAZARDOUS MATERIALS, OR ANY OTHER ITEMS OF WHICH NO EVIDENCE CAN BE FOUND ON THE SURFACE BY A REASONABLE INSPECTION.

PURSUANT TO WISCONSIN STATUTE 182.0175, AVAILABLE DATA ON JNDERGROUND STRUCTURES, CONDUIT AND PIPES HAS BEEN SHOWN ON THI MAP. THE LOCATIONS SHOWN HAVE BEEN COMPILED FROM A COMBINATION OF EXISTING UTILITY MAPS AND MARKINGS PLACED IN THE FIELD FOR THE VARIOUS FACILITIES BY "DIGGERS HOTLINE" (TICKET NO. 20251100875) SHALL NOT BE TAKEN AS CONCLUSIVE. FIELD VERIFICATION SHALL BE REQUIRED BEFORE ANY EXCAVATION.

DESCRIPTION

COVER SHEET GENERAL NOTES

EXISTING CONDITIONS

EROSION CONTROL PLAN

SITE AND UTILITY PLAN

GRADING PLAN

CONSTRUCTION DETAILS



FOX CITIES

N216 State Road 55 3308 Nursery Drive P.O. Box 620 Kaukauna, WI 54130 Phone (608)445-224 Phone (920)766-5795 1-800-236-2534 Fax (920)766-5004

MILWAUKEE W204 N11509 Goldendale Rd 1-800-236-2534 Fax (262)250-9740

5605 Lilac Ave Wausau, WI 54401 Germantown, WI 53022 Phone (715)849-3141 Phone (262)250-9710 Fax (715)849-3181

Middleton, WI 53562



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REVISIONS

PROJECT MANAGER:

DESIGNER:

C. MEISEL

DRAWN BY:

G. DEPREY

133

--/--/2025

PRELIMINARY NO:

CONTRACT NO:

DATE:

COVER SHEET C1.0

"ISSUED NOT FOR CONSTRUCTION"

SUPERVISOR:

EXPEDITOR:

R & M PROJECT #8341-10203.200 PROJECT MANAGER: COLIN MEISEL, PE

RUEKERT-MIELKE INC. 1400 LOMBARDI AVE, GREEN BAY, WI

ENGINEER:

PHONE: (920) 876-6382

G&G MACHINE, INC.

1801 PROGRESS WAY

KAUKANUNA, WI 54130

LOCATION MAP IS NOT TO SCALE

SITE:

Dawes I Rental,

GENERAL NOTES:

- 1. CONTRACTOR TO CONTACT DIGGERS HOTLINE FOR UTILITY LOCATES PRIOR TO CONSTRUCTION. CONTRACTOR SHALL VERIFY WITH UTILITY COMPANIES IF UTILITY COMPANY'S STAFF IS REQUIRED TO BE ON SITE WHEN CONSTRUCTION ACTIVITIES ARE NEAR UTILITY FACILITIES.
- 2. LOCATION OF ALL STRUCTURES, OBSTACLES, AND EXISTING FACILITIES SHOWN SHALL NOT BE TAKEN AS CONCLUSIVE. CONTRACTOR SHALL VERIFY LOCATIONS AS A CONDITION OF THEIR BID AND BE RESPONSIBLE FOR ALL DAMAGES RESULTING FROM THEIR ACTIVITIES.
- 3. CONTRACTOR SHALL TAKE CARE WHEN EXCAVATING AROUND EXISTING UTILITY LINES AND EQUIPMENT. VERIFY COVERAGE REQUIREMENTS WITH UTILITY COMPANIES.
- 4. EXISTING UTILITIES SHOWN ARE APPROXIMATE AND HAVE BEEN OBTAINED FROM AVAILABLE RESOURCES FOR FIELD LOCATES. THERE MAY BE ADDITIONAL UTILITIES NOT SHOWN. CONTRACTOR IS REQUIRED TO VERIFY LOCATION OF EXISTING UTILITIES.
- 5. CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS NECESSARY TO CARRY OUT THEIR WORK, UNLESS OTHERWISE NOTED.
- 6. CONTRACTOR SHALL PROVIDE STAKING AS NECESSARY TO LAYOUT AND PROVIDE GRADES FOR ANY SECTION OF THE WORK.
- 7. A COMPETENT REPRESENTATIVE WHO HAS AUTHORITY TO ACT FOR THE CONTRACTOR MUST BE AT THE SITE AT ALL TIMES.
- 8. STAGING AND MATERIAL STORAGE AREAS SHALL BE COORDINATED WITH THE OWNER AND SHALL BE DONE IN A MANNER TO AVOID INTERFERENCE WITH THE OWNER'S ACTIVITIES.
- 9. CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING ALL EXISTING PROPERTY CORNERS AND SURVEY MONUMENTS.
- 10. CONTRACTOR SHALL BE RESPONSIBLE FOR BARRICADING AREAS OF CONSTRUCTION TO PROTECT AGAINST PERSONAL INJURY.
- 11. EXISTING FACILITIES TO REMAIN INCLUDING PAVEMENT, SIDEWALKS, BUILDINGS, LANDSCAPING AND TREES SHALL BE PROTECTED DURING CONSTRUCTION.
- 12. CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING THEIR WORK FROM ALL DAMAGE INCLUDING THE PUBLIC. OTHER CONTRACTORS, AND THE ENVIRONMENT.
- 13. EXCESS MATERIALS SHALL BE REMOVED FROM THE SITE UNLESS OTHERWISE DIRECTED BY THE OWNER.

UTILITIES AND STORM SEWER:

- 1. ALL EXISTING SURFACE INFRASTRUCTURE INCLUDING HYDRANTS, VALVES, HANDHOLES, CASTINGS, IRRIGATION SYSTEMS, AND UTILITY PEDESTALS ARE REQUIRED TO BE ADJUSTED TO PROPOSED GRADE BY CONTRACTOR.
- 2. WHERE NOT IDENTIFIED IN THE CIVIL SPECIFICATION MANUAL, UTILITY MATERIALS AND INSTALLATION SHALL CONFORM TO LOC/ UTILITY COMPANIES HAVING JURISDICTION.
- 3. CONTRACTOR SHALL COORDINATE INSTALLATION OF UTILITIES AND CONDUITS TO AVOID CONFLICTS AND TO PROVIDE MINIMUM ADDITIONAL BENDS AND ASSOCIATED MATERIALS ARE TO BE INSTALLED AS REQUIRED FOR WATER MAINS AND LATERALS.
- 4. ALL STORM AND SANITARY SEWER PIPES AND STRUCTURES SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE PROJECT CIVIL SUPPLEMENTAL PLAN NOTES HEREIN.
- 5. STORM SEWER STRUCTURES SHALL BE PRECAST CONCRETE AND THE SIZE AS NOTED ON THE PLANS.
- 6. TRENCHES SHALL BE BACKFILLED WITH CRUSHED STONE BEDDING WITHIN 1:1 OF PAVEMENT AREAS AND WITH SPOIL IN LANDSC
- 7. STORM SEWER 8-INCHES OR SMALLER CONNECTED TO MAINS OR LATERALS SHALL BE PLACED HORIZONTALLY AT THE SPRING LI CONNECTION.
- 8. CONNECTIONS TO EXISTING MANHOLES SHALL BE CORED AND A WATER TIGHT SEAL PROVIDED.
- 9. TRACER WIRE OR OTHER MEANS OF LOCATING UNDERGROUND PIPES SHALL BE INSTALLED ON ALL PIPING PER THE PROJECT CIV
- 10. ALL DIMENSIONS ARE TO THE CENTERLINE OF UTILITIES AND STRUCTURES.

EROSION CONTROL:

- 1. CONSTRUCTION ACTIVITIES SHALL NOT COMMENCE UNTIL EROSION CONTROL DEVICES HAVE BEEN INSTALLED.
- 2. EROSION CONTROL DEVICES SHALL BE INSTALLED ACCORDING TO WDNR BEST MANAGEMENT PRACTICES AND THE CIVIL SPECIFI
- 3. EXISTING LANDSCAPING AND TREES TO REMAIN SHALL BE PRUNED TO REMOVE LOW HANGING, BROKEN, AND UNDESIRABLE GRO SYMMETRICAL NEW GROWTH.
- 4. ALL AREAS DISTURBED BY CONTRACTOR OPERATIONS SHALL BE PREPARED FOR GRASS SEED BY LOOSENING RUTS AND WORKII OF 6-INCHES PRIOR TO THE FINE GRADING AND SEEDING. AREAS SHALL HAVE A MINIMUM OF 4-INCHES OF TOPSOIL PLACED, SEE OTHERWISE INDICATED.
- 5. INSPECTION OF ALL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE DONE BY CONTRACTOR ONCE PER WEEK AND WITH PRECIPITATION EVENT OF 1/2-INCH OR GREATER. EROSION CONTROL REPORTS WILL BE SENT TO EROSIONCONTROL@GREENBAY
- 6. CONTRACTOR SHALL REPAIR DEFICIENT EROSION AND SEDIMENT CONTROL MEASURES WITHIN 24-HOURS AFTER INSPECTION. AD SEDIMENT CONTROL DEVICES NOT SHOWN ON THIS PLAN MAY BE NECESSARY AS A RESULT OF CONSTRUCTION PRACTICES OR A ENGINEER
- 7. CONTRACTOR SHALL NOTIFY AND OBTAIN WRITTEN ACCEPTANCE FROM ENGINEER OF PROPOSED CHANGES TO THE EROSION CO PRIOR TO IMPLEMENTING THE CHANGE.
- 8. EXCESS MATERIAL THAT IS HAULED OFF SITE SHALL BE CONTRACTOR'S RESPONSIBILITY. CONTRACTOR SHALL OBTAIN PROPER F FILL SITE. EROSION AND SEDIMENT CONTROL MEASURES, RESTORATION, AND STABILIZATION AT FILL SITE IS CONTRACTOR'S RES NOTIFY OWNER OF ALL FILL AND BORROW SITES.
- 9. CONTRACTOR SHALL SWEEP STREETS ADJACENT TO PROJECT AS NEEDED.
- 10. ALL INSTALLATION, MAINTENANCE, AND REMOVAL OF EROSION AND SEDIMENT CONTROL MEASURES SHALL COMPLY WITH THE W
- 11. IF DEWATERING IS NECESSARY, CONTRACTOR SHALL PROVIDE PROPER DEWATERING SEDIMENT CONTROL DEVICES. DISCHARGE THE STORM OR SURFACE WATER IS PROHIBITED.
- 12. STABILIZE NEWLY GRADED AREAS WITHIN 3 DAYS OF BEING INACTIVE.
- 13. REMOVE EROSION AND SEDIMENT CONTROL DEVICES AFTER 80% OF VEGETATION HAS BEEN ESTABLISHED IN RESTORED AREAS.



Global Water Center • Fox Valley www.ruekertmielke.com

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GRADING AND PAVING:

- 1. ALL SITE CONSTRUCTION INCLUDING GRADING, EXCAVATION, AND PAVEMENT CONSTRUCTION SHALL BE DONE IN ACCORDANCE WITH THE CORRESPONDING PROJECT CIVIL SPECIFICATIONS MANUAL AND SUPPLEMENTAL PLAN NOTES HEREIN.
- 2. CONTRACTOR SHALL STRIP AND REMOVE TOPSOIL AND ORGANIC MATERIALS FOUND WITH THE SITE IN ACCORDANCE WITH THE PROJECT CIVIL SPECIFICATIONS. LANDSCAPING AREAS SHALL BE GRADED LOW TO ALLOW FOR TOPSOIL PLACEMENT.
- 3. ALL MATERIAL TESTING SHALL CONFORM TO THE PROJECT CIVIL SPECIFICATIONS. RETESTING AND CORRECTION OF FAILING MATERIAL SHALL BE COMPLETED AT THE CONTRACTOR'S EXPENSE.
- 4. SUBGRADE SHALL BE COMPACTED PRIOR TO PLACEMENT OF BASE AGGREGATE AS REQUIRED IN THE STANDARD SPECIFICATIONS. SUBGRADE SHALL BE PROOF ROLLED PRIOF TO PLACEMENT OF BASE COURSE. AREAS IDENTIFIED AS SOFT AND YIELDING SHALL BE IDENTIFIED FOR REMOVAL PRIOR TO PLACEMENT OF BASE AGGREGATE.
- 5. BACKFILL AND FILL MATERIALS SHALL BE PLACED IN LAYERS NOT MORE THAN 8-INCHES LOOSE IF COMPACTED WITH HEAVY EQUIPMENT AND NOT MORE THAN 4-INCHES LOOSE COMPACTED BY HAND EQUIPMENT.
- ACCESSIBLE ROUTES AND HANDICAP PARKING SHALL BE CONSTRUCTED IN ACCORDANCE WITH ADA STANDARDS.
- 7. ALL CONCRETE FLATWORK SHALL HAVE A LIGHT BROOMED FINISH
- EXTERIOR CONCRETE SHALL BE SEPARATED FROM BUILDINGS WITH A CONTINUOUS 0.5-INCH FIBER EXPANSION JOINT 8
- 9. TWO PAINT COATS SHALL BE APPLIED ON NEW PAVEMENT. THE FIRST COAT SHALL BE AFTER PAVING OPERATIONS HAVE BEEN COMPLETED. THE SECOND COAT SHALL BE APPLIED 30 CALENDAR DAYS AFTER PAVING HAS BEEN COMPLETED.
- 10. ELEVATIONS ARE TO FLANGE.

AL	STANDARDS A	ND SPECI	FICATIONS	FOR

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REQUIRED DEPTHS OF COVER.		BENCHMARK	<u>6</u> 8	SANITARY MANHOLE	С	CABLE BOX	Q	EXISTING HYDRANT	
	+ ^{CHIS}	CHISELED MARK	SEPTIC	SEPTIC SYSTEM	C	CABLE MANHOLE	Δ	EXISTING LOCATOR BOX	
SPECIFICATIONS MANUAL AND	 	CONTROL POINT	⊙ ⊙ ^{SEPC}	SEPTIC TANK COVER	C	UNDERGROUND TV CABLE	¥	EXISTING SPRINKLER HEAD	
L SPECIFICATIONS MANUAL AND	● ^{IP}	IRON PIPE (GENERIC)	⊙ ^{SEPV}	SEPTIC VENT	E	ELECTRIC BOX	- O I	EXISTING WATER CURB STOP	
	o ^{IP}	IRON PIPE (1" DIA)	?" SAN SWR	SANITARY SEWER	E	ELECTRIC MANHOLE	Ţ	EXISTING WATER MAIN BEND	
	⊗ _{IP}	IRON PIPE (2" DIA)	•	PROPOSED SANITARY MANHOLE	X E	ELECTRIC METER	ı <u> </u>	EXISTING WATER MAIN CROSS	
	oËT	IRON PIPE SET	-	PROPOSED SANITARY RISER	EP	ELECTRIC PAD	\sim	EXISTING WATER MAIN OFFSET	
AFING AREAS.	● ^{IR}	IRON ROD (GENERIC)		PROPOSED SANITARY LATERAL	Ē	ELECTRIC TRANFORMER	ш	EXISTING WATER MAIN PLUG	
INE OF THE PIPE WITH A WATER TIGHT	⊗ ^{IP}	IRON ROD (3/4" DIA)		PROPOSED SANITARY SEWER	Ŀ	POWER POLE	ب ي ا	EXISTING WATER MAIN PLUG W/ AIR RELEASE	
	o ^{IP}	IRON ROD (1 1/4" DIA)	STORM	<u>I SEWER</u>	———E———	UNDERGROUND ELECTRIC	∇	EXISTING WATER MAIN REDUCER	
	$\overline{\cdot}$	MONUMENT		STORM CATCH BASIN	——Е——	PROPOSED UNDERGROUND ELECTRIC	X	EXISTING WATER VALVE	
	NAIL	NAIL		STORM FIELD INLET	€□	PROPOSED UNDERGROUND ELECTRIC	\otimes	EXISTING WATER VALVE MANHOLE	
VIL SPECIFICATIONS MANUAL.	● ^{PM}	PAINT MARK	E	STORM INLET	6	GAS CURB STOP	\circ^{WELL}	EXISTING WELL	
	° PK	PK NAIL	S	STORM MANHOLE	G	GAS MANHOLE	-0-	EXISTING YARD HYDRANT	~
	RX •	RAILROAD SPIKE	*	STORM YARD DRAIN	X G	GAS METER	<u>?" WM</u>	EXISTING WATER MAIN	Ю
	RTIE	REFERENCE TIE	?" STO SWR	STORM SEWER	© GTST	GAS TEST STATION		EXISTING WATER SERVICE	LL O
		SECTION CORNER		CULVERT (SIZE & TYPE NOTED)	Ch	GAS VALVE	o l	PROPOSED CURB STOP	
	SIXT	SECTION CORNER MONUMENT		PROPOSED STORM INL/CB	o ^{GVT}	GAS VALVE TEST	A	PROPOSED HYDRANT	S
	∆	TEMPORARY CONTROL POINT		PROPOSED STORM MANHOLE	GAS VENT	GAS VENT	A	PROPOSED LOCATOR BOX	P
	🙆 USGS	USGS MONUMENT		PROPOSED STORM LATERAL	G	UNDERGROUND GAS MAIN	V	PROPOSED WATER MAIN BEND (ANGLE NOTED)	õ
	GRO	DUND		PROPOSED STORM SEWER	G	PROPOSED UNDERGROUND GAS MAIN	+	PROPOSED WATER MAIN CROSS	H L
CATIONS MANUAL.	WZ 000.00	WATER ELEVATION	GEN	NERAL	Т	TELEPHONE BOX		PROPOSED WATER MAIN OFFSET	
	¥	WETLANDS (SURVEYED LOCATION)	A/C	AIR CONDITIONER	T	TELEPHONE MANHOLE	Т	PROPOSED WATER MAIN PLUG	"COPY
WTH TO ENSURE HEALTHY AND		CROPFIELD	o ^{co}	CLEAN OUT	——— T ———	UNDERGROUND TELEPHONE	Ť	PROPOSED WATER MAIN PLUG W/AIR RELEASE	This design, dr copyrighted pro
		DITCH	O DP	DELINEATOR POST	——F0——	UNDERGROUND FIBER OPTIC	*	PROPOSED WATER MAIN REDUCER	No part hereof distributed, dis
NG THE SOIL AREAS TO A MINIMUM $-$	<u>n n n</u>	GUARD RAIL	oFP	FLAG POLE	——F0——	PROPOSED UNDERGROUND FIBER OPTIC	4	PROPOSED WATER MAIN TEE	anyone without consent of KEI
DED, AND MULCHED UNLESS —		TOE OF SLOPE		GENERIC HAND HOLE		DSCAPE	\otimes	PROPOSED WATER VALVE	
—	·	TOP OF BANK	MB	GENERIC MANHOLE		CONIFEROUS MULTIPLE TRUNK TREE		PROPOSED WATER VALVE MANHOLE	
	· — · — · —	WETLAND BOUNDARY	X	GENERIC METER	*	CONIFEROUS TREE	-	PROPOSED YARD HYDRANT	
YWI.GOV	EROSION	N CONTROL	Ä	GENERIC PEDESTAL	\odot	DECIDUOUS TREE		PROPOSED WATER MAIN	
		PROPOSED DITCH CHECK		GENERIC VALVE	E.	DECIDUOUS MULTIPLE TRUNK TREE		PROPOSED WATER SERVICE	
DDITIONAL EROSION AND		PROPOSED EROSION LOGS/EROSION WADDLES	o ^{vent}	GENERIC VENT	\bigcirc	DECORATIVE ROCK	PAVE	EMENT	
AS DIRECTED BY OWNER AND/OR	A	PROPOSED INLET PROTECTION TYPE A	OUY POLE	GUY POLE	Ø	STUMP	þ	EXISTING SIGN	
	B	PROPOSED INLET PROTECTION TYPE B	© GUY	GUY WIRE	· · · ·	EDGE OF PLANTER/LANDSCAPE BED		EDGE OF ASPHALT PAVEMENT	
ONTROL PLAN AND/OR SEQUENCE	C	PROPOSED INLET PROTECTION TYPE C	×	LIGHT POLE		EDGE OF TREES & BRUSH		EDGE OF GRAVEL PAVEMENT	
	D	PROPOSED INLET PROTECTION TYPE D	□ MB	MAIL BOX	—XX	FENCE		EDGE OF CONCRETE PAVEMENT	PROJECT M
		PROPOSED BARRIER FENCE	ΘMP	MARKER POST	RAII	LROAD	©©©©©©©©©©©©©©©©©©©©©©©©©©©©©©©©©©©©©©	PROPOSED DETECTABLE WARNING FIELD	
SPONSIBILITY. CONTRACTOR TO		PROPOSED SILT FENCE	⊙ MWEL	MONITORING WELL	RRSB	RAILROAD SIGNAL BOX	<u> </u>	PROPOSED SIGN	
		PROPOSED EROSION MAT	opile	PILING	-¢-	RAILROAD SIGNAL FLASHER		PROPOSED EDGE OF ASPHALT PAVEMENT	DESIGNER.
	KYXYX X	PROPOSED RIP RAP	0 Γ Λ	POST	• ^{RX}	RAILROAD SPIKE		PROPOSED EDGE OF CONCRETE PAVEMENT	
		PROPOSED TRACKING PAD	$\frac{1}{1}$	REVISION LABEL	++++++	RAILROAD TRACKS		PROPOSED EDGE OF GRAVEL PAVEMENT	NKAWN RI:
DNR TECHNICAL STANDARDS.			\bullet	SOIL BORING	<u>TRAFFI</u>	IC SIGNAL		PROPOSED SLOPE INTERCEPT	
			Ø	UTILITY POLE		CONTROL BOX/LIGHTING CABINET			EXPEDITOR:
E OF SEDIMENT LADEN WATER TO			*	YARD LIGHT	O	PULL BOX			
					₽	TRAFFIC SIGNAL			SUPERVISO
					LD	UNDERGROUND LOOP DETECTOR			





FOX CITIES N216 State Road 55 3308 Nursery Drive P.O. Box 620 Kaukauna, W 54130 Phone (608)445-2245 Phone (920)766-5795 1-800-236-2534 Fax (920)766-5004

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WAUSAU 5605 Lilac Ave |Wausau, WI 54401 Germantown, WI 53022|Phone (715)849-3141



LEGEND - CIVIL ENGINEERING DRAWINGS

ECT MANAGER: ____ SNER: C. MEISEL WN BY: G. DEPREY DITOR: _____ RVISOR: ____ PRELIMINARY NO: ____ CONTRACT NO: ____ DATE:

--/--/2025

C2.0

GENERAL NOTES



EXISTING CONDITIONS C3.0



	Point #	Raw Description	Elevation	Northing	Easting
	11	CP 11 MAG NAIL IN ASPH PAVEMENT	704.63	577270.65	864104.5
	12	CP 12 MAG NAIL IN ASPH PAVEMENT	704.25	577459.08	864114.34
	100	BM HYD TOP NUT	704.82	577434.09	864492.3
	13	CP 13 MAG NAIL IN CONC PAVEMENT	703.65	577550.43	864130.4 ⁻
	14	CP 14 MAG NAIL IN CONC PAVEMENT	701.67	577468.90	864311.5
	102	BM HYD TOP NUT	702.67	577665.12	864118.4
	15	CP 15 MAG NAIL IN CONC PAVEMENT	700.96	577462.52	864390.2
	16	CP 16 MAG NAIL IN ASPH PAVEMENT	703.54	577398.49	864436.89
1					

Point Table

CELEBRATION INVESTMENTS II LLC 1901 PROGRESS WAY TAX KEY: 322098801

COORDINATE SYSTEM AND VERTICAL DATUM: UNITED STATES/NAD83/WISCRS

PROJECT MANAGER:	
DESIGNER:	C. MEISE
DRAWN BY:	G. DEPRE`
EXPEDITOR:	
SUPERVISOR:	
PRELIMINARY NO:	
CONTRACT NO:	

--/--/2025

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Fax (262)250-9740 www.kellerbuilds.com







EROSION CONTROL



PROPOSED INLET PROTECTION TYPE B PROPOSED SILT FENCE PROPOSED TRACKING PAD

NOTES:

1. ESTIMATED AREA OF DISTURBANCE: 0.24 ACRES.

CONSTRUCTION SEQUENCE

- 1. INSTALL AND MAINTAIN EROSION AND SEDIMENT CONTROL MEASURES PRIOR TO ANY LAND DISTURBING ACTIVITIES, AS SHOWN ON DRAWINGS AND DIRECTED BY ENGINEER.
- 2. CLEAR AND GRUB VEGETATION AS SHOWN ON DRAWINGS OR AS DIRECTED BY ENGINEER.
- 3. STRIP TOPSOIL AND STOCKPILE IN LOCATION AS SHOWN ON DRAWINGS AND AS DIRECTED BY OWNER. INSTALL PERIMETER SILT FENCE AROUND DOWN SLOPE SIDE OF STOCKPILE. STABILIZE STOCKPILE IMMEDIATELY UPON LAYUP.
- INSTALL, ADJUST AND MAINTAIN ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES NECESSARY FOR EACH ACTIVE STAGE. MAINTAIN MEASURES AT THE END OF EACH DAY.
- 5. STAGE CONSTRUCTION BY WORK LOCATION. SUBSEQUENT STEPS MAY BE COMPLETED IN ALTERNATE SEQUENCE DEPENDING UPON CONTRACTOR OPERATIONS.
- 6. COMPLETE ROUGH GRADING.
- 7. PREPARE PAVEMENT SUBGRADE.
- 8. REMOVE AND REPLACE EXISTING BIOFILTER ENGINEERING SOIL.
- 9. INSTALL NEW PAVEMENT AND BASE LAYERS.
- 10. INSTALL LAWN LANDSCAPING.
- 11. AREAS PLANNED TO BE INACTIVE FOR 7 DAYS OR LONGER SHALL BE TEMPORARILY STABILIZED FOLLOWING DNR TECHNICAL STANDARD 1059 SEEDING. THESE AREAS SHALL BE STABILIZED WITHIN 7 DAYS OF BEING INACTIVE.
- 12. AREAS BROUGHT TO FINAL GRADE SHALL BE PERMANENTLY STABILIZED WITHIN 7 DAYS.
- 13. COMPLETE FINAL RESTORATIONS INCLUDING BUT NOT LIMITED TO: TOPSOIL, TURF GRASS SEED, AND CLASS I TYPE B URBAN EROSION MATTING FOR ALL LAWN RESTORATION AND TOPSOIL, TURF GRASS SEED, AND CLASS II TYPE B EROSION MATTING FOR THE DRAINAGE SWALES AND SLOPES OF 4:1 OR MORE.
- 14. REMOVE TEMPORARY EROSION CONTROL DEVICES AFTER 80% GROWTH DENSITY HAS OCCURRED IN 100% OF RESTORATION AREAS. CONTRACTOR SHALL OBTAIN OWNER AND ENGINEER APPROVAL PRIOR TO REMOVING THE MEASURE(S). RESTORE DISTURBED AREAS AROUND REMOVED DEVICES AND CLEAN SITE.



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REVISIONS

PROJECT MANAGER: ____

DESIGNER: C. MEISEL

--/--/2025

C4.0

DRAWN BY: G. DEPREY

EXPEDITOR:

SUPERVISOR:

PRELIMINARY NO:

CONTRACT NO:

DATE:

SHEET: EROSION CONTROL PLAN

"ISSUED NOT FOR CONSTRUCTION"



"ISSUED NOT FOR CONSTRUCTION"

SITE AND UTILITY PLAN C5.0

PROJECT INF	JRIMATION
ZONING	INDUSTRIAL
LOT AREA	9.1 AC
EXISTING IMPERVIOUS	2.2 AC
PROPOSED IMPERVIOUS	0.2 AC
LAND DISTURBANCE	0.2 AC
GREEN SPACE	6.7 AC
PARKING SPACES	55
PARKING SPACES	55

<u>LEGEND</u>

. 4 4 4. 4

PROPOSED BUILDING

ASPHALT PAVEMENT

PROPOSED CONCRETE PAVEMENT



137

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CELEBRATION INVESTMENTS II LLC 1901 PROGRESS WAY

TAX KEY: 322098801

- PROPERTY LINE



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"ISSUED NOT FOR CONSTRUCTION"





RUEKERT/MIELKE TAKES NO RESPONSIBILITY FOR ANY UNDERGROUND STRUCTURES OR BURIED MATERIALS SUCH AS, BUT NOT LIMITED TO, FOUNDATIONS, WELLS, SEPTIC, HOLDING TANKS, UTILITIES, HAZARDOUS MATERIALS. OR ANY OTHER ITEMS OF WHICH NO EVIDENCE CAN BE FOUND ON THE SURFACE BY A REASONABLE INSPECTION.







PURSUANT TO WISCONSIN STATUTE 182.0175, AVAILABLE DATA ON JNDERGROUND STRUCTURES, CONDUIT AND PIPES HAS BEEN SHOWN ON THIS MAP. THE LOCATIONS SHOWN HAVE BEEN COMPILED FROM A COMBINATION OF EXISTING UTILITY MAPS AND MARKINGS PLACED IN THE FIELD FOR THE VARIOUS FACILITIES BY "DIGGERS HOTLINE" (TICKET NO. 20251100875) SHALL NOT BE TAKEN AS CONCLUSIVE. FIELD VERIFICATION SHALL BE REQUIRED BEFORE ANY EXCAVATION.



MEMO

PLANNING AND COMMUNITY DEVELOPMENT

To:	Plan Commission
From:	David Kittel Director of Planning and Community Development
Date:	5/16/2025
Re:	Site Plan Review – 101 Kelso Rd (Co Vantage)

Co Vantage is finalizing plans to redevelop the property along Kelso Rd adjacent form NEW Prosperity Industrial park. The new development will be a for a Co Vantage Bank of approximately 3,479 square feet. The plans are attached to this report for review and summarized below:

Site Plan Review:

Site/Architectural: 17.32 (10) Supplementary District Regulations & applicable zoning

All set back requirements are met and all applicable ordinances are being complied with to include zoning requirements. Note that the property is going through rezoning process to Commercial Highway District(CHD) from Industrial (IND).

Landscape: 17.52 Landscaping Requirements

The proposed Landscape plan submitted meets all requirements per ordinance.

Lighting:

Lighting is being added in the parking lot area, the submitted photometric plan confirms there should be no light directly shining off the property.

Stormwater: 22 Stormwater Management

The developer will work with Engineering Department to complete Erosion Control and Stormwater Management permitting.

Ingress/Egress: No Concerns noted

Public Safety:

No concerns noted at this time.

Façade: 17.53 Façade Standards

The proposed site elevations meet façade requirements

Staff Recommendation:

Staff recommend approval of the development with the following conditions:

- Prior to issuance of building permits, must obtain Stormwater and Erosion Control permits from the Engineering Department
- Approval contingent final adoption of rezoning from IND to CHD



CONSULTANT

Name: Martin Riley, Attn.: Kim Greene Mailing Address: 404 N. Main St., Oshkosh, WI 54901

SITE PLAN REVIEW APPLICATION

Phone: 920-267-3600 Email: kgreene@martin-riley.com



PROPERTY OWNER	APPLICANT (IF DIFFERENT PARTY THAN OWNER)
Name:	Name:
VanEpern Family Trust, Attn: Roxanna Gebert	CoVantage Credit Union, Attn.: Bryan Ermeling
Mailing Address:	Mailing Address:
N2638 Bodde Rd., Kaukauna, WI 54130	PO Box 1986, Wausau, WI 54402-1986
Phone:	Phone:
920-427-9110	715-845-4351x4650
Email:	Email:
jandrgebert@sbcglobal.net	bermelin@covantagecu.org

PROPERTY INFORMATION

Describe the Proposed Project in Detail:

Construction plans for a bank with off-street parking.

Propety Parcel (#):

322111500

Site Address/Location:

101 E. Kelso Rd.

Current Zoning and Use:

IND - Agricultural

Proposed Zoning and Use:

CHD - Bank

Existing Gross Floor Area of Building:	Proposed Gross Floor Area of Building:
None	3479 SF
Existing Building Height:	Proposed Building Height:
None	25'
Existing Number of Off-Street Parking Spaces:	Proposed Number of Off-Street Parking Spaces:
None	30
Existing Impervious Surface Coverage	Proposed Impervious Surface Coverage Percentage:
Percentage:	9%
None	

I certify that the attached drawings are, to the best of my knowledge, complete and drawn in accordance with all City of Kaukauna codes.

berley Greene Cault, England of martin-day con, O-Mar Ring Church and Cault, England Grantin-day con, O-Mar Ring, Church and States Owner/Agent Signature:

Kimberley Greene Owner/Agent Name (printed):

CITY OF KAUKAUNA

SITE PLAN REVIEW PROCEDURE

The plan review process is required for all new commercial, industrial, or multifamily buildings, as well as for building expansions/additions to structures.

Early in the process, consult the site application checklist shown below for a complete list of plan requirements and contact staff in the Planning and Community Development Department for initial direction and assistance. In addition, it is your responsibility to notify utility companies regarding your proposed development.

Completed site plans must be submitted 14 business days prior to the intended Plan Commission meeting. Those plans will be distributed amongst various city departments for an initial review. After review, questions, comments, and requested revisions will be returned to the applicant in advance of the Plan Commission meeting.

SITE PLAN CHECKLIST

- ✓ Completed site plan application
- ✓ Completed erosion control and stormwater management permit application and necessary fees
- ✓ Calculations for sanitary sewer and water
- ✓ Calculations for storm sewer design
- ✓ Site plan set to include:
 - Site plan layout and streets, including designated fire lanes
 - Utilities, grading, and drainage plan
 - o Erosion control plan
 - Landscape and lighting plan
 - o Architectural elevation and construction details
 - Floor plan set
 - Any other plans or information deemed necessary by the Planning and Community Development Department

SITE PLAN SUBMISSION

- 1. Email the Planning and Community Development Department at planning@kaukauna.gov
- OR
- Drop off in-person or send by mail to City of Kaukauna, Attn: Planning and Community Development Department, 144 W. 2nd Street, Kaukauna, WI 54130



General Demoiltion Notes

1. LITTERING STREETS- THE CONTRACTOR SHALL REMOVE ANY DEMOLITION DEBRIS OR MUD FROM ANY STREET, ALLEY, RIGHT OF WAY RESULTING FROM THE EXECUTION OF THE DEMOLITION WORK. LITTERING OF THE SITE SHALL NOT BE PERMITTED. ALL WASTE MATERIALS SHALL BE PROMPTLY REMOVED FROM THE SITE.

2. STREET CLOSURES- IF IT SHOULD BECOME NECESSARY TO CLOSE ANY TRAFFIC OR PARKING LANES, CONTRACTOR SHALL BE RESPONSIBLE TO ACQUIRE NECESSARY PERMITS AND PLACE ADEQUATE BARRICADES AND WARNING SIGNS AS REQUIRED BY THE CITY OF KAUKAUNA and/or OUTAGAMIE COUNTY. STREET OR LANE CLOSURES SHALL BE COORDINATED WITH THE APPROPRIATE JURISDICTIONAL AUTHORITY.

3. GENERAL PROTECTION- WHERE APPLICABLE

A. SIDEWALKS- THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE TO PUBLIC SIDEWALKS, IF SCHEDULED TO REMAIN, ABUTTING OR ADJACENT TO THE PROJECT SITE. REPAIR OR REPLACEMENT IS THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE CONSIDERED INCIDENTAL TO THE WORK (REPLACEMENT PER THE CITY OF KAUKAUNA and/or OUTAGAMIE COUNTY STANDARDS).

B. PEDESTRIAN ACCESS/ VEHICULAR TRAFFIC- IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PLACE AND CONSTRUCT NECESSARY WARNING SIGNS, BARRICADES FENCING OR TEMPORARY ACCESS AS DIRECTED BY OWNER OR LOCAL AUTHORITY.

C. DEMOLITION HOURS- CONTRACTOR SHALL COMPLY WITH ANY RESTRICTIONS TO WORKING HOURS AS DIRECTED BY LOCAL AUTHORITY.

D. NOISE POLLUTION- ALL CONSTRUCTION EQUIPMENT SHALL BE IN GOOD REPAIR AND ADEQUATELY MUFFLED, OR AS DIRECTED BY LOCAL AUTHORITY

E. DUST CONTROL- THE CONTRACTOR SHALL TAKE APPROPRIATE ACTIONS TO MINIMIZE ATMOSPHERIC POLLUTION. SUCH PRECAUTIONS SHALL INCLUDE, BUT NOT LIMITED TO, USE OF WATER OR CHEMICALS FOR DUST CONTROL IN THE DEMOLITION OF BUILDING STRUCTURES, PAVING OR CLEARING OF LAND AND AS REQUIRED BY LOCAL AUTHORITY. OPEN-BODY TRUCKS LIKELY OF CREATING AIRBORNE DUSTS SHALL BE COVERED.

4. **REQUIREMENTS FOR THE REDUCTIONS OF FIRE HAZARDS-** THE CONTRACTOR SHALL BE RESPONSIBLE FOR HAVING AND MAINTAINING THE CORRECT TYPE AND CLASS OF FIRE EXTINGUISHER ON SITE. NO MATERIAL OBSTRUCTIONS OR DEBRIS SHALL BE PLACED OR ALLOWED TO ACCUMULATE WITHIN 15 FEET OF ANY FIRE HYDRANT.

5. **PROTECTION OF PUBLIC UTILITIES-** THE CONTRACTOR SHALL NOT DAMAGE EXISTING FIRE HYDRANTS, TRAFFIC SIGNALS, POWER POLES, TELEPHONE POLES, FIRE ALARM BOXES, WIRE CABLES AND/ OR UNDERGROUND UTILITIES TO REMAIN OR OTHER APPURTENANCES IN THE VICINITY OF THE SITE.

6. **PROTECTION OF ADJACENT PROPERTIES-** THE CONTRACTOR SHALL NOT DAMAGE OR CAUSE TO BE DAMAGED ANY PUBLIC RIGHT-OF WAY, STRUCTURES, PARKING LOTS, DRIVES, STREETS, SIDEWALKS, UTILITIES, LAWNS OR ANY OTHER PROPERTY ADJACENT TO THE PROJECT SITE.

7. GENERAL DEMOLITION NOTE- THE CONTRACTOR SHALL ACCEPT THE SITE IN ITS PRESENT CONDITION AND SHALL INSPECT THE SITE FOR ITS CHARACTER AND THE TYPE OF IMPROVEMENTS TO BE DEMOLISHED. THE DEMOLITION LIMITS SHALL BE RELEASED TO THE CONTRACTOR UPON AWARD OF CONTRACT AND NOTICE TO PROCEED. THE CONTRACTOR SHALL HAVE FULL CONTROL OF DEMOLITION PROGRESS AND CLEARANCE OF THE SITE, SUBJECT TO THE PROJECT MANUAL AND SPECIFICATIONS.

C Typical Site Demolition Notes

- SAWCUT EXISTING CONCRETE CURB, SEE DETAIL 8/C800
 CLEARING AND GRUBBING, TOPSOIL REMOVAL.
- DISTURBED WETLANDS.
 CONTRACTOR TO FIELD VERIFY DIMENSION OF EXISTING CASTING. REMOVE EXISTING CURB BOX & GRATE.

Note: ALL DEMOLISHED MATERIAL FROM CONSTRUCTION ACTIVITIES SHALL BE REMOVED OFF-SITE AND DISPOSED OF IN A LEGAL MANNER.











Oshkosh, Wisconsin 54901

www.martin-riley.com ph 920.267.3600







DATE: 2025-05-09

C101

DEMOLITION PLAN
			Point #	Northing	Easting	Elevation	Point Name	
			300	576811.380	868538.190	695.39	BM PN0644	
			303	582821.391	877764.049	681.78	BM DE7781	
			304	576591 /82	870663.055	710.62 696.97		W
			306	579251.028	867783.649	701.52	SC RRS	R
			307	579194.639	869458.065	694.62	СРТ РК	4
			308	579206.657	870177.713	696.91	СРТ РК	
			309	579187.763	869306.072	697.39	BM BURY BOLT	
			310	579197.937	868854.394	696.37	BM BURY BOLT	
			311	579231.406	869679.943	689.44	BM NAIL TOP CULVERT	
			312	579209.340	868616 025	094.70 706.71		
			313	579133 069	868498 478	700.71	СРТРК	
			315	579106.154	870282.125	705.48	BM NW BOLT ON SGN BASE	
			316	578734.830	869508.958	687.92	IP1 LEAN SHOT BASE	
			317	578772.282	868678.033	691.85	IP1	
			318	579087.857	870444.640	710.79	IP1	
			319	579156.502	868525.971	697.56	IP1	
			330	579158.478	870417.984	706.08	IP1	
			331	578684.372	868582.267	691.83 603.87	IP1	
			333	579200 152	868571 104	699.46	ABM RB6	
			340	579170.376	869887.541	689.20	ABM RB6	
			341	578883.051	869881.022	694.61	ABM RB6	
			342	579099.953	870440.100	709.69	ABM RB6	
			343	578729.808	869446.623	688.08	ABM RB6	
			344	578768.417	869566.933	688.85	ABM RB6	
			345	579114.756	869657.435	687.36	ABM RB6	
			346	570175 550	860657 794	00/.46		
			1011	579166 049	870079 179	0.00	RB6S	
			1012	578958.106	870074.473	0.00	RB6S	
	\		10688	578789.492	869472.024	691.30	СР	
			11215	579186.279	868517.499	698.65	CP UC1	
		M/님		м/ч			<i>.</i>	
		· · · ·				м/Я —	M/a	
\		371.2' TO CENTER OF INTER	SECTION					
/		F0	F0			0		.88'
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								25' FRONT
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	LAME STF (ASPHA			299	9.64'			
	LANNE STREE (ASPHALT)			299	9.64'			
	LANNE STREET (ASPHALT)			299	9.64'			
R/W	LANNE STREET (ASPHALT)			299	9.64'			
R/W	LAWE STREET (ASPHALT)			299	9.64'			
R/W	LAWE STREET (ASPHALT)			299	9.64' 299.4	5		
R/W	LAME STREET (ASPHALT)			299	9.64' 299.4	5		
R/W	LAME STREET (ASPHALT)			299	9.64' 299.4	5		
R/W	LAME STREET (ASPHALT)			299	299.4	5		
R/W	LAME STREET (ASPHALT)			299 30' SIDE	9.64'	5		
R/W	LANNE STREET (ASPHALT)			299 30' SIDE SETBACK	9.64' 299.4			
R/W R/W	LAME STREET (ASPHALT)			299 30' SIDE SETBACK	9.64'			
R/W R/W	LANNE STREET (ASPHALT)			299 30' SIDE SETBACK	9.64' 299.4	5		
R/W R/W	LAME STREET (ASPHALT)			299 30' SIDE SETBACK	9.64' 299.4	5		326.48'
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R/W	LAME STREET (ASPHALT)		Z CL	299 30' SIDE SETBACK	299.44		APPROX.75' STREAM SETBACK	326.48'



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PROPOSED SANITARY MANHOLE

EXISTING SANITARY MANHOLE

PROPOSED STORMSEWER MANHOLE

EXISTING STORMSEWER MANHOLE

GRASS SEED

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EX. ASPHALT PAVEMENT SECTION

STANDARD DUTY ASPHALT SECTION

PR. DETENTION BASIN

CONCRETE PAVEMENT SECTION

RIP RAP

LANDSCAPE STONE / PEA GRAVEL

MULCH / LANDSCAPE BED

↓ DELINEATED WETLANDS

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Credit Union Branch for Work CoVantage 101 E. Kelso Rd Kaukauna, WI 54130 Kaukauna ŭ New

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REVIEWED

DATE: 2025-05-09

BY:

Overall Site Layout Plan





DRAWN BY

KAG/CAS

O25006

OVERALL SITE LAYOUT PLAN



SKU: 7ZT7041 BK LENGTH: 42.25" HEIGHT: 36" FRAME: 1 ⁷/₈" DIAMETER TUBE

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General Construction Notes





GRASS SEED
EX. ASPHALT PAVEMENT SECTION
STANDARD DUTY ASPHALT SECTION
PR. DETENTION BASIN
CONCRETE PAVEMENT SECTION
RIP RAP
LANDSCAPE STONE / PEA GRAVEL

MULCH / LANDSCAPE BED

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PROPOSED SANITARY MANHOLE

EXISTING SANITARY MANHOLE

PROPOSED STORMSEWER MANHOLE

EXISTING STORMSEWER MANHOLE



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UTILITY PLAN





1. ENGINEERED SOIL SHALL BE A MIXTURE ACCORDING TO TABLE 1 AND PLACED IN MULTIPLE LIFTS @ 2. CONTRACTOR SHALL KEEP HEAVY EQUIPMENT OFF BIORETENTION AREAS TO PREVENT OVER COMPACTION OF SUBGRADE SOILS OF REMEDIATE SUBGRADE SOILS THOUGH CHISEL PLOWING OR DEEP TILLING PRIOR TO

6. GRAVEL SHALL BE DOUBLE WASHED AND MEET THE STANDARDS IDENTIFIED IN WDNR TECHNICAL STANDARD

7. SURFACE LAYER SHALL BE 2-3 INCHES OF SHREDDED HARDWOOD MULCH, AGED A MINIMUM OF 12 MONTHS, OR A WISDOT CLASS 2 TYPE B COCONUT FIBER EROSION CONTROL MAT. EROSION CONTROL MAT SHALL ALSO BE PLACED OVER THE HARDWOOD MULCH TO PREVENT MULCH FROM FLOATING. EROSION CONTROL



1.5" HMA SURFACE (4 LT 58-28 S) 3.0" HMA INTERMEDIATE (4 LT 58-28 S)

6" AGGREGATE BASE (³/₄" COMPACTED CRUSHED STONE) NOTE: 2-3" LIFTS

PREPARED SUBGRADE



















404 N. Main Street, Suite 601

LL IDEAS, DESIGNS, ARRANGEMENTS AND PLANS INDICATE ROPERTY OF MARTIN RILEY, INC., AND WERE CREATED, EV "ECIFIED PROJECT. NONE OF THE IDEAS, DESIGNS, ARRAM IN OR COMPORTION FOR ANY RIDBOSE WILATSOURCE VE

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B Fence Elevation
1" = 1'-0"
<u>ALTERNATE #1</u>





ENCLOSURE

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6 Signage Key Plan





4 LED Message Sign Elevations

O25006 CoVantage Kaukauna Branch 5/8/2025 4:50:03 PM C:\Users\GWynveen\D CD

32" BRICK

SIDE VIEW

.4









- LOGO TO BE PRINTED VINYL W/ GLOSS LAMINATE - COLORS: PMS 130c AND PMS 167c

- COLOR: PMS 2756c

- COLOR: 5650-010







FACE VIEW



SIDE VIEW





ltem 4.d.





C804

SITE SIGNAGE



- SHALL BE STABILIZED BY COVERING OR BY EQUIVALENT 6. FOR AREAS TO BE PERMANENTLY SEEDED, COMPLETE
- SEEDING WITHIN 24 HOURS OF REACHING FINAL GRADE. SEE EROSION CONTROL SPECIFICATIONS FOR TEMPORARY

-FABRIC DROP INLET PROTECTION (INITIAL) and -FABRIC BASKET INLET PROTECTION

-ROCK CHECK DAM PROTECTION

-EROSION CONTROL BLANKETS

EXISTING MINOR CONTOUR PROPOSED MAJOR CONTOUR





Ne



EROSION CONTROL PLAN



3. WOOD OR METAL STAKES TO SECURE THE STRAW BALES (2 PER BALE) 4. ALTERNATIVE MATERIALS CAN BE SUBSTITUTED FOR THE STRAW BALES TO PROVIDE STRUCTURAL CONTAINMENT WITH PRIOR APPROVAL FROM CITY UTILITIES PROGRAM MANAGER.

THE CONCRETE WASHOUT SIGN SHALL BE INSTALLED WITHIN 10 FT. OF THE TEMPORARY CONCRETE

- HOLD POLYETHYLENE LINING DOWN.
- PREFABRICATED WASHOUT SYSTEMS MAY BE USED WITH PRIOR APPROVAL FROM PROJECT ENGINEER.
- 5. SANDBAGS, GRAVEL-FILLED BAGS OR OTHER APPROPRIATE ANCHORING SYSTEM SHALL BE USED TO

- 7. THE SYMBOL "*" INDICATES A DIMENSION THAT IS TO BE DESIGNED BASED ON EXPECTED QUANTITY OF

Temporary Concrete Washout

- MATERIAL
- 2. THE POLYETHYLENE LINER SHALL BE SECURED TO STRAW BALES WITH METAL PINS OR STAPLES.

BLACK LETTERS OF 6" HEIGHT PLYWOOD — 4'-0" X 2'-0" PAINTED WHITE CONCRETE WASHOUT

CONCRETE WASHOUT

SIGN DETAIL

ABBREVIATIONS: *L = INSIDE LENGTH

*W = INSIDE WIDTH

(SEE NOTE 2) TOP-MOST STRAW BALES STRAW BALES (SEE NOTE 4) SANDBAGS WOOD OR (SEE NOTE 5) METAL STAKES ---10 MIL (SEE NOTE 3) POLYETHYLENE LINER SECTION A-A

OR STAPLES -

SANDBAGS

(SEE NOTE 5)

<u>PLAN</u>

(SEE NOTE 2)

10 MIL POLYETHYLENE LINING TO

WRAP OVER AND AROUND -

METAL PINS

OR STAPLES –

WOOD OR

METAL STAKES

(SEE NOTE 3)

STRAW BALES

(SEE NOTE 4)

WASHOUT UNIT.

2

INSET

16 GAUGE STEEL STAPLE DETAIL

OVERFLOW OPENING

(FOR INLETS w/ CURB

BOXES)

REBAR

FLAP POCKET

(SEE NOTE #5)

(OR EQUIVALENT)

- OVER AND REINFORCE WITH MULTIPLE STITCHES. 5. FLAP POCKETS SHALL BE LARGE ENOUGH TO ACCEPT WOOD 2" x 2". THE REBAR, STEEL PIPE, OR WOOD SHALL BE INSTALLED IN THE REAR FLAP AND SHALL NOT BLOCK THE TOP HALF OF THE CURB FACE OPENING.
- 4. SIDE FLAPS SHALL BE A MAXIMUM OF TWO INCHES LONG. FOLD THE FABRIC
- FILTER BAG.
- FRONT, BACK, AND BOTTOM BEING ONE PIECE. 3. FRONT LIFTING FLAP IS TO BE USED WHEN REMOVING AND MAINTAINING
- BETWEEN THE BAG AND THE STRUCTURE, MEASURED FROM THE BOTTOM OF THE OVERFLOW OPENINGS TO THE STRUCTURE WALL. 2. GEOTEXTILE FABRIC, TYPE FF FOR FLAPS AND TOP HALF OF FILTER BAG. GEOTEXTILE FABRIC, TYPE HR FOR BOTTOM HALF OF FILTER BAG WITH
- NOTES: 1. TAPER BOTTOM OF BAG TO MAINTAIN THREE INCHES OF CLEARANCE

- - FRONT LIFTING FLAP (SEE NOTE #3)
- TYPE FF GEOTEXTILE FABRIC (SEE NOTE #2)

INLET SHALL BE IMMEDIATELY REMOVED.

4 Temporary Inlet Protection Sediment Control Sack

Note 1 Use hard, durable, angular stone or recycled concrete meeting the gradation in Table 1. Where this gradation is not available, meet the gradation in Wisconsin Department of Transportation (DOT) 2018 Standard Specification, Section 312, Select Crushed Material.

Note 2 Slope the stone tracking pad in a manner to direct runoff to an approved treatment practice.

Note 3 Select fabric type based on soil conditions and vehicles loading.

Note 4 Install tracking pad across full width of the access point, or restrict existing traffic to a dedicated egress lane at least 12 feet wide across the top of the pad.

Note 5 If a 50' pad length is not possible due to site geometry, install the maximum length practicable and supplement with additional practices as needed.

Temporary Construction Entrance

Union Credit Branch for と Φ Vor Kelso Rd una, WI 54130 **CoVantage** Kaukauna Str පි 101 E. Kaukai New CoVantage CREDIT UNION MARTINRILEY architects • engineers 404 N. Main Street, Suite 601 www.martin-riley.com ph 920.267.3600 Oshkosh, Wisconsin 54901 MARTINRILEY DOCUMENT IN PROGRESS NOT FOR CONSTRUCTION REVISION REVIEWED DRAWN BY KAG/CAS BY: DATE: MMISSION 025006 2025-05-09

ltem 4.d.

153

PLANT LIST

General Construction Notes

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EXISTING SANITARY MANHOLE

PROPOSED STORMSEWER MANHOLE

EXISTING STORMSEWER MANHOLE

GRASS SEED

EX. ASPHALT PAVEMENT SECTION

STANDARD DUTY ASPHALT SECTION

PR. DETENTION BASIN

CONCRETE PAVEMENT SECTION

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LANDSCAPE STONE / PEA GRAVEL

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Union Credit Branch for Work coVantage . Kelso Rd iuna, WI 54130 Kaukauna 101 E. Kaukau Õ New \bigcirc

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-AA MARTINRILEY DOCUMENT IN PROGRESS NOT FOR CONSTRUCTION **REVISION:**

DRAWN BY: REVIEWED KAG/CAS COMMISSION O25006 DATE: 2025-05-09 L100

SITE LANDSCAPE PLAN

154

Site Landscape Plan

		Ge	eneral Co	nstructi	on Notes					
		1. AL COUI 2. AL 3. CO BUILI WALI ACTI SHAL ADDI 4. PR CON MEAS ON-S 5. CO GRAI PATH 6. CO ADJA 7. CO TOPS SEEL 8. CO TRAI	LL WORK TO BE F NTY AND LOCAL L PERMITTING FE DNTRACTOR SHA DINGS, INFRASTF KS, GRASS, ETC IVITIES. ANY DAM LL BE REPAIRED/ ITIONAL COST. RIOR TO THE STA TRACTOR SHALL SURES TO ENSUI SITE OR PUBLIC S DNTRACTOR SHA ACENT TO THE W DNTRACTOR SHA SOIL, IF REQUIRE DING AND MULCH DNTRACTOR SHA DONTRACTOR SHA DONTRACTOR SHA	PERFORMED I CODES INCLU EES SHALL BE LL PROTECT RUCTURE, PA DURING DEM IAGE CAUSED REPLACED B RT OF CONST PLACE APPR RE NO SEDIM STORM SYSTE MATCH CONS DE POSITIVE D LL ADJUST AI ORK. LL RESTORE E, GRADING I. LL BE RESPO INTY JURISDI	N ACCORDANCE W JDING ALL AMENDM E PAID FOR BY THE ALL ADJACENT IMF VEMENTS, PAVEME OLITION AND CONS D BY CONSTRUCTION Y THE CONTRACTO TRUCTION ACTIVITI OPRIATE EROSION ENT LEAVES THE S EMS (SEE C900). TRUCTION LIMITS TO ORAINAGE TO EXIS LL CASTINGS TO GI ALL DISTURBED AF TO ESTABLISH POS NSIBLE FOR COOR CTIONS AND UTILIT	ATTIC AND A CONTRACTOR. A CONTRACTOR. A CONTRACTOR. A CONTRACTOR. A CONTRACTOR. A CONTROL A CTIVITIES A CONTROL B		dit Union	ch	
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	SCIENTIFIC NAME CORNUS RACEMOSA	NATIVE/ ADAPTED SOIL Y DRY	. MOISTURE Y - MOIST	SIZE 1 1/2" - 2" CALIF	ROOT ER B&B	NOTES 35' O.C. MIN.	ALL IDEAS, DESIGNS, ARRA PROPERTY OF MARTIN RILE SPECIFIED PROJECT. NOIN FIRM OR CORPORATION FC DIMENSIONS ON THESE DR RESPONSIBLE FOR ALL DIM DIMENSIONS AND CONDITI THE LIMITED PURPOSE OF CONTRACT DOCUMENTS BI	NGEMENTS AND PLANS INDICAT Y, INC., AND WERE CREATED, EV E OF THE IDEAS, DESIGNS, ARRAI RANY PURPOSE WHATSOEVER I AWINGS SHALL HAVE PRECEDEN HENSIONS AND CONDITIONS ON T ONS SHOWN BY THESE DRAWING CHECKING FOR CONFORMANCE EFORE PROCEEDING WITH FABRI	ED OR REPRESENTED BY OLVED, AND DEVELOPEI VIGENENTS OR PLANS SY THOUT THE WRITTEN I CE OVER SCALE DIMENS E JOB AND THIS OFFICI S. SHOP DETALS MUST WITH INFORMATION GIVE CATION.	¹ THIS DRAWING ARE OWNED BY, AND THE D FOR USE ON, AND IN CONNECTION WITH THE HALL BE USED BY OR DISCLOSED TO ANY PER PERMISSION OF MARTIN RIEV, INC. WRITTEN IONS. CONTRACTORS SHALL VERIFY AND BE SUBST BE NOTFIED OF ANY VARIATION ROM BE SUBMITTED TO THIS OFFICE FOR REVIEW I NAND THE DESIGN CONCEPT EXPRESSED IN
RBORVITAE	SCIENTIFIC NAME THUJA OCCIDENTALIS	NATIVE/ ADAPTED SOIL Y DRY	. MOISTURE Y - MOIST	SIZE 1' HT. MIN.	ROOT B & B	NOTES 3' O.C. MIN.				
	SCIENTIFIC NAME RUDBECKIA HIRTA	NATIVE/ ADAPTED SOIL Y DRY	MOISTURE Y, MEDIUM, MOIST	SIZE 15" HT. MIN.	ROOT CONTAINER	NOTES 12" - 18" O.C. MIN.		MAR DOCUMENT NOT FOR C	TINRILEY F IN PROG ONSTRU	GRESS CTION
	AQUILEGIA CANADENIS ASARUM CANADENSE	Y MED	י, meduum DIUM	тэ нт. міN. 4" НТ. МІN.	CONTAINER	12" - 18" O.C. MIN. 12" - 18" O.C. MIN.	REVISION:		DATE:	

SITE LANDSCAPE PLAN

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	Lumens Per Lamp	Light Loss Factor	Wattage
P1 T 80 CRI	6968	0.9	50.9

Plan View Scale - 1" = 30ft

Statistics					
Description	Symbol	Avg	Max	Min	Max/Min
East TRespass	+	0.0 fc	0.0 fc	0.0 fc	N/A
Front Parking Lot	+	1.1 fc	1.7 fc	0.3 fc	5.7:1
North Trespass	+	0.0 fc	0.0 fc	0.0 fc	N/A
Rear Parking Lot	+	0.7 fc	1.5 fc	0.2 fc	7.5:1
South Trespass	+	0.0 fc	0.0 fc	0.0 fc	N/A
West Trespass	+	0.0 fc	0.0 fc	0.0 fc	N/A

ELECTRICAL SITE PHOTOMETRIC PLAN Item 4.d.

AREA MAP

CoVantage Credit Union Kaukauna Branch

101 E Kelso Rd Kaukauna, WI 54130

INDEX OF DRAWINGS

T101	TITLE SHEET

		A101	FLOOR PLAN, WALL TYPES, MISC DETAILS
C101	DEMOLITION PLAN	A201	ELEVATIONS/DOOR SCHEDULE/WINDOW EL
C200	OVERALL SITE LAYOUT PLAN	A301	REFLECTED CEILING PLANS/DETAILS
C201	SITE LAYOUT PLAN	A401	BUILDING SECTION/WALL SECTIONS
C300	GRADING PLAN	A402	WALL SECTIONS/DETAILS
C400	UTILITY PLAN	A403	WALL SECTION DETAILS
C800	SITE DETAILS	A410	DRIVE-UP ITM CANOPY
C801	SITE DETAILS	A501	DETAILS
C802	SITE DETAILS/SIGNAGE/TRASH ENCLOSURE	A601	FINISH PLAN/SCHEDULE
C900	EROSION CONTROL PLAN	A602	INTERIOR ELEVATIONS AND DETAILS
C901	POST CONSTRUCTION EROSION CONTROL PLAN	A603	INTERIOR ELEVATIONS AND DETAILS
C902	EROSION CONTROL DETAILS	M201	MECHANICAL PIPING PLAN
L100	LANDSCAPE PLAN	R101	ROOF PLAN/DETAILS
L101	LANDSCAPE PLAN		
		M101	MECHANICAL PLAN
S001	STRUCTURAL SPECIFICATIONS	M301	MECHANICAL SCHEDULES AND DETAILS
S002	STRUCTURAL SPECIFICATIONS		
S101	FOUNDATION PLANS	E001	ELECTRICAL SITE PLAN
S102	FOUNDATION PLANS	E101	ELECTRICAL POWER PLAN
S201	FRAMING PLANS	E201	ELECTRICAL LIGHTING PLAN
S202	FRAMING PLANS	E301	IT, SECURITY, & AUDIO PLAN
S203	HEADER PLANS & SCHEDULES	E501	ELECTRICAL SCHEDULES & DETAILS
S301	STRUCTURAL SECTIONS		
S302	STRUCTURAL SECTIONS	P101	PLUMBING PLAN
S401	FOUNDATION DETAILS	P201	ENLARGED PLUMBING PLANS
S402	FOUNDATION DETAILS	P301	PLUMBING SCHEDULES & DETAILS

A001 LIFE SAFETY PLAN

P302 PLUMBING ISOMETRICS

- S402 FOUNDATION DETAILS S501 STRUCTURAL DETAILS

DULE/WINDOW ELEVATIONS

157

O25006 CoVantage Cr Kaukauna Branch 4/25/2025 10:17:04 AM C:\Users\GWynveen\Doc CD

LEGEND	
PATH OF EGRESS	$\bullet \rightarrow \rightarrow \rightarrow -$
DRAFT STOP	
SMOKE PARTITION	— · — · — · —
SMOKE BARRIER	· · · · · ·
SHAFT WALL	
FIRE PARTITION	
FIRE WALL	
FIRE BARRIER	

NORTH

			Item -
Code Summary Code: Wisconsin Building Code - 2018 Edition (International Building Code, 2015 Edition, with Wisconsin Amendments) Wisconsin Fire Code - 2014 Edition Plumbing Code - 2012 Edition International Mechanical Code - 2015 Edition International Fuel Gas Code - 2012 Edition Electrical Code - 2009 Edition			
Electrical Code - 2009 Edition International Energy Code - 2010 Edition Occupancy Classifications: B: Office and Administrative [304.1] B: Accessory [304.1] Construction Type: Type VB Construction - Two or more buildings on the same lot are within the limits of Table 503 [503.1.2] Building Areas: Total Existing: 0 Square Feet New construction: Office 3,479 Square Feet New construction: Office 3,479 Square Feet Total Complete: 3,479 Square Feet Addition: 0 Square Feet Addition: 0 Square Feet Addition: 0 Square Feet Total Complete: 3,479 Square Feet Allowable Height & Area: Allowable area per floor: 15,750 square feet Frontage increase allowed due to 60 feet wide open space surrounding building [506] Allowable height of 2 stories / 40 feet [T503] Building Elements: Structural Frame - Any material permitted by code [T601, 602.5] Bearing Walls - Any material permitted by code [T601, 602.5] Nonbearing walls (Exterior) - Any material permitted by code [T601, 602.5]	onstruction Work for :	/antage Credit Union Jkauna Branch	Kelso Rd una, WI 54130
Nonbearing walls (Interior) - Any material permitted by code [T601, 602.5] Roof Assembly - [T601] Exterior Walls - Any material permitted by code [T601, 602.5] Occupancy Separations: Separations not required between S & B Occupancies based upon nonseparated occupancies. [508.3] Incidental Uses: Incidental Uses: Incidental use - No separations required [T509] Boiler/Mechanical Room: Equipment is under 400,000 btu's and boiler pumps are less than 10 hp and system less than 15 psi [T509]	New C	O E CoVa C R E D I T	и и и и и и и и и и и и и и и и и и и
Lgress. Maximum travel distance to an exterior exit: 200' [1016.2] Occupant Load: 1 occupant per 100 gross sf [T1004.1.2] Occupant Load: 1 st floor - 34 occupants (calculated) Accessible means of egress: Yes [1007.1, EX1] Door Swing in direction of egress: Yes [1008.1.2] Enclosed Stair: Not Applicable [1009.3, EX1] Maximum Travel Distance: 200' [T1016.2] Maximum Common Path of Egress Travel: 75' [T1014.3] Corridor Fire Resistance: 0 0 Hour [T1018.1] Corridor Minimum width: 36" 36" [T1018.2] Automatic Sprinklers: Automatic sprinklers are not required based upon an area building and occupant load. [903.2.1, 507.4, Table 903.2]			
Not required [907.2.1, 907.2.2, Table 907.2] Fire Alarm: Not Required Not Required [907.2.1, 907.2.2, Table 907.2] Attic Draftstopping: Requirement - subdivide attic into areas <3000 SF	404 N. Main Str Oshkosh, WI 54	RTINF itects - eng reet, Suite 601 v 4901	RILEY gineers vww.martin-riley.com pho 920.267.3600
	ALL DEAS, DESIGNS, ARRANGEME PROPERTY OF MARTIN RLEY, NC. SPECIFED PROJECT. NONE OF TH FIRM OR CORPORTION FOR ANY DMENSIONS ON THESE DRAWING RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS SH THE LIMITED PURPOSE OF CHECK CONTRACT DOCUMENTS BEFORE	ENTS AND PLANS INDICATED OR REPRESENTED BY TH AND WERE CREATED, EVOLVED, AND DEVELOPED F IE DEAS, DESIGNS, ARRANGEMENTS OR PLANS SHAL DURPOSE WHATSOEVER WITHOUT THE WRITEN PEI S SHALL HAVE PRECEDENCE OVER SCALE DIMENSIO IN SAND CONDITIONS ON THE JOB AND THIS OFFICE M OWN BY THESE DRAWINGS. SHOP DETALS MUST BE IN 6 FOR CONFORMANCE WITH INFORMATION GIVEN A PROCEEDING WITH FABRICATION.	IS DRAWING ARE OWNED BY, AND THE OR USE ON, AND IN CONNECTION WITH THE LIE USED BY OR DISCLOSED TO ANY PERSON, MISSION OF MARTIN RILEY, NO. WITTEN S. CONTRACTORS SHALL VERFY AND BE SUBMITTED TO THIS OFFOE FOR REVIEW FOR ND THE DESIGN CONCEPT EXPRESSED IN THE
EXIT	REVISION:	DOCUMENT IN PRO NOT FOR CONSTRU	GRESS
	DRAWN BY: GW COMMISSION NUMBER: 0250	REVIEWE BY: DATE:	^D NH 2025-04-24

LIFE SAFETY PLAN

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FLOOD LIGHT AT ENDS OF

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CoVantage Credit Union Kaukauna Branch - for Work : Kelso Rd auna, WI 54130 101 E Kaukai New

Item 4.d.

A401

O25006

NUMBER:

DATE: 2025-04-24

BUILDING SECTION/WALL SECTIONS

162

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Credit Union

CoVantage

- -

for

ction Work

New

Kaukauna Branch

101 E Kelso Rd Kaukauna, WI 54130

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166

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ALIGN T	THE SIDE WALL TILE JOINTS WITH OPPOSITE SIDE WALL TILE JOINTS IN VESTIBULE 100. C	CENTER WALL HEATER IN TILE.
NISH	ILEGEND	
(A) CE 1. 2.	ILING USG "MARS" ACOUSTICAL PANELS WITH CLIMAPLUS PERFORMANCE #86985 a. SIZE: 2' x 2' x 3/4" b. EDGE: FLB b. SUSPENSION SYSTEM: DONN CENTRICITEE DXT; COLOR: WHITE PAINTED GYPSUM BOARD a. COLOR: SHERWIN WILLIAMS SW7004 SNOWBOUND (PNT-5) b. FINISH: FLAT	 (MISC) MISCELLANEOUS CONTINUED 9. FIBERGLASS REINFORCED PLASTIC PANELS a. MANUF: CRANE COMPOSITES b. STYLE: GLASBORD FRP PANEL c. COLOR: NO. 85 WHITE d. TRIM: 10. SCHLUTER SYSTEMS ALUMINUM TRIM a. QUADEC: 5/16" b. IOL M: 210"
3.	USG "SHEETROCK BRAND" LAY-IN CEILING PANEL CLIMAPLUS #3260 a. SIZE: 2' x 2' x 1/2" b. EDGE: SQUARE b. SUSPENSION SYSTEM: DONN CENTRICITEE DX/DXL; COLOR WHITE	c. DECO-SG: 3/8" d. SCHIENE: AE100 e. FINISH: SATIN ANODIZED ALUMINUM (PT) PAINT
(B) BA	SE JOHNSONITE 4" THERMOPLASTIC VINYL 1/8" (TYPE TV) COVE BASE a. COLOR: STERLING SILVER #69 b. ADHESIVE: MANUFACTURER RECOMMENDED	 LOBBY WALLS & SOFFITS MANUF: SHERWIN WILLIAMS COLOR: KNITTING NEEDLES SW7672 FINISH: EG-SHEL
2.	 CEASAR CERAMICS a. PATTERN: FOUNDRY 26 b. COLOR: OXIDE c. SIZE: 4" x 12" d. TRIM: SCHLUTER-SYSTEMS "JOLLY" AT TOP OF TILE; SATIN ANODIZED ALUMINUM 	 LOBBY WALLS & CURVED SOFFIT MANUF: SHERWIN WILLIAMS COLOR: HONORABLE BLUE SW6811 FINISH: EG-SHEL OFFICE WALLS
(BRK) E (CONC)	 e. GROUT COLOR/JOINT: CUSTOM BUILDING PRODUCTS; COLOR: WINTER GRAY #335 WITH 1/8" GROUT JOINT BRICK) SEALED CONCRETE a. STANDARD WATERBORNE, LOW GLOSS CURING AND SEALING COMPOUND 	 a. MANUF: SHERWIN WILLIAMS b. COLOR: PUSSYWILLOW SW7643 c. FINISH: EG-SHEL 4. DOOR FRAMES & VESTIBULE a. MANUF: RUSTOLEUM
(CPT) 1.	CARPET J&J FLOORING "EVOLVE" CARPET TILE #7981 a. SIZE: 24" x 24" b. COLOR: SURGE #1652 c. BACKING: NEXUS d. INSTALLATION: QUARTER TURN e. ADHESIVE: J&J FLOORING "TILETABS"	 b. COLOR: ALUMI-NON c. FINISH: SEMI-GLOSS d. NOTE: PROVIDE A MOCK UP AREA OF PAINT ON TEXTURED IN VESTIBULE; TO BE APPROVED BY OWNER BEFORE PROC 5. CEILING a. SHERWIN WILLIAMS b. COLOR: SNOWBOUND SW7004 c. FINISH: FLAT (SEMI-GLOSS IN RESTROOMS)
2. (MISC)	FLOR "TUXEDO POCKET" AREA RUG a. SIZE: 19.7" x 19.7". SEE A8.0 b. COLORS: MARIGOLD, COLBALT AND BONE/GOLD (SEE DETAIL 4/A601) c. NOTE: TAPE SEEMS MISCELLANEOUS DI ACTIO LAMINATE FACED CUSTOM CADINETS	 (PL) PLASTIC LAMINATE 1. CABINETS & SHELVES a. MANUF: NEVAMAR HPL b. COLOR: CLEAR MAPLE WM8340-T c. FINISH: ARP
2.	 a. DETAILS AND FINISH: SEE ELEVATIONS ON SHEET A602. b. SPECIFICATIONS: REFER TO PROJECT MANUAL MDC "DIMENSION WALL" TEXTURED WALL PANEL a. PATTERN: LINE THEM UP; DWP3408/4559 b. COLOR: SILVER c. SIZE: 4'x8' d. TRIM: SCHLUTER "QUADEC" 5/16" IN SATIN ANODIZED ALUMINUM 	 EMPLOYEE COUNTERTOP & WINDOW SILLS MANUF: PIONITE HPL COLOR: COOKIES & CREAM AG130-SM FINISH: SUPER MATTE TELLER ASSISTANT MANUF: NEVAMAR COLOR: SUMMER NIGHT GLOW S-8003-T (THRUCOLOR) EINISH: ADD
3.	 e. NOTE: SEE ELEVATION 1/A603 FOR PANEL LAYOUT AND TRIM LOCATIONS MDC "DIMENSION WALL" TEXTURED WALL PANEL a. PATTERN: LINE THEM UP VERTICAL; DWP4308XL b. COLOR: SILVER c. SIZE: 4'x8' d. TRIM: SCHLUTER, QUADEC 5/16" IN SATIN ANODIZED ALUMINUM e. NOTE: SEE ELEVATION 1/A603 FOR PANEL LAYOUT AND TRIM LOCATIONS f. INSTALLATION: RAILROAD TO MINIMIZE SEEMS 	 c. FINISH: ARP 4. TELLER ASSISTANT a. MANUF: PIONITE "THRUCOLOR" HPL b. COLOR: FORSYNTHIA SY913-SD c. FINISH: TEXTURED/SUEDE 5. TELLER ASSISTANT BASE a. MANUF: CHEMETAL b. EINISHED: BPLISHED ALLIMINUM #702
4.	 LUMICOR "LUMINOUS" ACRYLIC PANEL a. COLOR: 2 LAYERS OF SUNFLOWER & 1 LAYER OF SUNSET b. THICKNESS: 1/2" c. ADD-ONS: ILLUME d. NOTE: ALL EXPOSED EDGES TO BE FINISHED; CANOPY TO HAVE SQUARED EASED EDGES; CABLE SUPPORTS BY ACRYLIC PANEL MANUFACTURER; SEAM PANEL INTERSECTIONS. SEE ELEVATIONS AND DETAILS ON SHEETS A602 AND A603 FOR QUANTITY AND LOCATIONS (VERIFY THAT MEETS MANUFACTURER'S BECOMMENDATIONS) 	 6. VESTIBULE WINDOW SILLS a. MANUF: NEVAMAR HPL b. COLOR: SILVER DOLLAR AG8000-SD c. FINISH: TEXTURED/ SUEDE d. SEE SHEET A402 FOR DETAILS (T) TILE 1. ELOOP THE
5.	MOZ DESIGNS "LASER CUT METAL SHEET" a. PATTERN: REEDS b. COLOR: "BLENDZ PATINA COLLECTION"- #162 c. GRAIN: FOG d. FINISH: POLYCOAT MATTE e. PANEL SIZE(S): SEE 7A/A602	 a. MANUF: CAESAR CERAMICS b. PATTERN: FOUNDRY 26 c. COLOR: OXIDE d. SIZE: 24" x 24" x 9MM THICK; SEE FINISH PLAN FOR TILE LAY e. GROUT/JOINT: CUSTOM BLDG PRODUCTS; COLOR: DRIFTW WITH 1/8" GROUT JOINT f. TRIM: SCHLUTER-SYSTEMS IN SATIN ANODIZED ALUMINUM
6. 7.	COAK SHELF a. REESE #536 RUBBER HINGE MAT WITH CARPET INSERTS a. RAILS: VINYL-ACRYLIC TREAD RAILS 2" WIDE BY 3/8" THICK WITH SLOTTED OR PERFORATED VINYL HINGES b. TREAD INSERTS: 1/4" HIG, 28 OZ/SQ YD WEIGHT, LEVEL-CUT NYLON, FUSION BONDED CARPET c. CARPET INSERT COLOR: GREY d. VINYL COLOR: BROWN e. SERRATED COLOR: CLEAR f. SEE DETAIL 2/A601	 WALL TILE AND WINDOW SILLS MANUF: CASALGRANDE PADANA PATTERN: METALLICA COLOR: INOX LAPPATO SIZE: 23-5/8" x 23-5/8" GROUT/JOINT: CUSTOM BLDG PRODUCTS; COLOR: PEWTEF 1/8" GROUT JOINT TRIM: SCHLUTER JOLLY AT EXPOSED TILE EDGE AND DECO CHANNEL TRIM; ALL IN SATIN ANODIZED ALUMINUM; SEE EI 7S/A602 FOR DETAILS. SEE 6/A402 FOR WINDOW SILL DETAIL
8.	 ADA COMPLIANT INTERIOR PANEL SIGNAGE (RESTROOMS) a. MANUF/PRODUCT: INPRO CORP "PHOENIX"; 8" x 8" ADA RESTROOM SIGN (NO BACKPLATE) b. STYLE: TACTILE AND BRAILLE SIGN COMPLYING WITH ADA-ABA ACCESSIBILTY GUIDELINES AND WITH ICC/ANSI A117.1. b. TACTILE CHARACTERS: CHARACTERS AND TEXT SHALL BE ACCOMPANIED BY GRADE 2 BRAILLE RAISED 1/32" ABOVE SURFACE WITH CONTRASTING COLORS c. MATERIAL: ACRYLIC d. FONT: HELVETICA REGULAR e. FACE COLOR: ELM GRAY 0388 f. TEXT COLOR: BLACK 29920 	

GENERAL NOTES

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- FLOOR COVERING TRANSITIONS SHALL BE CENTERED BELOW DOORS IN THE CLOSED POSITION. FLOORING SHALL EXTEND WALL TO WALL, UNDER EQUIPMENT.
- SEE FINISH PLANS FOR CERAMIC TILE (T-1) PATTERN DETAILS. NO BASE AT BRICK WALLS.
- REFER TO PROJECT MANUAL FOR FINISH MATERIALS SPECIFICATIONS.
- REFER TO REFLECTED CEILING PLAN A301 FOR CHANGES IN CEILING HEIGHTS AND MATERIALS. WALL TILE AND FLOOR TILE JOINTS ARE TO ALIGN. GC TO COORDINATE BEFORE INSTALLATION.

	vvail	Finish				
	South	East	West	Ceiling Finish	Ceiling Height	Remarks
	SEE DETAIL 7S/A602	SEE DETAIL 7W/A602	SEE DETAIL 7W/A602	A-2	SEE A301	1,2,3,6
‡3	SEE DETAIL 1/A603	PT-2	PT-2	A-1/A-2	SEE A301	1,2,3
	PT-3	PT-3	PT-3	A-1	SEE A301	
	PT-3	PT-3	PT-3	A-1	SEE A301	
	PT-3	PT-3	PT-3	A-1	SEE A301	
	PT-3	PT-3	PT-3	A-1	SEE A301	
	PT-3	PT-3	PT-3	A-1	SEE A301	
	PT-3	PT-3	PT-3	A-1/A-2	SEE A301	4
	PT-3	PT-3	PT-3	A-1	SEE A301	2
	PT-3	PT-3	PT-3	A-2	SEE A301	
	PT-3	PT-3	PT-3	A-2	SEE A301	
	PT-3	PT-3	PT-3	A-2	SEE A301	
	PT-3	PT-3	PT-3	A-2	SEE A301	
	PT-3	PT-3	PT-3	A-2	SEE A301	
	PT-3	PT-3	PT-3	A-2	SEE A301	
	PT-3	PT-3	PT-3	A-1	SEE A301	2
	PT-3	PT-3	PT-3	A-1	SEE A301	
	PT-3	PT-3	PT-3	A-3	SEE A301	5
	PT-3	PT-3	PT-3	A-3	SEE A301	5
	PT-3	PT-3	PT-3	A-1	SEE A301	
	PT-3	PT-3	PT-3	A-1	SEE A301	

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FINISH PLAN/SCHEDULE

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HIGH DENSITY NEOPRENE FOAM CLOSURE BETWEEN ITM CABINET AND ADJACENT

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INTERIOR ELEVATIONS AND DETAILS

- Roof Work Description Notes
 PROVIDE NEW PLYWOOD SHEATHING
- 2 PROVIDE NEW CONTINUOUS SELF ADHERING (BUTYL BASED) WATERPROOFING UNDERLAYMENT EXTENDING 24" PASSED INSIDE OF EXTERIOR WALL MINIMUM UP SLOPE AT EAVES, 36" WIDE AT RIDGES, 72" WIDE AT VALLEYS, 36" WIDE AT VERTICAL TRANSITIONS, 36" WIDE AT PENETRATIONS
- 3 PROVIDE NEW (2) LAYERS 30# FELT UNDERLAYMENT - EXTEND OVER SELF ADHEREING (BUTYL BASED) WATERPROOFING UNDERLAYMENT
- 4 PROVIDE NEW ARCHITECTURAL LAMINATE MODIFIED ASPHALT SHINGLE
- 5 PROVIDE NEW STAINLESS STEEL COMPRESSION DRAW BAND
- 6 NEW ROOF PENETRATION
- 7 PROVIDE NEW RUBBER PIPE BOOT WITH SPUN ALUMINUM BASE

NET FREE VENTILATING AREA (NFVA) CALCS

(Sq. ft. of attic space) / 300 = Total min. sq. ft. of NFVA needed 3074 / 300 = 10.25 ft² (1475 in²)

Intake ventilation required = (1/2) NFVA = 737.5 in² Exhaust ventilation required = (1/2) NFVA = 737.5 in²

Ridge vent = **18 in²/lf** x 5 lf = 90 in² Hip vent = **9 in²/lf** x 65.75 lf = 591.75 in² Louver Vent = 60 in²

Total designed ventilation = 741.75 in²

Note: Provide ridge and hip vents with NFVA greater than or equal to those used in vent calcs.

6 Roof Plan - Tower

SEE 3/A410 FOR DRIVE-UP ROOF PLAN

ROOF PLAN/DETAILS

ltem 4.d.

MEMO

PLANNING AND COMMUNITY DEVELOPMENT

To:	Plan Commission
From:	Dave Kittel, Director of Planning and Community Development
Date:	5/16/2025
Re:	Site Plan Review- 1800 Crooks

On December 5th, 2024 the Plan Commission approved a site plan for the redevelopment of 1800 Crooks. The site plan was for the demolition of the old Family video building and then building a Taco Bell on the property. After some discussions with the adjacent commercial property the Taco Bell developers have agreed to make some minor changes to the site plan to help both entities. The new updates are attached to the memo for review. The Changes that are proposed are to allow for some angle parking that currently exists to remain and a slightly different dumpster setup. These changes allow for a shared dumpster facility and help maintain the parking available onsite to prevent on street parking for uses at the adjacent property. The link below will take you to the meeting agenda when this was originally discussed to be able to view the original plans as well.

https://meetings.municode.com/adaHtmlDocument/index?cc=KAUKAUNAWI&me=487890 4af1624f019309b4028bdf5a6d&ip=True

Staffs only comment on the updated plan is to possibly eliminate one or two of the angled parking stalls nearest to Crooks Ave to help prevent a conflict point when a car is backing out of the stall. This condition existed previously on site as well see image below for clarity:

Staff Recommendation:

To review the updated site plan and approve the updated plans with one stall of angle parking being removed to ease potential traffic conflict.

NEW BUILDING FOR: PACIFIC BELLS, LLC

KAUKAUNA

PROJECT INFORMA

SITE INFORMATION:

A PARCEL OF LAND BEING PART OF PARCEL "A" AND "B", OUTAGAMIE COUNTY CERTIFIED SURVEY MAP NO. 68, RECORDED IN VOLUME 1 OF CERTIFIED SURVEY MAPS, PAGE 68, OUTAGAMIE COUNTY RECORDS, ORIGINALLY BEING A PART OF SUB-LOT "A" OF GOVERNMENT LOT FIVE (5)

APN: 324047300

PROPERTY AREA: 29,875 S.F. (0.686 ACRES)

EXISTING ZONING: CHD (COMMERCIAL HIGHWAY DISTRICT)

PROPOSED ZONING: CHD (COMMERCIAL HIGHWAY DISTRICT)

PROPOSED USE: QUICK SERVE RESTAURANT W/ DRIVE-THRU

AREA OF SITE DISTURBANCE: 22,701 (0.52 ACRES)

AREA OF IMPERVIOUS SURFACE DISTURBANCE: 19,221 S.F. (0.44 ACRES)

SETBACKS:

BUILDING: FRONT (WEST, NORTH, EAST) = 25' SIDE (SOUTH) = 10'

PAVEMENT: FRONT (WEST, NORTH, EAST) = 0' SIDE (SOUTH) = 0'

BUFFERYARDS:

FRONT (WEST, NORTH, EAST) = 0' SIDE (SOUTH) = 0'

PROPOSED BUILDING HEIGHT: 23' (MAX. HEIGHT ALLOWED: 56' OR FOUR STORIES)

PARKING REQUIRED: SUFFICIENT SUCH THAT NO PUBLIC STREET SHALL BE USED PARKING PROVIDED: 32 SPACES (2 H.C. ACCESSIBLE)

HANDICAP STALLS REQUIRED: 2, HANDICAP STALLS PROVIDED: 2

MAXIMUM LOT COVERAGE - BUILDING ONLY: 35%

PROJECT CONTACTS

OWNER INFORMATION: ANCHOR POINT MANAGEMENT, LLC KARI KELLER 111 W. 39TH STREET VANCOUVER, WA 98660 Phone: (714) 724-9415 Email: KKeller@anchorpointmg.com

CIVIL: GRANT DUCHAC, P.E. Phone: (920) 926-9800 E-mail: grant.duchac@excelengineer.com

LOCATION MAP

			NOTE: ALL SYMBOLS SHOWN MAY NOT APPEAR ON DRAWINGS.	
			SYM. IDENTIFICATION SPOT ELEVATIONS	SYM. IDENTIFICATION
ΓΙΟΝΙ			PROPOSED SPOT ELEVATIONS (FLOW LINE OF CURB UNLESS OTHERWISE SPECIFIED) 000.00 EXISTING GRADE SPOT ELEVATIONS	O00.00 TC PROPOSED SPOT ELEVATIONS (TOP OF CURB, FLOWLINE OF CURB)
			PROPOSED SPOT ELEVATIONS (REFERENCE R-WALL DETAIL) BG-FINISHED SURFACE GRADE AT BACK OF WALL FG-FINISHED SURFACE GRADE AT FRONT OF WALL	O00.00 TW PROPOSED SPOT ELEVATIONS (TOP OF WALK, BOTTOM OF WALK @ FLOWLINE)
EXISTING SITE DATA			EXISTING SITE SYMBOLS	
BUILDING FLOOR AREA (AC) AREA (SF) 6,021	RATIO 20.2%	EAST 18TH STREET	EXISTING SIGN	Ø EXISTING UTILITY POLE
PAVEMENT (ASP. & CONC.) 0.40 17,508 TOTAL IMPERVIOUS 22,520	58.6%	N 89*1620 W 198.75(R) N 89*33709*E 198.75(M) 凶战	EXISTING HANDICAP PARKING STALL	$\swarrow \longrightarrow$ EXISTING UTILITY POLE WITH GUY WIRE
LANDSCAPE/ OPEN SPACE 0.54 23,529 0.15 6,346			EXISTING WATER VALVE IN BOX	O→ EXISTING STREET LIGHT
PROJECT SITE 0.69 29,875			EXISTING WATER VALVE IN MANHOLE	EXISTING TELEPHONE PEDESTAL
			EXISTING WATER SERVICE VALVE	E EXISTING ELECTRIC PEDESTAL
PROPOSED SITE DATA		PROPOSED TACO BELL	EXISTING WELL	EXISTING ELECTRIC BOX
AREA (AC) AREA (SF) BUILDING FLOOR AREA 0.05 2.241			EXISTING STORM CATCH BASIN	EXISTING FLOOD LIGHT
PAVEMENT (ASP. & CONC.) 0.43 18,850	63.1%		EXISTING STORM CURB INLET	EXISTING TELEPHONE MANHOLE
TOTAL IMPERVIOUS0.4821,091LANDSCAPE/ OPEN SPACE0.208.784			EXISTING SQUARE CATCH BASIN	C EXISTING CABLE TV PEDESTAL
PROJECT SITE 0.69 29,875	100.0%		EXISTING LIGHT POLE	EXISTING GAS VALVE
			■ 1-1/4" REBAR SET WEIGHING 4.30 LB/FT.	EXISTING HEDGE
~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~			3/4" REBAR SET WEIGHING 1.50 LB/FT.	EXISTING WOODED AREA
			□ 1-1/4" REBAR FOUND	الله EXISTING MARSH AREA
PARTICIPANTS' UNDERGROUND			O         3/4" REBAR FOUND	EXISTING DECIDUOUS TREE WITH TRUNK DIAMETER
			2" IRON PIPE FOUND	EXISTING CONIFEROUS TREE
CALL DIGGERS HOTLINE 1-800-242-8511			1" IRON PIPE FOUND	EXISTING SHRUB
TOLL FREE TELEFAX (414) 259-0947			SECTION CORNER	戶 EXISTING STUMP
1-800 542-2289		NF: BEATEN'S MINI MALL LLC 1810 CROOKS Ave, KAUKAUNA WISCONSIN 54130	PROPOSED SITE SYMBOLS	
REQUIRES MINIMUM OF 3 WORK DAYS			PROPOSED SIGN	PROPOSED STORM FIELD INLET - ST FI
			டூ PROPOSED HANDICAP PARKING STALL	O-D PROPOSED LIGHT POLE
BENCHMARK NOTE:	NORTH 50'	0 50' 100'	PROPOSED WATER VALVE IN BOX	PROPOSED DRAINAGE FLOW
BENCHMARKS SHOWN ON THIS			PROPOSED WATER VALVE IN MANHOLE	PROPOSED APRON END SECTION
PLAN ARE ON NAVD 88 DATUM.			PROPOSED WATER SERVICE VALVE	SOIL BORING
			PROPOSED WELL	
			PROPOSED STORM CATCH BASIN - ST CB	CO PROPOSED CLEANOUT
DAVID KITTEL	JAKE CARREL BRETT JENSEN		PROPOSED STORM CURB INLET - ST CI	DSG PROPOSED DOWNSPOUT TO GRADE
Phone: (920) 766-6370 JOHN NEUMEIER E-mail: dkittel@kaukauna.gov Phone: (920) 766-6305	Phone: (920) 766-6320 ext. 2 Phone: (920) 766-6325 E-mail: buildinginspector@kauka	na.gov	2CO PROPOSED 2-WAY CLEANOUT	DSR PROPOSED DOWNSPOUT TO RISER
E-mail: jneumeier@kaukauna.go	ov			
		CLIEFT TRIDEV	EXISTING CURB AND GUTTER	
	PROJECT NOTES	SHEEL INDEX		
		SHEETS BELOW INTENDED TO BE PRINTED IN: COLOR. REFER TO DIGITAL FORMAT	• 800 FXISTING GROUND CONTOUR	
Avve H	GENERAL PROJECT NOTES	DRAWINGS IF PRINTED GRAYSCALE TO ENSURE SCOPE CLARITY.		
W 15th St W 15th St W 15th St G E 15th St	<ol> <li>ALL DRIVEWAYS AND CURB CUTS TO BE CONSTRUCTED ACCORDING TO LOCAL ORDINANCES. CONTRACTOR TO OBTAIN ALL NECESSARY PERMITS.</li> </ol>			
Sulfram A	2. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL WORK IN ROW PERMITS.			
Glerview Park O Unison Credit Union	3. THE CONTRACTOR SHALL COORDINATE CONSTRUCTION WITH THE			
W 17/n St E 17/h St E 17/h St E 17/h St	NEIGHBORING PROPERTY OWNER TO THE SOUTH.	C0.2 CIVIL SPECIFICATIONS		
Dick's Drivelin 😔		C1.0 CIVIL EXISTING SITE AND DEMOLITION PLAN C1.1 CIVIL SITE PLAN		
W Henry St E Henry St E Henry St Henry		C1.2 CIVIL GRADING AND EROSION CONTROL PLAN		
Centre 2 State		C1.4 CIVIL CIVIL LANDSCAPE AND RESTORATION PLAN		
		C2.0         CIVIL DETAILS           C2.1         CIVIL DETAILS		
		C2.2 CIVIL DETAILS		
W Ann St V				
NTS			W	
			PROPOSED WATER LINE AND HYDRANT	
				CIVIL COVER SHEET

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# LEGEND

NUMBER	SHEET NAME / DESCRIPTION
C0.1	CIVIL COVER SHEET
C0.2	CIVIL SPECIFICATIONS
C1.0	CIVIL EXISTING SITE AND DEMOLITION PLAN
C1.1	CIVIL SITE PLAN
C1.2	CIVIL GRADING AND EROSION CONTROL PLAN
C1.3	CIVIL UTILITY PLAN
C1.4	CIVIL LANDSCAPE AND RESTORATION PLAN
C2.0	CIVIL DETAILS
C2.1	CIVIL DETAILS
C2.2	CIVIL DETAILS
C3.1	CIVIL SITE PHOTOMETRIC PLAN & DETAILS

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# **CIVIL SPECIFICATIONS**

### **DIVISION 31 EARTH WORK**

### 31 10 00 SITE CLEARING (DEMOLITION)

- A. CONTRACTOR SHALL CALL DIGGER'S HOT LINE AND CONDUCT A PRIVATE UTILITY LOCATE AS REQUIRED TO ENSURE THAT ALL UTILITIES HAVE BEEN LOCATED BEFORE STARTING SITE DEMOLITION. DESIGN ENGINEER SHALL BE NOTIFIED OF ANY
- DISCREPANCIES BETWEEN PLAN AND FIELD CONDITIONS PRIOR TO CONSTRUCTION. B. CONTRACTOR TO FIELD TELEVISE ALL EXISTING SANITARY AND STORM LATERALS THAT ARE SCHEDULED TO BE RE-USED AND/OR CONNECTED TO ON SITE AT TIME OF DEMOLITION. THE TELEVISING SHALL BE COMPLETED TO ENSURE THE EXISTING LATERAL(S) ARE FREE OF OBSTRUCTIONS AND IN SOUND STRUCTURAL CONDITION. TELEVISING OF THESE LATERAL(S) SHOULD BE COMPLETED AT BEGINNING OF
- CONSTRUCTION AND DESIGN ENGINEER SHALL BE NOTIFIED OF ANY PIPE OBSTRUCTIONS AND/OR STRUCTURAL DEFICIENCIES IMMEDIATELY AFTER COMPLETION OF FIELD TELEVISING. C. DEMOLITION PLAN IS AN OVERVIEW OF DEMOLITION TO TAKE PLACE ON SITE.
- CONTRACTOR TO FIELD VERIFY EXISTING SITE CONDITIONS PRIOR TO BIDDING. CONTRACTOR SHALL REMOVE, REPLACE, OR DEMOLISH ALL ITEMS AS NEEDED DURING CONSTRUCTION
- D. CONTRACTOR TO PROTECT EXISTING IMPROVEMENTS THAT ARE SCHEDULED TO REMAIN. ANY DAMAGE TO EXISTING FACILITIES SHALL BE REPLACED AT CONTRACTORS FXPFNSF
- E. ALL CONCRETE NOTED TO BE REMOVED SHALL BE REMOVED TO THE NEAREST CONTROL JOINT.

### 31 20 00 EARTH MOVING

- A. CONTRACTOR SHALL CALL DIGGER'S HOT LINE AND CONDUCT A PRIVATE UTILITY LOCATE AS REQUIRED TO ENSURE THAT ALL UTILITIES HAVE BEEN LOCATED BEFORE STARTING EXCAVATION. DESIGN ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCIES BETWEEN PLAN AND FIELD CONDITIONS PRIOR TO CONSTRUCTION.
- B. PROVIDE ALL LABOR, MATERIALS AND EQUIPMENT FOR ALL EXCAVATION, GRADING, FILL, AND BACKFILL WORK AS REQUIRED TO COMPLETE THE GENERAL CONSTRUCTION WORK. ALL EXCAVATION AND BACKFILL FOR ELECTRICALS AND MECHANICALS ARE THE RESPONSIBILITY OF THE RESPECTIVE CONTRACTOR UNLESS OTHERWISE SPECIFIED IN THE BID DOCUMENTS.
- C. ALL ORGANIC TOPSOIL INSIDE THE BUILDING AREA, UNDER PAVED AREAS, AND AT SITE FILL AREAS SHALL BE REMOVED. PROOF ROLL SUBGRADES BEFORE PLACING FILL WITH HEAVY PNEUMATIC-TIRED EQUIPMENT, SUCH AS A FULLY-LOADED TANDEM AXLE DUMP TRUCK, TO IDENTIFY SOFT POCKETS AND AREAS OF EXCESS YIELDING. CONTRACTOR SHALL VERIFY TOPSOIL DEPTHS PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL REVIEW AND FOLLOW THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT AND ACCOUNT FOR EXISTING CONDITIONS PRIOR TO SUBMITTING BID FOR THE PROJECT. EXCESS MATERIALS SHALL BE REMOVED FROM THE SITE UNLESS OTHERWISE DIRECTED IN THE PLANS OR BY LOCAL ZONING REOUIREMENTS.
- D. PLACE AND COMPACT FILL MATERIAL IN LAYERS TO REQUIRED ELEVATIONS. UNIFORMLY MOISTEN OR AERATE SUBGRADE AND EACH SUBSEQUENT FILL OR BACKFILL LAYER BEFORE COMPACTION AS RECOMMENDED TO ACHIEVE SPECIFIED DRY DENSITY. REMOVE AND REPLACE, OR SCARIFY AND AIR DRY, OTHERWISE SATISFACTORY SOIL MATERIAL THAT IS TOO WET TO COMPACT TO SPECIFIED DRY DENSITY.
- E. PLACE BACKFILL AND FILL MATERIALS IN LAYERS NOT MORE THAN 8" IN LOOSE DEPTH FOR MATERIAL COMPACTED BY HEAVY COMPACTION EQUIPMENT, AND NOT MORE
- THAN 4" IN LOOSE DEPTH FOR MATERIAL COMPACTED BY HAND-OPERATED TAMPERS. F. COMPACT THE SOIL TO NOT LESS THAN THE FOLLOWING PERCENTAGES OF MAXIMUM DRY DENSITY ACCORDING TO ASTM D 698, STANDARD PROCTOR TEST. FILL MAY NOT BE PLACED ON FROZEN GROUND AND NO FROZEN MATERIALS MAY BE USED FOR BACKFILL. APPLY THE MORE STRINGENT REOUIREMENTS WHEN COMPARING BETWEEN THE FOLLOWING AND THE GEOTECHNICAL REPORT. 1. UNDER FOUNDATIONS - SUBGRADE, AND EACH LAYER OF BACKFILL OR FILL
- MATERIAL, TO NOT LESS THAN 98 PERCENT. 2. UNDER INTERIOR SLAB-ON-GRADE WHERE GROUNDWATER IS MORE THAN 3 FEET BELOW THE SLAB - PLACE A DRAINAGE COURSE LAYER OF 3/4" CRUSHED STONE, WITH 5% TO 12% FINES, PER THICKNESS INDICATED ON FOUNDATION PLANS ON
- PREPARED SUBGRADE. COMPACT THE SUBGRADE AND DRAINAGE COURSE TO NOT LESS THAN 95 PERCENT. 3. UNDER INTERIOR SLAB-ON-GRADE WHERE GROUNDWATER IS WITHIN 3 FEET OF THE SLAB SURFACE- PLACE A DRAINAGE COURSE LAYER OF CLEAN 3/4" CRUSHED STONE,
- WITH NO MORE THAN 5% FINES, PER THICKNESS INDICATED ON FOUNDATION PLANS ON PREPARED SUBGRADE. COMPACT THE SUBGRADE AND DRAINAGE COURSE TO NOT LESS THAN 95 PERCENT 4. UNDER EXTERIOR CONCRETE AND ASPHALT PAVEMENTS - COMPACT THE SUBGRADE
- AND EACH LAYER OF BACKFILL OR FILL MATERIAL TO NOT LESS THAN 95 PERCENT. 5. UNDER WALKWAYS - COMPACT SUBGRADE AND EACH LAYER OF BACKFILL OR FILL MATERIAL TO NOT LESS THAN 95 PERCENT.
- 6. UNDER LAWN OR UNPAVED AREAS COMPACT SUBGRADE AND EACH LAYER OF BACKFILL OR FILL MATERIAL, TO NOT LESS THAN 85 PERCENT.
- G. CONTRACTOR SHALL ENGAGE A QUALIFIED INDEPENDENT TESTING AND INSPECTING AGENCY TO PERFORM FIELD TESTS AND INSPECTIONS. CONTRACTOR SHALL PROVID DOCUMENTATION OF PASSING DENSITY TESTING AND PROOF-ROLLING TO ENGINEER UPON COMPLETION. IT IS SUGGESTED THAT THE GEOTECHNICAL FIRM USED TO PERFORM THE SUBSURFACE SOIL INVESTIGATION BE ENGAGED FOR THE FIELD QUALITY CONTROL TESTS. THE GEOTECHNICAL REPORT WAS PERFORMED BY PROFESSIONAL SERVICE INDUSTRIES, INC.
- H. ALLOW THE TESTING AGENCY TO TEST AND INSPECT SUBGRADES AND EACH FILL OR BACKFILL LAYER. PROCEED WITH SUBSEQUENT EARTHWORK ONLY AFTER TEST RESULTS FOR PREVIOUSLY COMPLETED WORK COMPLY WITH REQUIREMENTS. PROVIDE ONE TEST FOR EVERY 2000 SOUARE FEET OF PAVED AREA OR BUILDING SLAB, ONE TEST FOR EACH SPREAD FOOTING, AND ONE TEST FOR EVERY 50 LINEAR FEET OF WALL STRIP FOOTING.
- I. WHEN THE TESTING AGENCY REPORTS THAT SUBGRADES, FILLS, OR BACKFILLS HAVE NOT ACHIEVED DEGREE OF COMPACTION SPECIFIED, SCARIFY AND MOISTEN OR AERATE, OR REMOVE AND REPLACE SOIL TO DEPTH REQUIRED: RECOMPACT AND RETEST UNTIL SPECIFIED COMPACTION IS OBTAINED.
- J. THE BUILDING SITE SHALL BE GRADED TO PROVIDE DRAINAGE AWAY FROM THE BUILDING AS INDICATED ON THE PLANS. SITE EARTHWORK SHALL BE GRADED TO WITHIN 0.10' OF REQUIRED EARTHWORK ELEVATIONS ASSUMING POSITIVE DRAINAGE IS MAINTAINED IN ACCORDANCE WITH THE GRADING PLAN.

### 31 30 00 EROSION CONTROL

- A. THE GRADING PLAN REFLECTS LESS THAN 1 ACRE OF DISTURBED AREA. THE SITE IS THEREFORE EXEMPT FROM WISCONSIN DEPARTMENT OF NATURAL RESOURCES NR 216 NOTICE OF INTENT REQUIREMENTS. THE DESIGN ENGINEER SHALL PREPARE AN EROSION CONTROL PLAN TO MEET NR 151.105 CONSTRUCTION SITE PERFORMANCE STANDARDS FOR NON-PERMITTED SITES.
- B. EROSION AND SEDIMENT CONTROL IMPLEMENTED DURING CONSTRUCTION SHALL STRICTLY COMPLY WITH THE GUIDELINES AND REQUIREMENTS SET FORTH IN WISCONSIN ADMINISTRATIVE CODE (W.A.C.) NR 151, THE STATE OF WISCONSIN DEPARTMENT OF NATURAL RESOURCES RUNOFF MANAGEMENT PERFORMANCE STANDARDS. TECHNICAL STANDARDS PUBLISHED BY THE WISCONSIN DNR SHALL ALSO BE UTILIZED TO IMPLEMENT THE REQUIRED PERFORMANCE STANDARDS. THE METHODS AND TYPES OF EROSION CONTROL WILL BE DEPENDENT ON THE LOCATION AND TYPE OF WORK INVOLVED. ALL SEDIMENT CONTROL MEASURES SHALL BE ADJUSTED TO MEET FIELD CONDITIONS AT THE TIME OF CONSTRUCTION, AND INSTALLED PRIOR TO ANY GRADING OR DISTURBANCE OF EXISTING SURFACE MATERIAL. BELOW IS A LIST OF EROSION AND SEDIMENT CONTROL BEST MANAGEMENT PRACTICES TO ACHIEVE THE PERFORMANCE STANDARDS REQUIRED
- 1. SILT FENCE SHALL BE PLACED ON SITE AT LOCATIONS SHOWN ON THE EROSION CONTROL PLAN. SILT FENCE SHALL ALSO BE PROVIDED AROUND THE PERIMETER OF ALL SOIL STOCKPILES THAT WILL EXIST FOR MORE THAN 7 DAYS. FOLLOW PROCEDURES FOUND IN WISCONSIN DNR TECHNICAL STANDARD 1056 (CURRENT EDITION).
- 2. DITCH CHECKS SHALL BE PROVIDED TO REDUCE THE VELOCITY OF WATER FLOWING IN DITCH BOTTOMS. PLACE AT LOCATIONS SHOWN ON THE EROSION CONTROL PLAN. FOLLOW PROCEDURES FOUND IN WISCONSIN DNR TECHNICAL STANDARD 1062 (CURRENT EDITION).

- 3. STONE TRACKING PADS AND TRACKOUT CONTROL PRACTICES SHALL BE PLACED AT ALL CONSTRUCTION SITE ENTRANCES AND SHALL BE INSTALLED PRIOR TO ANY TRAFFIC LEAVING THE CONSTRUCTION SITE. SEE THE EROSION CONTROL PLAN FOR LOCATIONS. THE AGGREGATE USED FOR THE STONE TRACKING PAD SHALL BE 3/8" TO 3 INCH CLEAR OR WASHED STONE AND SHALL BE PLACED IN A LAYER AT LEAST 12 INCHES THICK. THE STONE SHALL BE UNDERLAIN WITH A WISDOT TYPE R GEOTEXTILE FABRIC AS NEEDED. THE TRACKING PAD SHALL BE THE FULL WIDTH OF THE EGRESS POINT (12' MIN WIDTH) AND SHALL BE A MINIMUM OF 50 FEET LONG. SURFACE WATER MUST BE PREVENTED FROM PASSING THROUGH THE TRACKING PAD. OTHER TRACKOUT CONTROL PRACTICES INCLUDING STABILIZED WORK SURFACES, MANUFACTURED TRACKOUT CONTROL DEVICES, TIRE WASHING, AND STREET/PAVEMENT CLEANING SHALL BE IMPLEMENTED AS NECESSARY TO MITIGATE THE TRACKOUT OF SEDIMENT OFFSITE. FOLLOW PROCEDURES FOUND IN WISCONSIN DNR TECHNICAL STANDARD 1057 (CURRENT EDITION).
- 4. STORM DRAIN INLET PROTECTION SHALL BE PROVIDED FOR ALL NEW AND DOWNSTREAM STORM CATCH BASINS AND CURB INLETS. TYPE B OR C PROTECTION SHOULD BE PROVIDED AND SHALL BE IN CONFORMANCE WITH WISCONSIN DNR TECHNICAL STANDARD 1060 (CURRENT EDITION).
- 5. DUST CONTROL MEASURES SHALL BE PROVIDED TO REDUCE OR PREVENT THE SURFACE AND AIR TRANSPORT OF DUST DURING CONSTRUCTION. CONTROL MEASURES INCLUDE APPLYING MULCH AND ESTABLISHING VEGETATION, WATER SPRAYING, SURFACE ROUGHENING, APPLYING POLYMERS, SPRAY-ON TACKIFIERS, CHLORIDES, AND BARRIERS. SOME SITES MAY REQUIRE AN APPROACH THAT UTILIZES A COMBINATION OF MEASURES FOR DUST CONTROL. FOLLOW PROCEDURES FOUND IN WISCONSIN DNR TECHNICAL STANDARD 1068 (CURRENT EDITION).
- 6. THE USE, STORAGE, AND DISPOSAL OF CHEMICALS, CEMENT, AND OTHER COMPOUNDS AND MATERIALS USED ON SITE SHALL BE MANAGED DURING THE CONSTRUCTION PERIOD TO PREVENT THEIR TRANSPORT BY RUNOFF INTO WATERS OF THE STATE
- 7. CONTRACTOR SHALL PROVIDE AN OPEN AGGREGATE CONCRETE TRUCK WASHOUT AREA ON SITE. CONTRACTOR TO ENSURE THAT CONCRETE WASHOUT SHALL BE CONTAINED TO THIS DESIGNATED AREA AND NOT BE ALLOWED TO RUN INTO STORM INLETS OR INTO THE OVERLAND STORMWATER DRAINAGE SYSTEM. WASHOUT AREA SHALL BE REMOVED UPON COMPLETION OF CONSTRUCTION.
- 8. TEMPORARY SITE RESTORATION SHALL TAKE PLACE IN DISTURBED AREAS THAT WILL NOT BE BROUGHT TO FINAL GRADE OR ON WHICH LAND DISTURBING ACTIVITIES WILL NOT BE PERFORMED FOR A PERIOD GREATER THAN 14 DAYS AND REQUIRES VEGETATIVE COVER FOR LESS THAN ONE YEAR. THIS TEMPORARY SITE RESTORATION REQUIREMENT ALSO APPLIES TO SOIL STOCKPILES THAT EXIST FOR MORE THAN 7 DAYS. PERMANENT RESTORATION APPLIES TO AREAS WHERE PERENNIAL VEGETATIVE COVER IS NEEDED TO PERMANENTLY STABILIZE AREAS OF EXPOSED SOIL PERMANENT STABILIZATION SHALL OCCUR WITHIN 3 WORKING DAYS OF FINAL GRADING. TOPSOIL, SEED, AND MULCH SHALL BE IN GENERAL CONFORMANCE WITH TECHNICAL STANDARDS 1058 AND 1059 AND SHALL MEET THE SPECIFICATIONS FOUND IN THE LANDSCAPING AND SITE STABILIZATION SECTION OF THIS CONSTRUCTION DOCUMENT. ANY SOIL EROSION THAT OCCURS AFTER FINAL GRADING AND/OR FINAL STABILIZATION MUST BE REPAIRED AND THE STABILIZATION WORK REDONE.
- 9. IF SITE DEWATERING IS REQUIRED FOR PROPOSED CONSTRUCTION ACTIVITIES, ALL SEDIMENT LADEN WATER GENERATED DURING THE DEWATERING PROCESS SHALL BE TREATED TO REMOVE SEDIMENT PRIOR TO DISCHARGING OFF-SITE OR TO WATERS OF THE STATE. FOLLOW ALL PROCEDURES FOUND IN TECHNICAL STANDARD 1061. 10. ALL OFF-SITE SEDIMENT DEPOSITS OCCURRING AS A RESULT OF CONSTRUCTION
- WORK OR A STORM EVENT SHALL BE CLEANED UP BY THE END OF EACH WORKING DAY. DUST CONTROL REQUIREMENTS SHALL BE FOLLOWED PER WI DNR TECHNICAL STANDARD 1068 (CURRENT EDITION). FLUSHING SHALL NOT BE ALLOWED. C. ALL EROSION CONTROL DEVICES SHALL AT A MINIMUM BE INSPECTED EVERY 7 CALENDAR DAYS OR EVERY 14 DAYS AND WITHIN 24 HOURS OF THE END OF A RAIN
- EVENT OF 0.5" OR MORE. MAINTENANCE SHALL BE PERFORMED PER WISCONSIN ADMINISTRATIVE CODE (W.A.C.) NR 151 STORMWATER MANAGEMENT TECHNICAL STANDARD REOUIREMENTS.
- D. EROSION CONTROL MEASURES SHALL NOT BE REMOVED UNTIL THE AREA(S) SERVED HAVE ESTABLISHED VEGETATIVE COVER. E. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL LOCAL EROSION CONTROL

### **DIVISION 32 EXTERIOR IMPROVEMENTS**

### 32 10 00 AGGREGATE BASE & ASPHALT PAVEMENT

A. CONTRACTOR TO PROVIDE COMPACTED AGGREGATE BASE AND HOT MIX ASPHALT PAVEMENT WHERE INDICATED ON THE PLANS. ALL AGGREGATE PROVIDED MUST COMPLY WITH SECTION 305 OF THE WISCONSIN STANDARD SPECIFICATIONS FOR HIGHWAY AND STRUCTURE CONSTRUCTION PROVIDE HOT MIX ASPHALT MIXTURE TYPES PER SECTION 460 OF THE WISCONSIN STANDARD SPECIFICATIONS FOR HIGHWAY AND STRUCTURE CONSTRUCTION. CONTRACTOR SHALL OBTAIN AND REVIEW SOILS REPORT FOR RECOMMENDATIONS FOR GEO-GRID / GEOTEXTILE BELOW CRUSHED AGGREGATE (IF APPLICABLE). CONTRACTOR TO PROVIDE AGGREGATE BASE AND HOT MIX ASPHALT PAVEMENT TYPES AND DEPTHS AS INDICATED BELOW:

STANDARD ASPHALT PAVING SECTION 1-3/4" SURFACE COURSE (5 LT 58-28S) (WISDOT 455.2.5 TACK COAT (STAGED PAVING) 1-3/4" BINDER COURSE (4 LT 58-28S) 9" OF 1-1/4" CRUSHED AGGREGATE

HEAVY ASPHALT PAVING SECTION 2" SURFACE COURSE (5 LT 58-28S) WISDOT 455.2.5 TACK COAT (STAGED PAVING) 1-3/4" BINDER COURSE (4 LT 58-28S) 12" OF 1-1/4" CRUSHED AGGREGATE

MILL & OVERLAY MILL 2" OF SURFACE ASPHALT PAVEMENT WISDOT 455.2.5 TACK COAT 2" SURFACE COURSE (5 LT 58-28S)

- B. CONTRACTOR TO COMPACT THE AGGREGATE BASE, ASPHALT BINDER COURSE, AND ASPHALT SURFACE COURSE TO AN AVERAGE DENSITY PER WISCONSIN STANDARD SPECIFICATIONS FOR HIGHWAY AND STRUCTURE CONSTRUCTION. ALL ASPHALT PAVEMENT AREAS SHALL BE PAVED TO WITHIN 0.05' OF DESIGN SURFACE GRADES WITH POSITIVE DRAINAGE BEING MAINTAINED IN ACCORDANCE WITH DESIGN PLANS. A MINIMUM OF 1.5% SLOPE SHALL BE MAINTAINED IN ALL ASPHALT PAVEMENT AREA.
- C. HOT MIX ASPHALT CONSTRUCTION TO BE PROVIDED PER MORE STRINGENT
- REQUIREMENTS OF GEOTECHNICAL REPORT OR CONSTRUCTION DOCUMENTS. D. CONTRACTOR TO PROVIDE 4" WIDE YELLOW PAINTED STRIPING FOR PARKING STALLS, TRAFFIC LANES, AND NO PARKING AREAS. YELLOW PAINT MARKINGS SHALL ALSO BE PROVIDED FOR H.C. ACCESSIBLE SYMBOLS, TRAFFIC ARROWS, AND TRAFFIC MESSAGES.

### 32 20 00 CONCRETE AND AGGREGATE BASE

- A. CONTRACTOR TO PROVIDE CRUSHED AGGREGATE BASE AND CONCRETE WHERE
- INDICATED ON THE PLANS. B. ALL AGGREGATE PROVIDED MUST COMPLY WITH SECTION 305 OF THE WISCONSIN STANDARD SPECIFICATIONS FOR HIGHWAY AND STRUCTURE CONSTRUCTION. ALL AGGREGATE PLACED MUST BE COMPACTED TO AN AVERAGE DENSITY PER WISCONSIN
- STANDARD SPECIFICATIONS FOR HIGHWAY AND STRUCTURE CONSTRUCTION. C. DESIGN AND CONSTRUCTION OF ALL CAST-IN-PLACE EXTERIOR CONCRETE FLAT WORK SHALL CONFORM TO ACI 330R-08 & ACI 318-08.
- D. EXTERIOR CONCRETE FLAT WORK CONSTRUCTION TO BE PROVIDED PER MORE STRINGENT REQUIREMENTS OF THE GEOTECHNICAL REPORT OR THIS SPECIFICATION. CONCRETE FLAT WORK CONSTRUCTION IS AS FOLLOWS: 1. SIDEWALK CONCRETE - 4" OF CONCRETE OVER 4" OF 3/4" CRUSHED AGGREGATE
- BASE. CONTRACTION JOINTS SHALL CONSIST OF 1/8" WIDE BY 1" DEEP TOOLED JOINT WHERE INDICATED ON THE PLANS. 2. DUMPSTER PAD/APRON CONCRETE - 8" OF CONCRETE OVER 6" OF AGGREGATE BASE. a. CONCRETE SHALL BE STEEL REINFORCED WITH THE FOLLOWING AND PLACED IN
- THE UPPER 1/3 TO 1/2 OF THE SLAB 1) TIE BARS AT ALL CONTRACTION JOINTS OF THE CONCRETE. TIE BARS SHALL BE #4 REBAR 30" LONG PLACED AT 30" O.C.
- b. DUMPSTER PAD CONCRETE JOINTING SHALL BE AS FOLLOWS:
- 1) CONTRACTION SAWCUT JOINT CONTRACTOR SHALL PROVIDE A SAWCUT JOINT AT MAXIMUM SPACING OF 15' ON CENTER. SAWCUT SHALL BE 2" IN DFPTH
- 2) TYPICAL POUR CONTROL JOINT POUR CONTROL JOINT SHALL BE PROVIDED WITH 1-1/4" DIAMETER BY 20" LONG SMOOTH DOWEL PLACED AT 12" O.C. ONE HALF OF THE DOWEL SHALL BE GREASED. GREENSTREAK 9" SPEED DOWEL TUBES SHALL BE USED.

- 3. HEAVY DUTY/DRIVE-THRU CONCRETE 7" OF CONCRETE OVER 6" OF 3/4" CRUSHED AGGREGATE. CONCRETE SHALL BE REINFORCED WITH #3 REBARS ON CHAIRS AT 3' O.C. REBAR SHALL BE PLACED PLACED IN THE UPPER 1/3 TO 1/2 OF THE SLAB. CONTRACTION JOINTS SHALL BE SAWCUT 1.75" IN DEPTH AND BE SPACED A MAXIMUM OF 15' ON CENTER.
- E. DESIGN MIXES SHALL BE IN ACCORDANCE WITH ASTM C94 1. STRENGTH TO BE MINIMUM OF 4,500 PSI AT 28 DAYS FOR EXTERIOR CONCRETE. 2. MAXIMUM WATER/CEMENT RATIO SHALL BE 0.45.
- 3. SLUMP SHALL NOT EXCEED 4" FOR EXTERIOR CONCRETE FLAT WORK
- 4. SLUMP SHALL BE 2.5" OR LESS FOR SLIP-FORMED CURB AND GUTTER 5. SLUMP SHALL BE BETWEEN 1.5" TO 3" FOR NON SLIP-FORMED CURB AND GUTTER.
- 6. ALL EXTERIOR CONCRETE SHALL BE AIR ENTRAINED WITH 4% TO 7% AIR CONTENT. NO OTHER ADMIXTURES SHALL BE USED WITHOUT APPROVAL OF EXCEL ENGINEERING, INC. CALCIUM CHLORIDE SHALL NOT BE USED.
- 7. MAXIMUM AGGREGATE SIZE FOR ALL EXTERIOR CONCRETE SHALL BE 0.75 INCHES. F. VERIFY EQUIPMENT CONCRETE PAD SIZES WITH CONTRACTOR REQUIRING PAD. PADS SHALL HAVE FIBERMESH 300 FIBERS AT A RATE OF 1.5 LBS/CU. YD. OR 6 X 6-W1.4 X W1.4 WELDED WIRE MESH WITH MINIMUM 1 INCH COVER FOUIPMENT PADS SHALL BE 5.5 INCHES THICK WITH 1 INCH CHAMFER UNLESS SPECIFIED OTHERWISE. COORDINATE ADDITIONAL PAD REQUIREMENTS WITH RESPECTIVE CONTRACTOR.
- G.ALL CONCRETE FLAT WORK SURFACES AND CONCRETE CURB FLOWLINES SHALL BE CONSTRUCTED TO WITHIN 0.05' OF DESIGN SURFACE AND FLOWLINE GRADES ASSUMING POSITIVE DRAINAGE IS MAINTAINED IN ACCORDANCE WITH THE DESIGN PLANS
- H. CONCRETE FLAT WORK SHALL HAVE CONSTRUCTION JOINTS OR SAW CUT JOINTS PLACED AS INDICATED ON THE PLANS OR PER THIS SPECIFICATION. SAWCUTS SHALL BE DONE AS SOON AS POSSIBLE, BUT NO LATER THAN 24 HOURS AFTER CONCRETE IS PLACED. CONCRETE CURB AND GUTTER JOINTING SHALL BE PLACED EVERY 10' OR CLOSER (6' MIN.). IF CONCRETE PAVEMENT IS ADJACENT TO CONCRETE CURB, JOINTING IN THE PAVEMENT AND CURB SHALL ALIGN. ALL EXTERIOR CONCRETE SHALL HAVE A BROOM FINISH UNLESS NOTED OTHERWISE. A UNIFORM COAT OF A HIGH SOLIDS CURING COMPOUND MEETING ASTM C309 SHALL BE APPLIED TO ALL EXPOSED CONCRETE SURFACES. ALL CONCRETE IS TO BE CURED FOR 7 DAYS. EXTERIOR CONCRETE SHALL BE SEPARATED FROM BUILDINGS WITH CONTINUOUS 0.5 INCH FIBER EXPANSION JOINT AND/OR 0.25 INCH FIBER EXPANSION JOINT AT DECORATIVE MASONRY UNITS
- I. ALL REINFORCING BARS SHALL BE ASTM A615 GRADE 60. THICKNESS OF CONCRETE COVER OVER REINFORCEMENT SHALL BE NOT LESS THAN 3" WHERE CONCRETE IS DEPOSITED AGAINST THE GROUND WITHOUT THE USE OF FORMS AND NOT LESS THAN 1.5" FOR UP TO #5 BARS AND 2" FOR #6 TO #10 BARS IN ALL OTHER LOCATIONS. ALL REINFORCING SHALL BE LAPPED 48 DIAMETERS FOR UP TO #6 BARS, 62 DIAMETERS FOR #7 TO #9 BARS 68 DIAMETERS FOR #10 BARS OR AS NOTED ON THE DRAWINGS AND EXTENDED AROUND CORNERS WITH CORNER BARS. PLACING AND DETAILING OF STEEL REINFORCING AND REINFORCING SUPPORTS SHALL BE IN ACCORDANCE WITH CRSI AND ACI MANUAL AND STANDARD PRACTICES. THE REINFORCEMENT SHALL NOT BE PAINTED AND MUST BE FREE OF GREASE/OIL, DIRT, OR DEEP RUST WHEN PLACED IN THE WORK. ALL WELDED WIRE FABRIC SHALL MEET THE REQUIREMENTS OF ASTM A 1064. WELDED WIRE FABRIC SHALL BE PLACED 2" FROM TOP OF SLAB, UNLESS INDICATED OTHERWISE.
- J. CONTRACTOR SHALL ENGAGE A QUALIFIED INDEPENDENT TESTING AND INSPECTING AGENCY TO SAMPLE MATERIALS, PERFORM TESTS, AND SUBMIT TEST REPORTS DURING CONCRETE PLACEMENT, TESTS WILL BE PERFORMED ACCORDING TO ACI 301, CAST AND LABORATORY CURF ONE SET OF FOUR STANDARD CYLINDERS FOR EACH COMPOSITE SAMPLE FOR EACH DAY'S POUR OF EACH CONCRETE MIX EXCEEDING 5 CU. YD., BUT LESS THAN 25 CU. YD., PLUS ONE SET FOR EACH ADDITIONAL 50 CU. YD. OR FRACTION THEREOF. PERFORM COMPRESSIVE-STRENGTH TESTS ACCORDING TO ASTM C 39. TEST TWO SPECIMENS AT 7 DAYS AND TWO SPECIMENS AT 28 DAYS. PERFORM SLUMP TESTING ACCORDING TO ASTM C 143. PROVIDE ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIX. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.
- K. PROTECT FRESHLY PLACED CONCRETE FROM PREMATURE DRYING AND EXCESSIVE COLD OR HOT TEMPERATURES. IN HOT, DRY, AND WINDY WEATHER, APPLY AN EVAPORATION-CONTROL COMPOUND ACCORDING TO MANUFACTURER'S INSTRUCTIONS AFTER SCREEDING AND BULL FLOATING, BUT BEFORE POWER FLOATING AND TROWELLING.
- L. LIMIT MAXIMUM WATER-CEMENTITIOUS RATIO OF CONCRETE EXPOSED TO FREEZING, THAWING, AND DEICING SALTS TO 0.45.
- M.TEST RESULTS WILL BE REPORTED IN WRITING TO THE DESIGN ENGINEER, READY-MIX PRODUCER, AND CONTRACTOR WITHIN 24 HOURS AFTER TESTS. REPORTS OF COMPRESSIVE STRENGTH TESTS SHALL CONTAIN THE PROJECT IDENTIFICATION NAME AND NUMBER, DATE OF CONCRETE PLACEMENT, NAME OF CONCRETE TESTING SERVICE CONCRETE TYPE AND CLASS, LOCATION OF CONCRETE BATCH ON SITE, DESIGN COMPRESSIVE STRENGTH AT 28 DAYS, CONCRETE MIX PROPORTIONS AND MATERIALS, COMPRESSIVE BREAKING STRENGTH, AND TYPE OF BREAK FOR BOTH 7-DAY TESTS AND 28-DAY TESTS.

### 32 30 00 LANDSCAPING AND SITE STABILIZATION

A. TOPSOIL: CONTRACTOR TO PROVIDE A MINIMUM OF 6" OF TOPSOIL FOR ALL DISTURBED OPEN AREAS, OTHER THAN LANDSCAPE ISLANDS SHALL BE PROVIDED WITH A MINIMUM OF 10" OF TOPSOIL. REUSE SURFACE SOIL STOCKPILED ON SITE AND SUPPLEMENT WITH IMPORTED OR MANUFACTURED TOPSOIL FROM OFF SITE SOURCES WHEN OUANTITIES ARE INSUFFICIENT. EXCAVATOR SHALL BE RESPONSIBLE FOR ROUGH PLACEMENT OF TOPSOIL TO WITHIN 1" OF FINAL GRADE PRIOR TO LANDSCAPER FINAL GRADING. LANDSCAPER TO PROVIDE PULVERIZING AND FINAL GRADING OF TOPSOIL PROVIDE SOIL ANALYSIS BY A QUALIFIED SOIL TESTING LABORATORY AS REQUIRED TO VERIFY THE SUITABILITY OF SOIL TO BE USED AS TOPSOIL AND TO DETERMINE THE NECESSARY SOIL AMENDMENTS. TEST SOIL FOR THE PRESENCE OF ATRAZINE AND INFORM EXCEL ENGINEERING, INC. IF PRESENT PRIOR TO BIDDING PROJECT TOPSOIL SHALL HAVE A PH RANGE OF 5.5 TO 8. CONTAIN A MINIMUM OF 5 PERCENT ORGANIC MATERIAL CONTENT, AND SHALL BE FREE OF STONES 1 INCH OR LARGER IN DIAMETER. ALL MATERIALS HARMFUL TO PLANT GROWTH SHALL ALSO BE REMOVED. TOPSOIL INSTALLATION: LOOSEN SUBGRADE TO A MINIMUM DEPTH OF 6 INCHES AND REMOVE STONES LARGER THAN 1" IN DIAMETER. ALSO REMOVE ANY STICKS, ROOTS, RUBBISH, AND OTHER EXTRANEOUS MATTER AND DISPOSE OF THEM OFF THE PROPERTY SPREAD TOPSOIL TO A DEPTH OF 6" BUT NOT LESS THAN WHAT IS REQUIRED. TO MEET FINISHED GRADES AFTER LIGHT ROLLING AND NATURAL SETTLEMENT. DO NOT SPREAD TOPSOIL IF SUBGRADE IS FROZEN, MUDDY, OR EXCESSIVELY WET. GRADE PLANTING AREAS TO A SMOOTH, UNIFORM SURFACE PLANE WITH LOOSE, UNIFORMLY FINE TEXTURE. GRADE TO WITHIN 0.05 FEET OF FINISHED GRADE ELEVATION.

- B. SEEDED LAWNS
- 1. PERMANENT LAWN AREAS SHALL BE SEEDED WITH THE FOLLOWING MIXTURE: 65% KENTUCKY BLUEGRASS BLEND (2.0-2.6 LBS./1,000 S.F.), 20% PERENNIAL RYEGRASS (0.6-0.8 LBS./1.000 S.F.), 15% FINE FESCUE (0.4-0.6 LBS/1.000 S.F.), STRAW AND MULCH SHALL BE LAID AT 100LBS/1.000 S.F. FERTILIZE AS PER SOIL TEST OR APPLY 5-10-10 OR EQUIVALENT AT 5-6 LBS/1,000 S.F. SEE EROSION MATTING SPECIFICATIONS AS REQUIRED. ALL SITE DISTURBED AREAS NOT DESIGNATED FOR OTHER LANDSCAPING AND SITE STABILIZATION METHODS SHALL BE SEEDED AS PERMANENT LAWN. NO BARE TOPSOIL SHALL BE LEFT ONSITE. FOLLOW PROCEDURES FOUND IN WDNR TECHNICAL STANDARDS 1058 & 1059.
- 2. ALL PERMANENT AND TEMPORARY STORM WATER CONVEYANCE SWALE BOTTOMS AND SIDE SLOPES SHALL BE SEEDED WITH THE FOLLOWING MIXTURE: 45% KENTUCKY BLUEGRASS (0.60 LBS./1000 S.F.), 40% CREEPING RED FESCUE (0.50 LBS./1,000 S.F.), AND 15% PERENNIAL RYEGRASS (0.20 LBS./1,000 S.F.). FERTILIZE AS PER SOIL TEST OR APPLY 5-10-10 OR EQUIVALENT AT 5-6 LBS./1,000 S.F. SEE EROSION MATTING SPECIFICATIONS AS REQUIRED. FOLLOW PROCEDURES FOUND IN WDNR TECHNICAL STANDARDS 1058 & 1059.
- 3. ALL TEMPORARY SEEDING SHALL CONSIST OF THE FOLLOWING MIXTURE: 100% RYEGRASS AT 1.9 LBS./1,000 S.F. STRAW AND MULCH SHALL BE LAID AT 100 LBS./1,000 S.F. FERTILIZE AS PER SOIL TEST OR APPLY 5-10-10 OR EQUIVALENT AT 5-6 LBS./1,000 S.F. SEE EROSION MATTING SPECIFICATIONS AS REQUIRED. FOLLOW PROCEDURES FOUND IN WDNR TECHNICAL STANDARDS 1058 & 1059.
- C. SEEDED LAWN MAINTENANCE: CONTRACTOR TO PROVIDE MAINTENANCE OF ALL LANDSCAPING FOR A PERIOD OF 90 DAYS FROM THE DATE OF INSTALLATION. AT THE END OF THE MAINTENANCE PERIOD, A HEALTHY, UNIFORM, CLOSE STAND OF GRASS SHOULD BE ESTABLISHED FREE OF WEEDS AND SURFACE IRREGULARITIES. LAWN COVERAGE SHOULD EXCEED 90% AND BARE SPOTS SHOULD NOT EXCEED 5"X5". CONTRACTOR SHOULD REESTABLISH LAWNS THAT DO NOT COMPLY WITH THESE
- REQUIREMENTS AND CONTINUE MAINTENANCE UNTIL LAWNS ARE SATISFACTORY. D. EROSION MATTING: 1. CONTRACTOR TO PROVIDE EROSION CONTROL MATTING (NORTH AMERICAN GREEN S150) OR FOUTVALENT ON ALL SLOPES THAT ARE 4-1 AND GREATER OUTSIDE OF
- STORMWATER CONVEYANCE SWALES AND STORMWATER MANAGEMENT BASINS. LAWN SEED SHALL BE PLACED BELOW MATTING IN ACCORDANCE WITH SEEDING REQUIREMENTS AND MANUFACTURER SPECIFICATIONS.
- 2. CONTRACTOR TO PROVIDE EROSION MATTING (NORTH AMERICAN GREEN C125) OR EQUIVALENT IN ALL SWALE BOTTOMS AND SIDE SLOPES AS REQUIRED. LAWN SEED SHALL BE PLACED BELOW MATTING IN ACCORDANCE WITH SEEDING REQUIREMENTS AND MANUFACTURER SPECIFICATIONS.
- E. TREES AND SHRUBS: FURNISH NURSERY-GROWN TREES AND SHRUBS WITH HEALTHY ROOT SYSTEMS DEVELOPED BY TRANSPLANTING OR ROOT PRUNING. PROVIDE WELL-SHAPED, FULLY BRANCHED, AND HEALTHY LOOKING STOCK. STOCK SHOULD ALSO BE FREE OF DISEASE, INSECTS, EGGS, LARVAE, AND DEFECTS SUCH AS KNOTS, SUN SCALD, INJURIES, ABRASIONS, AND DISFIGUREMENT. SEE THE LANDSCAPE PLAN FOR
- SPECIFIC SPECIE TYPE, SIZE, AND LOCATION. F. TREE AND SHRUB INSTALLATION: EXCAVATE CIRCULAR PITS WITH SIDES SLOPED INWARD. TRIM BASE LEAVING CENTER AREA RAISED SLIGHTLY TO SUPPORT ROOT BALL. EXCAVATE PIT APPROXIMATELY THREE TIMES AS WIDE AS THE ROOT BALL DIAMETER. SET TREES AND SHRUBS PLUMB AND IN CENTER OF PIT WITH TOP OF BALL 1" ABOVE ADJACENT FINISHED GRADES. PLACE PLANTING SOIL MIX AROUND ROOT BALL IN LAYERS AND TAMP TO SETTLE MIX. WATER ALL PLANTS THOROUGHLY. PROVIDE TEMPORARY STAKING FOR TREES AS REOUIRED.
- G. TREE AND SHRUB MAINTENANCE/WARRANTY: CONTRACTOR TO PROVIDE MAINTENANCE OF ALL LANDSCAPING FOR A PERIOD OF 90 DAYS FROM THE DATE OF INSTALLATION. MAINTENANCE TO INCLUDE REGULAR WATERING AS REQUIRED FOR SUCCESSFUL PLANT ESTABLISHMENT. CONTRACTOR TO PROVIDE 1 YEAR WARRANTY ON ALL TREES, SHRUBS, AND PERENNIALS.
- H.MINERAL MULCH: PROVIDE 4" MINIMUM THICK BLANKET OF 1.5" MINIMUM TO 2.5" MAXIMUM CRUSHED DECORATIVE STONE AT ALL PLANTING AREAS INDICATED ON THE LANDSCAPE PLAN. INSTALL OVER NON-WOVEN WEED BARRIER FABRIC. COLOR BY OW/NFR
- I. PLASTIC EDGING: INSTALL VALLEY VIEW INDUSTRIES BLACK DIAMOND LAWN EDGING TO SEPARATE ALL PLANTING BEDS FROM LAWN AREAS. EDGING TO BE 5.5" TALL WITH METAL STAKES INSTALLED PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
- J. LANDSCAPE AND LAWN IRRIGATION: CONTRACTOR TO PROVIDE DESIGN AND INSTALLATION OF IRRIGATION SYSTEM PIPING, VALVES, VALVE BOXES, SPRINKLERS, EMITTERS, DRIP TUBES, AND CONTROLS IN COMBINATIONS THAT BEST SUIT THE LANDSCAPE PLAN LAYOUT. ALL LAWN AND LANDSCAPING AREAS SHALL BE PROVIDED WITH IRRIGATION AS DELINEATED ON THE PLAN. THE DESIGN SHOULD MINIMIZE THE AMOUNT OF WATER THAT EXTENDS BEYOND THE PROPERTY AND ON PAVED AREAS. THE SYSTEM SHALL BE DESIGNED FOR FULLY AUTOMATIC OPERATION AND PROVIDE ALL NECESSARY CONTROLS, VALVES, AND WIRING TO OPERATE THE SYSTEM. THE CONTROL UNIT SHALL BE INSTALLED IN A MECHANICAL ROOM OR AT A LOCATION AGREED TO WITH THE OWNER. THE CONTROL UNIT SHOULD BE PROVIDED WITH A LOCKING COVER.
- POP-UP SPRAY OR ROTARY SPRINKLERS SHALL BE USED AT LAWN AREAS TO PROVIDE A UNIFORM COVERAGE OF 1 TO 2 INCHES OF WATER PER HOUR. EMITTERS AND DRIP TUBES OR SHRUBBERY SPRINKLERS SHALL BE USED AT PLANTS AND SHRUBS AS APPROPRIATE FOR THE PLANTING DENSITY AND SPECIES TYPE. ALL SPRINKLER HEADS SHALL BE COMMERCIAL GRADE. THE SYSTEM SHALL BE CIRCUITED AS REQUIRED TO PROVIDE ADEQUATE WATER FLOW TO EACH SPRINKLER HEAD. THE CONTROL SYSTEM MUST INCLUDE A RAIN SENSING SHUT OFF DEVICE. THE ENTIRE SYSTEM IS TO BE INSTALLED WITH A MINIMUM UNIFORM SLOPE OF 0.5 PERCENT TOWARD DRAIN VALVES.

### **DIVISION 33 UTILITIES**

### 33 10 00 SITE UTILITIES

A. CONTRACTOR TO FIELD VERIFY ALL EXISTING UNDERGROUND UTILITIES ON SITE. CONTRACTOR TO VERIFY PIPE LOCATIONS, SIZES, AND DEPTHS AT POINT OF PROPOSED CONNECTIONS AND VERIFY PROPOSED UTILITY ROUTES ARE CLEAR (PER CODE) OF ALL EXISTING UTILITIES AND OTHER OBSTRUCTIONS PRIOR TO CONSTRUCTION. COSTS INCURRED FOR FAILURE TO DO SO SHALL BE THE CONTRACTORS RESPONSIBILITY.

## **Table A: Allowable Pipe Material Schedule**

Utility	Material	Pipe Code	Fitting Code	Joint Code
Water Lateral	C901/906 PE	AWWA C901/C906	ASTM D2609, ASTM D2683, ASTM D3261	Heat fusion: ASTM D2657
Sanitary Sewer	SDR 35 PVC	ASTM D1785, ASTM D2665, ASTM D3034, ASTM F891	ASTM F1336	Push On: ASTM D3212 for Tightness Elastomeric Gasket: ASTM F477
Sanitary Sewer	SCH.40 PVC	ASTM D1785, ASTM D2665, ASTM F891	ASTM F1336	Primer: ASTM F656 Solvent Cement: ASTM D2564
Storm Sewer	ндре	ASTM F2648, ASTM F2306, AASHTO M252, TYPE S (4 IN - 10 IN), AASHTO M294, TYPE S (12 IN - 60 IN)	ASTM F2648, ASTM F2306, AASHTO M252, or AASHTO M294	Joint: ASTM F2648, ASTM F2306, AASHTO M252, or AASHTO M294 Elastomeric Seal: ASTM F477
Storm Sewer	SDR 35 PVC	ASTM D1785, ASTM D2665, ASTM D3034, ASTM F891	ASTM F1336	Push On: ASTM D3212 for Tightness Elastomeric Seal: ASTM F477
Pavement Underdrain	Single Wall Perforated HDPE-Socked	ASTM F667	ASTM F667	ASTM D1056 Grade 2A2 Gasketed
1) See Table DSPS 384.30-7 for Water 2) See Table DSPS 384.30-2 for Underg 3) See Table DSPS 384.30-10 for Pipe f 4) See Section SPS 384.40 for Joints ar	Services and Private Water Mai ground Drain and Vent Pipe an Fittings	ns d Tubing		

- B. CONTRACTOR TO FIELD TELEVISE ALL EXISTING SANITARY AND STORM LATERALS THAT ARE SCHEDULED TO BE RE-USED AND/OR CONNECTED TO ON SITE. THE TELEVISING SHALL BE COMPLETED TO ENSURE THE EXISTING LATERAL(S) ARE FREE OF OBSTRUCTIONS AND IN SOUND STRUCTURAL CONDITION. TELEVISING OF THESE LATERAL(S) SHOULD BE COMPLETED AT BEGINNING OF CONSTRUCTION AND DESIGN ENGINEER SHALL BE NOTIFIED OF ANY PIPE OBSTRUCTIONS AND/OR STRUCTURAL DEFICIENCIES IMMEDIATELY AFTER COMPLETION OF FIELD TELEVISING.
- C. ALL SANITARY PIPE SHALL BE IN ACCORDANCE WITH MATERIALS SPECIFIED IN TABLE A: ALLOWABLE PIPE MATERIAL SCHEDULE. INSULATION SHALL BE PROVIDED PER STATE PLUMBING CODES AS NECESSARY BASED ON PROPOSED DEPTH PER PLANS.
- D. CLEANOUTS SHALL BE PROVIDED FOR THE SANITARY & STORM SERVICES AT LOCATIONS INDICATED ON THE UTILITY PLAN. THE CLEANOUT SHALL CONSIST OF A COMBINATION WYE FITTING IN LINE WITH THE SANITARY/STORM SERVICE WITH THE CLEANOUT LEG OF THE COMBINATION WYE FACING STRAIGHT UP. THE CLEANOUT SHALL CONSIST OF A 4" OR 6" VERTICAL PVC PIPE WITH A WATERTIGHT REMOVABLE CLEANOUT PLUG. THE PVC CLEANOUT SHALL BE 4" IF THE SANITARY LINE IS 5" IN DIAMETER OR SMALLER AND 6" IF THE SANITARY LINE IS 6" IN DIAMETER OR BIGGER. AN 8" PVC FROST SLEEVE SHALL BE PROVIDED. THE BOTTOM OF THE FROST SLEEVE SHALL TERMINATE 12" ABOVE THE TOP OF THE SANITARY LATERAL OR AT LEAST 6" BELOW THE PREDICTED FROST DEPTH, WHICHEVER IS SHALLOWER. THE CLEANOUT SHALL EXTEND JUST ABOVE THE SURFACE GRADE IN LAWN OR LANDSCAPE AREAS WITH THE FROST SLEEVE TERMINATING AT THE GRADE SURFACE. THE CLEANOUT SHALL EXTEND TO 4 INCHES BELOW SURFACE GRADE IN PAVED SURFACES WITH A ZURN (Z-1474-N) HEAVY DUTY CLEANOUT HOUSING PLACED OVER THE TOP OF THE CLEANOUT FLUSH WITH THE SURFACE GRADE. IN PAVED SURFACES, THE FROST SLEEVE SHALL TERMINATE IN A CONCRETE PAD AT LEAST 6" THICK AND EXTENDING AT LEAST 9" FROM THE SLEEVE ON ALL SIDES, SLOPING AWAY FROM THE SLEEVE. THE CLEANOUT HOUSING SHALL BE CONSTRUCTED PER MANUFACTURERS' REQUIREMENTS. E. ALL PROPOSED WATER PIPE SHALL BE IN ACCORDANCE WITH MATERIALS SPECIFIED IN
- TABLE A: ALLOWABLE PIPE MATERIAL SCHEDULE. 6.5' MINIMUM COVER SHALL BE PROVIDED OVER ALL WATER PIPING UNLESS OTHERWISE SPECIFIED. F. ALL PROPOSED STORM PIPE SHALL BE IN ACCORDANCE WITH MATERIALS SPECIFIED IN
- TABLE A: ALLOWABLE PIPE MATERIAL SCHEDULE. SEE UTILITY PLANS FOR ALL STORM PIPE MATERIAL TYPES TO BE USED. PIPE SHALL BE PLACED MIN. 8' HORIZONTALLY FROM FOUNDATION WALLS.
- G. SANITARY, STORM, AND WATER UTILITY PIPE INVERTS SHALL BE CONSTRUCTED WITHIN 0.10' OF DESIGN INVERT ELEVATIONS ASSUMING PIPE SLOPE AND SEPARATION IS MAINTAINED PER THE UTILITY DESIGN PLANS AND STATE REQUIREMENTS. H. SITE UTILITY CONTRACTOR SHALL RUN SANITARY SERVICE TO A POINT WHICH IS A
- MAXIMUM OF 5' FROM THE EXTERIOR WALL OF THE FOUNDATION. SITE UTILITY CONTRACTOR SHALL RUN STORM SEWER FOR INTERNALLY DRAINED BUILDINGS TO A POINT WHICH IS A MAXIMUM OF 5' FROM THE EXTERIOR WALL OF THE FOUNDATION. SITE UTILITY CONTRACTOR SHALL RUN DOWNSPOUT LEADS TO BUILDING FOUNDATION AND UP 6" ABOVE SURFACE GRADE FOR CONNECTION TO DOWNSPOUT FOR ALL DOWNSPOUT TO RISER (DSR) CONNECTIONS. DOWNSPOUTS TO GRADE (DSG) SHALL BE PROVIDED WITH SPLASH BLOCKS AT THE DISCHARGE LOCATION. ALL DOWNSPOUT LOCATIONS SHOULD BE VERIFIED WITH ARCHITECTURAL PLANS AND DOWNSPOUT CONTRACTOR/GC PRIOR TO INSTALLATION OF DOWNSPOUT LEADS. DOWNSPOUT LEADS SHALL NOT UNDERMINE BUILDING FOUNDATIONS. SITE UTILITY CONTRACTOR SHALL RUN WATER SERVICE TO A POINT WITHIN THE FOUNDATION SPECIFIED BY THE PLUMBING PLANS. CONTRACTOR TO CUT AND CAP WATER SERVICE 12" ABOVE FINISHED FLOOR ELEVATION.
- I. ALL UTILITIES SHALL BE INSTALLED WITH PLASTIC COATED TRACER WIRE (10 TO 14 GAUGE SOLID COPPER, OR COPPER COATED STEEL WIRE). PLASTIC WIRE MAY BE TAPED TO PLASTIC WATER OR SEWER PIPE. IF ATTACHED, THE TRACER WIRE SHALL BE SECURED EVERY 6 TO 20 FEET AND AT ALL BENDS. TRACER WIRE SHALL HAVE ACCESS POINTS AT LEAST EVERY 300 FEET. TRACER WIRE SHALL TERMINATE IN ACCORDANCE WITH MANUFACTURER SPECIFICATIONS AT GRADE OR IN TERMINATION BOX PER LOCAL/STATE REQUIREMENTS.
- J. ALL UTILITIES SHALL BE INSTALLED PER STATE, LOCAL, AND INDUSTRY STANDARDS. WATER, SANITARY, AND STORM SEWER SHALL BE INSTALLED PER "STANDARD SPECIFICATION FOR SEWER AND WATER CONSTRUCTION IN WISCONSIN". THE EXCEL ENGINEERING DESIGN ENGINEER SHALL BE RESPONSIBLE FOR OBTAINING STATE PLUMBING REVIEW APPROVAL (IF REQUIRED). THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL OTHER PERMITS REQUIRED TO INSTALL WATER, SANITARY, AND STORM
- K. SEE PLANS FOR ALL OTHER UTILITY SPECIFICATIONS AND DETAILS.

# SHOP DRAWING SUBMITTALS **MATERIAL / INFORMATION** 31.10.00 - TELEVISING REPORTS OF EXISTING LATERALS

		• SANITARY
	2.	<u>31.20.00 - FILL</u>
		PRODUCT DATA
		SOURCE MATERIAL
	3.	32.10.00 (A) - AGGREGATE BASE & ASPHALT PAVEMENT
		HOT MIX ASPHALT SPECIFICATIONS
		AGGREGATE BASE
		PAVEMENT MARKINGS
	4.	32.20.00-CONCRETE AND AGGREGATE BASE
		DESIGN MIX
		AGGREGATE BASE
		COMPRESSION TEST RESULTS
		DETECTABLE WARNING PLATES
	5.	32.30.00 LANDSCAPING
		AMENDED SOIL MIX
		SEEDING PRODUCT DATA
		PLANTING SUBSTITUTION SCHEDULE
		MULCH PRODUCT DATA
λ		EROSION MATTING
		IRRIGATION CONTROL PRODUCT DATA
		IRRIGATION LAYOUT
	6.	<u>33.10.00 - SITE UTILITIES</u>
		STORM MANHOLES
		SANITARY PIPING MATERIALS
		GREASE INTERCEPTOR SHOP DRAWINGS
Ś		WATER FITTINGS & APPORTENANCES
$\langle$	7	
5	7.	
5		

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![](_page_174_Figure_0.jpeg)

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	BENCHMARKS BY B	BLEW & ASSOCIATES:	SUR
> > >	TBM #1 SET MAG-NAIL NORTHING: 162017.97 EASTING: 2420921.08 ELEVATION: 714.85'	TBM #2 SET MAG-NAIL NORTHING: 161816.80 EASTING: 2420774.15 ELEVATION: 717.81'	EXISTI ASSO ASSO REGA
			Ş
	BENCHMARK BY HE	EBERT ASSOCIATES:	< _
	BURY BOLT ON FIRE HYDRA CROOKS AVENUE AND EAST ELEV.=717.68	NT AT SOUTHEAST CORNER OF I 18TH STREET	
$\overline{}$			/
	GENERAL NO	OTE:	

CONTRACTOR TO FIELD VERIFY EXISTING UTILITIES AND PROVIDE INFORMATION TO DESIGN ENGINEER PRIOR TO CONSTRUCTION. DOWNSTREAM UTILITY CONNECTIONS **MUST** BE VERIFIED PRIOR TO CONSTRUCTION AND PROVIDED TO ENGINEER

# GENERAL NOTE:

BY GRAPHIC PLOTTING ONLY, THIS PROPERTY IS IN ZONE "X" OF THE FLOOD INSURANCE

OBTAIN APPROVAL FROM NEIGHBORING PROPERTY OWNER TO THE SOUTH FOR WORK ON THEIR PROPERTY PRIOR TO CONSTRUCTION.

# LEGEND & SYMBOLS

	FOUND MONUMENT AS NOTED
<ul><li>↓</li></ul>	SET MONUMENT AS NOTED
↔ P	COMPUTED POINT
6.	HANDICAP PARKING
×.	FIRE HYDRANT
*	LIGHT
S	SANITARY MANHOLE (SMH)
•	BOLLARD
Ø	
EIVI	
	ELECTRIC BOX
E	ELECTRIC MANHOLE
$\mathbf{W}$	MONITORING WELL
₩¥ ₩	WATER VALVE
	CIRCLE INLET (CI)
	GRATED INLET (GI)
$\bigcirc$	STORM MANHOLE (DMH)
T	TELEPHONE PEDESTAL
CO	
G	GASMETER
	SIGN
	FLAG POLE
₩ N	GAS METER
TP	TRAFFIC POLE
BOC	BACK OF CURB
FL	FLOW LINE
ТС	TOP OF CONCRETE
EC	EDGE OF CONCRETE
TA	TOP OF ASPHALT
FΔ	EDGE OF ASPHALT
NG	
FVC	
CMP	CORRUGATED METAL PIPE
RCP	REINFORCED CONCRETE PIPE
(M)	MEASURED/CALCULATED DIMENS
(R)	RECORD DIMENSION
N/F	NOW OR FORMERLY
BHL	BUILDING HEIGHT LOCATION
P.O.B.	POINT OF BEGINNING
	BOUNDARY LINE
	ADJOINER/TIE LINE
	FASEMENTLINE
	RIGHT-OF-WAY LINE
	OVERHEAD POWER LINE
- SS	UNDERGROUND SANITARY SEWE
- SD	UNDERGROUND STORM DRAIN LI
- UGE	UNDERGROUND ELECTRIC LINE
- BTL	UNDERGROUND TELEPHONE LINE
GAS ——	UNDERGROUND GAS LINE
- WL	UNDERGROUND WATER LINE
	MAJOR CONTOUR
	MINOR CONTOUR

P.O.

RVEY NOTE:

TING CONDITIONS SURVEY WAS COMPLETED BY BLEW & OCIATES, P.A. ON SEPTEMBER 30, 2024. CONTACT BLEW & OCIATES, P.A. AT (479) 443-4506 WITH ANY QUESTIONS ARDING SURVEY OR EXISTING CONDITIONS INFORMATION. LEGEND:

REMOVE PAVEMENT & BASE MILL EXISTING ASPHALT SURFACE, PROTECT BASE

ltem 4.e.

Always a Better Plan

100 Camelot Drive

Fond du Lac, WI 54935 920-926-9800

excelengineer.com

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В

**PROJECT INFORMATION** 

REMOVE FEATURES

![](_page_174_Picture_12.jpeg)

Έ	Y١	10	TES
$\langle$	A	$\rangle$	REMOVE BOLLARD
<	В	$\rangle$	FIELD VERIFY EXISTING STORM PIPE IS NOT IN USE. ABANDON IN PLACE. INFORM ENGINEER OF ANY DISCREPANCIES.
$\langle$	D	$\rangle$	MILL EXISTING ASPHALT SURFACE AND PROTECT BASE
<	E	$\rangle$	SAWCUT (AS NECESSARY) AND REMOVE CONCRETE AND BASE
<	F	$\rangle$	REMOVE GLASS TOWER
<	G	$\rangle$	REMOVE CURB. SAWCUT (AS NECESSARY)
<	Н	$\rangle$	REMOVE FLAG POLE
<	Ι	$\rangle$	PROTECT EXISTING CURB
<		$\rangle$	REMOVE STORM PIPING TO EXTENTS
5	к	$\mathbf{\Sigma}$	REMOVE LIGHT POLE
			REMOVE AND REPLACE PARKING STALL PAVEMENT MARKINGS (TYP) (SEE LAYOUT ON SHEET C1.1)
<	м	$\sum$	RECYCLE GRAVEL AS NEEDED
<	N	$\rangle$	REMOVE STORM STRUCTURE
$\langle$	0	$\rangle$	REMOVE POLE SIGN
<	P	$\rangle$	DEMOLISH BUILDING. CAP EXISTING UTILTIES. FOLLOW LOCAL AND STATE REQUIREMENTS FOR REMOVAL OF POSSIBLE ASBESTOS MATERIALS.
<	Q	$\rangle$	REMOVE ELECTRICAL EQUIPMENT. COORDINATE WITH UTILITY COMPANY.
<	R	$\rangle$	FIELD VERIFY AND TELEVISE EXISTING SANITARY LINE. INFORM ENGINEER C ANY DISCREPANCIES.
$\langle$	S	$\rangle$	REMOVE TREE IF NECESSARY.
$\langle$	T	$\rangle$	PROTECT STORM STRUCTURE
<	U	$\rangle$	REMOVE SANITARY LINE TO EXTENTS

 $\underline{\phantom{a}}$ CAP AND ABANDON EXISTING GAS SERVICE PER UTILITY COMPANY STANDARDS. COORDINATE WITH UTILITY COMPANY. 

GI#7

GI#8

GI#9 RIM ELEVATION: 716.51'

12" RCP INVERT N: 712.2'

GI#10

12" RCP INVERT E: 712.8'

GI#11

10" PVC INVERT S: 714.2'

4" PVC INVERT S: 712.9'

RIM ELEVATION: 716.32' 12" RCP INVERT W: 712.1'

# INVERT TABLE

*RIM ELEVATION: 718.67'* 10" *PVC INVERT E: 712.0'* 

RIM ELEVATION: 716.83' 12" RCP INVERT W: 712.2'

12" PVC INVERT N: 712.8' 12" RCP INVERT E: 712.2'

12" PVC INVERT S: 712.2' RIM ELEVATION: 716.03'

RIM ELEVATION: 714.29' RIM ELEVATION: 717.20'

30" RCP INVERT W: 707.8' 12" CMP INVERT E: 712.3'

30" CMP INVERT E: 707.2' 12" PVC INVERT N: 712.8' 18" RCP INVERT S: 708.0' 4" PVC INVERT N: 712.8'

SMH#2

RIM FI EVATION: 716 83

GI#1

12" RCP INVERT W: 715.5'

GI#2

GI#3

GI#4

GI#5

RIM ELEVATION: 716.19'

 
 DMH#1
 SMH#1
 GI#6
 GI#12

 RIM ELEVATION: 716.61'
 RIM ELEVATION: 714.44'
 RIM ELEVATION: 716.17'
 RIM ELEVATION: 717.33'

 30" CMP INVERT W: 706.6'
 8" PVC INVERT N: 705.9'
 12" CMP INVERT W: 711.6'
 12" CMP INVERT W: 712.6'
 30" CMP INVERT E: 706.6' 8" PVC INVERT E: 705.9' 24" CMP INVERT S: 708.8' 8" PVC INVERT S: 705.9'

V

DMH#2 FI EVATION: 715 03' 30" CMP INVERT W: 705.4' 8" PVC INVERT W: 708.4' 30" CMP INVERT E: 705.4' 8" PVC INVERT E: 708.5' 12" CMP INVERT S: 709.2' 12" CMP INVERT N: 709.2' DMH#3

RIM ELEVATION: 714.78' 12" CMP INVERT W: 709.8' 12" CMP INVERT E: 709.6' 24" CMP INVERT S: 707.8' 24" CMP INVERT N: 707.8'

DMH#4 RIM ELEVATION: 717.50' 12" CMP INVERT W: 711.3' 18" RCP INVERT N: 712.1' 12" CMP INVERT E: 711.5' 24" CMP INVERT S: 709.7 24" CMP INVERT N: 709.7'

DMH#5 RIM ELEVATION: 717.37' 18" RCP INVERT S: 708.0' 12" CMP INVERT W: 711.8' 12" CMP INVERT E: 711.9' 12" CMP INVERT S: 712.3' RIM ELEVATION: 716.43' 10" PVC INVERT W: 714.2' 12" CMP INVERT N: 712.0' 12" CMP INVERT N: 711.7'

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	$\mathbf{I}\mathbf{A}$	DL	

LINE	BEARING	DISTANCE	LINE	BEARING	DISTANCE
L1	N 89°33'09" E	50.01'	L1	S 89°16'20" E	50.00'
L2	N 00°31′01" W	50.00'	L2	N 00°39'30" E	50.00'
L3	N 89°33'09" E	100.00'	L3	S 89°16'20" E	100.00'
L4	N 00°31′01" W	50.00'	L4	N 00°39'30" E	50.00'
L5	N 89°29'33" E	48.74'	L5	S 89°16'20" E	48.75'
L6	N 89°29'33" E	5.01'	L6		
L7	S 89°36'55" W	60.00'	L7		
L8	S 89°29'33" W	55.29'	L8		
L9	S 89°36′21″ W	8.39'	L9		
L10	S 89°33'09" W	26.43'	L10		
L11	N 89°24'02" E	35.09'	L11		

ER LINE INE

![](_page_174_Picture_27.jpeg)

# CIVIL EXISTING SITE AND DEMOLITION PLAN

SCALE: 1"= 20'

![](_page_175_Figure_0.jpeg)

LEGEND:				
НАТСН	PAVEMENT SECTION	НАТСН	PAVEMENT SECTION	
	STANDARD ASPHALT		HEAVY DUTY CONCRETE	
	HEAVY DUTY ASPHALT		DUMPSTER PAD / APRON CONCRETE	
	SIDEWALK CONCRETE		MILL & OVERLAY EXISTING ASPHALT	

					Item 4.e.
		НАТСИ			
					Бусги
	STANDARD ASPHALT				EXCEL
	HEAVY DUTY ASPHALT		CONCRETE	RON	Always a Better Plan
	SIDEWALK CONCRETE		MILL & OVERLAY EXI ASPHALT	STING	100 Camelot Drive
YNOT	ËS				Fond du Lac, WI 54935 920-926-9800 excelengineer.com
$\langle 1 \rangle$	CONCRETE STOOP (SEE STRUCTUF	RAL PLANS FOR D	ETAILS)		
$\langle 2 \rangle$	RAISED WALK (SEE DETAIL)				PROJECT INFORMATION
$\overline{\left\langle 3\right\rangle}$	FLUSH WALK (SEE DETAIL)				
$\overbrace{6}$	ADA CURB RAMP (SEE DETAIL)				
$\sqrt{7}$	6" VERTICAL CURB (SEE DETAIL)				
$\overline{\ }$			)		
$\frac{\circ}{\circ}$					
					[30
			- D		<b>O</b> 241
	(CONTRACTOR TO VERIFY FINAL L	OCATION & DESI	GN PRIOR TO CONSTRU	ICTION)	
(12)	HANDICAP SIGN PER STATE CODE	(SEE DETAIL)			
	HANDICAP STALL & STRIPING PER	STATE CODES			
	PRECAST CONCRETE WHEEL STOP	(TYP.)	_		
15	PYLON SIGN (DETAILS, FINAL LOCATION, & APF	PROVAL BY SIGN	VENDOR)		
(16)	DUMPSTER ENCLOSURE (SEE ARCH	H PLANS FOR DET	TAILS)		
$\langle 17 \rangle$	6" CONCRETE BOLLARDS (TYP.) (SI	EE DETAIL)			
$\overline{)}$	CONCRETE FLUME (TYP)				
21					KS X
$\left  \begin{array}{c} 21 \\ \hline 22 \end{array} \right $					
		IOR TO MATCH	PARKING STALL STRIPIN	G	C B P
23	PAINT STRIPING (TYP). COLOR TO	MATCH PARKING	STALL STRIPING		
26	MENU BOARD				1 7
27	SPEAKER POST, CANOPY, AND BO	LLARD			
28	CLEARANCE BAR AND BOLLARD W	VITH DRIVE-THRU	SIGN		
29	BUILDING CANOPY (TYP.) (SEE ARG	CH PLANS)			
30	DRIVE-THRU LOOP (TYP.)				
31	DO NOT ENTER/THANK YOU SIGN				
$\overline{\langle}$ 32 $\rangle$	DIRECTIONAL SIGNAGE FOR DRIVI	E-THRU			PROFESSIONAL SEAL
33	MOBILE PICKUP PARKING STALL W (TYP.) SIGN BY SIGN VENDOR	/ITH ASSOCIATED	) SIGNAGE AND PAINT S	TRIPING	
$\overline{\langle} 34 \rangle$	LIGHT POLE (SEE SHEET C2.2, C3.1,	AND ELEC. PLAN	S FOR DETAILS)		
35	ENTRANCE SIGN				
27					
		~~~~~	~~~~~		SHEET DATES
38				{	SHEET ISSUE DEC. 6, 2024
<u></u>				{	
EXISTI	NG SITE DATA			2	AD1 JAN. 9, 2025 AD2 JAN. 27, 2025
		area (AC) 0.14	akea (SF) 6,021	RATIO 20.2%	CB1 MAY 7, 2025
OTAL IMPE	RVIOUS	0.40 0.54	17,508 23,529	58.6% 78.8%	
ANDSCAPE,	/ OPEN SPACE E	0.15	6,346 29,875	21.2%	
5-1		0.09	<i></i> کان ک	100.0%	
PROPO	DSED SITE DATA			2	
BUILDING FL	OOR AREA	area (AC) 0.05	AREA (SF) 2,241	RATIO 7.5%	
PAVEMENT (FOTAL IMPE	(ASP. & CONC.) RVIOUS	0.43 0.48	18,850 21,091	63.1% 70.6%	
ANDSCAPE, PROJECT SIT	/ OPEN SPACE E	0.20	8,784 29 875	29.4%	240206000
		0.09		100.0%	
					SHEET NUMBER
	SC/ 201	ALE: 1"= 20'	20'	IORTH 10	
				+0	
		CI	VIL SITE PI	_AN	

К	FYNOT	FS			
	$\overline{\langle 1 \rangle}$	CONCRETE STOOP (SEE	STRUCTURAL PLANS FOR DET	AILS)	
	$\left\langle 2 \right\rangle$	RAISED WALK (SEE DETAIL)			
	$\overline{\langle}_{3}$	FLUSH WALK (SEE DET)	AIL)		
		ADA CURB RAMP (SEE	DETAIL)		
	$\sqrt{\frac{3}{7}}$	6" VERTICAL CURB (SEE			
	$\frac{1}{8}$				
	$\frac{3}{10}$				
		CONCRETE TRANSFORM	MER PAD BY UTILITY SUPPLIER		
			IFY FINAL LOCATION & DESIGN	N PRIOR TO CONSTRU	ICTION)
		HANDICAP SIGN PER S	IATE CODE (SEE DETAIL)		
	$\langle 13 \rangle$	HANDICAP STALL & ST	RIPING PER STATE CODES		
		PRECAST CONCRETE W	HEEL STOP (TYP.)		
	(15)	PYLON SIGN (DETAILS, FINAL LOCAT	ION, & APPROVAL BY SIGN VE	NDOR)	
		DUMPSTER ENCLOSUR	E (SEE ARCH PLANS FOR DETAI	iLS)	
		6" CONCRETE BOLLARE	DS (TYP.) (SEE DETAIL)		
		CONCRETE FLUME (TYP)		
	21	DETECTABLE WARNING	PLATE PER STATE CODE		
	22	TRAFFIC FLOW ARROW	S (TYP). COLOR TO MATCH PA	RKING STALL STRIPIN	G
	23	PAINT STRIPING (TYP). COLOR TO MATCH PARKING STALL STRIPING			
	26	MENU BOARD			
	27	SPEAKER POST, CANOPY, AND BOLLARD			
	28	CLEARANCE BAR AND BOLLARD WITH DRIVE-THRU SIGN			
	29	BUILDING CANOPY (TY	P.) (SEE ARCH PLANS)		
	30	DRIVE-THRU LOOP (TYP	2.)		
	31	DO NOT ENTER/THANK	YOU SIGN		
	32	DIRECTIONAL SIGNAGE	FOR DRIVE-THRU		
	33	MOBILE PICKUP PARKIN (TYP.) SIGN BY SIGN VE	NG STALL WITH ASSOCIATED S NDOR	IGNAGE AND PAINT S	TRIPING
	34	LIGHT POLE (SEE SHEET	C2.2, C3.1, AND ELEC. PLANS F	FOR DETAILS)	
	35	ENTRANCE SIGN			
	36	NO EXIT SIGN			
	37	STOP SIGN WITH NO RIGHT TURN			
	38	REVISED EXISTING PARKING (TYP.)			
K	39	18" CONCRETE CURB A	ND GUTTER (SEE DETAIL)		
<	EXISTIN	NG SITE DAT	۹		
5			AREA (AC)	AREA (SF)	RATIC
\$	PAVEMENT (A	ASP. & CONC.)	0.14 0.40	6,021 17.508	20.2% 58.6%
>	TOTAL IMPER	RVIOUS	0.54	23,529	78.8%
>	LANDSCAPE/	OPEN SPACE	0.15	6,346	21.2%
5	PROJECT SITE		0.69	29,875	100.0%
ζ					
5	PROPC	<u>SED SITE</u> DA	TA		
>			AREA (AC)	AREA (SF)	RATIC
5	BUILDING FLO	DOR AREA	0.05	2,241	7.5%
)	PAVEMENT (A	ASP. & CONC.)	0.43	18,850	63.1%
(0.48	21,091	70.6%
(LANDSCAPE/	ST LIN SFACE	0.20	8,/84	29.4%

PROJECT SITE

GENERAL NOTES:

- HANDICAP STALL AND ACCESS AISLES SHALL NOT EXCEED A SLOPE OF
 1.50% IN ANY DIRECTION. HANDICAP STALL & ACCESS AISLES SHALL CONFORM TO ADA REQUIREMENTS (CURRENT EDITION)
- ALL SIDEWALKS SHALL NOT EXCEED A MAXIMUM CROSS SLOPE OF 1.50% AND RUNNING SLOPE OF 4.50% UNLESS OTHERWISE SPECIFIED.
- CONTRACTOR SHALL PROVIDE STABILIZED CONSTRUCTION ENTRANCE AT CONSTRUCTION ENTRANCE FOR PROPOSED IMPROVEMENTS AS REQUIRED PER CODE.
- CONTRACTOR SHALL PROVIDE CONCRETE WASHOUT AS REQUIRED PER CODE. FINAL LOCATION TBD BY CONTRACTOR.
- CONTRACTOR SHALL PROVIDE TEMPORARY INLET PROTECTION FOR ALL CURB INLETS & CATCH BASINS ONSITE & OFFSITE IMMEDIATELY DOWNSTREAM OF THE PROJECT SITE PER LOCAL CODE.
- CONTRACTOR SHALL NOTIFY THE DIRECTOR OF PUBLIC WORKS WITHIN 48 HOURS OF COMMENCING ANY LAND DISTURBING CONSTRUCTION ACTIVITY.

KEYNOTES				
EC 1	SILT FENCE			
EC 2	DITCH CHECK			
EC 3	STABILIZED CONSTRUCTION ENTRANCE			
EC 4	INLET PROTECTION			
EC 5	CONCRETE WASHOUT			
EC 6	SEDIMENT LOG			

SCALE:	1"= 20'		NORTH
20'	() 2	0' 40'

CIVIL GRADING AND EROSION CONTROL PLAN

SHEET NUMBER

C1.2

GENERAL NOTES:

• CONTRACTOR TO FIELD VERIFY EXISTING UTILITIES AND PROVIDE INFORMATION TO DESIGN ENGINEER PRIOR TO CONSTRUCTION. DOWNSTREAM UTILITY CONNECTIONS **MUST** BE VERIFIED PRIOR TO CONSTRUCTION AND PROVIDED TO ENGINEER.

GAS:

WE ENERGIES

KATHY MEYER

800-714-7777 (OFFICE) 262-305-4772 (CELL)

ELECTRIC & WATER:

920-766-5721 kumail@ku-wi.org

SCALE: 1"= 20'

KAUKAUNA UTILITIES

CIVIL UTILITY PLAN

kathy.meyer@we-energies.com

FOLLOW GEOTECH RECOMMENDATIONS FOR UNDERDRAINS AT INLET LOCATIONS.

TREE PLANTING DETAIL NOT TO SCALE

	LANDSCA	PING PLANTING SCHEDU	JLE	
SYMBOL	COMMON NAME	BOTANICAL NAME	PLANTED SIZE	QUANTITY
	DEC	CIDUOUS TREES		
Ó	Red Maple	Acer rubrum	2"	2
÷	Red Oak	Quercus rubra	2"	1
\odot	Crape Myrtle	Lagerstroemia indica	2"	2
	DEC	IDUOUS SHRUBS		
畿	Bush Morning Glory	Conolvulus cneorum	5 gal pot	8
Ŵ	Adams Needle	Yucca flaccida	5 gal pot	8
0	Barberry	Berberis spp.	1 gal pot	12
		PERENNIALS		
**	Ajuga	Ajuga repans	1 gal pot	7
*	Canadian Juniper	Juniperus communifs	1 gal pot	(7)
*	Daylilies 'Stella de Oro'	Hemerocallis 'Stella de Oro'	1 gal pot	5
	<u>E</u> 2	XISTING TREES		
\odot	EXISTING TREE			3
	GRA	ANITE BOULDERS		
	24"-30" Diameter Granite Boulder		24"-30"	2

	DEAD PLANTINGS SHALL BE REMOVED AND REPLACED WITHIN 30 DAYS. IN CASES WHERE PLANTINGS CAN'T BE ESTABLISHED IN 30 DAYS, AN APPROPRIATE TIMELINE SHALL BE APPROVED BY THE COMMUNITY DEVELOPMENT DEPARTMENT DESIGNEE.			
		НАТСН К	EY:	
		HATCH	LANDSCAPE MATERIAL	
			MINERAL MULCH	
MU	LCH		SEEDED LAWN	
PEA	T MOSS		EROSION MATTING (NAG C125) OVER SEEDED LAWN (SWALE BOTTOMS)	

ltem 4.e.

LANDSCAPING CALCULATIONS				
REQ. PLANTS	PLANTS PROVIDED			
TREE/75 LF WITHIN FRONT YARD SETBACK DJACENT TO STREET, INGRESS/EGRESS EXCLUDED 99 LF-53 LF)/75 LF=6 TREES	3 TREES PROPOSED, 3 TREES EXISTING 6 TREES TOTAL			
SHRUB/5 LF OF BUILDING STREET FRONTAGE 36 LF/5 LF=28 SHRUBS	28 SHRUBS			



INLET PROTECTION DETAIL NOT TO SCALE

GENERAL NOTES

DETAILS OF CONSTRUCTION, MATERIALS AND WORKMANSHIP NOT SHOWN ON THIS DRAWING SHALL CONFORM TO THE PERTINENT REQUIREMENTS OF THE WISCONSIN DEPARTMENT OF NATURAL RESOURCES TECHNICAL STANDARD NO. 1053 (CHANNEL EROSION MAT).

VARIATIONS IN THE DIMENSIONS OR MATERIALS SHOWN HEREON SHALL BE PERMITTED IF THEY PROVIDE EQUIVALENT PROTECTION AND MATERIAL STRENGTH AND IF PRIOR APPROVAL OF THE ENGINEER IS OBTAINED.

LAP JOINTS SHALL NOT BE PLACED IN THE BOTTOM OF V-SHAPED DITCHES.

JUNCTION SLOTS ON ADJACENT STRIPS OF MATTING SHALL BE STAGGERED A MINIMUM OF 4 FEET APART.

EDGES OF EROSION MAT SHALL BE IMPRESSED IN THE SOIL. EROSION MAT SHALL PAID BY THE SQUARE YARD INSTALLED.

EROSION MAT OVER SEEDING

JUNCTION OR ANCHOR SLOTS SHALL BE AT MINIMUM INTERVALS OF 100 FEET ON GRADES UP TO AND INCLUDING 3%, AND 50 FEET ON GRADES EXCEEDING 3%.

NOTE: SEE SPECIFICATIONS FOR MATTING TYPE









CIVIL DETAILS

ltem 4.e.





LIGHT POLE FOOTING

PORTAL PLACEMENT DETAIL

CIVIL DETAILS