

# Development Review Committee

October 12, 2023 at 2:00 PM Howey-in the-Hills Town Hall 101 N. Palm Ave., Howey-in-the-Hills, FL 34737

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## **AGENDA**

CALL TO ORDER ATTENDANCE

#### **NEW BUSINESS**

1. Discussion: Mission Rise Development PUD Rezoning Submittal

#### **PUBLIC COMMENTS**

Any person wishing to address the Development Review Committee and who is not on the agenda is asked to speak their name and address. Three (3) minutes is allocated per speaker.

#### **ADJOURNMENT**

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**Topic: Development Review Committee** 

Time: Oct 12, 2023 02:00 PM Eastern Time (US and Canada)

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Please Note: In accordance with F.S. 286.0105: Any person who desires to appeal any decision or recommendation at this meeting will need a record of the proceedings, and that for such purposes may need to ensure that a verbatim record of the proceedings is made, which includes the testimony and evidence upon which the appeal is based. The Town of Howey-in-the-Hills does not prepare or provide this verbatim record. Note: In accordance with the F.S. 286.26: Persons with disabilities needing assistance to participate in any of these proceedings should contact Town Hall, 101 N. Palm Avenue, Howey-in-the-Hills, FL 34737, (352) 324-2290 at least 48 business hours in advance of the meeting.

# GRIFFEY ENGINEERING, INC.

October 9, 2023 Mission Rise PUD Engineering Review Comments Page 1

## Traffic Study

- 1. Figures in the report are missing. They need to be included.
- 2. For the future condition analysis of the intersection of SR 19 & CR 48, evaluate for a roundabout as well as signal timing adjustment.

## Concept Plan

1. The county has expressed concerns regarding the connection to Orange Blossom Road. Even though it is a county maintained, public road, Orange Blossom is structurally substandard with insufficient right-of-way for improvements. While they recognize and support the practice of interconnecting new roads to existing roads, in this case the additional traffic would accelerate the degradation of Orange Blossom. The tie-in of this development to Orange Blossom should be as an emergency only connection until such time that Orange Blossom meets county standards.

## **Development Agreement**

1. Section 1. (j) Transportation, Streets and Sidewalks: Revels Road and the Spine Road must have a minimum 90-foot right-of-way, 2' curb and gutter, and a minimum 32-foot-wide pavement with 12-foot travel lanes and 4' curb lanes.

### Recommended Improvements

- 1. The traffic study identifies three intersections along SR 19 that will need to be signalized in the future (SR 19 & Central Ave., SR 19 & Revels Rd., and SR 19 & CR 455). The Development Agreement has a section that addresses proportionate share payment for off-site impacts. In the study's mitigation analysis it states: "In lieu of contributing a proportionate share to the three (3) intersections needing new traffic signals, the developer is recommending to construct the new traffic signal at SR 19 and Revels Road, which serves as the main access to the project." This is a reasonable mitigation alternative provided that there is a binding commitment for the developer to construct (or fund) the signal when it is deemed warranted by FDOT. This would be in addition to the turn lanes that the development will need to install at the intersection (right & left on SR 19, and right & through/left on EB Revels).
- 2. The right and left turn lane improvements along Number 2 Road will result in 12' through lanes along most of the projects frontage. The paving work would normally stop just 256' feet short of the project's eastern property line. This additional length should also be widened to 12' lanes. This would result in 12' through lanes across the projects entire frontage.



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PH: 386.316.8426

### **MEMORANDUM**

TO: Howey-in-the-Hills Development Review Committee

CC: J. Brock, Town Clerk

FROM: Thomas Harowski, AICP, Planning Consultant SUBJECT: Mission Rise Development Proposal 4<sup>th</sup> Revision

DATE: October 9, 2023

The Town has received the fourth revision of the Mission Rise development proposal which incorporates comments from the Development Review Committee into the conceptual development plan and the development agreement. This should be the final DRC review of the plan before referring the item to the Planning Board for consideration. The review is based on the fourth submittal plus the comments from the September DRC meeting.

## **Concept Plan**

- 1. The staff recommended moving the stormwater retention out of the central green space and park area. The current concpet plan proposal retains these items without revision.
- 2. The staff has commented on the proposed lot sizes relative to the lot size ranges in projects recently approved by the Town Council.
- 3. The staff requested the applicant address the design of through lots which abut the central collector but access from a different street. The applicants propose a 10-footlandscaped buffer.
- The staff asked for design proposals for mitigation of the garage-scape view of units on 55-foot wide lots with front entry. No comments were received on this item.
- 5. The applicant was asked to provide a timing for the construction of the centralcollector road. Will the road be constructed by phase or will the road be constructed entirely within one phase?

The applicants are requested to provide a response to items four and five. The other comments will become part of the staff report for Planning Board.

# **Development Agreement**

The revisions made to the development agreement track the recommendations from the September DRC meeting.

If the Town Council approves the development agreement and project on first reading, the development agreement will be provided to the Town Attorney for a legal review.



September 28, 2023

Thomas A. Harowski, AICP Town of Howey-in-the-Hills 101 N. Palm Ave., P.O. Box128, Howey-In-The-Hills, Florida 34737

RE: Mission Rise PUD

Dear: Mr. Harowski

Enclosed please find responses to Staff's comments below in bold. The following items are resubmitted in response to Staff's comments:

- 1. Revised Conceptual Land Use Plan
- 2. Revised Development Agreement
- 3. Revised Traffic Impact Analysis

# PLANNING REVIEW COMMENTS: CONCEPT PLAN:

1. The project still fails to meet the 15% non-residential land area requirements of the Village Mixed Use land use classification. The stormwater areas allocated to the non-residential use calculation are in fact engineering elements of other land uses. The civic land use, the amenity centers and the park areas can count toward the non-residential land use as proposed. Staff is willing to include the major trail area that falls outside the central collector road right-of-way (so long as this area is not already counted as park area).

RESPONSE: Please see page 4 of the Conceptual Land Use Plan, which provides distinct details of the non-residential land area proposed within the development. Stormwater areas have been excluded from the calculation. An additional park area is proposed in the southern part of Phase 2.

2. The proposed recreational facilities have been better detailed, but the "regional" park still fails to meet the definition included in the comprehensive plan. Perhaps revising the name to a neighborhood facility is more appropriate given that the park is unlikely to draw significant interest from residents outside the neighborhood.

RESPONSE: The "regional" park has been renamed to "neighborhood" parks. In turn, the previous "neighborhood parks" have been renamed to "mini" parks. The mini parks are planned as recreational space for the use of the residents of the community. The neighborhood parks are intended to serve the larger community and facilitate access and use of the multiuse trail system.

3. The area in the center designated as regional park is a bonafide park area. The highlighted areas in Phase 3 and at the south end of Phase 2 are just open space and should not be

counted as park area.

RESPONSE: The proposed park areas have been detailed, in terms of the proposed features/amenities on page 3 of the Conceptual Land Use Plan.

4. The applicant has elected to retain stormwater retention areas within the central core area which staff recommended for tree preservation and green space. As noted in our comments last time, the retention ponds are part of the residential land use and should be located there. Be advised this item will be a comment in the staff report.

**RESPONSE:** Acknowledged.

5. The park area developments have been detailed but outside of the amenity centers are essentially passive designs. As an additional item, the applicant could consider including some court activities as part of the overall program. We renew our suggestions for repurposing the small residential development at the southeast corner of Phase 2 as a central community facility.

RESPONSE: Active recreational amenities may be provided in the park area in the southern part of Phase 2. The planned facilities/amenities and design of the park areas are intended to be further detailed at the subdivision/site plan process.

6. The applicant needs to address how the double-frontage lots located in Phase 2 and Phase 3 will be addressed. These lots have access from a parallel street so that the rear yards of these properties will front on the central collector road. Perhaps some sort of buffer such as a landscaped berm or wall is appropriate.

RESPONSE: The double-frontage lots will have a 10' landscaped buffer along the Collector Road to protect views from this roadway.

7. For the 55-foot-wide lots where no alley access is proposed, what design options are suggested to reduce the impact of a garage-dominate streetscape.

RESPONSE: In accordance with LDC Section 4.06.02.A.3., at least 25% of the lots in the development will have to provide recessed garages. Further, side-loaded garages are encouraged, as stated in the proposed Development Agreement.

**8.** The unit totals provided for the phase allocations do not add correctly on the table provided.

RESPONSE: The unit totals have been revised on the Phase Development Table. Please see page 2 of the Conceptual Land Use Plan.

**9.** The note to the table needs to be removed. Movement of units between phases will be considered a major amendment of the development agreement. As an alternative the applicant could propose language in the development agreement allowing for a specific level of shifting units between phases for Town Council consideration.

RESPONSE: Acknowledged. The note has been removed and language related to movement of units between phases will be added to the Development Agreement.

10. At the last DRC meeting the applicant was requested to provide a timing proposal for construction of the central collector road. The agreement needs to include a proposed timing.

**RESPONSE: Please see the revised Development Agreement.** 

11. Map 2 seems to be unclear. Phase lines are similar to the symbols for pathways, parking, non-residential areas etc. Perhaps the information can be divided into more maps that will present a clearer summary.

RESPONSE: Please see page 2 of the Conceptual Land Use Plan where the phase line type has been updated for better readability.

### **PUD/DEVELOPMENT AGREEMENT:**

1. On page two the development agreement states the project is 592 units while the concept plan has 499. These documents need to be in agreement.

**RESPONSE: Please see the revised Development Agreement.** 

2. On page three the minimum lot width at the building line needs to be 75 feet for the 75 x 120 lot size.

**RESPONSE: Please see the revised Development Agreement.** 

3. On page three the wetland buffer needs to reflect the town requirements in Sec. 3.02.03C as well as the water management district and DEP requirements. The Town's requirements vary in some respects from the state requirements.

**RESPONSE:** Please see the revised Development Agreement.

4. On page four, the language setting the timing for the Town to ask for utility upgrades is still not satisfactory. The proposed 270 days from approval of the plan is still not what we need. The timing should be triggered by the application for final subdivision approval for the phase of the project proposed. The final subdivision approval gives authorization to construct subdivision improvements. The Town should be required to make its needs and commitments at this point. If final subdivision approval is sought by phase, then the Town's opportunity to seek utility line upgrades should attach to each phase.

**RESPONSE: Please see the revised Development Agreement.** 

5. On page 6, the Town is not requiring all roads to be public. The applicant has the choice to use gated access for the project or for sub-areas within the project. While the collector road should remain with full public access, the applicant may wish to revise the proposed language to preserve the option for gated areas.

**RESPONSE: Please see the revised Development Agreement.** 

6. On page eleven, the termination language related to sewer service acquisition should be modified to include other options than the CLCDD.

**RESPONSE:** Please see the revised Development Agreement.

### TRAFFIC IMPACT ASSESSMENT:

1. Defer to the Town engineer comments

**RESPONSE:** Acknowledged.

# ENGINEERING REVIEW COMMENTS: TRAFFIC STUDY:

1. The conceptual land use plan states the maximum number of lots is 499. The traffic study and the development agreement states 592 lots. All three need to be the same.

RESPONSE: The Traffic Impact Analysis and Development Agreement have been revised to state a maximum of 499 units.

2. The methodology states that Lake Hills & Watermark are to be included in the background traffic projection. The submitted study left these developments out.

**RESPONSE:** Please see the revised Traffic Impact Analysis.

3. For the future condition intersection analysis for SR 19 & Revels Rd. include right & left turn lanes on SR 19 and a right turn lane on revels.

**RESPONSE:** Please see the revised Traffic Impact Analysis.

4. For the future condition intersection analysis for the Spine Rd. and Number 2 Rd., include right & left turn lanes on Number 2 Rd.

**RESPONSE:** Please see the revised Traffic Impact Analysis.

5. Per the MPO TIS Guidelines the study needs to include a section for Mitigation Strategies. This needs to address the road segments and intersections with deficiencies. For unsignalized intersections, side streets with deficient delays need to be evaluated for mitigation. Also, the narrow width of Number 2 Road needs to be addressed in this section. While capacity is not an issue, operational safety is.

**RESPONSE: Please see the revised Traffic Impact Analysis.** 

6. There is no proposed widening of SR 19 at Central Avenue as stated in the study.

**RESPONSE: Please see the revised Traffic Impact Analysis.** 

7. Based on Lake County's requirement for turn lane widening on Number 2 Road (all on the south side) the length of tapers will need to be twice the standard length.

**RESPONSE: Please see the revised Traffic Impact Analysis.** 

## **CONCEPT PLAN:**

1. The main N-S spine road and realigned Revels Road should not have driveway connections or on-street parking. They should have full pedestrian accommodation including the multi-use trail and raised crosswalks/speed tables at key points along its length connecting the trail and sidewalks to amenity, open space, and park areas.

RESPONSE: On-street parking/driveway connections along the Collector Road have been removed from the plan. All lots abutting the Collector Road will have access from another local street or alley.

2. The curb & gutter for the neighborhood roads should 2' wide Type F or Drop Curb.

RESPONSE: Please see page 6 of the Conceptual Land Use Plan, where the curb and gutter has been updated to 2' width.

### **Development Agreement**

1. Section 1. (f) Wetlands: Wetland impacts and buffering shall also be subject to the Town's land development regulations as well as the St Johns River Water Management District.

**RESPONSE: Please see the revised Development Agreement.** 

2. Section 1. (j) Transportation, Streets and Sidewalks: Revels Road and the Spine Road must have a minimum 90-foot right-of-way, 2' curb and gutter, and a minimum 32-foot-wide pavement with 12-foot travel lanes and 4' curb lanes.

**RESPONSE:** Please see the revised Development Agreement.

Thank you in advance for your consideration of the above information. If you require further information, please do not hesitate to contact me at 607.216.2390 or <a href="mailto:rlopes@rviplanning.com">rlopes@rviplanning.com</a> Sincerely,

RVi Planning + Landscape Architecture

Rhea Lopes, AICP Project Manager

#### **Enclosures**

cc: Alexis Crespo, RVi Planning + Landscape Architecture
Jason Humm, ASF TAP FL I LLC
Jonathan Huels, Lowndes Law Group

•

### **MISSION RISE**

Project № 23017.1, v1.2 September 2023

# TRAFFIC IMPACT ANALYSIS TOWN OF HOWEY-IN-THE HILLS FLORIDA

# Prepared by:



# **Traffic & Mobility Consultants**

3101 Maguire Boulevard, Suite 265 Orlando, Florida 32803 www.trafficmobility.com (407) 531-5332

# Prepared for:

ASF TAP Florida I, LLC 1170 Peachtree Street Northeast, Suite 1150 Atlanta, Georgia 30309

#### **EXECUTIVE SUMMARY**

**Project Information** 

Name: Mission Rise

Location: West of SR 19 (South Palm Avenue), east of Silverwood Lane, and south

of Number 2 Road in the Town of Howey-in-the-Hills, Lake County, Florida

Description: 499 Single Family Residential Units

Access Plan: One (1) full access at the intersection of Number 2 Road and Spine Road

One (1) full access at the intersection of SR 19 and Revels Road

One (1) full access at the intersection of Revels Road and Orange Blossom

Road (expected to carry limited traffic)

**Findings** 

Trip Generation: 4,428 Daily Trips / 322 AM Peak Hour Trips / 451 PM Peak Hour Trips

Roadway Capacity: The segments of SR 19, from Lane Park Road to Central Avenue and from

CR 455 to CR 478 are projected to operate over their capacities at the

project buildout.

Intersection Capacity: The intersections of SR 19 and CR 48, SR 19 and Central Avenue, SR 19

and Revels Road, and SR 19 and CR 455 are projected to experience delays in the buildout condition. The project does not have a significant

impact on the intersections.

Recommendations

Intersection Improvements:

Retime the signal at the intersections of SR 19 and CR 48 to maintain

LOS standards.

Provide traffic signals on SR 19 at Central Avenue, Revels Road, and CR 455 to maintain LOS standards. A signal warrant analysis is

recommended and should be provided in separate reports.

Construct a 430-foot northbound left turn lane and a 405-foot southbound

right turn lane at the intersection of SR 19 and Revels Road.

Construct a 655-foot westbound left turn lane and a 420-foot eastbound right turn lane at the intersection of Number 2 Road and Spine Road.



### PROFESSIONAL ENGINEERING CERTIFICATION

I hereby certify that I am a Professional Engineer properly registered in the State of Florida practicing with Traffic & Mobility Consultants LLC, a corporation authorized to operate as an engineering business, CA-30024, by the State of Florida Department of Professional Regulation, Board of Professional Engineers, and that I have prepared or approved the evaluations, findings, opinions, conclusions, or technical advice attached hereto for:

PROJECT: Mission Rise

**LOCATION:** Town of Howey-in-the-Hills, Florida

CLIENT: ASF TAP Florida, LLC

I hereby acknowledge that the procedures and references used to develop the results contained in these computations are standard to the professional practice of Transportation Engineering as applied through professional judgment and experience.



THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY

Davidson

Charlotte N Digitally signed by Charlotte N Davidson Date: 2023.09.22 16:18:56 -04'00'

ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

TRAFFIC & MOBILITY CONSULTANTS LLC 3101 MAGUIRE BOULEVARD, SUITE 265 ORLANDO, FLORIDA 32803 CERTIFICATE OF AUTHORIZATION CA-30024 CHARLOTTE N. DAVIDSON, P.E. NO 50725

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### 1.0 INTRODUCTION

This Traffic Impact Analysis (TIA) was conducted to assess the impact of the proposed Mission Rise development in the town of Howey-in-the-Hills, Florida. The proposed development consists of 499 single-family units with an anticipated buildout year of 2033. This study conforms to the Tier 2 TIA requirements of the Town of Howey-in-the-Hills and Lake County. The analysis was prepared in accordance with the approved methodology. The study has been updated to incorporate comments received from the Town of Howey-in-the-Hills. The methodology and the response to comments letter are included in **Appendix A**.

The site is located east of Silverwood Lane, west of SR 19 (South Palm Avenue), and south of Number 2 Road. **Figure 1** depicts the site location and the surrounding transportation network.

The development will be accessed via the intersections of Number 2 Road and Spine Road (future road), SR 19 and Revels Road, and Revels Road and Orange Blossom Road. The preliminary development site plan is provided in **Appendix B**.

Data used in the analysis consisted of site plan/development information provided by the project engineers, AM and PM peak hour intersection traffic counts obtained by Traffic & Mobility Consultants LLC, FDOT's 2023 Multimodal Quality/Level of Service (MQ/LOS) Handbook and roadway capacities obtained from the 2022 Lake County Congestion Management Process (CMP) Database.

### 1.1 Study Area

The project study area was established based on the standard requirements of the Lake Sumter Metropolitan Planning Organization (LSMPO) methodology and the Town of Howey-in-the-Hills Land Development Code (LDC). In accordance with the requirements of Tier 2 TIA methodology, the impact area includes roadway segments and intersections within a 4.55-mile radius of the site in addition to roadways where the development traffic is expected to consume 5% or more of their adopted Level of Service (LOS) capacities. The roadway segments characteristics were obtained from the 2022 Lake County Congestion Management Process (CMP) Database and 2023 FDOT Multimodal Quality/Level of Service (Q/LOS) Handbook Appendix B, included in Appendix C. The project study area determination is provided in Table 1, as determined in the approved methodology.



Figure 1 Site Location Map



# Table 1 **Study Area**

No Area   Median   Type   Limit   Std   Cap   Dir   Dist   Trips   1-Mile? ** Cap   S	udy?
SR 19 to	
CR 561	
CR 561 to   CR 48   CR 48   CR 48   CR 48   CR 48   CR 48   CR 561 to   CR 5	NO
CR 561A	NO
CR 561A	NO
US 27 to Lime Ave	NO
Lime Ave         1240         2         U         Undivided         40         D         1,080         WB         15%         25         NO         2.3%           Lime Ave to SR 19         1250         2         U         Undivided         40         D         1,080         EB WB         2%         6 WB         NO         0.6%           SR 19         1260         2         U         Undivided         40         D         840         EB WB         3%         5 WB         NO         0.6%           Ranch Rd to CR 448A         1270         2         R         Undivided         40         C         410         EB WB         3%         5 WB         NO         1.2%           CR 448 to CR 448 to South Astatula City Limit         1420         2         U         Undivided         40         D         620         NB SB         3%         9 WB         NO         0.8%           South Astatula City Limit         1430         2         III         Undivided         40         D         1080         NB SB         3%         9 WB         NO         0.8%	
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SR 19         1250         2         U         Undivided         40         D         1,080         WB         2%         3         NO         0.3%           CR 561 to Ranch Rd         1260         2         U         Undivided         40         D         840         EB WB         3%         5 9         NO         0.6%           Ranch Rd to CR 448A         1270         2         R         Undivided         40         C         410         EB WB         3%         5 9         NO         1.2%           CR 448 to CR 48 to South Astatula City Limit         1410         2         U         Undivided         50         D         1,080         NB SB         0%         0 0         NO         0.0%           South Astatula City Limit         1430         2         U         Undivided         40         D         1080         NB SB         3%         9 9         NO         0.8%	NO
SR 19	NO
Ranch Rd	NO
Ranch Rd Ranch Rd to CR 448A	NO
CR 448A	NO
CR 448A  CR 561  CR 448 to	NO
CR 448 to CR 48       1410       2       U       Undivided       50       D       1,080       NB SB       0%       0       NO       0.0%         CR 48 to South Astatula City Limit       1420       2       U       Undivided       40       D       620       NB SB       3%       9       NO       1.5%         South Astatula City Limit       1430       2       II       Undivided       40       D       1,080       NB SB       3%       9       NO       0.8%	NO
CR 48	
CR 48       1410       2       U       Undivided       50       D       1,080       SB       0%       0       NO       0.0%         CR 48 to       South Astatula City Limit       1420       2       U       Undivided       40       D       620       NB       3%       9       NO       1.5%         South Astatula City Limit       1430       2       II       Undivided       40       D       1 080       NB       3%       9       NO       0.8%	NO
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	NO
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Howey Cross Rd 1440 2 R Undivided 35 C 470 SB 2% 3 NO 0.6%	NO
Howey Chase Pd to NR 6 0.0%	
Turnpike Rd / CR 561A 1450 2 R Undivided 40 C 640 SB 2% 3 NO 0.5%	NO
SR 19	
Lane Park Pd to NR 38 4.1%	
CR 48 3040 2 U Undivided 55 D 920 SB 23% 65 NO 7.1%	YES
CR 48 to NB 42 6.0%	
Central Ave 3050 2 U Undivided 40 D 700 SB 25% 71 NO 10.1%	YES
Central Ave to NB 1/2 118%	
CR 455 3060 2 U Undivided 35 D 1,200 SB 50% 84 YES 7.0%	YES
CD 455 to NB 00 22 09/	
US 27 / SR 25 3070 2 R Undivided 55 C 450 SB 35% 58 NO 12.9%	YES
US 27 / SR 25	
to CR 478 3080 2 R Undivided 55 C 450 SB 20% 33 NO 7.3%	YES
SR 91 (Florida Turnpike)	
LIS 27/SP 25 to ER 17 0.8%	
US 27/SR 25 to US 27/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   B   2,230   US 27/SR 25/SR 19 Interchange   3566   4   U   Freeway   70   US 27/SR 25/SR 25	NO
US 27/SR 25	
SR 19 to 0.8%	
CR 561 3830 4 U Divided 55 D 3,280 WB 15% 43 NO 1.3%	NO
Central Ave	
N/A   2   U   Undivided   30   U   1/0 *   → 10%   → YES   →	YES
Number 2 Rd	
Mare Ave to   N/A   2   U   Undivided   30   D   730 *   EB   35%   58   YES   7.9%   13.69/	YES
Silverwood Ln	
Silverwood Ln to	
CR 48	YES



Source: 2022 Lake County CMP Database
\*2023 FDOT Multimodal Quality/Level of Service Handbook, Appendix B: Florida's Generalized Service Volume Tables Bold numbers represent capacity equal or higher than 5%.

Based on the study area analysis presented in **Table 1**, the following roadway segments were analyzed for the PM peak hour:

- SR 19
  - Lane Park Road to CR 48
  - o CR 48 to Central Avenue
  - Central Avenue to CR 455
  - o CR 455 to US 27 / SR 25
  - US 27 / SR 25 to CR 478
- Central Avenue
  - o SR 19 to Mare Avenue
- Number 2 Road
  - Mare Avenue to Silverwood Lane
  - Silverwood Lane to CR 48

The following intersections were analyzed for the AM and PM peak hours:

- SR 19 and CR 48 (Signalized)
- SR 19 and Central Avenue (Unsignalized)
- Central Avenue and South Florida Avenue (Unsignalized)
- SR 19 and Revels Road (Unsignalized) (East Project Access)
- SR 19 and CR 455 (Unsignalized)
- Spine Road and Interconnect Road (Proposed)
- Number 2 Road and Spine Road (North Project Access) (Proposed)
- Revels Road and Spine Road (Proposed)
- Revels Road and Orange Blossom Road (South Project Access)



### 2.0 EXISTING CONDITIONS ANALYSIS

Existing conditions in the vicinity of the site were analyzed to establish a baseline for the traffic conditions prevailing in the vicinity of the proposed development. The analysis included a review of existing roadway segment capacity and analysis of the intersection operations at the study intersections.

# 2.1 Roadway Segment Capacity

Existing roadway conditions were analyzed by comparing the existing traffic volumes within the study area and the adopted level of service (LOS) standards for the roadway segments. **Table 2** summarizes the roadway segment capacity analysis.

Table 2
Existing Roadway Segment Capacity Analysis

	•	-	•	-	-	-			
Roadway Segment	Seg ID	No Lns	LOS Std	Pk Dir Cap	Dir	Existing Vol	LOS	V/C	Deficient?
*Central Ave									
SR 19 to Mare Ave	N/A	2	D	F20	EB	57	С	0.11	NO
SK 19 to Mare Ave	IN/A	-	D	530	WB	59	С	0.11	NO
SR 19									
Lane Park Rd to CR 48	3040	2	D	920	NB	610	С	0.66	NO
Lane Park Rd to CR 46	3040		U	920	SB	656	С	0.71	NO
CR 48 to Central Ave	3050	2	D	700	NB	433	С	0.62	NO
CR 48 to Certifal Ave	3030		U	700	SB	372	С	0.53	NO
Central Ave to CR 455	3060	2	D	1,200	NB	433	В	0.36	NO
Certifal Ave to CR 455	3000		U	1,200	SB	372	В	0.31	NO
CR 455 to US 27 / SR 25	3070	2	С	450	NB	507	D	1.13	YES
CR 499 to 09 27 / SR 29	3070		C	450	SB	435	С	0.97	NO
US 27 / SR 25 to CR 478	3080	2	С	450	NB	466	D	1.04	YES
03 27 / SR 25 to CR 476	3000		C	450	SB	519	D	1.15	YES
Number 2 Rd									
Mare Avenue to Silverwood Ln	N/A	2	D	400	EB	57	С	0.14	NO
Ivale Avenue to Silverwood En	IN/A			400	WB	59	С	0.15	NO
Silverwood Ln to CR 48	N/A	2	D	400	EB	57	С	0.14	NO
Silver wood Lit to CR 40	IN/A		D	400	WB	59	С	0.15	NO

Source: 2022 Lake County CMP Database

The analysis indicates that all study roadway segments currently operate adequately within their capacities except the segments of SR 19 from CR 455 to CR 478 which currently operate over capacity.



<sup>\*</sup> Counts were obtained from PM Peak Turning Movement Counts

<sup>\*\*</sup>A reduction of 25% was applied to the Peak Hour Directional Capacity of 530, as Number 2 Road is a substandard road

## 2.2 Intersection Capacity

The intersection capacity analysis was performed for the AM and PM peak hour periods. The capacity analysis was performed using *Synchro* and the methods of the *Highway Capacity Manual (HCM)*. Turning movement volumes obtained during the AM and PM peak hour are displayed in **Figure 2** and **Figure 3**, respectively. The counts at SR 19 and CR 455 were collected on January 24, 2023, which coincides with a seasonal factor of 1.0. The remaining intersection turning movement counts were collected on July 19, 2023, during the off-peak season; therefore, a seasonal factor of 1.06 was applied to these counts. The turning movement counts and the seasonal factor report are included in **Appendix D**.

The results of the intersection capacity analysis, summarized in **Table 3**, reveal that all study intersections are currently operating at adequate LOS. Detailed *HCM* analysis worksheets are included in **Appendix E**.

Table 3
Existing Intersection Capacity Analysis

Intersection	Traffic	Traffic Time EB WE				3	N	3	SI	В	Overall	
intersection	Control	Period	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
SR 19 & CR 48	Signal	AM			50.7	D	20.3	С	11.2	В	29.5	С
SK 19 & CK 46	Signal	PM			87.5	F	17.1	В	10.7	В	55.7	Е
SR 19 & Central Ave	TWSC	AM	20.7	С	15.1	С	8.9	Α	8.8	Α	ı	
SK 19 & Certifal Ave	10050	PM	22.6	С	17.9	С	9.0	Α	8.8	Α		
W Central Ave & S Florida Ave	TWSC	AM	7.3	Α	7.3	Α	8.8	Α	0.0	Α	-	
VV Certifal Ave & 3 Florida Ave	10030	PM	0.0	Α	7.3	Α	8.8	Α	9.4	Α	-	
SR 19 & Revels Rd	TWSC	AM	13.3	В	15.0	С	8.3	Α	8.0	Α	ı	
SK 19 & Reveis Ru	10050	PM	14.0	В	16.1	С	8.1	Α	8.2	Α	1	
SR 19 & CR 455	TWSC	AM			25.1	D			8.9	Α	ı	
SK 19 & CK 400	10050	PM			26.7	D			9.0	Α		

Average delay is in seconds



# Figure 2 Existing AM Peak Hour Intersection Volumes



# Figure 3 Existing PM Peak Hour Intersection Volumes



### 3.0 PROJECT TRAFFIC

### 3.1 Trip Generation

The Trip Generation Analysis was conducted using the Institute of Transportation Engineers (ITE) *Trip Generation Manual, 11<sup>th</sup> Edition.* The ITE Information sheets are included in **Appendix F**. **Table 4** summarizes the resulting trip generation analysis.

Table 4
Trip Generation Analysis

ΙΤΕ	ITE		Da	aily	A	M Pea	k Hour		PM Peak Hour					
Code	Land Use	Size	Rate	Trips	Rate	Total	Enter	Exit	Rate	Total	Enter	Exit		
210	Single Family Residential (Detached)	499 DU	8.87	4,428	0.64	322	81	241	0.90	451	284	167		

Trip Generation analysis based on ITE Trip Generation Manual, 11th Edition.

The proposed development is projected to generate 4,428 new daily trips, of which 322 trips occur during the AM peak hour and 451 trips occur during the PM peak hour.

## 3.2 Trip Distribution

A trip distribution pattern was developed using the *Central Florida Regional Planning Model (CFRPMv7)*. The model distribution was slightly adjusted based on local knowledge, professional engineering judgement, and the location of the development with respect to the study area attractions and activity centers to reflect the prevailing travel patterns in the study area and the surrounding transportation network. The raw model plots are provided in **Appendix G**, and the project trip distribution pattern is shown in **Figure 4**. Detailed trip distribution near the project site is shown in **Figure 5**.



# Figure 4 Project Trip Distribution



Figure 5 Project Trip Distribution Near Project Site



#### 4.0 PROJECTED CONDITIONS ANALYSIS

An analysis of projected conditions was conducted to determine the impact of the proposed development on the roadway segments capacity, as well as the proposed access connections and intersections to the site. The project buildout year for the analysis is 2033.

## 4.1 Planned and Programmed Improvements

The Lake-Sumter Metropolitan Planning Organization (LSMPO) 2023-2027 Transportation Improvement Program (TIP), as well as LSMPO 2022 List of Priority Projects (LOPP) were reviewed to identify any planned or programmed improvements to the transportation facilities in this area. The improvements are listed in **Table 5**. Construction is not planned to be completed within the next three (3) years for either improvement. Excerpts from the LSMPO TIP and LSMPO LOPP are provided in **Appendix H**.

Table 5
Planned and Programmed Improvements

FM#	Project Name	- From			Proposed Phase FY	Description of Improvement
2383191	SR 19 *	CR 48	CR 561	PDE-PE-ENV	2023	Add Lanes & Reconstruct
238319-1	SR 19 **	Howey Bridge	CR 561	-	-	Road Widening

<sup>\*</sup>LSMPO TIP Fiscal Year 2023-2027

### 4.2 Background Traffic Projection

Projected traffic includes background traffic volumes, the project trips, and committed trips. Projected background traffic for the buildout year of 2033 was estimated by applying the growth rates obtained from 2022 Lake County CMP Database to the existing traffic volumes. A minimum of 2% annual growth rate was applied to existing traffic volumes for which published annual growth rates are below 2%. The committed trips for the following approved developments within the study area are included in **Appendix I**:



<sup>\*\*</sup> LSMPO 2022 LOPP Tier 2 project

- Whispering Hills
- Talichet Phase 1 and Phase 2
- The Reserve at Howey in the Hills
- Lake Hills (Four Seasons). Trips were estimated based on the trip generation analysis and the trip distribution obtained from the methodology.
- Watermark (Simpson)

### 4.3 Roadway Segment Capacity

Projected roadway conditions were analyzed by comparing the projected traffic volumes on the study segments to their service volumes and adopted LOS standards. The total projected traffic volume is composed of background traffic, vested trips and project trips. **Table 6** summarizes the roadway segment capacity analysis, which reveals the following:

- SR 19 from Lane Park Road to Central Avenue and from CR 455 to CR 478 are projected to operate over their capacities due to background traffic.
- All remaining roadway segments are projected to continue to operate adequately at project buildout.

#### Roadway Segment Capacity Analysis with Recommended Mitigation

Number 2 Road is a substandard road with reduced capacity. It is projected to operate at an acceptable LOS; however, operational safety is a concern due to its narrow width. Lake County would need to improve it in the future to achieve safety.

SR 19 from CR 48 to CR 561 is programmed in the *TIP* to be widened to four (4) lanes. The roadway segment capacity analysis reveals that the widening of SR 19 to 4-lanes is projected to improve the capacity of the segment from Lane Park Road to CR 48. The segments of SR 19 from CR 48 to Central Avenue and from CR 455 to CR 478 would need to be widened to 4-lanes to achieve acceptable LOS conditions at project buildout, as summarized in **Table 7**.



Table 6
Projected Roadway Segment Capacity Analysis

Dandway Carrinant	No	LOS	PH Dir	D:-	Exist	Growth	2033	Vested	Total Backg'd		Backg'd		Proj Dir		Total	Final	
Roadway Segment	Lns	Std	Capacity	Dir	Vol	Rate	Backg'd	Trips	Volume	LOS	V/C	Distr	DIL	volume	Volume	LOS	V/C
*Central Ave																	
SR 19 to Mare Ave	2	D	530	NB/EB	57	2.00%	70	53	123	С	0.23	10%	OUT	17	140	С	0.26
				SB/WB	59		72	85	157	С	0.30		IN	28	185	С	0.35
SR 19																	
Lane Park Rd to CR 48	2	D	920	NB/EB	610	2.00%	744	125	869	С	0.94	23%	OUT	38	907	D	0.99
Lane Faik Nu to Civ 40			920	SB/WB	656	2.00 /0	800	264	1,064	F	1.16	23 /0	IN	65	1,129	F	1.23
CR 48 to Central Ave	2	D	700	NB/EB	433	2.00%	528	266	794	F	1.13	25%	OUT	42	836	F	1.19
CIV 40 to Central Ave			700	SB/WB	372	2.0070	454	355	809	F	1.16	2370	IN	71	880	F	1.26
Central Ave to CR 455	2	D	1,200	NB/EB	433	2.00%	528	437	965	D	0.80	50%	IN	142	1,107	D	0.92
Central Ave to Cit 455	_		1,200	SB/WB	372	2.0070	454	272	726	С	0.61	30 70	OUT	84	810	С	0.68
CR 455 to US 27/ SR 25	2	С	450	NB/EB	507	2.00%	619	286	905	Е	2.01	35%	IN	99	1,004	Е	2.23
CIV 433 to 03 217 SIV 23	_		430	SB/WB	435	2.0070	531	178	709	D	1.58	3370	OUT	58	767	Е	1.70
US 27/ SR 25 to CR 478	2	С	450	NB/EB	466	2.00%	569	286	855	Е	1.90	10%	IN	28	883	Е	1.96
00 21/ 51( 25 to 51( 4/6	_		430	SB/WB	519	2.0070	633	178	811	E	1.80	1070	OUT	17	828	Ε	1.84
**Number 2 Rd																	
Mare Ave to Silverwood Ln	2	D	400	NB/EB	57	2.00%	70	53	123	С	0.31	35%	OUT	58	181	С	0.45
Wate Ave to Sliverwood En			400	SB/WB	59	2.0070	72	53	125	С	0.31	0070	IN	99	224	D	0.56
Silverwood Ln to CR 48	2	D	400	NB/EB	57	2.00%	70	53	123	С	0.31	15%	IN	43	166	С	0.42
Silverwood En to CIV 48		U	400	SB/WB	59	2.00 /0	72	53	125	С	0.31	1370	OUT	25	150	С	0.38

Source: 2022 Lake County Annual Traffic Counts



<sup>\*</sup>Exiting Counts were obtained from PM Peak Turning Movement Counts

<sup>\*\*</sup>A reduction of 25% was applied to the Peak Hour Directional Capacity of 530, as Number 2 Road is a substandard road

Table 7
Projected Roadway Segment Capacity Analysis with Mitigation

Roadway Segment	No Lns	LOS Std	PH Dir Capacity	Dir	Exist Vol	Growth Rate	2033 Backg'd	Vested Trips	Total Backg'd Volume	Backg'd LOS	Backg'd V/C	Trip Distr			Total Volume	Final LOS	Final V/C	Project Responsible ?
SR 19							J											
Lane Park Rd to CR 48	4	D	1,480	NB/EB SB/WB	610 656	2.00%	744 800	125 264	869 1.064	C D	0.59 0.72	23%	OUT IN	38 65	907 1.129	D D	0.61 0.76	NO NO
CR 48 to Central Ave	4	D	1,480	NB/EB SB/WB	433 372	2.00%	528 454	266 355	794 809	D D	0.54 0.55	25%	OUT	42 71	836 880	D D	0.56 0.59	NO NO
CR 455 to US 27/ SR 25	4	С	1,360	NB/EB SB/WB	507 435	2.00%	619 531	286 178	905 709	C C	0.67 0.52	35%	IN OUT	99 58	1,004 767	υo	0.74 0.56	NO NO
US 27/ SR 25 to CR 478	4	С	1,360	NB/EB SB/WB	466 519	2.00%	569 633	286 178	855 811	C C	0.63 0.60	10%	IN OUT	28 17	883 828	OO	0.65 0.61	NO NO
**Number 2 Rd	•	•		•		•			•	•	•		•	•				
Mare Ave to Silverwood Ln	2	D	530	NB/EB SB/WB	57 59	2.00%	70 72	53 53	123 125	C C	0.23 0.24	35%	OUT IN	58 99	181 224	C D	0.34 0.42	NO NO
Silverwood Ln to CR 48	2	D	530	NB/EB SB/WB	57 59	2.00%	70 72	53 53	123 125	C C	0.23 0.24	15%	IN OUT	43 25	166 150	υo	0.31 0.28	NO NO

Source: 2022 Lake County Annual Traffic Counts

Note: Roadway mitigations are necessitated by background traffic. Number 2 Road is an existing substandard facility.

The development is not responsible for these improvements, per Florida Statutes.



<sup>\*</sup>Exiting Counts were obtained from PM Peak Turning Movement Counts

<sup>\*\*</sup>A reduction of 25% was applied to the Peak Hour Directional Capacity of 530, as Number 2 Road is a substandard road

## 4.4 Intersection Capacity Analysis

The projected volumes for the intersection capacity and operations analysis were calculated by assigning the project trips to the project driveways and adding those volumes to the background volumes and vested trips at the study intersections. Projected background traffic was estimated as discussed in the previous section. Projected background traffic on the proposed Spine Road and Revels Road were estimated based on the *CFRPMv7* model daily volumes. The AADT model plots are included in **Appendix J**.

### Intersection Capacity Analysis

The projected AM and PM peak hour volumes are illustrated in **Figure 6** and **Figure 7**, respectively. The analysis includes right and left turn lanes on SR 19, and a right turn lane on Revels Road at the intersection of SR 19 and Revels Road. It also includes right and left turn lanes on Number 2 Road at the intersection of Spine Road and Number 2 Road. The results of the analysis are summarized in **Table 8**, and the analysis worksheets are included in **Appendix K**. The intersection volume projection sheets are included in **Appendix L**.

Table 8
Projected Intersection Capacity Analysis

Intersection	Traffic	Time	EE	3	W	WB		NB		SB		rall
Intersection	Control	Period	Delay	LOS								
SR 19 & CR 48	Signal	AM			177.1	F	29.7	С	22.1	С	87.2	F
31 19 & CI 48	Signal	PM			>300	F	21.5	В	12.1	В	234.3	F
SR 19 & Central Ave	TWSC	AM	>300	F	26.5	D	10.1	В	10.3	В		
SK 19 & Cellifal Ave	10030	PM	>300	F	89.7	F	11.4	В	10.3	В		-
W Central Ave & S Florida Ave	TWSC	AM	7.3	Α	7.4	Α	9.2	Α	0.0	Α		
VV Certifal Ave & 3 Florida Ave	17730	PM	0.0	Α	7.4	Α	9.3	Α	10.6	В		
SR 19 & Revels Rd / Project Entrance	TWSC	AM	51.2	F	>300	F	10.1	В	8.8	Α		
SK 19 & Revels Ru / Project Entrance	10030	PM	135.1	F	>300	F	9.9	Α	10.7	В		
SR 19 & CR 455	TWSC	AM			>300	F			10.7	В		
3K 19 & CK 433	17730	PM			>300	F			12.7	В		
Spine Rd & Interconnect Rd / Proposed	TWSC	AM			8.8	Α			7.4	Α		
Spirie Ru & interconnect Ru / Proposed	10030	PM			8.8	Α			7.4	Α		-
Number 2 Rd and Spine Rd / Project	TWSC	AM			7.5	Α	9.8	Α				
Entrance	10030	PM			7.6	Α	9.9	Α	ı			
Spine Dd & Dovele Dd	TWSC	AM			9.1	Α			7.5	Α		
Spine Rd & Revels Rd	1 1 1 1 1 1 1	PM			9.3	Α			7.5	Α		
Revels Rd & Orange Blossom Rd /	TWSC	AM	7.2	Α					8.6	Α		
Project Entrance	1 11/50	PM	7.3	Α					8.6	Α		

Average delay is in seconds



# Figure 6 Projected AM Peak Hour Intersection Volumes



# Figure 7 Projected PM Peak Hour Intersection Volumes



The analysis reveals the following:

 The intersection of SR 19 and CR 48 is projected to operate with delay during the AM and the PM peak hour. Further review is needed.

The intersection of SR 19 and Central Avenue is projected to operate with delay in the
eastbound and westbound directions. The westbound movement does not carry any
project traffic and it is projected to operate at volume to capacity ratio less than 1.0. Further
review is needed.

 The intersection of SR 19 and Revels Road is projected to operate with delay in the eastbound and westbound directions. The westbound movement does not carry any project traffic. Further review is needed.

 The intersection of SR 19 and CR 455 is projected to operate with delay for the westbound left movement. Project trips contribute no traffic to the movement. Further review is needed.

The remaining study intersections are projected to operate adequately at the project buildout.

### Intersection Capacity Analysis with Recommended Mitigation

The proposed project does not significantly impact study area intersections. Four (4) intersections have been reviewed further. The intersections are determined to need the following improvements to achieve acceptable LOS conditions at project buildout:

- Retiming the signal is recommended at the intersection of SR 19 and CR 48.
- Installing a signal is recommended at the intersection of SR 19 and Central Avenue.
- Installing a signal is recommended at the intersection of SR 19 and Revels Road.
- Installing a signal is recommended at the intersection of SR 19 and CR 455.



The traffic operations for the mitigated intersections are projected to have acceptable LOS, as detailed in **Table 9**. The background conditions and the buildout conditions with the mitigation analysis worksheets are included in **Appendix M**.

Table 9
Projected Intersection Capacity Analysis with Mitigation

Intersection	Peak		Е	В	W	/B	N	В	S	В	Ove	rall
intersection	Period	Scenario	Delay	LOS								
		Background			161.9	F	29.5	С	21.8	С	80.1	F
	AM	Buildout			177.1	F	29.7	С	22.1	С	87.2	F
SR 19 & CR 48		Mitigation			59.4	E	72.4	E	54.1	D	60.9	D
SK 19 & CK 46		Background			>300	F	21.5	С	12.1	В	187.5	F
	PM	Buildout			>300	F	21.5	С	12.1	В	233.7	F
		Mitigation			48.7	D	56.5	E	58.2	E	52.6	D
		Background	>300	F	24.5	С	9.9	Α	10.1	Α		
	AM	Buildout	>300	F	26.5	D	10.1	В	10.3	В		
SR 19 & Central Ave		Mitigation	21.0	С	18.3	В	8.2	Α	8.2	Α	9.9	Α
		Background	>300	F	65.2	Е	11.0	В	10.2	В		
	PM	Buildout	>300	F	89.7	F	11.4	В	10.3	Α		
		Mitigation	13.3	В	12.0	В	6.8	Α	24.7	С	16.9	В
		Background	22.5	С	>300	F	9.7	Α	8.8	Α		
	AM	Buildout	51.2	F	>300	F	10.1	В	8.8	Α		
SR 19 & Revels Road		Mitigation	18.2	В	16.0	В	5.0	Α	6.2	Α	7.3	Α
SK 19 & Reveis Road		Background	30.0	D	>300	F	9.0	Α	10.6	В		
	PM	Buildout	135.1	F	>300	F	9.9	Α	10.7	В		
		Mitigation	30.0	С	26.7	С	6.5	Α	3.8	Α	7.3	Α
		Background			>300	F			10.3	В		
	AM	Buildout			>300	F			10.7	В		
SR 19 & CR 455		Mitigation			78.2	Е	2.3	Α	30.8	С	24.3	С
SK 19 & CK 455		Background			>300	F			11.6	В		
	PM	Buildout			>300	F			12.7	В		
		Mitigation			130.1	F	6.4	Α	62.3	E	44.1	D

Average delay is in seconds

### The analysis reveals the following:

- The intersection of SR 19 and CR 48 is projected to operate at an acceptable overall LOS by optimizing the signal timing.
- The intersection of SR 19 and Central Avenue is projected to operate adequately at buildout with a signal. The westbound movement does not carry any project traffic. Project contribute 5.9% of the total traffic.
- The intersection of SR 19 and Revels Road is projected to operate adequately at buildout with a signal. The westbound movement does not carry any project traffic. Project contributes 13.6% of the total traffic.
- The intersection of SR 19 and CR 455 is projected to operate adequately at buildout with a signal. The westbound movement does not carry any project traffic. Project contribute 9.0% of the total traffic.

In lieu of contributing a proportionate share to the three (3) intersections needing new traffic signals, the developer is recommending to construct the new traffic signal at SR 19 and Revels Road, which serves as the main access to the project.



#### 5.0 ACCESS REVIEW

The development will be accessed via the intersections of Number 2 Road and Spine Road (future road), SR 19 and Revels Road, and Revels Road and Orange Blossom Road. SR 19 is a 2-lane undivided facility with a posted speed limit of 55 miles per hour (mph) near the project entrance. Number 2 Road is a 2-lane undivided facility with a posted speed limit of 30 mph in the east direction and 45 mph in the west direction near the project entrance. Orange Blossom Road is a 2-lane undivided facility with a posted speed limit of 30 mph near the project entrance.

#### 5.1 Turn Lane Review

A review of the need for turn lanes at the project entrance intersections was conducted based on the Lake County *Land Development Code (LDC)* guidelines, which are provided in **Appendix N**. In accordance with the *LDC* guidelines, right and left turn lanes are warranted at the intersections of SR 19 and Revels Road, and at Number 2 Road and Spine Road. The intersection of Orange Blossom Road and Revels Road is expected to carry limited traffic; therefore, exclusive turn lanes are not recommended.

The recommended lengths of the turn lanes on SR 19 were calculated based on the requirements of the *FDOT Design Manual Exhibit 212-1*, provided in **Appendix O**, and the recommended lengths of the turn lanes on Number 2 Road were calculated based on the Lake County *LDC* guidelines. Per Lake County requirement for turn lane widening on Number 2 Road, the length of tapers will need to be twice the standard length. The calculations are provided as follows:

### SR 19 and Revels Road

Left Turn Lane Length = Deceleration Distance + Queue Length Deceleration at 60 mph (design speed) = 405 feet 95<sup>th</sup> Percentile Queue Length = 1 x 25 = 25 feet

Northbound Left Turn Lane = 405 feet + 25 feet = 430 feet (including a 50-foot taper)

Right Turn Lane Length = Deceleration Distance Deceleration at 60 mph (design speed) = 405 feet Southbound Right Turn Lane = 405 feet



#### Number 2 Road and Spine Road

Left Turn Lane Length = Taper Length + Storage Length
Taper Length at 50 mph (design speed) = 230 feet x 2 = 460 feet
Storage Length at 50 mph (design speed) = 195 feet
Westbound Left Turn Lane = 460 feet + 195 feet = 655 feet

Right Turn Lane Length = Taper Length + Storage Length
Taper Length at 35 mph (design speed) = 170 feet x 2 = 340 feet
Storage Length at 35 mph (design speed) = 80 feet
Eastbound Right Turn Lane = 340 feet + 80 feet = 420 feet



#### 6.0 STUDY CONCLUSIONS

This traffic analysis was conducted to assess the impact of the proposed Mission Rise development in the Town of Howey-in-the-Hills, Florida. The project will include 499 single family residential units. The analysis included a determination of project trip generation, a review of existing and projected roadway and intersection capacity.

The results of the traffic analysis are summarized as follows:

- The proposed development is projected to generate 4,428 trips per day, of which 322 trips occur during the AM peak hour and 451 trips occur during the PM peak hour.
- SR 19 SR 19 from Lane Park Road to Central Avenue and from CR 455 to CR 478 are
  projected to operate over their capacities due to background traffic. The development is
  not responsible for mitigating background deficiencies, per Florida's Statutes.
- SR 19 from CR 48 to CR 561 is programmed in the TIP to be widened to 4 lanes.
- All remaining roadway segments are projected to continue to operate adequately at project buildout.
- The intersection of SR 19 and CR 48 is projected to operate with delay during the AM and the PM peak hour. It is recommended to retime the signal to maintain LOS standards.
- The intersection of SR 19 and Central Avenue is projected to operate with delay in the eastbound and the westbound movement. The westbound movement does not carry any project traffic.
- The intersection of SR 19 and Revels Road is projected to operate with delay in the eastbound and westbound directions. The westbound movement does not carry any project traffic.



- The intersection of SR 19 and CR 455 is projected to operate with delay for the westbound left movement. Project trips contribute no traffic to the movement.
- In lieu of contributing a proportional share to the three (3) intersections where traffic signals are projected to be needed, the developer is recommending to construct the traffic signal at the intersection of SR 19 and Revels Road.
- A traffic signal at SR 19 and Revels Road traffic signal needs to be warranted based on a signal warrant study of the in-field traffic volumes. An Intersection Control Evaluation (ICE) study will also need to be coordinated with FDOT.
- All remaining study intersections are projected to operate adequately at project buildout.
- The turn lane recommendations are as follows:
  - Construct a 430-foot northbound left turn lane and a 405-foot southbound right turn lane at the intersection of SR 19 and Revels Road.
  - Construct a 655-foot westbound left turn lane and a 420-foot eastbound right turn lane at the intersection of Number 2 Road and Spine Road.



### **APPENDICES**

**Appendix A**Study Methodology and Response to Comments Letter



#### **MEMORANDUM**

May 23, 2023

Re: Mission Rise

Traffic Impact Analysis Methodology, v1.1 Town of Howey-In-The-Hills, Florida

Project № 23017.1

This methodology outlines the proposed Traffic Impact Analysis (TIA) for the above referenced project. This methodology was prepared in accordance with the requirements of the Town of Howey-In-The-Hills and the Lake~Sumter Metropolitan Planning Organization (LSMPO) TIA guidelines for a Tier 2 TIA. This methodology has been revised in accordance with the comments provided by the Town of Howey-In-The-Hills. The comments and response to comments letter are included in the **Attachments**.

#### **Project Description**

The  $\pm 243.3$ -acre site is a single-family residential development consisting of 592 dwelling units. The project site consists of parcels 34-20-25-0001-000-00100, 34-20-25-0004-000-01003, 02-21-25-0002-000-04800, and 27-20-25-0004-000-01200. The anticipated buildout year is 2033. A preliminary site plan is included in the **Attachments**.

#### **Project Location**

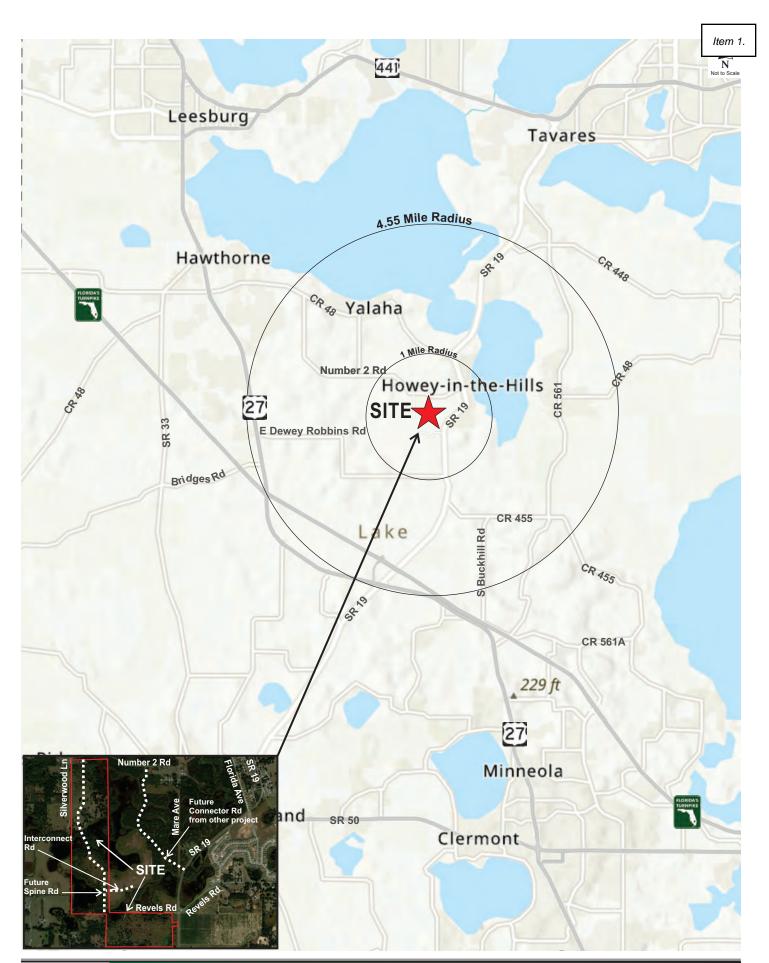
The site is located east of Silverwood Lane, west of SR 19 (South Palm Avenue), and south of Number 2 Road in the Town of Howey-in-the-Hills, Florida. The site will be crossed from north to south by a future two-lane spine road that will connect Number 2 Roadway with Revels Road, as shown in **Figure 1**.

#### **Project Access**

The project has access to the external network via one (1) full access driveway on Number 2 Road and one (1) full access driveway on SR 19. In addition, there is an emergency access to the south via Orange Blossom Road. The access configuration is depicted in the preliminary site plan included in the **Attachments**.

#### Trip Generation

A trip generation analysis was performed for the development using the trip generation information from the Institute of Transportation Engineers (ITE) *Trip Generation Manual, 11<sup>th</sup> Edition.* The ITE information sheets are included in the **Attachments**. The trip generation of the proposed development is summarized in **Table 1**.





Traffic Impact Analysis Methodology, v1.1 Project № 23017.1 May 23, 2023 Page 3 of 9

## Table 1 Trip Generation Analysis

ITE			Da	aily	Д	M Pea	k Hour	,		РМ Реа	k Hour	
			Eqvlt		Eqvlt				Eqvlt			
Code	Land Use	Size	Rate	Trips	Rate	Total	Enter	Exit	Rate	Total	Enter	Exit
	Single Family Residential (Detached)	592 DU	8.75	5,181	0.63	376	94	282	0.89	529	333	196

Trip Generation analysis based on ITE Trip Generation Manual, 11th Edition.

The proposed development at project buildout is projected to generate 5,181 new daily trips of which 376 trips occur during the AM peak hour, and 529 trips occur during the PM peak hour.

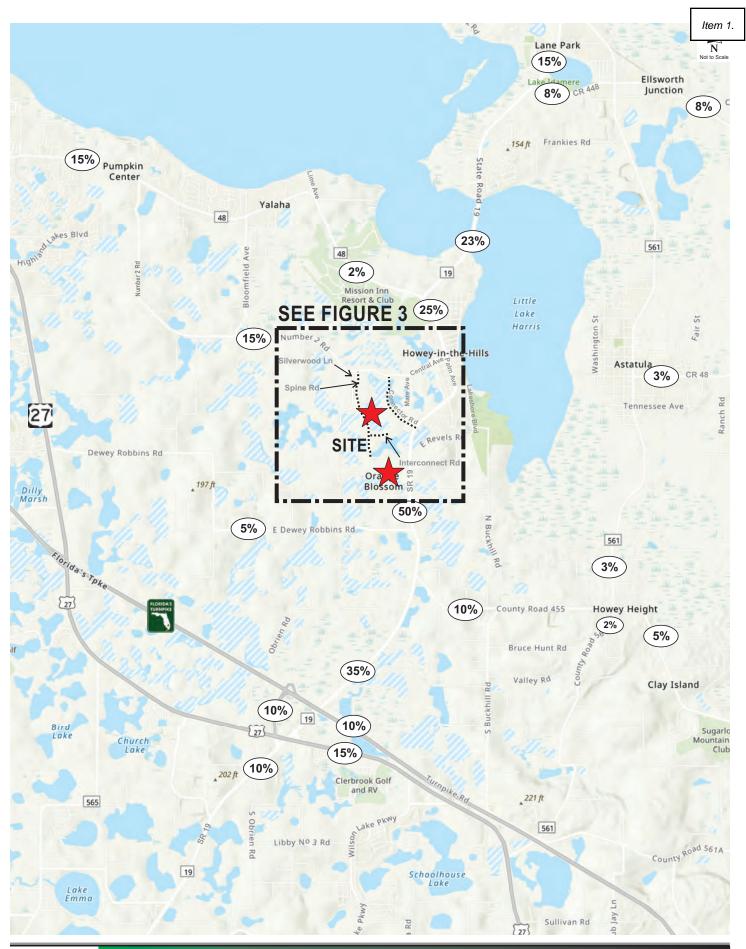
#### **Trip Distribution**

A trip distribution pattern in the general vicinity of the project site was initially determined based on the *Central Florida Regional Planning Model (CFRPM v7)*. Two (2) future connections (Spine Road and Connector Road) from SR 19 to Number 2 Road were included in the model for this project. The model distribution was modified to reflect the local network and prevailing traffic patterns. The proposed trip distribution pattern is provided in **Figure 2**. Detailed trip distribution near the project site is shown in **Figure 3**. The model distribution plots are included in the **Attachments**.

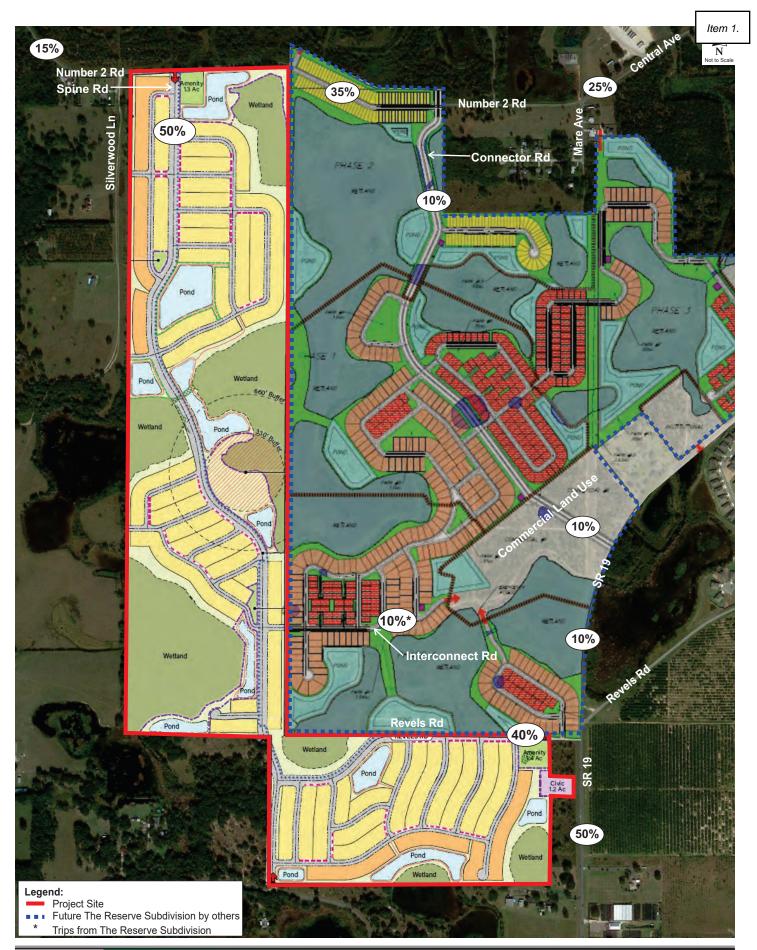
#### Study Area

In accordance with the LSMPO requirements for a Tier 2 TIA methodology, the study area will include a minimum 1-mile radius plus all roadway segments within a 4.55-mile radius in addition to roadways where the development is projected to consume 5% or more of their adopted Level of Service (LOS), unless otherwise specified by the City/LSMPO.

The extent of the study impact area shall be determined by the area of influence of the project. The area of influence shall be established as one-half (½) the total trip length associated with the land use of the proposed development, based upon the 2021 Lake County Transportation Impact Fee Update Study Final Report. The total trip length for single-family is 9.1-miles. Accordingly, the area of influence will encompass all roadway segments within 4.55-mile radius. Excerpts of the 2022 Lake County Congestion Management Process (CMP) Database, the 2021 Lake County Transportation Impact Fee Update Study Final Report, and the 2023 FDOT Multimodal Quality/Level of Service (Q/LOS) Handbook Appendix B are included in the Attachments. Table 2 lists all roadway segments within the area of influence along with their capacities and percentages consumed by the project trips.









Traffic Impact Analysis Methodology, v1.1 Project № 23017.1 May 23, 2023 Page 6 of 9

Table 2 **Study Area** 

		No	Area	Median	Speed					ject	Within	%	In
Roadway Segment	SEG ID	Lns	Type	Type	Limit	Std	Сар	Dir	Dist	Trips	1-Mile? **	Сар	Study?
CR 455	1				1								
SR 19 to	950	2	R	Undivided	45	С	740	EB	10%	20	NO	2.7%	NO
CR 561						Ŭ		WB		33		4.5%	
CR 561 to	960	2	R	Undivided	25	С	410	EB	5%	10	NO	2.4%	NO
CR 561A	300		1.	Ondivided	20		710	WB	070	17	110	4.1%	110
CR 48													
US 27 to	1240	2	U	Undivided	40	D	1,080	EB	15%	50	NO	4.6%	NO
Lime Ave	1240	-	U	Offdivided	40		1,000	WB	1376	29	NO	2.7%	INO
Lime Ave to	1250	2	U	Undivided	40	D	1,080	EB	2%	7	NO	0.6%	NO
SR 19	1230	-	U	Orlaividea	40	الا	1,000	WB	2 70	4	NO	0.4%	INO
CR 561 to	1260	2	U	Undivided	40	D	840	EB	3%	6	NO	0.7%	NO
Ranch Rd	1200	-	U	Undivided	40	ן ט	040	WB	370	10	NO	1.2%	INO
Ranch Rd to	4070		1		40		440	EB	00/	6	NO	1.5%	
CR 448A	1270	2	R	Undivided	40	С	410	WB	3%	10	NO	2.4%	NO
CR 561													
CR 448 to	4					_		NB	001	0		0.0%	
CR 48	1410	2	U	Undivided	50	D	1,080	SB	0%	0	NO	0.0%	NO
CR 48 to								NB		10		1.6%	
South Astatula City Limit	1420	2	U	Undivided	40	D	620	SB	3%	6	NO	1.0%	NO
South Astatula City Limit								NB		10		0.9%	
to CR 455	1430	2	U	Undivided	40	D	1,080	SB	3%	6	NO	0.6%	NO
CR 455 to								NB		7		1.5%	
Howey Cross Rd	1440	2	R	Undivided	35	С	470	SB	2%	4	NO	0.9%	NO
Howey CRoss Rd to										7		1.1%	
	1450	2	R	Undivided	40	С	640	NB SB	2%	4	NO	0.6%	NO
Turnpike Rd / CR 561A								28		4		0.6%	
SR 19	1	1			1			L NID		1 45 1		1.00/	
Lane Park Rd to	3040	2	U	Undivided	55	D	920	NB	23%	45	NO	4.9%	YES
CR 48								SB		77		8.4%	
CR 48 to	3050	2	υ	Undivided	40	D	700	NB	25%	49	NO	7.0%	YES
Central Ave								SB		83		11.9%	
Central Ave to	3060	2	υ	Undivided	35	D	1,200	NB	50%	167	YES	13.9%	YES
CR 455			_	• • • • • • • • • • • • • • • • • • • •			.,	SB		98		8.2%	
CR 455 to	3070	2	R	Undivided	55	С	450	NB	35%	117	NO	26.0%	YES
US 27 / SR 25	0070		1.	Ondivided	00	Ŭ	400	SB	0070	69	110	15.3%	
US 27 / SR 25	3080	2	R	Undivided	55	С	450	NB	20%	67	NO	14.9%	YES
to CR 478	3000		11	Ondivided	55		730	SB	20 /0	39	110	8.7%	1.20
SR 91 (Florida Turnpike)													
US 27/SR 25 to	3566	4	C	Freeway	70	В	2,230	EB	10%	20	NO	0.9%	NO
US 27/SR 25/SR 19 Interchange	3300	4	U	rieeway	70		2,230	WB	10 76	33	NO	1.5%	INO
US 27/SR 25													
SR 19 to	2020	4	11	Divided	E F	_	2 000	EB	150/	29	NO	0.9%	NO
CR 561	3830	4	U	Divided	55	D	3,280	WB	15%	50	NO	1.5%	NO
Central Ave													
SR 19 to								EB	0=0:	49		6.4%	
Mare Ave	N/A	2	U	Undivided	30	D	770 *	WB	25%	83	YES	10.8%	YES
Number 2 Rd	1			Ī	I			]				/ 0	
Mare Ave to		_						EB		69		9.5%	
Silverwood Ln	N/A	2	U	Undivided	30	D	730 *	WB	35%	117	YES	16.0%	YES
Silverwood Ln to								EB		29		4.0%	
CR 48	N/A	2	U	Undivided	45	D	730 *	WB	15%	50	YES	6.8%	YES
Source: 2022 Lake County CMP Datal	h000	<u> </u>	<u> </u>		<u> </u>			440		50		0.0 /0	l

Source: 2022 Lake County CMP Database
\*2023 FDOT Multimodal Quality/Level of Service Handbook, Appendix B: Florida's Generalized Service Volume Tables Bold numbers represent capacity equal or higher than 5%.

Traffic Impact Analysis Methodology, v1.1 Project № 23017.1 May 23, 2023 Page 7 of 9

Based on the study area analysis, the following roadway segments will be analyzed for the PM peak hour:

- SR 19
  - o Lane Park Road to CR 48
  - o CR 48 to Central Avenue
  - Central Avenue to CR 455
  - o CR 455 to US 27 / SR 25
  - o US 27 / SR 25 to CR 478
- Central Avenue
  - o SR 19 to Mare Avenue
- Number 2 Road
  - Mare Avenue to Silverwood Lane
  - Silverwood Lane to CR 48

The following intersections will be analyzed for the AM and PM peak hours:

- SR 19 and CR 48 (Signalized)
- SR 19 and Central Avenue (Unsignalized)
- SR 19 and South Florida Avenue (Unsignalized)
- SR 19 and Revels Road (Unsignalized)
- SR 19 and CR 455 (Unsignalized)
- Spine Road and Interconnect Road (Proposed)
- Number 2 Road and Spine Road (North Project Access) (Proposed)
- Revels Road and Spine Road (South Project Access) (Proposed)

Traffic Impact Analysis Methodology, v1.1 Project № 23017.1 May 23, 2023 Page 8 of 9

#### **Projected Traffic**

Projected traffic includes background traffic volumes, the project trips, and committed trips. Projected background traffic will be calculated using the historical growth rates obtained from the *Lake County CMP* database and *FDOT Florida Traffic Online* web-based database. A 2%, minimum growth rate will be applied if the calculated growth rates are lower than 2%. The committed trips for the following approved developments within the study area will be added to the background traffic:

- The Reserve (traffic study obtained)
- Talichet Phase 2 (traffic study obtained)
- Whispering Hills (traffic study obtained)
- Lake Hills (City to provide traffic study)
- Watermark (City to provide traffic study)

#### <u>Planned and Programmed Improvements</u>

The Lake-Sumter Metropolitan Planning Organization (LSMPO) 2023-2027 Transportation Improvement Program (TIP), as well as LSMPO 2022 List of Priority Projects (LOPP) were reviewed to identify any planned or programmed improvements to the transportation facilities in this area. As shown in **Table 3**, construction is not planned to be completed within the next three (3) years for either improvement. Excerpts from the LSMPO TIP and LSMPO LOPP are provided in the **Attachments**.

Table 3
Planned and Programmed Improvements

FM#	Project Name	From	То	Proposed Phase	Proposed Phase FY	Description of Improvement
2383191	SR 19 *	CR 48	CR 561	PDE-PE-ENV	2023	Add Lanes & Reconstruct
238319-1	SR 19 **	Howey Bridge	CR 561	-	-	Road Widening

<sup>\*</sup> LSMPO TIP Fiscal Year 2023-2027

#### Capacity Analysis

The traffic study will include existing and 2033 buildout conditions for the roadway segment and intersection capacity analyses. A capacity analysis of the study roadway segments will be conducted for the PM peak hour under existing and projected conditions. The capacity analysis will be based on service volumes, capacities, and existing volumes, as documented in 2022 Lake County CMP Database and the FDOT's 2023 Multimodal Quality/Level of Service (MQ/LOS) Handbook, included in the **Attachments**.

<sup>\*\*</sup> LSMPO 2022 LOPP Tier 2 project

Traffic Impact Analysis Methodology, v1.1 Project № 23017.1 May 23, 2023 Page 9 of 9

The intersection turning movement counts will be seasonally adjusted, if needed, using the 2022 FDOT Peak Season Factor Category Report obtained from the Florida Traffic Online (FTO) website.

Right and left turn lane warrant reviews will be performed at the Spine Road accesses on Number 2 Road and at SR 19 and Revels Road in accordance with the Lake County requirements for turn lanes.

In cases where projected conditions require mitigation as a result of the proposed development, an analysis including the recommended mitigation will be conducted.

#### Alternative Mode Analysis

A review of transit, pedestrian, and bicycle facilities will be conducted in accordance with the LSMPO requirements.

#### Report

A TIA report detailing the methods and findings of the study, including all associated graphics, tables, calculations, and supporting information will be prepared for submittal to the Town of Howey-In-The-Hills.

### **ATTACHMENTS**



May 23, 2023

Mr. John Brock Town Clerk PO Box 125 Howey-In-The-Hills, Florida 34737 jbrock@howey.org

Re: Mission Rise

Response to Methodology Comments

TMC Project № 23017.1

Town Howey-In-The-Hills, Florida

Dear Mr. Brock.

Please find below our responses to the review comments prepared on behalf of The Town of Howey-In-The-Hills by TMH Consulting Inc dated May 8, 2023, regarding the above referenced Methodology dated April 28, 2023. The comments are listed in **bold** typeface and the TMC responses follow in *italic* typeface. Additionally, a revised Methodology is provided under cover reflecting the changes resulting from these comments.

1. The Revels Road access to the south cannot be limited to emergency access as this is a public road now. Since we have received comments from residents to the south, it will be very useful to get some type of prediction about how many trips are likely to use this access point as opposed to SR 19 and Number 2 Road.

TMC Response: The emergency access on Orange Blossom Road will be restricted to emergency vehicles only; therefore, no trips were assigned to that access.

2. There is an interconnect between the Mission Rise parcel and The Reserve parcel. Is the model sensitive enough to determine if this interconnect will impact trip assignments? The Reserve has an approved connecting road which is discussed in the TMC methodology. The Reserve also includes a future commercial development area that might be an attractor.

TMC Response: Noted. The Reserve Subdivision includes a future commercial development, therefore, 10% of the trips are assumed to originate from The Reserve's commercial development and use the interconnect road to access the project site.

- 3. The study needs to include those projects that have some level of approval. TMC has done the traffic studies for several of these and been provided with traffic studies from others. The projects that need to be included are:
  - The Reserve
  - Watermark
  - Talichet Phase 2 (Phase 1 is mostly in the background traffic by now.)
  - Whispering Heights
  - Lake Hills

Mr. John Brock
Mission Rise
Response to Methodology Comments
TMC Project № 23017.1
May 23, 2023
Page 2 of 3

TMC Response: Noted. The vested trips from The Reserve, Watermark, Talichet Phase 2, Whispering Heights [Whispering Hills], and Lake Hills will be included in the traffic study as indicated in the revised methodology (attached).

4. The study needs to include CFRPM distributions that show the percentages of future background through traffic that will use the new roads in Mission Rise and The Reserve that link No 2 Road to SR 19. Use that data to project future background traffic volumes on those links.

TMC Response: Noted. As reflected in Figure 2, the future Spine Road, which transverses the project site from north to south and connects Number 2 Road with Revels Road, and the future Connector Road, which connects SR 19 and Number 2 Road are included in the project trip distribution Figure 2 in the revised methodology (attached).

5. The project trip distribution map is basically unreadable. They need to provide a graphic that someone can review and understand.

TMC Response: Noted. The distribution map has been revised to show an inset with the detail project distribution within the project site. See Figure 2 in the revised methodology (attached).

6. SR 19 at Central Avenue is listed as signalized, but it is only a flashing light. The analysis cannot assume it is a true signal.

TMC Response: Noted. SR 19 at Central Avenue intersection is listed as an unsignalized intersection in the revised methodology (attached).

7. The ITE land use, code 210, shows traffic generation as 9.43 trips per unit with 0.70% for the AM Peak and 0.94% for the PM Peak. Why did they use 8.75, 0.63 and 0.89 respectively for the project traffic generation?

TMC Response: Per the Trip Generation Handbook, 3rd Edition Figure 4.2 (Process for selecting average rate or equation in trip generation manual data) linear curve equations should be used for the weekday, AM, and PM peak period trip generation calculation. The linear curve equations have an R<sup>2</sup> equal to 0.75 or greater, therefore, the fitted curve equations were used instead of average rate.

The linear curve equations used for the 592 dwelling residential units corresponding to the weekday, AM, and PM trips are as follows:

Weekday: Ln(T)=0.92 Ln(X)+2.68 which is equivalent to an average rate of 8.75 (5,181/592). AM: Ln(T)=0.91 Ln(X)+0.12 which is equivalent to an average rate of 0.63 (376/592). PM: Ln(T)=0.94 Ln(X)+0.27 which is equivalent to an average rate of 0.89 (529/592).

Item 1.

Mr. John Brock
Mission Rise
Response to Methodology Comments
TMC Project № 23017.1
May 23, 2023
Page 3 of 3

#### **END OF COMMENTS**

We trust these responses and the revised Methodology adequately address the review comments. We remain available to discuss this matter further or to answer any questions you may have.

Kind regards,

TRAFFIC & MOBILITY CONSULTANTS LLC

Charlotte N. Davidson, PE Senior Transportation Engineer



September 22, 2023

Mr. J. Brock
Town Clerk
Howey-in-the-Hills /Development Review Committee
101 North Palm Avenue
Howey-in-the-Hills, FL 34737
jbrock@howey.org

Re: Mission Rise

Response to Traffic Impact Analysis Comments

TMC Project № 23017.1 Howey-in-the-Hills, Florida

Dear Mr. Brock,

Please find below our responses to the review comments prepared by TMH Consulting on behalf of Howey-in-the-Hills /Development Review Committee dated September 6, 2023, regarding the above referenced Traffic Impact Analysis dated August 2023. The comments are listed in **bold** typeface and the TMC responses follow in *italic* typeface. Additionally, a revised Traffic Impact Analysis is provided under cover reflecting the changes resulting from these comments.

1. The conceptual land use plan states the maximum number of lots is 499. The traffic study and the development agreement states 592 lots. All three need to be the same.

TMC Response: The TIA has been updated to reference 499 single family residential units.

2. The methodology states that Lake Hills & Watermark are to be included in the background traffic projection. The submitted study left these developments out.

TMC Response: Vested trips from Lake Hills (Four Seasons) and Watermark (Simpson) developments have been included in the background traffic projection.

3. For the future condition intersection analysis for SR 19 & Revels Rd. include right & left turn lanes on SR 19 and a right turn lane on revels.

TMC Response: Turn lanes on SR 19 and Revels Road have been included in the projected intersection capacity analysis.

4. For the future condition intersection analysis for the Spine Rd. and Number 2 Rd., include right & left turn lanes on Number 2 Rd.

TMC Response: Turn lanes on Number 2 Road have been included in the projected intersection capacity analysis.

Mr. J. Brock
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5. Per the MPO TIS Guidelines the study needs to include a section for Mitigation Strategies. This needs to address the road segments and intersections with deficiencies. For unsignalized intersections, side streets with deficient delays need to be evaluated for mitigation. Also, the narrow width of Number 2 Road needs to be addressed in this section. While capacity is not an issue, operational safety is.

TMC Response: Text has been added to discuss the recommended segments and intersection mitigation measures.

6. There is no proposed widening of SR 19 at Central Avenue as stated in the study.

TMC Response: Acknowledged. Text has been updated accordingly.

7. Based on Lake County's requirement for turn lane widening on Number 2 Road (all on the south side) the length of tapers will need to be twice the standard length.

TMC Response: Taper lengths have been updated to ensure they are consistent with Lake County requirements.

#### **END OF COMMENTS**

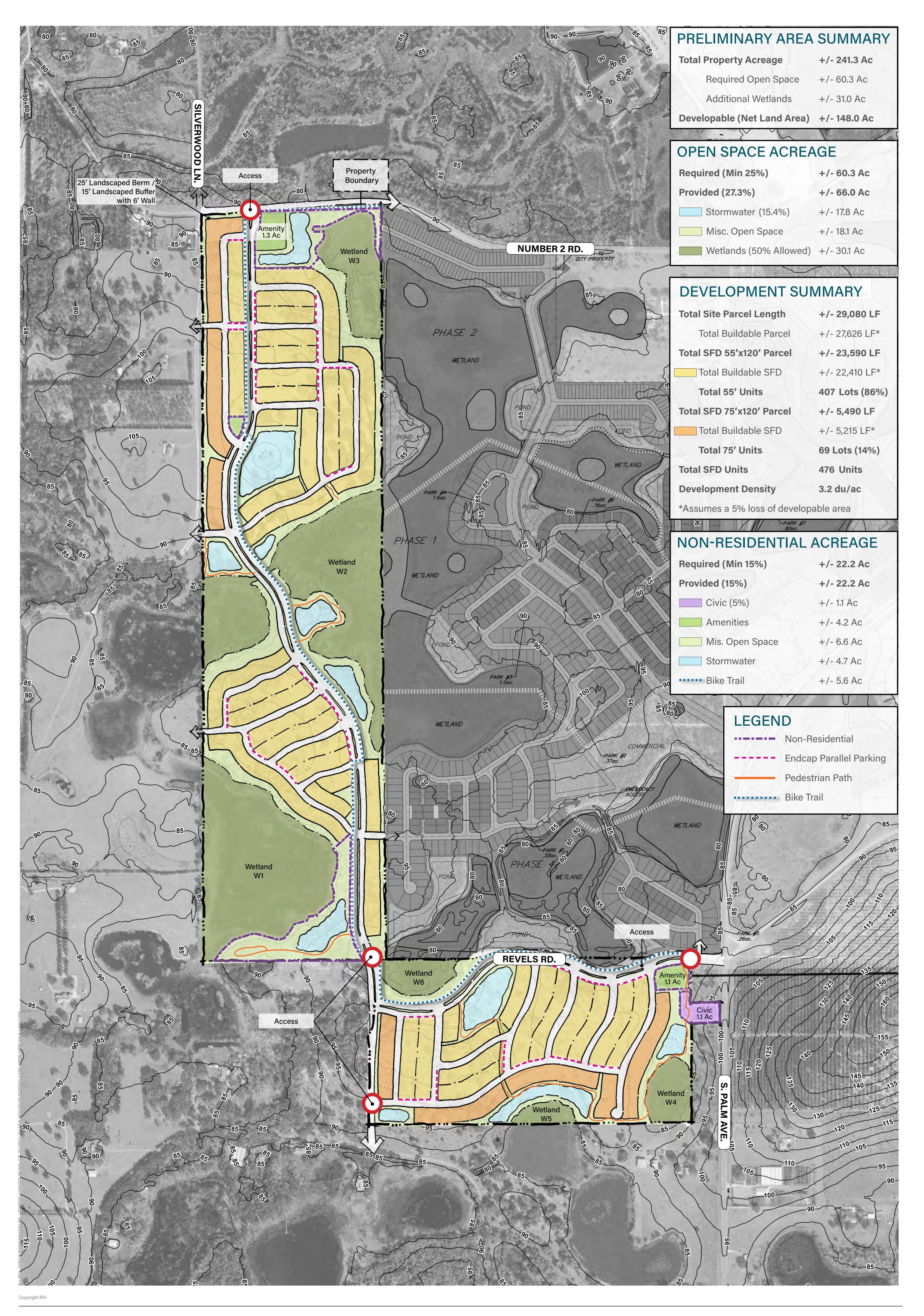
We trust these responses and the revised Traffic Impact Analysis adequately address the review comments. We remain available to discuss this matter further or to answer any questions you may have.

Kind regards,

TRAFFIC & MOBILITY CONSULTANTS LLC

Charlotte N. Davidson, PE Senior Transportation Engineer

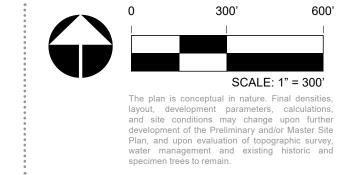
**Appendix B**Preliminary Development Plan





# MISSION RISE • CONCEPTUAL PLAN

- **♀** Town of Howey Hills, FL
- December 22, 2022
- **#** 22003786
- Turnstone Group



**Appendix C**Lake County CMP Database and 2023 FDOT Q/LOS

SEGMENT ID	COUNTY FDOT STATION STATION	DATA SOURCE SPEED LIMIT	SEGMENT LENGTH (MI)	ROAD NAME	FROM	то		INES URBAN / 027) RURAL	DIVIDED / UNDIVIDED	MAINTAINING AGENCY	( JURISDICTION	ADOPTED LOS DAILY SE STANDARD VOLU		2022 DAILY 20 V/C	U22 DAILY D	PEAK HOUR DIRECTIONAL RVICE VOLUME	2022 PEAK HOUR NB/EB VOLUME	2022 PEAK HOUR SB/WB VOLUME		2 PEAK UR LOS GROWTH RAT	DAILY E SERVICE VOLUME (2027)	2027 AADT 2027 D.	AILY 2027 DAILY L	LOS PEAK HOUR DIRECTIONAL SERVICE VOLUME (2027		2027 PEAK HOUR SB/WB VOLUME	2027 PEAK 2 HOUR V/C H	
1100 1110	497 490	County 35 County 35	1.75 0.55	C.R. 466B C.R. 468	EAGLE NEST ROAD CR 466A	CR 466A PINE RIDGE DAIRY ROAD			UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY FRUITLAND PARK	D 10,36			C C	530 530	193 190	233 213	0.44	C 1.25% C 1.25%	10,360 10,360	5,385 0.5 5,021 0.4		530 530	205 202	248 227	0.47	C C
1120	480 436	County 35 County 45		C.R. 468	PINE RIDGE DAIRY ROAD GRIFFIN ROAD	GRIFFIN ROAD SR 44			UNDIVIDED	COUNTY	FRUITLAND PARK	D 13,33		0.58	D C	680 620	343 440	384 404	0.56	D 3.00%	13,320	8,968 0.67 10,005 0.87		680 620	398	445 440	0.65	D
1145 1150	612 267	County 55 County 55		C.R. 46A REALIGNMENT	SR 44 SUMTER COUNTY LINE	SR 46 FLORIDA TURNPIKE			UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	C 7,74		2.14	E D	410 690	663 530	857 376		E 3.50% D 8.50%	7,740 28,880	19,687 2.54 16,996 0.51		410 1.500	788 797	1,018 566	2.48 0.53	E C
1155	266	County 55	2.39	C.R. 470	FLORIDA TURNPIKE	BAY AVENUE	2	2 RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D 12,60	0 8,826	0.70	D	660	436	278	0.66	D 1.00%	12,600	9,276 0.74	D D	660	458	292	0.69	D
1160 1170	266 499	ADJACENT 55 County 35	2.99	C.R. 473	BAY AVENUE CR 44	CR 33 FOUNTAIN LAKE BOULEVARD	2 :		UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	D 12,39	0 6,957	0.71	C D	620 710	436 322	278 242	0.70	C 1.00%	12,390 14,060	9,276 0.75 7,312 0.5		620 710	458 338	292 255	0.74	C
1180 1190	443	County 40 County 55	1.03 5.21	C.R. 473 C.R. 474	FOUNTAIN LAKE BOULEVARD SR 33	US 441 GREEN SWAMP ROAD	2		DIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	D 35,83		0.41	c c	1,800 410	811 151	461 240	0.45	C 1.00% C 2.50%	35,820 7,740	15,464 0.43 6,745 0.83		1,800 410	852 171	485 272	0.47	
1200 1210	3 222	County 55 County 45	3.35 5.99	C.R. 474	GREEN SWAMP ROAD SR 19	US 27 JAMARLY ROAD	2 :		UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY  CITY OF GROVELAND	C 7,74		0.70	C B	410 1.080	173 112	202 93	0.49	B 1.00% B 7.75%	7,740 21,780	5,713 0.74 3,259 0.15		410 1.080	182 162	212 135	0.52	B
1220 1225	259 248	County 55 County 55	3.17	C.R. 48 C.R. 48	SUMTER COUNTY LINE CLEARWATER LAKE RD	CLEARWATER LAKE RD CR 33	2	2 RURAL	UNDIVIDED	COUNTY	CITY OF LEESBURG	C 7,74	3,504	0.45	B B	410	112	180	0.44	B 4.25% B 1.75%	7,740	4,315 0.5i	C C	410	138	222	0.54	c
1230	263	County 45	0.46	C.R. 48	CR 33	HAYWOOD WORM FARM RD	2	2 URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D 15,93	0 8,836	0.55	С	790	370	297	0.47	C 2.75%	15,930	10,120 0.6	С	790	424	340	0.54	c
1235 1240	262 264	County 45 County 40		C.R. 48 C.R. 48	HAYWOOD WORM FARM RD US 27	US 27 LIME AVENUE	2		UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	D 16,83		0.54 0.45	C B	1,080	401 420	375 380	0.48	C 1.00% B 4.00%	16,820 21,780	9,536 0.5 11,949 0.5		840 1,080	421 511	394 462	0.50	В
1250 1260	255 253	County 40 County 40		C.R. 48 C.R. 48	LIME AVENUE CR 561	SR 19  RANCH ROAD	2	<ol> <li>URBAN</li> <li>URBAN</li> </ol>	UNDIVIDED	COUNTY	HOWEY-IN-THE-HILLS TOWN OF ASTATULA	D 21,78	,	0.46	B C	1,080 840	429 310	404 292	0.40	B 1.50% C 1.00%	21,780 16,820	10,754 0.49 6,847 0.49		1,080 840	462 326	435 307	0.43	C
1270 1280	253 217	ADJACENT 40 County 30		C.R. 48 C.R. 50 (SUNSET AVENUE)	RANCH ROAD CR 33	CR 448A SR 50			UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY  CITY OF MASCOTTE	C 7,74		0.84	c c	410 530	310 66	292 95	0.76 0.18	C 1.00% C 1.75%	7,740 10,360	6,847 0.8I 1,736 0.1		410 530	326 72	307 104	0.80	C
1290 1300	210 202	County 45 County 45	1.74	C.R. 50 C.R. 50	US 27 N HANCOCK ROAD	N HANCOCK ROAD CR 455			UNDIVIDED	COUNTY	CITY OF MINNEOLA UNINCORPORATED LAKE COUNTY	D 16,83 D 21,78		0.42 0.32	СВ	840 1,080	285 228	346 491	0.41 0.45	C 1.00% B 2.00%	16,820 21,780	7,337 0.44 7,593 0.3		840 1,080	299 251	363 542	0.43	С
1310	42	County 45	1.92	C.R. 50	CR 455	ORANGE COUNTY LINE	2	2 URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D 16,83	0 6,828	0.41	С	840	195	557	0.66	C 1.00%	16,820	7,176 0.43	С	840	205	585	0.70	c
1320 1325	417 417	County 35 County 35	1.08	C.R. 500A/ OLD 441 C.R. 500A/ OLD 441	SR 19 DORA AVENUE	DORA AVENUE SR 19	2	2 URBAN	DIVIDED	COUNTY	CITY OF TAVARES CITY OF TAVARES	D 8,39	9,907	1.18	F	870 870	367 367	450 450		D 1.00% D 1.00%	8,390 8,390	10,412 1.24 10,412 1.24	F F	870 870	386 386	473 473	0.54	D
1330 1340	413 115084 420	County 45 County 35		C.R. 500A/OLD 441/ALFRED ST C.R. 500A/OLD 441	DORA AVENUE BAY ROAD	BAY ROAD  CR 44C / EUDORA AVENUE			UNDIVIDED	COUNTY	CITY OF TAVARES CITY OF MOUNT DORA	D 16,80 D 10,36	0 9,558 0 9,917	0.57	C D	840 530	489 465	424 458	0.58	D 2.50%	16,820 10,360	10,045 0.6i 11,220 1.0i		840 530	514 526	446 518	0.61	D
1350 1360	421 415	County 35 County 35		C.R. 500A/OLD 441 C.R. 500A/OLD 441	CR 44C / EUDORA DRIVE LAKESHORE DRIVE	STH AVENUE			DIVIDED	COUNTY	CITY OF MOUNT DORA CITY OF MOUNT DORA	D 14,76	,	1.12	F F	750 530	725 469	761 505	1.01 0.95	E 4.25% D 4.25%	14,760 10,360	20,430 1.3i 13,800 1.3i		750 530	893 577	937 621	1.25	F
1370 1380	415 605	ADJACENT 25 ADJACENT 30		C.R. 500A/ 5TH AVENUE C.R. 500A (HIGHLAND STREET)	OLD 441 5TH AVENUE	N HIGHLAND STREET SR 46	2		UNDIVIDED	COUNTY	CITY OF MOUNT DORA CITY OF MOUNT DORA	D 10,36 D 13,33		1.08 0.21	F	530 680	469 179	505 127	0.95 0.26	D 4.25% C 3.50%	10,360 13,320	13,800 1.3 3,316 0.2		530 680	577 213	621 150	1.17 0.31	F
1390	602 115004	County 35	0.75	C.R. 500A/ OLD 441	SR 46	ORANGE COUNTY LINE	2	2 URBAN	UNDIVIDED	COUNTY	CITY OF MOUNT DORA	D 10,36	5,849	0.56	D	530	325	244	0.61	D 5.25%	10,360	7,555 0.73	B D	530	419 784	316	0.79	D
1400	401 257	County 45 County 50	3.93		SR 19 CR 448	CR 448  CR 48	2	2 URBAN	UNDIVIDED	COUNTY	CITY OF TAVARES  ASTATULA/TAVARES	D 16,83	10,160		В	1,080	622 507	825 590	0.98	D 4.75% C 1.00%	16,820 21,780	20,914 1.2d 10,678 0.4d	С	1,080	533		0.57	
1420 1430	252 252	County 40 ADJACENT 40			CR 48 SOUTH ASTATULA CITY LIMIT	SOUTH ASTATULA CITY LIMIT CR 455	2 2		UNDIVIDED	COUNTY	TOWN OF ASTATULA UNINCORPORATED LAKE COUNTY	D 12,39		0.96 0.55	C	620 1,080	570 570	558 558	0.92	C 1.00%	12,390 21,780	12,556 1.0° 12,556 0.56		620 1,080	599 599	586 586	0.97	
1440 1450	242	County 35 County 40		C.R. 561 C.R. 561	CR 455 HOWEY CROSS ROAD	HOWEY CROSS ROAD TURNPIKE ROAD / CR 561A	2 2		UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	C 9,03	1,000	0.85	C C	470 640	369 328	364 385	0.78	C 1.00%	9,030 12,260	8,090 0.91 8,529 0.70		470 640	387 345	382 405	0.82	C
1460 1470	235 214	County 45 County 30		C.R. 561 / C.R. 561A  FAST AVE/LAKE MINNEOLA DR/MAIN AVE	TURNPIKE ROAD / CR 561A	US 27 FAST AVENUE	2		UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY  CLERMONT/MINNEOLA	D 12,39		0.73 0.15	С	620 710	403 108	385 124	0.65	C 1.00% C 3.50%	12,390 14,060	9,538 0.77 2,555 0.18		620 710	423 128	405 147	0.68	С
1480	214	ADJACENT 30	1.05	8TH ST/OSCEOLA ST/4TH ST/CARROL ST/3RD S	S EAST AVENUE	W MINNEOLA AVENUE		2 URBAN	UNDIVIDED	COUNTY	CITY OF CLERMONT	D 10,36	2,151	0.21	c	530	108	124	0.23	C 3.50%	10,360	2,555 0.25	С	530	128	147	0.28	C
1490 1500	115065 115065 203	ADJACENT 35	0.23	C.R. 561 (W MINNEOLA AVENUE) C.R. 561	8TH STREET C.R. 561A	C.R. 561A SR 50	2	2 URBAN	UNDIVIDED	COUNTY	CITY OF CLERMONT CITY OF CLERMONT	D 12,39	5,175	0.09	С	620 710	179 278	186 212	0.30	C 1.00% C 6.50%	12,390 14,060	1,140 0.09 7,090 0.50	D D	620 710	188 381	195 290	0.31	C D
1510 1520	45 10	County 25 County 55	4.31 1.56	C.R. 561 C.R. 561	SR 50 LOG HOUSE ROAD	LOG HOUSE ROAD FLORIDA BOYS RANCH ROAD			UNDIVIDED	COUNTY	CITY OF CLERMONT UNINCORPORATED LAKE COUNTY	D 14,06		0.47	c	710 840	326 159	276 156	0.46	C 1.00% C 2.00%	14,060 16,820	6,934 0.49 4,159 0.29		710 840	342 175	290 172	0.48	C
1530 1540	6 237	County 55 County 55	5.87 1.16	C.R. 561 C.R. 561A	FLORIDA BOYS RANCH ROAD TURNPIKE ROAD / CR 561	SR 33 SCRUB JAY LN	2 :		UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	C 7,74		0.29 0.43	B C	410 620	106 199	100 308	0.26 0.50	B 2.25% C 1.25%	7,740 12,390	2,491 0.33 5,612 0.45		410 620	118 212	112 327	0.29	B C
1545 1546	234	County 55 ADJACENT 55	0.69	C.R. 561A C.R. 561A	SCRUB JAY LN N HANCOCK ROAD	N HANCOCK ROAD CR 455	2	2 URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	D 21,78	5,529	0.25	B B	1,080	201	307 307	0.28	B 1.00%	21,780 21,780	5,811 0.2 5,811 0.2	' В	1,080	211	322 322	0.30	В
1550	203	County 35	1.69	C.R. 561	W MINNEOLA AVE	C.R. 565A	2	2 URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D 13,33	5,175	0.39	С	680	278	212	0.41	C 6.50%	13,320	7,090 0.53	B D	680	381	290	0.56	D
1560 1570	213 223	County 40 County 40		C.R. 561A  C.R. 561 (LAKE MINNEOLA SHORES)	CR 565A JALARMY ROAD	JALARMY ROAD US 27			UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY  CITY OF MINNEOLA	D 16,83		0.39	С	840 840	316 397	258 491		C 4.50% C 3.00%	16,820 16,820	8,081 0.41 12,829 0.76		840 840	393 460	322 569	0.47	C
1580 1590	241	County 55 County 40		C.R. 565 C.R. 565 (VILLA CITY ROAD)	US 27 KJELLSTROM LANE	KJELLSTROM LANE SR 50			UNDIVIDED	COUNTY	GROVELAND/MASCOTTE CITY OF GROVELAND	C 14,13		0.17	B C	740 840	167 247	70 249	0.23	B 5.25% C 4.25%	14,130 16,820	3,032 0.2 6,608 0.3		740 840	215 305	90 307	0.29	C C
1600 1610	118063 118063 118063 118063		1.96 5.44	C.R. 565 C.R. 565	SR 50 SLOANS RIDGE	SLOANS RIDGE LAKE ERIE ROAD			UNDIVIDED	COUNTY	CITY OF MASCOTTE  UNINCORPORATED LAKE COUNTY	D 16,83	0 865 0 865	0.05	C B	840 410	44 44	42 42	0.05	C 2.00% B 2.00%	16,820 7,740	955 0.00 955 0.11		840 410	49 49	46 46	0.06	C B
1620 1630	201 47	County 40 County 55		C.R. 565A C.R. 565A	SR 50 SR 50	CR 561A CR 565B	2	2 URBAN	UNDIVIDED	COUNTY	CLERMONT/GROVELAND CITY OF GROVELAND	D 16,83	9,917	0.59	C B	840 1.080	407 82	348 133	0.48	C 2.25% B 3.25%	16,820 21,780	11,084 0.6i 2,991 0.14	G C	840 1,080	454 96	389 156	0.54	С
1640	18	County 45	3.66	C.R. 565B	SR 33	CR 561	2	2 RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	C 7,74	2,796	0.36	В	410	135	152	0.37	B 4.00%	7,740	3,401 0.44	В	410	164	185	0.45	В
1650 1660	434 426	County 25 County 25	0.30	CANAL STREET CANAL STREET	US 441 MAIN STREET	MAIN STREET SR 44			DIVIDED UNDIVIDED	CITY OF LEESBURG	CITY OF LEESBURG CITY OF LEESBURG	D 13,99		0.27 0.24	c	710 680	201 144	137 127	0.28 0.21	C 1.00%	13,990 13,320	3,957 0.21 3,331 0.25		710 680	211 151	144 134	0.30	C
1670 1680	205 44	County 35 County 30	1.80 0.47	CITRUS TOWER BOULEVARD  CITRUS TOWER BOULEVARD	US 27 OAKLEY SEAVER DRIVE	OAKLEY SEAVER DRIVE SR 50		2 URBAN 4 URBAN	UNDIVIDED	COUNTY	CITY OF CLERMONT CITY OF CLERMONT	D 14,06		0.87	D D	710 1,470	651 561	446 715	0.92	D 1.00%	14,060 29,160	12,923 0.93 17,068 0.50		710 1,470	684 590	469 752	0.96	D D
1690 1692	28	County 40 County 30	0.28	CITRUS TOWER BOULEVARD	SR 50 HOOKS STREET	HOOKS STREET	-		DIVIDED	COUNTY	CITY OF CLERMONT	D 35,82		0.60	C D	1,800	798 740	1,065	0.59	C 1.25% D 1.00%	35,820 30,780	22,846 0.6 21,284 0.6		1,800 1,550	849 778	1,134	0.63	C
1695	24	County 40	0.60	CITRUS TOWER BOULEVARD	JOHNS LAKE ROAD	US 27 CR 194	4		DIVIDED	COUNTY	CITY OF CLERMONT	D 37,8		0.47	С	1,900	738	629	0.39	C 1.50%	37,810 14,060	19,095 0.5		1,900	795	678	0.42	С
1710	442	County 35		DAVID WALKER DRIVE	CR 19A	US 441	2		UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D 14,06		0.61	D	710	388	367	0.55	D 1.00%	14,060	8,989 0.6		710	408	386	0.57	D
1720 1730	449 471	County 35 County 20	0.74	DAVID WALKER DRIVE DAVID WALKER DRIVE	US 441 MOUNT HOMER ROAD	MOUNT HOMER ROAD FLINKS AVE/KURT AVE		2 URBAN	UNDIVIDED	COUNTY	CITY OF EUSTIS CITY OF EUSTIS	D 14,00 D 10,36	0 6,537	0.63	C D	710 530	214 334	265 277	0.37 0.63	C 1.00% D 3.50%	14,060 10,360	5,984 0.43 7,763 0.75		710 530	225 397	279 329	0.39	D
1740 1750	406 117014 617	County 35 County 35	2.29 1.25	DEAD RIVER ROAD DONNELLY STREET	WEST TERMINI US 441	SR 19 11TH AVENUE			DIVIDED	COUNTY CITY OF MT. DORA	CITY OF TAVARES CITY OF MOUNT DORA	D 21,78		0.31	B D	1,080 750	276 535	355 474	0.33	B 1.00% D 1.00%	21,780 14,760	7,131 0.3 11,792 0.8		1,080 750	291 563	373 498	0.35	D D
1760 1770	617 258	ADJACENT 35 County 55		DONNELLY STREET  DUDA ROAD	11TH AVENUE CR 448A	5TH AVENUE ORANGE COUNTY LINE			UNDIVIDED	CITY OF MT. DORA COUNTY	CITY OF MOUNT DORA  UNINCORPORATED LAKE COUNTY	D 10,36 C 9,03		1.08	F C	530 470	535 293	474 323	1.01 0.69	E 1.00% C 1.50%	10,360 9,030	11,792 1.14 7,810 0.8i		530 470	563 316	498 348	1.06 0.74	F C
1780 1790	510 46	County 40 County 30		EAGLES NEST ROAD  EAST AVENUE	US 27 CR 561	CR 466B SR 50	2	2 URBAN	UNDIVIDED	COUNTY CITY OF CLERMONT	UNINCORPORATED LAKE COUNTY CITY OF CLERMONT	D 12,39		0.34 0.56	C D	620 530	198	133	0.32	C 3.75%	12,390 10,360	5,134 0.4 6,139 0.5		620 530	238	160	0.38	C
1800	454 454	ADJACENT 25	0.85	EAST CROOKED LAKE ROAD  EAST CROOKED LAKE ROAD	LAKEVIEW DRIVE BROADVIEW AVENUE	BROADVIEW AVENUE US 441	2	2 URBAN	UNDIVIDED	COUNTY	CITY OF EUSTIS	D 10,36		0.50	D D	530	273 273	167 167	0.52 0.52	D 1.00% D 1.00%	10,360	5,416 0.53 5,416 0.53	2 D	530 530	287	176 176	0.54	D D
1820	501	County 35	0.77	EMERALDA AVENUE	EMERALDA ISLAND ROAD	CR 44	2	2 URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D 13,33	0 4,265	0.32	С	530 680	273 266	167		C 2.50%	13,320	4,826 0.36	C C	680	301	176	0.54	С
1830 1840	41 622	ADJACENT 40	0.76		CR 565 CR 44A	ANDERSON ROAD  LAKE LINCOLN LANE	2	2 URBAN	UNDIVIDED	COUNTY	CITY OF GROVELAND UNINCORPORATED LAKE COUNTY	D 15,93	1,442	0.28	B C	410 790	146	262		- 1.00% C 2.75%	7,740 15,930	1,516 0.2i 5,021 0.3i	2 C	410 790	168	300		c c
1850 1860	622 452	County 40 County 35		ESTES ROAD EUDORA ROAD	OLD MT DORA ROAD	SR 44 US 441	2 2		UNDIVIDED	COUNTY CITY OF EUSTIS	UNINCORPORATED LAKE COUNTY CITY OF EUSTIS	D 16,83		0.26 0.29	C	840 530	146	262	0.31	C 2.75%	16,820 10,360	5,021 0.3i 3,151 0.3i		840 530	168	300	0.36	- C
1865 1870	30 508	County 35 County 35	0.73	EXCALLIBUR ROAD FISH CAMP ROAD	HOOKS STREET CR 452	CITRUS TOWER BOULEVARD CR 44	2 :	2 URBAN	DIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	D 14,76	5,301	0.36	C C	750 530	346 83	219 72	0.46 0.16	C 1.00% C 2.50%	14,760 10,360	5,572 0.3i 1,721 0.1i		750 530	364 94	230 82	0.49	C
1875	221 470	County 40  County 30	1.69	GRASSY LAKE ROAD/FOSGATE ROAD  GOLFLINKS AVENUE	CR 50 (WASHINGTON STREET) KURT STREET	HANCOCK ROAD SR 19 / BAY STREET	2	2 URBAN		CITY OF CLERMONT	UNINCORPORATED LAKE COUNTY	D 16,83	5,995	0.36	C	840 530	288 45	350	0.42	C 7.50%	16,820	8,606 0.5	ı c	840	414	503	0.60	C
1890	0	NO COUNT	0.38	GOLFLINKS AVENUE	SR 19 / BAY STREET	MARY STREET	2	2 URBAN	UNDIVIDED	CITY OF EUSTIS	CITY OF EUSTIS CITY OF EUSTIS	D 12,38	0 -	-	-	620	-	49	-	- N/A	12,390		-	530 620	-	-	-	-
1900 1910	514 40	County 45 County 35	1.23	GOOSE PRAIRIE ROAD GRAND HIGHWAY	EMERALDA AVENUE CITRUS TOWER BOULEVARD	CR 452 SR 50		2 URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY CITY OF CLERMONT	D 12,39	0 6,479	0.46	C C	620 710	196 268	111 273	0.39	C 3.25% C 1.00%	12,390 14,060	3,718 0.3i 6,809 0.4i	С	620 710	230 282	287	0.40	C
1915 1920	37 226	County 25 County 40		S. GRAND HIGHWAY CITRUS GROVE ROAD	SR 50 US 27	HOOKS STREET GRASSY LAKE ROAD			DIVIDED	COUNTY	CITY OF CLERMONT CITY OF MINNEOLA	D 29,16 D 12,39		0.18 0.43	C C	1,470 620	261 270	203 173	0.18 0.44	C 1.00% C 12.00%	29,160 12,390	5,469 0.11 9,373 0.76		1,470 620	275 476	213 305	0.19	C C
1930 1940	517 117007 517 117007	ADJACENT 45	1.76	GRAYS AIRPORT ROAD GRAYS AIRPORT ROAD	MARION COUNTY ROAD CR 466	CR 466 GRIFFIN VIEW DRIVE	2	2 URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	D 12,39	0 2,911	0.23 0.23	C C	620 620	173 173	118	0.28	C 3.25% C 3.25%	12,390 12,390	3,416 0.20 3,416 0.20	В С	620 620	203 203	138 138	0.33	C
1950	512 117007	County 45	1.75	S GRAYS AIRPORT ROAD	GRIFFIN VIEW DRIVE	EAGLES NEST ROAD	2	2 URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D 12,39	2,966	0.24	С	620	115	174	0.28	C 5.50%	12,390	3,877 0.3	С	620	150	228	0.37	C
1960 1970	505 536 117008		0.85	S GRAYS AIRPORT ROAD GRIFFIN AVENUE	EAGLES NEST ROAD US 27 / US 411	US 27 / US 412 CR 25		2 URBAN	UNDIVIDED	COUNTY	FRUITLAND PARK TOWN OF LADY LAKE	D 12,30 D 13,30	11,009		C D	620 680	55 599	28 378	0.88	C 1.00% D 1.75%	12,390 13,320	826 0.07 12,007 0.90	D D	620 680	58 653	30 412	0.09	C D
1980 1990	535 535	County 35 ADJACENT 35		GRIFFIN AVENUE GRIFFIN AVENUE	CR 25 UNCLE DONALDS LANE	UNCLE DONALDS LANE GRAYS AIRPORT ROAD			UNDIVIDED	COUNTY	TOWN OF LADY LAKE UNINCORPORATED LAKE COUNTY	D 10,36		0.33	C C	530 530	214 214	108 108	0.40	C 1.50%	10,360 10,360	3,737 0.3i 3,737 0.3i		530 530	230 230	116 116	0.43	c
2000 2010	462 515	County 25 County 45		GRIFFIN ROAD GRIFFIN VIEW DRIVE	US 27 US 27	LEE STREET GRAYS AIRPORT ROAD			UNDIVIDED	CITY OF LEESBURG COUNTY	CITY OF LEESBURG TOWN OF LADY LAKE	D 13,33 D 12,35		0.15 0.28	C C	680 620	202	124	0.33	- 1.00% C 1.00%	13,320 12,390	2,166 0.16 3,676 0.31		680 620	212	130	0.34	- C
2020 2030	516 479	County 45	1.64	GRIFFIN VIEW DRIVE GROVE STREET	GRAYS AIRPORT ROAD  SR 19 (BADGER AVENUE)	SULEN ROAD  LAKEVIEW AVENUE	2	2 RURAL	UNDIVIDED	COUNTY CITY OF EUSTIS	UNINCORPORATED LAKE COUNTY  CITY OF EUSTIS	C 9,03	1,715	0.19	С	470 530	113	75 106	0.24	C 1.00%	9,030	1,802 0.20 1,550 0.10	) с	470 530	118	78	0.25	C
2040	472	County 30 County 30	0.37	GROVE STREET	LAKEVIEW AVENUE	GOLFLINKS AVENUE	2	2 URBAN	UNDIVIDED	CITY OF EUSTIS	CITY OF EUSTIS	D 10,36	2,561	0.25	C	530	160	71	0.30	C 1.00%	10,360	2,692 0.20	C C	530	168	75	0.32	С
2045 2050	465 117017 21	County 25 County 35			GOLFLINKS AVENUE  LAKE SHORE DRIVE	OLD MT DORA ROAD US 27	4	2 URBAN 4 URBAN	UNDIVIDED	CITY OF EUSTIS COUNTY	CITY OF EUSTIS CITY OF CLERMONT		0 3,733 0 18,440	0.36	C B	530 2,950	140 479	250 1,149		C 1.00% B 2.25%	10,360 59,580	3,923 0.3i 20,610 0.3i		530 2,950	148 536	263 1,284		D B
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SEGMENT ID COUNTY FDOT STATION DATA SOURCE SPEED SEGMENT LENGTH (MI) ROAD NAME	FROM	то	LANES LANES (2022) (2027)	URBAN / DIVIDED / MA	AINTAINING AGENCY	JURISDICTION	ADOPTED LOS DAILY SERVIC STANDARD VOLUME	E 2022 AADT	2022 DAILY 2022 DAIL' V/C LOS	Y PEAK HOUR DIRECTIONAL SERVICE VOLUME	2022 PEAK HOUR NB/EB VOLUME	2022 PEAK HOUR SB/WB VOLUME	022 PEAK 2022 HOUR V/C HOU		DAILY SERVICE 2027 VOLUME (2027)	7 AADT 2027 DAILY 2	1027 DAILY LOS PEAK HOUR D SERVICE VOL	RECTIONAL HOUR	EAK 2027 PEA NB/EB HOUR SB/N VOLUME		2027 PEAK HOUR LOS
3020 110049 110049 State 45 1.38 SR 19 3030 110049 110049 ADJACENT 45 0.90 SR 19	CR 452 (MAIN STREET) CR 561	CR 561  LANE PARK ROAD		URBAN DIVIDED URBAN UNDIVIDED	STATE STATE	CITY OF TAVARES CITY OF TAVARES	D 41,790 D 18,590		1.09 F 2.45 F	2,100 920	2,203 2,203	1,892 1,892		F 4.50% F 4.50%	18,590 56	3,701 1.36 3,701 3.05	F 2,10				
3040 110494 110494 State 55 3.87 SR 19  3050 110495 110495 State 40 0.84 SR 19  3060 110495 110495 ADJACFAT 35 3.09 SR 19	LANE PARK ROAD  CR 48  CENTRAL AVENUE	CR 48 CENTRAL AVENUE	2 2 2	URBAN UNDIVIDED	STATE STATE	HOWEY-IN-THE-HILLS/TAVARES HOWEY-IN-THE-HILLS HOWEY-IN-THE-HILLS			0.86 C 0.63 C	920 700	610 433	372	0.62	1.00%	14,160 9	3,795 0.90 ,407 0.66	C 920	45	5 391	0.75	С
3060 110495 110495 ADJACENT 35 3.09 SR 19 3070 110255 110255 State 55 2.72 SR 19 3080 110376 110376 State 55 4.73 SR 19	CR 455 US 27 / SR 25	CR 455 US 27 / SR 25 CR 478	2 2	RURAL UNDIVIDED RURAL UNDIVIDED RURAL UNDIVIDED	STATE STATE STATE	CITY OF GROVELAND  CITY OF GROVELAND	D 24,200 C 8,600 C 8,600	9,910 9,350	0.37 B 1.15 D 1.09 D	1,200 450 450	433 507 466	435	1.13	B 1.00% D 1.00% D 1.00%	8,600 10	,407 0.39 0,416 1.21 .827 1.14	B 1,20 D 450 D 450	53	5 391 3 457 0 545	0.38 1.18 1.21	D
3090 110376 110376 ADJACENT 55 1.22 SR 19 3100 110097 110097 State 45 0.70 SR 19	CR 478  LAKE CATHERINE ROAD	LAKE CATHERINE ROAD SR 50/ SR 33	2 2	URBAN UNDIVIDED URBAN UNDIVIDED	STATE STATE	CITY OF GROVELAND CITY OF GROVELAND	D 17,700 D 17,700	9,350 12,950	0.53 C 0.73 C	880 880	466 449	519 533	0.59	1.00% 1.50%	17,700 9	,827 0.56 3,951 0.79	C 880	) 49	0 545	0.62	С
3110 115072 115072 State 40 0.52 SR 33 3120 110497 110497 State 60 3.16 SR 33	SR 50/ SR 33 ANDERSON ROAD	ANDERSON ROAD CR 565B		URBAN UNDIVIDED RURAL UNDIVIDED	STATE STATE	CITY OF GROVELAND CITY OF GROVELAND	D 18,590 C 8,600	14,760 10,428	0.79 C 1.21 D	920 450	470 533	667 458		C 4.25% D 3.75%		3,175 0.98 2,535 1.46	D 920 D 450			0.89 1.42	C D
3130 111002 111002 State 60 6.76 SR 33 3140 5 County 60 2.33 SR 33	CR 565B CR 561	CR 561 CR 474	2 2 2	RURAL UNDIVIDED	STATE STATE	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	C 8,600 C 8,600	8,242 13,084	0.96 C 1.52 D	450 450	421 452	362 415		1.75% 1.25%		,988 1.05 3,923 1.62	D 450			1.02	D D
3150 2 County 60 1.04 SR 33 3160 808 County 45 4.71 SR 40	CR 474 MARION COUNTY LINE	POLK COUNTY LINE CR 445A		RURAL UNDIVIDED RURAL UNDIVIDED	STATE STATE	UNINCORPORATED LAKE COUNTY  UNINCORPORATED LAKE COUNTY  UNINCORPORATED LAKE COUNTY	C 10,320 C 8,600	10,821 5,068	1.05 D 0.59 C	540 450	352 169	544 217	0.48	3 4.50% B 2.75%	8,600 5	3,485 1.31 ,805 0.68	F 540 C 450	) 19	3 248	1.26 0.55	F C
3170 110503 110503 State 55 1.61 SR 40 3180 110050 110050 State 45 1.43 SR 40 3190 110496 110496 State 55 2.38 SR 44	CR 445A RIVER ROAD SUMTER COUNTY LINE	RIVER ROAD  VOLUSIA COUNTY LINE  CR 468	2 2	RURAL DIVIDED URBAN DIVIDED	STATE STATE STATE	UNINCORPORATED LAKE COUNTY  UNINCORPORATED LAKE COUNTY  CITY OF LEESBURG	C 10,320 C 14,220 D 39,800	5,370 10,180 21,800	0.52 C 0.72 C 0.55 C	740 2.000	274 401 1,071	236 406 964	0.55	1.00% 2 4.75% 2 1.00%	14,220 12	,644 0.55 2,839 0.90 2,912 0.58	C 540 C 740 C 2.00	50	512	0.53 0.69 0.56	C
3190 110490 110490 State 35 2.30 Str44 3200 110487 110487 State 45 1.54 SR 44 3210 115147 115147 State 35 0.76 SR 44	CR 468 S LONE OAK DRIVE	S LONE OAK DRIVE US 27	4 4	URBAN DIVIDED URBAN DIVIDED	STATE STATE	UNINCORPORATED LAKE COUNTY  CITY OF LEESBURG	D 39,800 D 39,800 D 32,400	16,540 19,480	0.42 C 0.60 D	2,000 2,000 1,630	610	720 769	0.36	1.00% 1.00% 0 1.00%	39,800 17	7,384 0.44 0,474 0.63	C 2,00 D 1,63	0 64	1 757	0.38	C
3220 115179 115179 State 35 0.57 SR.44 (DIXIEAVENUE) 3230 115143 115143 ADJACENT 35 0.34 SR.44 (DIXIEAVENUE)	US 27 S 9TH STREET	S 9TH STREET CANAL STREET		URBAN DIVIDED URBAN DIVIDED	STATE STATE	CITY OF LEESBURG CITY OF LEESBURG	D 32,400 D 32,400	27,300	0.84 D 0.72 D	1,630 1,630	1,322	1,135 928		D 1.25%	32,400 29	9,049 0.90 1,383 0.75	D 1,63	0 1,4	7 1,208	0.86	D D
3240 115143 115143 State 40 0.41 SR 44 (DIXIE AVENUE) 3250 115142 115142 State 40 0.79 SR 44 (DIXIE AVENUE)	CANAL STREET S LAKE STREET	S LAKE STREET E MAIN STREET	4 4 4	URBAN DIVIDED URBAN DIVIDED	STATE STATE	CITY OF LEESBURG CITY OF LEESBURG	D 39,800 D 39,800	23,200 18,760	0.58 C 0.47 C	2,000 2,000	922 908	928 780	0.46	1.00% C 1.00%		1,383 0.61 9,717 0.50	C 2,00			0.49 0.48	C
3260         115183         State         40         0.11         SR 44 (DIXIE AVENUE)           3262         110005         State         45         0.45         SR 44 (OLD C.R. 44B)	E MAIN STREET US 441	US 441 WAYCROSS AVENUE		URBAN DIVIDED URBAN DIVIDED	STATE STATE	CITY OF LEESBURG CITY OF MOUNT DORA	D 41,790 D 19,510	18,760 25,500	0.45 C 1.31 F	2,100 970	908 1,235	780 1,060	0.43 1.27		19,510 26	9,717 0.47 3,801 1.37	C 2,10 F 970	1,2	98 1,114	0.45 1.34	C F
3268 110006 110006 State 45 1.65 SR 44 (OLD C.R. 44B) 3270 110500 110500 ADJACENT 55 2.27 SR 44	WAYCROSS AVENUE ABRAMS ROAD	ORANGE AVENUE THRILL HILL ROAD	2 2	URBAN UNDIVIDED URBAN UNDIVIDED	STATE STATE	EUSTIS/MOUNT DORA CITY OF EUSTIS	D 18,590 D 18,590	17,880 13,810	0.96 D 0.74 C	920 920	907 706	637 606	0.77	1.00% 1.00%	18,590 14	3,792 1.01 4,514 0.78	F 920 C 920	74	2 637	0.81	С
3280 110500 110500 ADJACENT 55 1.14 SR 44  3280 110500 110500 State 55 3.03 SR 44  3330 110500 110500 ADJACENT 55 1.15 SR 44	THRILL HILL ROAD CR 439 CR 437	CR 439 CR 437 CR 46A	2 2	URBAN UNDIVIDED RURAL UNDIVIDED RURAL UNDIVIDED	STATE STATE STATE	CITY OF MOUNT DORA  UNINCORPORATED LAKE COUNTY  UNINCORPORATED LAKE COUNTY	D 17,700 C 15,700 C 13,550	13,810 13,810 13,810	0.78 C 0.88 C 1.02 D	880 820 700	706 706 706	606 606 606	0.86	1.00% 1.00% 1.00%	15,700 14	4,514 0.82 4,514 0.92 4,514 1.07	C 880 C 820 D 700	74		0.84 0.90 1.06	С
3310 110010 110010 ADJACENT 55 3.43 SR.44 3320 110010 110010 ADJACENT 55 5.34 SR.44	CR 46A CR 44A	CR 44A OVERLOOK DRIVE		RURAL UNDIVIDED RURAL UNDIVIDED	STATE STATE	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	C 8,600 C 8,600	9,383	1.09 D 1.09 D	450 450	480	412 412		D 1.00%	8,600 9	,861 1.15 ,861 1.15	D 450	50	4 433		D
3330 110010 110010 State 55 5.64 SR 44  3340 110010 110010 ADJACENT 55 0.26 SR 44	OVERLOOK DRIVE CR 42	CR 42 VOLUSIA COUNTY LINE	2 2 2	RURAL UNDIVIDED RURAL UNDIVIDED	STATE STATE	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	C 15,700 C 13,550	9,383 9,383	0.60 B 0.69 C	820 700	480 480	412 412		B 1.00%	15,700 9	.861 0.63 .861 0.73	B 820 C 700		4 433	0.61	В
3344 110200 110200 State - 1.80 SR 429 (WEKIVA PKWY) 3345 610 County - 5.54 SR 46	ORANGE C/L CR 46A (REALIGNED)	CR 46A (REALIGNED) SEMINOLE C/L	4 4 4	URBAN DIVIDED URBAN DIVIDED	STATE STATE	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	D 66,200 D 66,200	6,200 17,646	0.09 B 0.27 B	3,280 3,280	422 657	322 874		3 4.25% 3 1.00%	66,200 18	,634 0.12 3,547 0.28	B 3,28			0.16 0.28	B B
3350 110501 110501 ADJACENT 45 1.08 SR 46 3360 110501 110501 State 55 0.94 SR 46	US 441 VISTA VIEW	VISTA VIEW ROUND LAKE ROAD		URBAN DIVIDED URBAN DIVIDED	STATE STATE	CITY OF MOUNT DORA CITY OF MOUNT DORA	D 62,900 D 62,900	13,420 13,420	0.21 C 0.21 C	3,170 3,170	650 650	558 558	0.21	3.25% 3.25%	62,900 15	5,747 0.25 5,747 0.25	C 3,17	0 76	3 655	0.24	
3370 110001 110001 ADJACENT 55 2.11 SR.46 3380 110001 110001 State 45 0.51 SR.46 3380 111019 111019 State 45 1.11 SR.46	ROUND LAKE ROAD CR 437 SOUTH CR 437 NORTH	CR 437 SOUTH CR 437 NORTH CR 435	2 2 2	URBAN UNDIVIDED URBAN UNDIVIDED	STATE STATE STATE	CITY OF MOUNT DORA  UNINCORPORATED LAKE COUNTY  LININGORPORATED LAKE COUNTY	D 24,200 D 17,700 D 17,700	14,950	0.62 C 0.84 C 0.75 C	1,200 880 880	600 600	600 600 550	0.50 0.68 0.73	1.50% 1.50%	17,700 16	3,105 0.67 3,105 0.91 3,905 0.79	C 1,20 C 880 C 886	) 64	6 646	0.54 0.73	C
3390 111019 111019 State 45 1.11 SR 46 3395 611 118115 County 45 0.87 SR 46 3420 110319 110319 State 55 3.64 SR 50	CR 437 NORTH  CR 435  SUMTER COUNTY LINE	CR 46A (REALIGNED) CR 565 / BAY LAKE ROAD		URBAN UNDIVIDED URBAN UNDIVIDED URBAN UNDIVIDED	STATE STATE	UNINCORPORATED LAKE COUNTY  UNINCORPORATED LAKE COUNTY  LININCORPORATED LAKE COUNTY	D 17,700 D 17,700 D 24,200	13,230 10,963 14,320	0.75 C 0.62 C 0.59 C	880 880 1,200	467 591	480 649	0.54	1.00% 1.00% 1.50%	17,700 1	3,905 0.79 1,522 0.65 5,427 0.64	C 880 C 120	) 49	0 504	0.77 0.57 0.58	C
3430 110319 110319 ADJACENT 35 0.77 SR 50 3440 110241 110241 State 45 0.96 SR 50	CR 565 / BAY LAKE ROAD CR 33	CR 33 GROVELAND FARMS ROAD	2 2	URBAN UNDIVIDED URBAN DIVIDED	STATE STATE	CITY OF MASCOTTE CITY OF MASCOTTE	D 14,800 D 39,800	14,320	0.97 D 0.65 C	750 2,000	591 942	649 1,013	0.87	D 1.50%	14,800 15	5,427 1.04 7,326 0.69	E 750		7 699	0.93	D
3450 110241 110241 ADJACENT 45 0.63 SR 50 3460 115182 115182 State 35 0.44 SR 50 (E)	GROVELAND FARMS ROAD SR 50 ONE WAY PAIRS	SR 50 ONE WAY PAIRS SR 19	4 4 4	URBAN DIVIDED URBAN DIVIDED	STATE STATE	CITY OF GROVELAND CITY OF GROVELAND	D 41,790 D 19,440	26,000 12,350	0.62 C 0.64 D	2,100 1,960	942 1,110	1,013	0.48	C 1.00%	41,790 27	7,326 0.65 2,980 0.67	C 2,10 D 1,96	0 99	0 1,065	0.51 0.60	C D
3470 115077 115077 State 35 0.44 SR 50 (W) 3481 115181 115181 State 35 0.33 SR 50 (E)	SR 19 SR 19	SR 50 ONE WAY PAIRS SR 33 SOUTH	4 4 4	URBAN DIVIDED URBAN DIVIDED	STATE STATE	CITY OF GROVELAND CITY OF GROVELAND	D 19,440 D 19,440	16,800 12,750	0.86 D 0.66 D	1,960 1,960	0 1,146	1,510 0	0.77	D 1.75% D 1.00%		3,322 0.94 3,400 0.69	D 1,96		1,647	0.84 0.61	D D
3491 115076 115076 State 35 0.34 SR 50 (W) 3500 115134 115134 State 55 1.53 SR 50	SR 33 SOUTH SR 33 SOUTH	SR 19 CR 565A NORTH		URBAN DIVIDED URBAN DIVIDED	STATE	CITY OF GROVELAND CITY OF GROVELAND	D 19,440 D 41,790	14,700 30,314	0.76 D 0.73 C	1,960 2,100	1,468	1,322 1,260	0.70	0 1.00% C 1.00%	41,790 31	5,450 0.79 1,861 0.76	D 1,96	0 1,5	1,324	0.71	
3510 110396 110396 State 55 3.15 SR 50 3520 115057 115057 State 40 1.19 SR 50 3530 115050 115050 State 40 0.92 SR 50	CR 565A NORTH CR 561 EAST AVENUE	CR 561 EAST AVENUE US 27	4 4	URBAN DIVIDED URBAN DIVIDED URBAN DIVIDED	STATE STATE STATE	CITY OF GROVELAND CITY OF CLERMONT CITY OF CLERMONT	D 41,790 D 39,800 D 41,790	29,500 35,600 32,650	0.71 C 0.89 C 0.78 C	2,100 2,000 2,100	1,059 1,724 1,581	2,242 1,480 1,358	0.86	F 1.00% C 1.50% C 1.00%	39,800 38	1,005 0.74 3,351 0.96 1,315 0.82	C 2,10 D 2,00 C 2.10	0 1,8	57 1,594	1.12 0.93 0.79	С
3540 110390 110390 State 55 2.14 SR 50 3550 110390 110390 ADJACENT 55 1.49 SR 50	US 27 HANCOCK ROAD	HANCOCK ROAD CR 455	6 6	URBAN DIVIDED URBAN DIVIDED	STATE STATE	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	D 62,900 D 62,900	54,629 54,629	0.87 C	3,170 3,170	2,645 2,645	2,271	0.83	1.00% 1.00%	62,900 57	7,415 0.91 7,415 0.91	C 3,17	0 2,7	30 2,387	0.88	С
3560 750572 750572 State 50 1.53 SR 50 3562 972200 972200 State 70 1.38 SR 91 (FLORIDA TURNPIKE)	CR 455 SUMTER COUNTY LINE	ORANGE COUNTY LINE CR 470	6 6 4 4	URBAN DIVIDED URBAN FREEWAY	STATE STATE	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	D 62,900 B 47,600	53,750 46,882	0.85 C 0.98 B	3,170 2,230	2,574 2,648	2,264 2,274	0.81	1.00% C 1.00%	62,900 56	5,492 0.90 9,273 1.04	C 3,17 C 2,23			0.85 1.25	С
3564 972160 972160 State 70 7.50 SR 91 (FLORIDA TURNPIKE) 3566 972006 972006 State 70 3.72 SR 91 (FLORIDA TURNPIKE)	CR 470 US 27/SR 25	US 27/SR 25 US 27/SR 25/SR 19 INTERCHANGE		URBAN FREEWAY URBAN FREEWAY	STATE STATE	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	B 47,600 B 47,600	49,600 43,670	1.04 C 0.92 B	2,230 2,230	2,661 2,733	1,803 1,852	1.19	1.00%	92,200 45	2,130 1.10 5,898 0.50	C 2,23 B 4,31	0 2,8	72 1,946	1.25 0.67	В
3568 972005 972005 State 70 10.82 SR 91 (FLORIDA TURNPIKE) 3569 29 County 30 0.84 STEVES ROAD	US 27/SR 25/SR 19 INTERCHANGE US 27	ORANGE COUNTY LINE CITRUS TOWER BOULEVARD	2 2	URBAN FREEWAY URBAN UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY CITY OF CLERMONT	C 66,400 D 14,060	66,200 7,625	1.00 C 0.54 D	3,100 710	3,551 335	2,407	0.62	0 1.00%	14,060 8	9,577 0.54 ,418 0.60	B 6,03	37	0 487	0.62	D
3570 429 County 20 1.46 SUNNYSIDE DRIVE 3580 423 117012 County 35 3.31 SUNNYSIDE DRIVE 3590 414 117013 County 35 1.14 SUNNYSIDE DRIVE	MAIN STREET/DR NICHOLS DRIVE SLEEPY HOLLOW ROAD BRIDGEWATER COURT	SLEEPY HOLLOW ROAD BRIDGEWATER COURT SUNNYSIDE DRIVE	2 2	URBAN UNDIVIDED URBAN UNDIVIDED URBAN UNDIVIDED	COUNTY COUNTY	CITY OF LEESBURG  CITY OF LEESBURG  UNINCORPORATED LAKE COUNTY	D 14,060 D 21,780 D 10,360	4,411 2,640 1,523	0.31 C 0.12 B 0.15 C	710 1,080 530	163 182 53	254 98 70	0.17	2.50% 3 2.00% C 1.00%	21,780 2	,990 0.35 ,915 0.13 .601 0.15	C 710 B 1,08	0 20	1 108	0.41 0.19 0.14	B C
3800 466 County 35 0.79 THOMAS AVENUE 3610 457 County 35 1.07 THOMAS AVENUE	CR 460 GRIFFIN ROAD (CR 44A)	CR 44A MAIN STREET	2 2	URBAN UNDIVIDED	COUNTY CITY OF LEESBURG	CITY OF LEESBURG CITY OF LEESBURG	D 10,360 D 10,360	9,755 7,696	0.94 D 0.74 D	530 530	405 393	529 340	1.00	D 1.00%	10,360 10	0,253 0.99 ,089 0.78	D 530	) 42	5 556	1.05	E D
3620         211         County         30         0.32         TURKEY FARM ROAD           3630         0         NO COUNT         35         4.19         TUSCANOOGA ROAD	OLD HWY 50 SUMTER COUNTY LINE	BRIMMING LAKE ROAD EGG ROAD	2 2 2	URBAN UNDIVIDED RURAL UNDIVIDED	COUNTY	CITY OF MINNEOLA UNINCORPORATED LAKE COUNTY	D 10,360 C 7,740	209	0.02 C	530 410	- 11	13	0.02	1.00% - N/A	10,360 :	220 0.02	C 530	) 1	13	0.02	C -
3640         216         County         40         0.54         TUSCANOOGA ROAD           3650         219         County         40         0.31         UNDERPASS ROAD	EGG ROAD CR 33	SR 50 AMERICAN LEGION ROAD	2 2	URBAN UNDIVIDED URBAN UNDIVIDED	COUNTY	CITY OF MASCOTTE CITY OF MASCOTTE	C 15,960 D 16,820	2,543 1,080	0.16 C 0.06 C	790 840	157 61	101 60	0.20	2.00%	16,820 1	,807 0.18 ,193 0.07	C 790	6	67	0.22	C
3680 110470 110470 State 55 1.01 US 192 3670 538 County 45 1.11 US 27/US441 3680 111012 111012 State 45 1.12 US 27/US441	US 27 SUMTER COUNTY LINE GRIFFIN AVENUE	ORANGE COUNTY LINE GRIFFIN AVENUE ALT US 441 / ALT US 27	6 6	URBAN DIVIDED URBAN DIVIDED URBAN DIVIDED	STATE STATE STATE	UNINCORPORATED LAKE COUNTY TOWN OF LADY LAKE TOWN OF LADY LAKE	D 62,900 D 59,900 D 41,790	47,750 35,295 30,300	0.76 C 0.59 C 0.73 C	3,170 3,020 2,100	2,312 1,446 1,467	1,985 1,484 1,260	0.49	1.00% 1.00% 1.50%	59,900 37	7,095 0.62 2,642 0.39	C 3,17 C 3,02 C 4,24	0 1,5	1,560	0.77 0.52 0.37	C
3680 111012 111012 State 45 1.12 US 27US441  3690 111012 111012 ADACENT 40 0.79 US 27US441  3700 111021 111021 State 55 2.27 US 27US441	ALT US 441 / ALT US 27 CR 466	CR 466 LAKE ELLA ROAD	4 6	URBAN DIVIDED URBAN DIVIDED	STATE STATE	TOWN OF LADY LAKE TOWN OF LADY LAKE	D 41,790 D 41,790	30,300	0.73 C 0.73 C 0.71 C	2,100 2,100 2,100	1,467	1,260	0.70	1.50% 1.50% 1.50%	62,900 32	2,642 0.52 1.320 0.50	C 4,24 C 3,17 C 3.17	0 1,5	30 1,357	0.50	c
3710 110430 110430 State 55 1.89 US 27/US441 3720 110431 110431 State 45 1.35 US 27/US441	LAKE ELLA ROAD CR 466A / MILLER BOULEVARD	CR 466A / MILLER BOULEVARD CR 460 (MARTIN LUTHER KING BLVD)	6 6	URBAN DIVIDED URBAN DIVIDED	STATE STATE	FRUITLAND PARK FRUITLAND PARK	D 59,900 D 59,900	29,350 37,800	0.49 C 0.63 C	3,020 3,020	1,421	1,220 1,572		1.00% C 1.00%		0,847 0.51 9,728 0.66	C 3,02			0.49	
3730 110109 110109 ADJACENT 45 0.51 US 27/US441 3740 110109 110109 State 45 0.67 US 27/US441	CR 460 (MARTIN LUTHER KING BLVD) CR 466A (LEE ROAD)	CR 466A (LEE ROAD) CR 44A/ GRIFFIN ROAD		URBAN DIVIDED URBAN DIVIDED	STATE STATE	CITY OF LEESBURG CITY OF LEESBURG	D 59,900 D 59,900	41,600 41,600	0.69 C 0.69 C	3,020 3,020	2,014 2,014	1,730 1,730		1.00% 1.00%	59,900 43	3,722 0.73 3,722 0.73	C 3,02		,		
3750 110109 110109 ADJACENT 35 0.15 US 27/US441 3760 115120 115120 State 35 1.04 US 27/US 25	CR 44A/ GRIFFIN ROAD US 27/US441 SPLIT	US 27/US441 SPLIT MAIN STREET	4 4	URBAN DIVIDED URBAN DIVIDED	STATE STATE	CITY OF LEESBURG	D 50,000 D 32,400	28,300	0.83 D 0.87 D	2,520 1,630	2,014 1,370	1,730 1,177	0.84	D 1.00%	32,400 29	3,722 0.87 9,744 0.92	D 2,52 D 1,63	0 1,4	1,237	0.88	D
3770 115119 115119 State 35 0.57 US.27/RR.25 3780 115116 115116 State 35 0.63 US.27/RR.25 3785 110014 110014 State 55 2.16 US.27/SR.25	MAIN STREET  SR 44  CR 25A (NORTH)	SR 44 CR 25A (NORTH) CR 33	4 4 4 4 4	URBAN DIVIDED URBAN DIVIDED URBAN DIVIDED	STATE STATE STATE	CITY OF LEESBURG CITY OF LEESBURG CITY OF LEESBURG	D 32,400 D 32,400 D 41,790	29,100 44,350 35,700	0.90 D 1.37 F 0.85 C	1,630 1,630 2,100	1,409 2,147 1,729	1,210 1,844 1,484	1.32	D 2.00% F 1.50% C 1.00%	32,400 47	2,129 0.99 7,778 1.47 7,521 0.90	D 1,63 F 1,63 C 2.10	0 2,3	1,987	0.95 1.42 0.87	
3765 110014 110014 State 35 2.16 US 27/SR 25 3780 110014 110014 ADACENT 55 1.12 US 27/SR 25 3800 110362 110362 State 55 2.54 US 27/SR 25	CR 33 CR 48	CR 48 PLANTATION BOULEVARD		URBAN DIVIDED URBAN DIVIDED	STATE STATE	UNINCORPORATED LAKE COUNTY UNINCORPORATED LAKE COUNTY	D 41,790 D 41,790 D 66,200	35,700 35,700 29,750	0.85 C 0.85 B	2,100 2,100 3,280	1,729	1,484		C 1.00% B 1.75%	41,790 37	7,521 0.90 7,521 0.90 2,446 0.49	C 2,10	0 1,8	17 1,560	0.87	C
3810 110362 110362 ADJACENT 55 2.67 US 27/SR 25 3820 240 110382 County 55 4.08 US 27/SR 25	PLANTATION BOULERVARD FLORIDA TURNPIKE	FLORIDA TURNPIKE SR 19	4 4	URBAN DIVIDED URBAN DIVIDED	STATE STATE	UNINCORPORATED LAKE COUNTY  CITY OF GROVELAND	D 66,200 D 41,790	29,750 26,086	0.45 B 0.62 C	3,280 2,100	1,222	1,260	0.38	B 1.75% C 1.00%	66,200 32	2,446 0.49 7,417 0.66	B 3,28 C 2,10	0 1,3	33 1,374	0.42	
3830         110363         110363         State         55         3.36         US 27/SR 25           3840         110468         110468         State         55         2.14         US 27/SR 25	SR 19 CR 561	CR 561 CR 561A	4 4 4	URBAN DIVIDED URBAN DIVIDED	STATE STATE	CITY OF GROVELAND CITY OF MINNEOLA		21,120 32,150	0.32 B 0.77 C	3,280 2,100	925 1,380	963 1,563		3 1.50% C 1.00%		2,752 0.34 3,790 0.81	B 3,28			0.32 0.78	
3850 110163 110163 State 50 0.38 US 27/SR 25 3860 110163 110163 ADJACENT 50 0.88 US 27/SR 25	CR 561A CR 561/ MAIN AVENUE	CR 561/ MAIN AVENUE CR 50		URBAN DIVIDED URBAN DIVIDED	STATE STATE	CITY OF MINNEOLA CITY OF MINNEOLA	D 62,900 D 59,900	41,100 41,100	0.65 C 0.69 C	3,170 3,020	1,990 1,990	1,709 1,709		C 1.00% C 1.00%	59,900 43	3,197 0.69 3,197 0.72	C 3,17 C 3,02	0 2,0	32 1,796		C C
3870 110423 110423 State 50 0.79 US 27/SR 25 3880 115047 115047 State 50 1.22 US 27/SR 25 2890 14100/13 14100/13 Chale 56 1.64 US 27/SR 25	CR 50 GRAND HIGHWAY	GRAND HIGHWAY  SR 50  JOHNS LAKE ROAD	6 6	URBAN DIVIDED URBAN DIVIDED	STATE STATE	CITY OF MINNEOLA CITY OF CLERMONT	D 59,900 D 62,900	29,000 31,500	0.48 C 0.50 C	3,020 3,170	1,084 1,322	1,040 1,455	0.36	C 1.00% C 4.00%	62,900 38	0,479 0.51 3,325 0.61	C 3,02 C 3,17	0 1,6	08 1,770	0.38 0.56	C
3890 110012 110012 State 55 1.54 US.27/SR.25 3900 110011 110011 State 55 2.06 US.27/SR.25 3910 110311 110311 State 55 0.95 US.27/SR.25	SR 50  JOHNS LAKE ROAD  HARDWOOD MARSH ROAD	JOHNS LAKE ROAD  HARDWOOD MARSH ROAD  LAKE LOUISA ROAD	6 6	URBAN DIVIDED URBAN DIVIDED URBAN DIVIDED	STATE STATE STATE	CITY OF CLERMONT  UNINCORPORATED LAKE COUNTY  UNINCORPORATED LAKE COUNTY	D 62,900 D 62,900 D 62,900	31,740 36,900 24,200	0.50 C 0.59 C 0.38 C	3,170 3,170 3,170	1,537 1,787 1,247	1,320 1,534 1,378	0.56	C 1.00% C 1.00% C 1.00%	62,900 38	3,359 0.53 3,782 0.62 5,434 0.40	C 3,17 C 3,17 C 3,17	0 1,8	78 1,612		
3910 110311 110311 State 55 0.55 0.57/SR 25 3920 110007 110007 State 65 6.51 US 27/SR 25 3927 110007 110007 ADJACENT 65 2.01 US 27/SR 25	LAKE LOUISA ROAD  BOGGY MARSH RD	BOGGY MARSH RD CR 474	6 6	RURAL DIVIDED URBAN DIVIDED	STATE STATE	UNINCORPORATED LAKE COUNTY  UNINCORPORATED LAKE COUNTY  UNINCORPORATED LAKE COUNTY	D 62,900 D 48,090 D 62,900	21,400 21,400	0.38 C 0.44 C 0.34 C	3,170 2,520 3,170	1,094 1,094	1,378 939 939	0.43	C 1.00%	48,090 22	2,492 0.47 2,492 0.36	C 3,17 C 2,52 C 3,17	0 1,1	50 987	0.46 0.46	С
3930 1 County 55 1.72 US 27/SR 25 3940 115096 115096 State 35 0.75 US 441/ SR 500	CR 474 US 27/US441 SPLIT	US 192 LEE STREET	6 6	URBAN DIVIDED URBAN DIVIDED	STATE STATE	UNINCORPORATED LAKE COUNTY CITY OF LEESBURG	D 62,900 D 34,020	55,383 29,150	0.88 C 0.86 D	3,170 1,710	1,945	1,878	0.61	1.00% D 1.00%	62,900 58	3,208 0.93 0,637 0.90	C 3,17 D 1,71	0 2,0	1,974	0.65	С
3950 110492 110492 State 35 0.42 US 441/ SR 500 3960 115093 115093 State 45 1.06 US 441/ SR 500	LEE STREET N CANAL STREET	N CANAL STREET E DIXIE AVENUE	4 4	URBAN DIVIDED URBAN DIVIDED	STATE STATE	CITY OF LEESBURG CITY OF LEESBURG	D 32,400 D 41,790	31,850 33,850	0.98 D 0.81 C	1,630 2,100	1,542 1,386	1,324 1,158	0.66	0 1.00% C 1.00%	41,790 35	3,475 1.03 5,577 0.85	E 1,63 C 2,10	0 1,4	57 1,217	0.69	С
3970 115092 115092 State 45 0.25 US 441/ SR 500 3980 110177 110177 State 45 1.41 US 441/ SR 500	E DIXIE AVENUE  E MAIN STREET	E MAIN STREET  CR 44	6 6	URBAN DIVIDED URBAN DIVIDED	STATE STATE	CITY OF LEESBURG CITY OF LEESBURG	D 59,900 D 59,900	34,100	0.74 C 0.57 C	3,020 3,020	2,157 1,654	1,852 1,415	0.55	1.00% 1.00%	59,900 35	5,822 0.78 5,839 0.60	C 3,02	0 1,7	38 1,487	0.58	С
3990 110177 110177 ADJACENT 45 3.07 US 441/ SR 500	CR 44	RADIO ROAD	6 6	URBAN DIVIDED	STATE	CITY OF LEESBURG	D 62,900	34,100	0.54 C	3,170	1,654	1,415	0.52	1.00%	0∠,900 35	5,839 0.57	C 3,17	u 1,7	38 1,487	0.55	



## C3C & C3R

## Motor Vehicle Arterial Generalized Service Volume Tables

#### **Peak Hour Directional**

	В	С	D	Е
1 Lane	*	760	1,070	**
2 Lane	*	1,520	1,810	**
3 Lane	*	2,360	2,680	**
4 Lane	*	3,170	3,180	**

#### **Peak Hour Two-Way**

В	С	D	Е
*	1,380	1,950	**
*	2,760	3,290	**
*	4,290	4,870	**
*	5,760	5,780	**
	* *	* 1,380 * 2,760 * 4,290	* 1,380 1,950 * 2,760 3,290 * 4,290 4,870

#### **AADT**

	В	С	D	Е
2 Lane	*	15,300	21,700	**
4 Lane	*	30,700	36,600	**
6 Lane	*	47,700	54,100	**
8 Lane	*	64,000	64,200	**



(C3C-Suburban Commercial)

(C3R-Suburban Residential)

	В	С	D	Е
1 Lane	*	970	1,110	**
2 Lane	*	1,700	1,850	**
3 Lane	*	2,620	2,730	**

	В	С	D	Е
2 Lane	*	1,760	2,020	**
4 Lane	*	3,090	3,360	**
6 Lane	*	4,760	4,960	**

	В	С	D	E
2 Lane	*	19,600	22,400	**
4 Lane	*	34,300	37,300	**
6 Lane	*	52,900	55,100	**

#### **Adjustment Factors**

The peak hour directional service volumes should be adjust by multiplying by 1.2 for one-way facilities. The AADT service volumes should be adjusted by multiplying 0.6 for one way facilities 2 Lane Divided Roadway with an Exclusive Left Turn Lane(s): Multiply by 1.05

2 lane Undivided Roadway with No Exclusive Left Turn Lane(s): Multiply by 0.80

Exclusive right turn lane(s): Multiply by 1.05
Multilane Undivided Roadway with an Exclusive Left Turn Lane(s): Multiply by 0.95
Multilane Roadway with No Exclusive Left Turn Lane(s): Multiply by 0.75
Non-State Signalized Roadway: Multiply by 0.90

This table does not constitute a standard and should be used only for general planning applications. The table should not be used for corridor or intersection design, where more refined techniques exist.

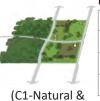
\* Cannot be achieved using table input value defaults.

<sup>\*\*</sup> Not applicable for that level of service letter grade. For the automobile mode, volumes greater than level of service D become F because intersection capacities have been reached.



## C1 & C2

## Motor Vehicle Highway Generalized Service Volume Tables



C2-Rural)

# Peak Hour Directional B C D E 1 Lane 240 430 730 1,490 2 Lane 1,670 2,390 2,910 3,340

3,570

2,510

Peak Hour	rwo-wa	У		
	В	С	D	E
2 Lane	440	780	1,330	2,710
4 Lane	3,040	4,350	5,290	6,070
6 Lane	4 560	6.490	7 950	9 110

Doole House True Man

AADT				
	В	С	D	Е
2 Lane	4,600	8,200	14,000	28,500
4 Lane	32,000	45,800	55,700	63,900
6 Lane	48,000	68,300	83,700	95,900

#### **Adjustment Factors**

3 Lane

2 Lane Divided Roadway with Exclusive Left Turn Adjustment: Multiply by 1.05 Multilane Undivided Highway with Exclusive Left Turn Adjustment: Multiply by 0.95 Multilane Undivided Highway without Exclusive Left Turn Adjustment:: Multiply by 0.75

4,370

5,010

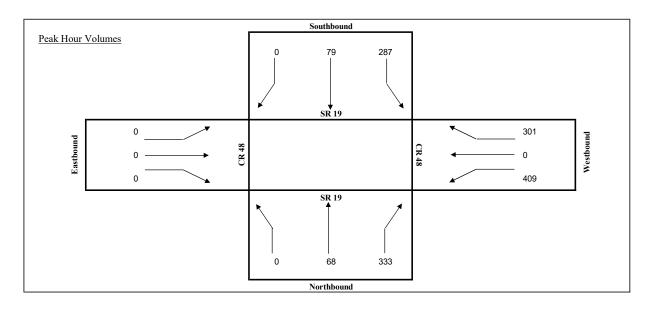
Appendix D
Turning Movement Counts and Seasonal Factor Data

## TURNING MOVEMENT COUNT ANALYSIS AUTOS & TRUCKS

Intersection (N/S): SR 19 Intersection (E/W): CR 48

Date: 7/19/2023

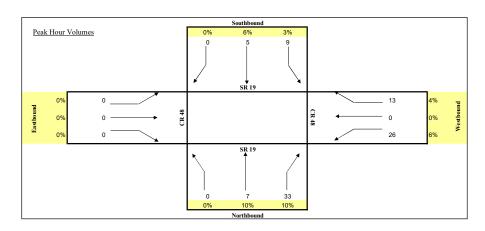
				SR 19			SR 19			CR 48			CR 48		
				NB			SB			EB			WB		
	Start	End	L	T	R	L	T	R	L	T	R	L	T	R	TOTAL
	4:00 PM	4:15 PM	0	19	82	68	13	0	0	0	0	84	0	65	331
	4:15 PM	4:30 PM	0	24	91	71	13	0	0	0	0	83	0	79	361
	4:30 PM	4:45 PM	0	18	72	68	17	0	0	0	0	93	0	76	344
	4:45 PM	5:00 PM	0	23	90	85	15	0	0	0	0	92	0	61	366
	5:00 PM	5:15 PM	0	18	71	73	23	0	0	0	0	88	0	73	346
	5:15 PM	5:30 PM	0	15	80	71	19	0	0	0	0	114	0	80	379
	5:30 PM	5:45 PM	0	12	92	58	22	0	0	0	0	115	0	87	386
	5:45 PM	6:00 PM	0	16	70	54	14	0	0	0	0	94	0	72	320
Total for:	4:00 PM	5:00 PM	0	84	335	292	58	0	0	0	0	352	0	281	1402
Total for:	5:00 PM	6:00 PM	0	61	313	256	78	0	0	0	0	411	0	312	1431
Tota Peak Hour:	4:45 PM	5:45 PM	0	68	333	287	79	0	0	0	0	409	0	301	1477
Overall PHF:	0.96														



## TURNING MOVEMENT COUNT ANALYSIS TRUCKS

Intersection (N/S): SR 19
Intersection (E/W): CR 48
Date: 7/19/2023

				SR 19			SR 19			CR 48			CR 48		
				NB			SB			EB			WB		
	Start	End	R	T	L	R	T	L	R	T	L	R	T	L	TOTAL
	4:00 PM	4:15 PM	0	3	10	5	0	0	0	0	0	4	0	6	28
	4:15 PM	4:30 PM	0	4	11	1	3	0	0	0	0	8	0	2	29
	4:30 PM	4:45 PM	0	0	8	2	1	0	0	0	0	7	0	4	22
	4:45 PM	5:00 PM	0	0	4	1	1	0	0	0	0	7	0	1	14
	5:00 PM	5:15 PM	0	1	7	2	2	0	0	0	0	6	0	0	18
	5:15 PM	5:30 PM	0	0	7	2	0	0	0	0	0	6	0	0	15
	5:30 PM	5:45 PM	0	0	2	0	0	0	0	0	0	2	0	1	5
	5:45 PM	6:00 PM	0	2	4	2	1	0	0	0	0	5	0	1	15
_															
Total for:	4:00 PM	5:00 PM	0	7	33	9	5	0	0	0	0	26	0	13	93
Total for:	5:00 PM	6:00 PM	0	3	20	6	3	0	0	0	0	19	0	2	53
Tota Peak Hour:	4:00 PM	5:00 PM	0	7	33	9	5	0	0	0	0	26	0	13	93
Overall PHF:	0.80														
O'CIANTINI.	0.00														

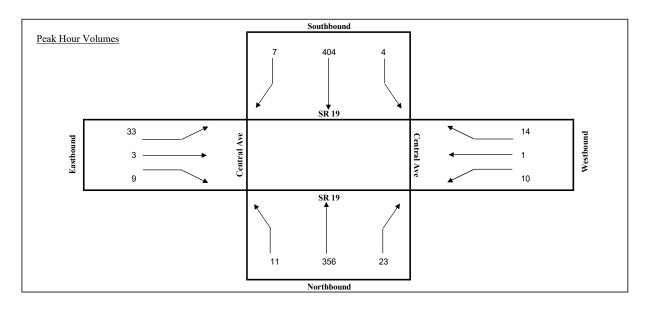


## TURNING MOVEMENT COUNT ANALYSIS AUTOS & TRUCKS

Intersection (N/S): SR 19 Intersection (E/W): Central Ave

Date: 7/19/2023

Date.	1/15/2025			SR 19			SR 19			Central Ave			Central Ave		
Γ				NB			SB			EB			WB		I
	Start	End	L	T	R	L	T	R	L	T	R	L	T	R	TOTAL
Γ	7:00 AM	7:15 AM	7	76	6	1	88	3	5	0	4	3	1	3	197
	7:15 AM	7:30 AM	3	92	4	1	101	0	15	1	1	1	0	2	221
	7:30 AM	7:45 AM	1	96	4	1	106	2	9	0	1	2	0	4	226
	7:45 AM	8:00 AM	5	85	4	2	93	2	4	1	4	4	0	3	207
	8:00 AM	8:15 AM	2	83	11	0	104	3	5	1	3	3	1	5	221
	8:15 AM	8:30 AM	8	70	1	1	91	5	7	2	0	0	0	4	189
	8:30 AM	8:45 AM	3	96	5	1	101	5	5	2	6	2	0	1	227
	8:45 AM	9:00 AM	3	77	10	4	68	2	13	0	1	2	0	4	184
Total for:	7:00 AM	8:00 AM	16	349	18	5	388	7	33	2	10	10	1	12	851
Total for:	8:00 AM	9:00 AM	16	326	27	6	364	15	30	5	10	7	1	14	821
Tota Peak Hour:	7:15 AM	8:15 AM	11	356	23	4	404	7	33	3	9	10	1	14	875
Overall PHF:	0.97														



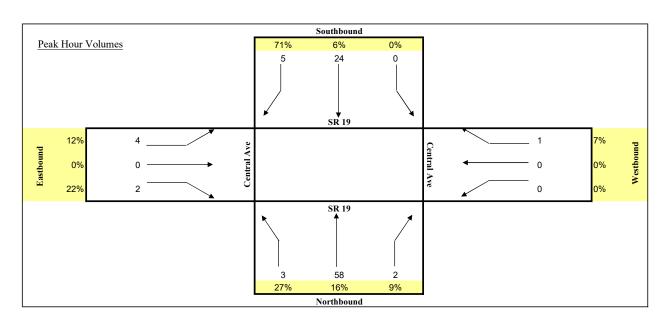
## TURNING MOVEMENT COUNT ANALYSIS TRUCKS

Intersection (N/S): SR 19 Intersection (E/W): Central Ave

Date: 7/19/2023

			SR 19			SR 19			Central Ave			Central Ave		
			NB			SB			EB			WB		
Start	End	R	T	L	R	T	L	R	T	L	R	T	L	TOTAL
7:00 AM	7:15 AM	1	13	0	0	10	0	1	0	0	0	0	0	25
7:15 AM	7:30 AM	1	15	1	1	13	0	1	0	0	0	0	0	32
7:30 AM	7:45 AM	0	9	0	0	7	0	0	0	0	0	0	2	18
7:45 AM	8:00 AM	1	12	1	0	2	0	0	0	0	1	0	0	17
8:00 AM	8:15 AM	0	14	1	0	5	0	0	0	0	0	0	1	21
8:15 AM	8:30 AM	2	7	1	0	8	1	2	0	0	0	0	0	21
8:30 AM	8:45 AM	1	19	0	0	6	2	0	0	2	0	0	0	30
8:45 AM	9:00 AM	0	18	0	0	5	2	2	0	0	0	0	0	27

												1			
Total for:	7:00 AM	8:00 AM	3	49	2	1	32	0	2	0	0	1	0	2	92
Total for:	8:00 AM	9:00 AM	3	58	2	0	24	5	4	0	2	0	0	1	99
Tota Peak Hour:	8:00 AM	9:00 AM	3	58	2	0	24	5	4	0	2	0	0	1	99
Overall PHF:	0.83														



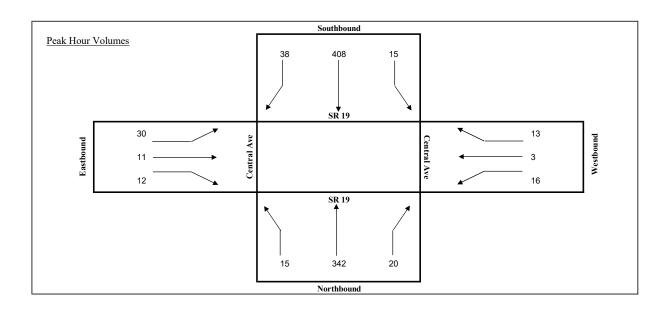
## TURNING MOVEMENT COUNT ANALYSIS AUTOS & TRUCKS

Intersection (N/S): SR 19
Intersection (E/W): Central Ave
Date: 7/19/2023

Overall PHF:

0.92

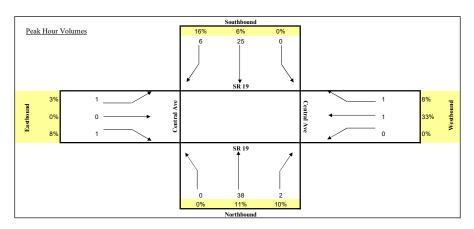
_				SR 19			SR 19			Central Ave			Central Ave		
				NB			SB			EB			WB		
	Start	End	L	T	R	L	T	R	L	T	R	L	T	R	TOTAL
Γ	4:00 PM	4:15 PM	2	88	5	3	81	10	7	1	2	1	0	4	204
	4:15 PM	4:30 PM	2	98	3	1	79	9	12	0	4	1	3	3	215
	4:30 PM	4:45 PM	2	75	7	6	89	10	11	3	4	4	1	1	213
	4:45 PM	5:00 PM	2	102	7	4	90	6	6	1	3	1	0	2	224
	5:00 PM	5:15 PM	5	66	5	0	96	10	12	5	5	5	0	6	215
	5:15 PM	5:30 PM	4	84	4	3	113	8	5	1	1	6	3	2	234
	5:30 PM	5:45 PM	4	90	4	8	109	14	7	4	3	4	0	3	250
	5:45 PM	6:00 PM	1	71	6	1	86	9	7	1	1	0	2	3	188
Total for:	4:00 PM	5:00 PM	8	363	22	14	339	35	36	5	13	7	4	10	856
Total for:	5:00 PM	6:00 PM	14	311	19	12	404	41	31	11	10	15	5	14	887
Tota Peak Hour:	4:45 PM	5:45 PM	15	342	20	15	408	38	30	11	12	16	3	13	923



## TURNING MOVEMENT COUNT ANALYSIS TRUCKS

Intersection (N/S): SR 19
Intersection (E/W): Central Ave
Date: 7/19/2023

_				SR 19			SR 19			Central Ave			Central Ave		
				NB			SB			EB			WB		
	Start	End	R	T	L	R	T	L	R	T	L	R	T	L	TOTAL
	4:00 PM	4:15 PM	0	13	2	0	2	2	0	0	0	0	0	0	19
	4:15 PM	4:30 PM	0	14	0	0	9	2	0	0	0	0	1	1	27
	4:30 PM	4:45 PM	0	8	0	0	8	0	0	0	0	0	0	0	16
	4:45 PM	5:00 PM	0	3	0	0	6	2	1	0	1	0	0	0	13
	5:00 PM	5:15 PM	1	7	0	0	8	0	1	0	0	0	0	0	17
	5:15 PM	5:30 PM	0	7	0	0	6	0	0	0	1	0	0	0	14
	5:30 PM	5:45 PM	1	2	0	1	0	1	0	0	1	1	0	0	7
	5:45 PM	6:00 PM	0	6	0	0	6	0	0	0	0	0	1	0	13
		-													
Total for:	4:00 PM	5:00 PM	0	38	2	0	25	6	1	0	1	0	1	1	75
Total for:	5:00 PM	6:00 PM	2	22	0	1	20	1	1	0	2	1	1	0	51
Tota Peak Hour:	4:00 PM	5:00 PM	0	38	2	0	25	6	1	0	1	0	1	1	75
Overall PHF:	0.69														

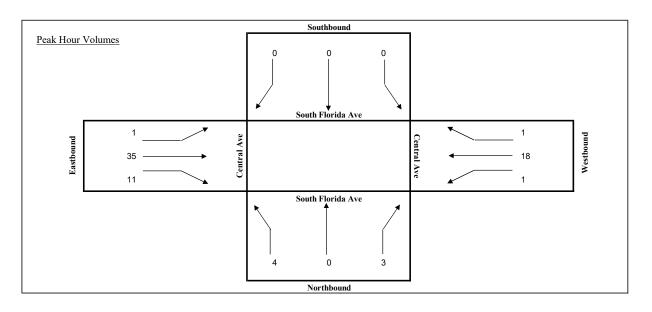


## TURNING MOVEMENT COUNT ANALYSIS AUTOS & TRUCKS

Intersection (N/S): South Florida Ave Intersection (E/W): Central Ave

Date:	7/19/2023
Date:	1/19/2023

			S	outh Florida A	ve	S	outh Florida A	ve		Central Ave			Central Ave		
				NB			SB			EB			WB		
	Start	End	L	T	R	L	T	R	L	T	R	L	T	R	TOTAL
	7:00 AM	7:15 AM	0	0	0	0	0	0	0	6	4	0	8	1	19
	7:15 AM	7:30 AM	2	0	1	0	0	0	1	13	2	0	4	0	23
	7:30 AM	7:45 AM	2	0	1	0	0	0	0	9	4	1	1	0	18
	7:45 AM	8:00 AM	0	0	1	0	0	0	0	7	1	0	5	0	14
	8:00 AM	8:15 AM	0	0	2	0	0	0	0	5	0	2	5	0	14
	8:15 AM	8:30 AM	0	0	3	0	0	0	0	8	2	1	3	2	19
	8:30 AM	8:45 AM	0	0	1	1	0	1	0	3	1	3	7	0	17
	8:45 AM	9:00 AM	1	0	2	0	0	0	0	7	2	1	6	1	20
Total for:	7:00 AM	8:00 AM	4	0	3	0	0	0	1	35	11	1	18	1	74
Total for:	8:00 AM	9:00 AM	1	0	8	1	0	1	0	23	5	7	21	3	70
Tota Peak Hour:	7:00 AM	8:00 AM	4	0	3	0	0	0	1	35	11	1	18	1	74
Overall PHF:	0.80														



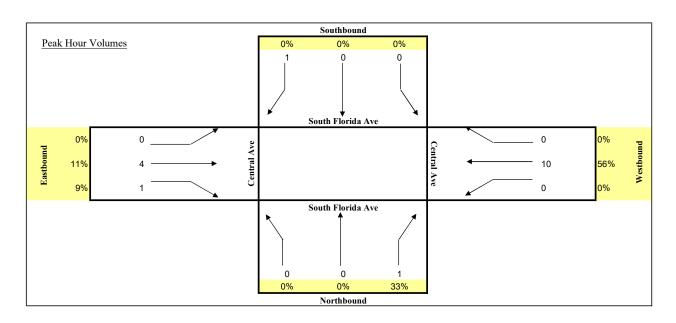
## TURNING MOVEMENT COUNT ANALYSIS TRUCKS

Intersection (N/S): South Florida Ave Intersection (E/W): Central Ave

Date: 7/19/2023

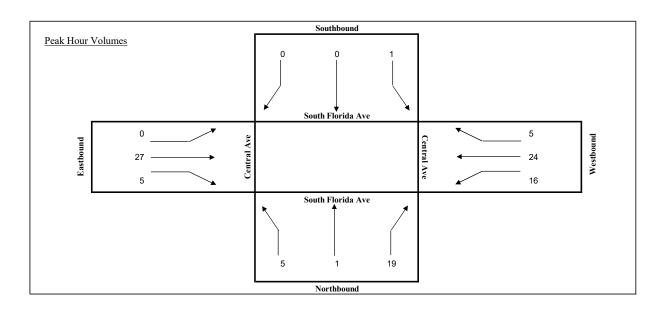
		S	outh Florida A	ve	S	outh Florida A	ve		Central Ave			Central Ave		
			NB			SB			EB			WB		T
Start	End	R	T	L	R	T	L	R	T	L	R	T	L	TOTAL
7:00 AM	7:15 AM	0	0	0	0	0	0	0	1	0	0	1	0	2
7:15 AM	7:30 AM	0	0	0	0	0	0	0	1	0	0	1	0	2
7:30 AM	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	8:00 AM	0	0	0	0	0	0	0	0	0	0	1	0	1
8:00 AM	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	8:30 AM	0	0	1	0	0	0	0	2	0	0	2	0	5
8:30 AM	8:45 AM	0	0	0	0	0	1	0	1	0	0	3	0	5
8:45 AM	9:00 AM	0	0	0	0	0	0	0	1	1	0	5	0	7

Total for:	7:00 AM	8:00 AM	0	0	0	0	0	0	0	2	0	0	3	0	5
Total for:	8:00 AM	9:00 AM	0	0	1	0	0	1	0	4	1	0	10	0	17
Tota Peak Hour:	8:00 AM	9:00 AM	0	0	1	0	0	1	0	4	1	0	10	0	17
Overall PHF:	0.61														



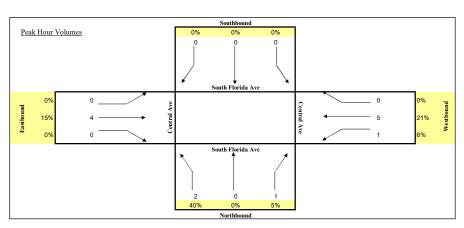
Intersection (N/S): South Florida Ave Intersection (E/W): Central Ave Date: 7/19/2023

			S	outh Florida Av	ve	S	outh Florida A	ve		Central Ave			Central Ave		
				NB			SB			EB			WB		
	Start	End	L	T	R	L	T	R	L	T	R	L	T	R	TOTAL
Γ	4:00 PM	4:15 PM	3	0	3	0	0	0	0	3	0	4	5	0	18
	4:15 PM	4:30 PM	3	0	5	0	0	0	0	6	2	4	8	0	28
	4:30 PM	4:45 PM	2	0	6	0	0	0	0	2	3	3	7	0	23
	4:45 PM	5:00 PM	1	0	4	0	0	0	0	5	1	1	4	0	16
	5:00 PM	5:15 PM	1	1	7	0	0	0	0	10	2	5	6	0	32
	5:15 PM	5:30 PM	1	0	4	0	0	0	0	5	1	0	4	4	19
	5:30 PM	5:45 PM	1	0	4	1	0	0	0	6	2	5	9	0	28
	5:45 PM	6:00 PM	2	0	4	0	0	0	0	6	0	6	5	1	24
_															
Total for:	4:00 PM	5:00 PM	9	0	18	0	0	0	0	16	6	12	24	0	85
Total for:	5:00 PM	6:00 PM	5	1	19	1	0	0	0	27	5	16	24	5	103
Tota Peak Hour:	5:00 PM	6:00 PM	5	1	19	1	0	0	0	27	5	16	24	5	103
Overall PHF:	0.80														



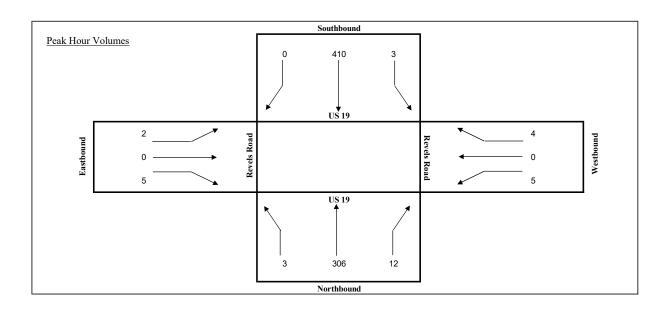
Intersection (N/S): South Florida Ave Intersection (E/W): Central Ave Date: 7/19/2023

_			s	outh Florida Av	ve	s	outh Florida A	ve		Central Ave			Central Ave		
				NB			SB			EB			WB		
	Start	End	R	T	L	R	T	L	R	T	L	R	T	L	TOTAL
	4:00 PM	4:15 PM	0	0	0	0	0	0	0	0	0	1	1	0	2
	4:15 PM	4:30 PM	1	0	0	0	0	0	0	0	0	1	2	0	4
	4:30 PM	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	5:00 PM	1	0	0	0	0	0	0	2	0	1	1	0	5
	5:00 PM	5:15 PM	0	0	1	0	0	0	0	0	0	0	1	0	2
	5:15 PM	5:30 PM	1	0	0	0	0	0	0	2	0	0	1	0	4
	5:30 PM	5:45 PM	0	0	0	0	0	0	0	0	0	0	2	0	2
	5:45 PM	6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
		-				-									
Total for:	4:00 PM	5:00 PM	2	0	0	0	0	0	0	2	0	3	4	0	11
Total for:	5:00 PM	6:00 PM	1	0	1	0	0	0	0	2	0	0	4	0	8
Tota Peak Hour:	4:45 PM	5:45 PM	2	0	1	0	0	0	0	4	0	1	5	0	13
Overall PHF:	0.65														



Intersection (N/S): US 19 Intersection (E/W): Revels Road Date: 7/19/2023

				US 19			US 19			Revels Road			Revels Road		
				NB			SB			EB			WB		
	Start	End	L	T	R	L	T	R	L	T	R	L	T	R	TOTAL
	7:00 AM	7:15 AM	3	80	1	0	74	2	0	0	0	3	0	2	165
	7:15 AM	7:30 AM	2	60	1	1	94	1	1	0	0	0	1	0	161
	7:30 AM	7:45 AM	1	72	0	1	107	0	0	0	2	1	0	1	185
	7:45 AM	8:00 AM	1	97	5	0	100	0	0	0	2	2	0	1	208
	8:00 AM	8:15 AM	0	71	2	2	110	0	2	0	0	2	0	2	191
	8:15 AM	8:30 AM	1	66	5	0	93	0	0	0	1	0	0	0	166
	8:30 AM	8:45 AM	0	58	1	0	60	1	1	0	2	4	0	2	129
	8:45 AM	9:00 AM	0	57	3	1	63	2	0	0	1	1	0	2	130
_															
Total for:	7:00 AM	8:00 AM	7	309	7	2	375	3	1	0	4	6	1	4	719
Total for:	8:00 AM	9:00 AM	1	252	11	3	326	3	3	0	4	7	0	6	616
Tota Peak Hour:	7:30 AM	8:30 AM	3	306	12	3	410	0	2	0	5	5	0	4	750
Overall PHF:	0.90														

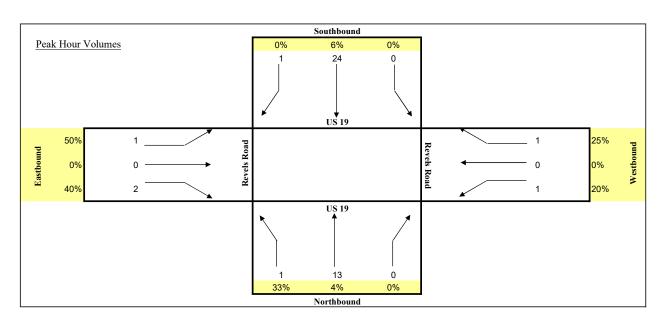


Intersection (N/S): US 19 Intersection (E/W): Revels Road

Date: 7/19/2023

			US 19			US 19			Revels Road			Revels Road		
			NB			SB			EB			WB		
Start	End	R	T	L	R	T	L	R	T	L	R	T	L	TOTAL
7:00 AM	7:15 AM	1	3	0	0	5	0	0	0	0	0	0	0	9
7:15 AM	7:30 AM	0	1	0	0	6	0	0	0	0	0	0	0	7
7:30 AM	7:45 AM	0	2	0	0	5	0	0	0	0	0	0	0	7
7:45 AM	8:00 AM	1	6	0	0	3	0	0	0	0	0	0	1	11
8:00 AM	8:15 AM	0	1	0	0	8	0	0	0	0	0	0	0	9
8:15 AM	8:30 AM	0	3	0	0	6	0	0	0	1	0	0	0	10
8:30 AM	8:45 AM	0	3	0	0	7	1	1	0	1	1	0	0	14
8:45 AM	9:00 AM	0	1	0	0	3	1	0	0	0	0	0	0	5
								_						
or: 7:00 AM	8:00 AM	2	12	0	0	19	0	0	0	0	0	0	1	34

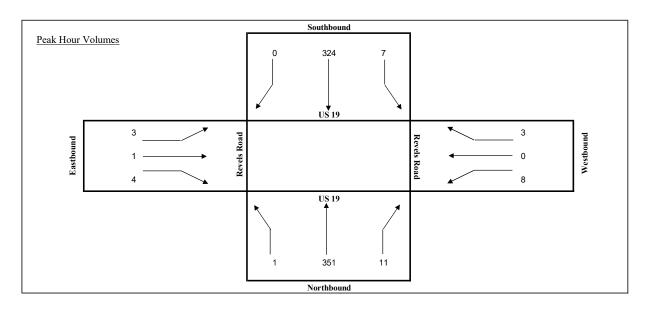
Total for:	7:00 AM	8:00 AM	2	12	0	0	19	0	0	0	0	0	0	1	34
Total for:	8:00 AM	9:00 AM	0	8	0	0	24	2	1	0	2	1	0	0	38
Tota Peak Hour:	7:45 AM	8:45 AM	1	13	0	0	24	1	1	0	2	1	0	1	44
Overall PHF:	0.79														



Intersection (N/S): US 19 Intersection (E/W): Revels Road

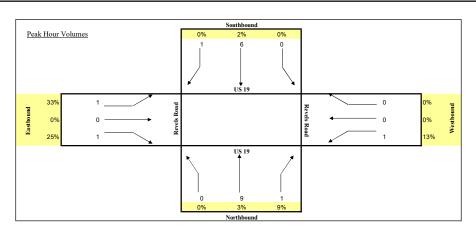
Date: 7/19/2023

_				US 19			US 19			Revels Road			Revels Road		
				NB			SB			EB			WB		
	Start	End	L	T	R	L	T	R	L	T	R	L	T	R	TOTAL
	4:00 PM	4:15 PM	2	89	6	5	61	3	1	0	0	1	1	2	171
	4:15 PM	4:30 PM	0	76	3	0	74	1	1	0	1	3	0	1	160
	4:30 PM	4:45 PM	1	78	1	2	88	0	0	0	1	2	0	0	173
	4:45 PM	5:00 PM	0	93	6	1	91	0	0	0	0	2	0	2	195
	5:00 PM	5:15 PM	0	88	3	2	70	0	1	0	2	2	0	0	168
	5:15 PM	5:30 PM	0	92	1	2	75	0	2	1	1	2	0	1	177
	5:30 PM	5:45 PM	0	92	2	1	70	0	0	0	1	0	0	0	166
	5:45 PM	6:00 PM	0	86	3	0	72	0	1	0	0	2	0	1	165
Total for:	4:00 PM	5:00 PM	3	336	16	8	314	4	2	0	2	8	1	5	699
Total for:	5:00 PM	6:00 PM	0	358	9	5	287	0	4	1	4	6	0	2	676
Tota Peak Hour:	4:30 PM	5:30 PM	1	351	11	7	324	0	3	1	4	8	0	3	713
Overall PHF:	0.91														



Intersection (N/S): US 19
Intersection (E/W): Revels Road
Date: 7/19/2023

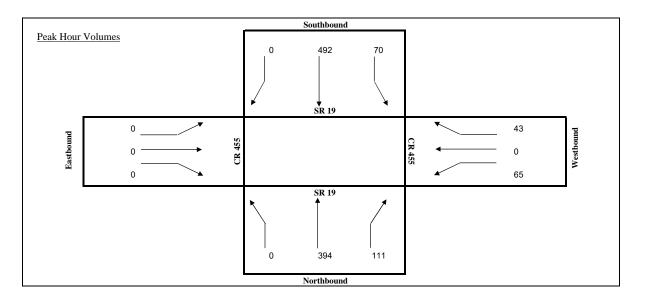
_				US 19			US 19			Revels Road			Revels Road		
				NB			SB			EB			WB		
	Start	End	R	T	L	R	T	L	R	T	L	R	T	L	TOTAL
	4:00 PM	4:15 PM	0	1	0	0	1	1	0	0	0	0	0	0	3
	4:15 PM	4:30 PM	0	4	1	0	2	0	1	0	0	0	0	0	8
	4:30 PM	4:45 PM	0	1	0	0	0	0	0	0	1	1	0	0	3
	4:45 PM	5:00 PM	0	3	0	0	3	0	0	0	0	0	0	0	6
	5:00 PM	5:15 PM	0	2	0	0	1	0	0	0	0	0	0	0	3
	5:15 PM	5:30 PM	0	1	0	0	1	0	0	0	0	0	0	0	2
	5:30 PM	5:45 PM	0	5	0	0	2	0	0	0	0	0	0	0	7
	5:45 PM	6:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	1
_						_			_						
Total for:	4:00 PM	5:00 PM	0	9	1	0	6	1	1	0	1	1	0	0	20
Total for:	5:00 PM	6:00 PM	0	9	0	0	4	0	0	0	0	0	0	0	13
Tota Peak Hour:	4:00 PM	5:00 PM	0	9	1	0	6	1	1	0	1	1	0	0	20
Overall PHF:	0.63														



Intersection (N/S): SR 19 Intersection (E/W): CR 455

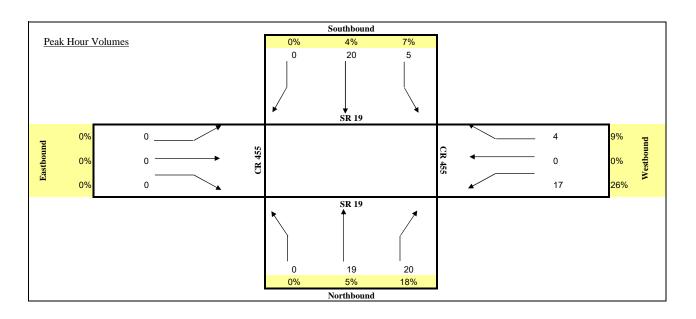
Date: 1/24/2023

_	1/2 1/2020			SR 19			SR 19			CR 455			CR 455		
				NB			SB			EB			WB		I
	Start	End	L	T	R	L	T	R	L	T	R	L	T	R	TOTAL
	7:00 AM	7:15 AM	0	92	15	11	131	0	0	0	0	7	0	4	260
	7:15 AM	7:30 AM	0	93	23	16	144	0	0	0	0	9	0	6	291
	7:30 AM	7:45 AM	0	111	27	21	105	0	0	0	0	13	0	11	288
	7:45 AM	8:00 AM	0	91	26	20	124	0	0	0	0	17	0	12	290
	8:00 AM	8:15 AM	0	99	35	13	119	0	0	0	0	26	0	14	306
	8:15 AM	8:30 AM	0	93	29	18	98	0	0	0	0	22	0	11	271
	8:30 AM	8:45 AM	0	74	27	11	94	0	0	0	0	22	0	12	240
	8:45 AM	9:00 AM	0	81	22	9	94	0	0	0	0	17	0	9	232
_															
Total for:	7:00 AM	8:00 AM	0	387	91	68	504	0	0	0	0	46	0	33	1129
Total for:	8:00 AM	9:00 AM	0	347	113	51	405	0	0	0	0	87	0	46	1049
Tota Peak Hour:	7:15 AM	8:15 AM	0	394	111	70	492	0	0	0	0	65	0	43	1175
Overall PHF:	0.96		•					•			•				



Intersection (N/S): SR 19 Intersection (E/W): CR 455 Date: 1/24/2023

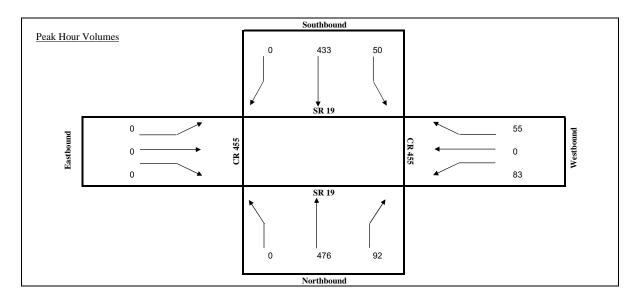
_				SR 19			SR 19			CR 455			CR 455		
				NB			SB			EB			WB		
	Start	End	R	T	L	R	T	L	R	T	L	R	T	L	TOTAL
	7:00 AM	7:15 AM	0	3	3	0	7	0	0	0	0	2	0	1	16
	7:15 AM	7:30 AM	0	6	1	1	8	0	0	0	0	2	0	0	18
	7:30 AM	7:45 AM	0	7	7	3	5	0	0	0	0	3	0	2	27
	7:45 AM	8:00 AM	0	3	2	1	3	0	0	0	0	1	0	0	10
	8:00 AM	8:15 AM	0	6	5	0	5	0	0	0	0	5	0	1	22
	8:15 AM	8:30 AM	0	3	6	3	6	0	0	0	0	3	0	2	23
	8:30 AM	8:45 AM	0	3	6	1	5	0	0	0	0	6	0	0	21
	8:45 AM	9:00 AM	0	7	3	1	4	0	0	0	0	3	0	1	19
_															
Total for:	7:00 AM	8:00 AM	0	19	13	5	23	0	0	0	0	8	0	3	71
Total for:	8:00 AM	9:00 AM	0	19	20	5	20	0	0	0	0	17	0	4	85
Tota Peak Hour:	8:00 AM	9:00 AM	0	19	20	5	20	0	0	0	0	17	0	4	85
Overall PHF:	0.92														



Intersection (N/S): SR 19 Intersection (E/W): CR 455

Date: 1/24/2023

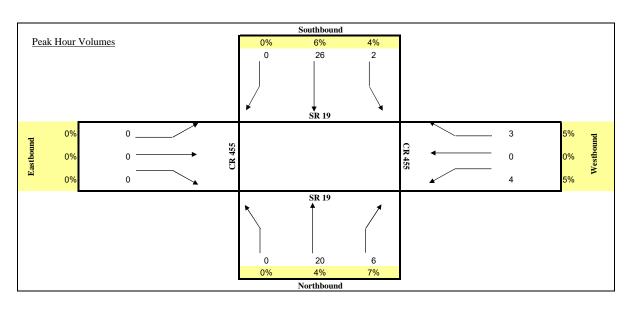
_	1/2 1/2020			SR 19			SR 19			CR 455			CR 455		
				NB			SB			EB			WB		
	Start	End	L	T	R	L	T	R	L	T	R	L	T	R	TOTAL
	4:00 PM	4:15 PM	0	97	20	6	117	0	0	0	0	18	0	14	272
	4:15 PM	4:30 PM	0	111	22	9	109	0	0	0	0	22	0	11	284
	4:30 PM	4:45 PM	0	114	25	13	108	0	0	0	0	19	0	16	295
	4:45 PM	5:00 PM	0	118	22	9	108	0	0	0	0	25	0	13	295
	5:00 PM	5:15 PM	0	131	21	14	104	0	0	0	0	18	0	10	298
	5:15 PM	5:30 PM	0	113	24	14	113	0	0	0	0	21	0	16	301
	5:30 PM	5:45 PM	0	96	28	17	94	0	0	0	0	17	0	19	271
	5:45 PM	6:00 PM	0	87	21	10	102	0	0	0	0	21	0	12	253
_															
Total for:	4:00 PM	5:00 PM	0	440	89	37	442	0	0	0	0	84	0	54	1146
Total for:	5:00 PM	6:00 PM	0	427	94	55	413	0	0	0	0	77	0	57	1123
Tota Peak Hour:	4:30 PM	5:30 PM	0	476	92	50	433	0	0	0	0	83	0	55	1189
Overall PHF:	0.99														



Intersection (N/S): SR 19 Intersection (E/W): CR 455

Date: 1/24/2023

				SR 19			SR 19			CR 455			CR 455		
				NB			SB			EB			WB		
	Start	End	R	T	L	R	T	L	R	T	L	R	T	L	TOTAL
	4:00 PM	4:15 PM	0	6	3	0	7	0	0	0	0	1	0	1	18
	4:15 PM	4:30 PM	0	5	0	1	7	0	0	0	0	1	0	1	15
	4:30 PM	4:45 PM	0	7	2	1	4	0	0	0	0	0	0	0	14
	4:45 PM	5:00 PM	0	2	1	0	8	0	0	0	0	2	0	1	14
	5:00 PM	5:15 PM	0	4	3	1	2	0	0	0	0	0	0	0	10
	5:15 PM	5:30 PM	0	3	1	0	7	0	0	0	0	1	0	0	12
	5:30 PM	5:45 PM	0	0	4	1	1	0	0	0	0	0	0	2	8
	5:45 PM	6:00 PM	0	0	1	0	5	0	0	0	0	1	0	1	8
_															
Total for:	4:00 PM	5:00 PM	0	20	6	2	26	0	0	0	0	4	0	3	61
Total for:	5:00 PM	6:00 PM	0	7	9	2	15	0	0	0	0	2	0	3	38
Tota Peak Hour:	4:00 PM	5:00 PM	0	20	6	2	26	0	0	0	0	4	0	3	61
Overall PHF:	0.85														



CATEG	ORY: 1100 LAKE COUNTYWIDE		W0GH 0 05
WEEK	DATES	SF	PSCF
	DATES  01/01/2022 - 01/01/2022 01/02/2022 - 01/08/2022 01/09/2022 - 01/15/2022 01/16/2022 - 01/29/2022 01/33/2022 - 01/29/2022 01/30/2022 - 02/05/2022 02/06/2022 - 02/12/2022 02/13/2022 - 02/19/2022 02/20/2022 - 02/19/2022 02/27/2022 - 03/05/2022 03/06/2022 - 02/19/2022 03/06/2022 - 03/19/2022 03/27/2022 - 03/12/2022 03/313/2022 - 03/12/2022 03/27/2022 - 03/19/2022 03/27/2022 - 03/19/2022 03/27/2022 - 04/02/2022 04/03/2022 - 04/02/2022 04/03/2022 - 04/02/2022 04/10/2022 - 04/16/2022 04/17/2022 - 04/30/2022 04/17/2022 - 04/30/2022 05/01/2022 - 05/14/2022 05/01/2022 - 05/14/2022 05/01/2022 - 05/14/2022 05/29/2022 - 05/14/2022 05/29/2022 - 06/14/2022 06/12/2022 - 06/11/2022 06/12/2022 - 06/11/2022 06/19/2022 - 06/11/2022 06/19/2022 - 06/11/2022 07/10/2022 - 07/09/2022 07/10/2022 - 07/09/2022 07/10/2022 - 07/09/2022 07/10/2022 - 07/16/2022 07/17/2022 - 08/13/2022 07/11/2022 - 08/13/2022 08/14/2022 - 08/27/2022 08/21/2022 - 08/27/2022 08/21/2022 - 08/27/2022 08/21/2022 - 08/27/2022 09/04/2022 - 09/10/2022 09/11/2022 - 09/10/2022 09/11/2022 - 09/17/2022		MOCF: 0.95 PSCF  1.04 1.06 1.08 1.07 1.05 1.03 1.02 1.00 0.99 0.99 0.99 0.99 1.00 1.00 1.00
32 33 34 35 36 37 38	07/31/2022 - 08/06/2022 08/07/2022 - 08/13/2022 08/14/2022 - 08/20/2022 08/21/2022 - 08/27/2022 08/28/2022 - 09/03/2022 09/04/2022 - 09/10/2022 09/11/2022 - 09/17/2022	1.05 1.04 1.04 1.05 1.06 1.07	1.11 1.09 1.09 1.11 1.12 1.13
45 46 47 48 49 50 51 52 53	10/30/2022 - 11/05/2022 11/06/2022 - 11/12/2022 11/13/2022 - 11/19/2022 11/20/2022 - 11/26/2022 11/27/2022 - 12/03/2022 12/04/2022 - 12/10/2022 12/11/2022 - 12/17/2022 12/18/2022 - 12/24/2022 12/25/2022 - 12/31/2022	0.99 1.00 1.01 1.00 1.00 0.99 0.99 1.01 1.03	1.04 1.05 1.06 1.05 1.05 1.04 1.04 1.06

\* PEAK SEASON

23-FEB-2023 09:11:22

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**Appendix E**HCM Analysis Worksheets - Existing Conditions

1	١٠	SI	R	1	g	ጴ	C	R	4	R

	•	•	<b>†</b>	~	-	Ţ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b>	7	7	<b>^</b>
Traffic Volume (veh/h)	346	229	316	455	277	98
Future Volume (veh/h)	346	229	316	455	277	98
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1752	1589	1767	1811	1737	1811
Adj Flow Rate, veh/h	357	117	326	0	286	101
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	10	21	9	6	11	6
Cap, veh/h	390	315	751	U	564	1114
Arrive On Green	0.23	0.23	0.42	0.00	0.12	0.62
	1668	1346	1767	1535	1654	1811
Sat Flow, veh/h						
Grp Volume(v), veh/h	357	117	326	0	286	101
Grp Sat Flow(s),veh/h/ln	1668	1346	1767	1535	1654	1811
Q Serve(g_s), s	18.9	6.6	11.8	0.0	8.2	2.1
Cycle Q Clear(g_c), s	18.9	6.6	11.8	0.0	8.2	2.1
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	390	315	751		564	1114
V/C Ratio(X)	0.91	0.37	0.43		0.51	0.09
Avail Cap(c_a), veh/h	417	336	751		705	1114
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	0.00	1.00	1.00
Uniform Delay (d), s/veh	33.9	29.2	18.4	0.0	11.8	7.1
Incr Delay (d2), s/veh	23.6	0.7	1.8	0.0	0.7	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	14.8	3.7	8.6	0.0	5.1	1.4
Unsig. Movement Delay, s/veh		3.1	3.0	3.0	J. 1	1.1
LnGrp Delay(d),s/veh	57.5	29.9	20.3	0.0	12.6	7.3
LnGrp LOS	57.5 E	29.9 C	20.5 C	0.0	12.0 B	7.5 A
	474	<u> </u>	326	А	ט	387
Approach Vol, veh/h				А		
Approach Delay, s/veh	50.7		20.3			11.2
Approach LOS	D		С			В
Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	17.3	45.0		28.6		62.3
Change Period (Y+Rc), s	6.5	6.4		7.3		6.4
Max Green Setting (Gmax), s	18.5	38.6		22.7		38.6
Max Q Clear Time (g_c+l1), s	10.2	13.8		20.9		4.1
Green Ext Time (p c), s	0.5	1.9		0.3		0.5
u = //	0.0	1.5		0.0		0.0
Intersection Summary						
HCM 6th Ctrl Delay			29.5			
HCM 6th LOS			С			
Notes						

Unsignalized Delay for [NBR] is excluded from calculations of the approach delay and intersection delay.

### 1: SR 19 & CR 48

	•	•	<b>†</b>	~	1	Ţ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	7	7	<b>↑</b>	7	7	<b>↑</b>
Traffic Volume (veh/h)	434	319	72	353	304	84
Future Volume (veh/h)	434	319	72	353	304	84
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1752	1589	1767	1811	1737	1811
Adj Flow Rate, veh/h	447	210	74	0	313	87
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	10	21	9	6	11	6
Cap, veh/h	405	327	729	U	767	1107
		0.24		0.00		
Arrive On Green	0.24		0.41	0.00	0.13	0.61
Sat Flow, veh/h	1668	1346	1767	1535	1654	1811
Grp Volume(v), veh/h	447	210	74	0	313	87
Grp Sat Flow(s),veh/h/ln	1668	1346	1767	1535	1654	1811
Q Serve(g_s), s	22.7	13.1	2.4	0.0	9.5	1.8
Cycle Q Clear(g_c), s	22.7	13.1	2.4	0.0	9.5	1.8
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	405	327	729		767	1107
V/C Ratio(X)	1.10	0.64	0.10		0.41	0.08
Avail Cap(c_a), veh/h	405	327	729		880	1107
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	0.00	1.00	1.00
Uniform Delay (d), s/veh	35.4	31.8	16.9	0.00	11.2	7.4
	76.1	4.3	0.3	0.0	0.3	0.1
Incr Delay (d2), s/veh						
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	25.4	7.8	1.8	0.0	5.8	1.2
Unsig. Movement Delay, s/veh		• • • •	45			
LnGrp Delay(d),s/veh	111.6	36.1	17.1	0.0	11.5	7.6
LnGrp LOS	F	D	В		В	Α
Approach Vol, veh/h	657		74	Α		400
Approach Delay, s/veh	87.5		17.1			10.7
Approach LOS	F		В			В
Timer - Assigned Phs	1	2		4		6
	10.0					
Phs Duration (G+Y+Rc), s	18.6	45.0		30.0		63.6
Change Period (Y+Rc), s	6.5	6.4		7.3		6.4
Max Green Setting (Gmax), s	18.5	38.6		22.7		38.6
Max Q Clear Time (g_c+l1), s	11.5	4.4		24.7		3.8
Green Ext Time (p_c), s	0.6	0.4		0.0		0.4
Intersection Summary						
HCM 6th Ctrl Delay			55.7			
HCM 6th LOS			E			
Notes						

Unsignalized Delay for [NBR] is excluded from calculations of the approach delay and intersection delay.

Intersection												
Int Delay, s/veh	1.7											
	EDI	EDT	EDD	WDI	WDT	WDD	NDI	NDT	NDD	CDI	CDT	CDD
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	25	4	40	4.4	4	45	40	4	0.4	4	400	7
Traffic Vol, veh/h	35	3	10	11	1	15	12	377	24	4	428	7
Future Vol, veh/h	35	3	10	11	1	15	12	377	24	4	428	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	_ 0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	9,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	12	33	2	2	2	2	38	10	2	42	2	11
Mvmt Flow	36	3	10	11	1	15	12	389	25	4	441	7
Major/Minor	Minor2			Minor1			Major1		N	Major2		
Conflicting Flow All	887	891	445	885	882	402	448	0	0	414	0	0
Stage 1	453	453	-	426	426	-	-	-	-	-	-	-
Stage 2	434	438	_	459	456	_	_	_	_	_	_	_
Critical Hdwy	7.22	6.83	6.22	7.12	6.52	6.22	4.48	_	_	4.52	_	_
Critical Hdwy Stg 1	6.22	5.83	- 0.22	6.12	5.52	0.22	-	_	_	-	_	_
Critical Hdwy Stg 2	6.22	5.83	_	6.12	5.52	_	_	_	_	_	_	_
Follow-up Hdwy	3.608	4.297	3.318	3.518	4.018	3.318	2.542	_	_	2.578	_	_
Pot Cap-1 Maneuver	254	251	613	266	285	648	946	_	_	960	_	_
Stage 1	568	521	-	606	586	U <del>1</del> U	J <del>-1</del> U	_	_	200		_
Stage 2	581	529	-	582	568	-				_		
Platoon blocked, %	JU 1	JZJ	_	JUZ	500	_	_		_			_
Mov Cap-1 Maneuver	243	245	613	255	278	648	946	<u>-</u>	<u>-</u>	960	-	<u>-</u>
Mov Cap-1 Maneuver	243	245	013	255	278	040	340	_	_	300		
Stage 1	558	518	-	596	576	<del>-</del>	<u>-</u>	<del>-</del>	<u>-</u>	<u>-</u>	_	<u>-</u>
	556	520	-	565	565	-	-	-	-	-	-	-
Stage 2	550	520	-	505	505	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	20.7			15.1			0.3			0.1		
HCM LOS	С			С								
Minor Lane/Major Mvm	nt	NBL	NBT	NRD	EBLn1V	WRI n1	SBL	SBT	SBR			
	ıι		NDT					ODT	אמט			
Capacity (veh/h)		946	-	-	278	386	960	-	-			
HCM Cantrol Dalay (a)		0.013	-	-	0.178			-	-			
HCM Control Delay (s)		8.9	0	-	20.7	15.1	8.8	0	-			
HCM Lane LOS	\	A	Α	-	С	С	A	Α	-			
HCM 95th %tile Q(veh)	)	0	-	-	0.6	0.2	0	-	-			

Intersection												
Int Delay, s/veh	2.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	32	12	13	17	3	14	16	363	21	16	432	40
Future Vol, veh/h	32	12	13	17	3	14	16	363	21	16	432	40
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	12	33	2	2	2	2	38	10	2	42	2	11
Mvmt Flow	33	12	13	18	3	14	16	374	22	16	445	41
Major/Minor	Minor2			Minor1			Major1		N	Major2		
-		000			025			^			^	^
Conflicting Flow All	924	926	466	927	935	385	486	0	0	396	0	0
Stage 1	498	498	-	417	417	-	-	-	-	-	-	-
Stage 2	426	428	- 00	510	518	6.00	4.40	-	-	4.50	-	-
Critical Hdwy	7.22	6.83	6.22	7.12	6.52	6.22	4.48	-	-	4.52	-	-
Critical Hdwy Stg 1	6.22	5.83	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.22	5.83	2 240	6.12	5.52	2.040	0.540	-	-	-	-	-
Follow-up Hdwy	3.608	4.297	3.318	3.518	4.018			-	-	2.578	-	-
Pot Cap-1 Maneuver	240	239	597	249	265	663	914	-	-	975	-	-
Stage 1	536	496	-	613	591	-	-	-	-	-	-	-
Stage 2	587	535	-	546	533	-	-	-	-	-	-	-
Platoon blocked, %	005	000	F07	005	050	000	044	-	-	075	-	-
Mov Cap-1 Maneuver	225	228	597	225	253	663	914	-	-	975	-	-
Mov Cap-2 Maneuver	225	228	-	225	253	-	-	-	-	-	-	-
Stage 1	524	485	-	599	577	-	-	-	-	-	-	-
Stage 2	558	523	-	508	521	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	22.6			17.9			0.4			0.3		
HCM LOS	C			С								
Minor Lane/Major Mvr	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		914		-	263	313	975	_	_			
HCM Lane V/C Ratio		0.018	_		0.223			<u>-</u>	<u>-</u>			
HCM Control Delay (s	)	9	0	_	22.6	17.9	8.8	0	_			
HCM Lane LOS	1	A	A	_	C	C	Α	A	_			
HCM 95th %tile Q(veh	1)	0.1	-		0.8	0.4	0.1	-				
HOW JOHN JOHNE W(VEI)	1)	0.1	_	_	0.0	0.4	0.1	_	_			

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	LDL	4	LDIX	VVDL	4	WDIX	INDL	4	NDIN	ODL	4	ODIT
Traffic Vol, veh/h	1	37	12	1	19	1	4	0	3	0	0	0
Future Vol, veh/h	1	37	12	1	19	1	4	0	3	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	_	_	None	_	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	80	80	80	80	80	80	80	80	80	80	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	46	15	1	24	1	5	0	4	0	0	0
Major/Minor N	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	25	0	0	61	0	0	83	83	54	85	90	25
Stage 1	-	-	-	-	-	_	56	56	-	27	27	-
Stage 2	-	-	-	-	-	-	27	27	-	58	63	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1589	-	-	1542	-	-	904	807	1013	901	800	1051
Stage 1	-	-	-	-	-	-	956	848	-	990	873	-
Stage 2	-	-	-	-	-	-	990	873	-	954	842	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1589	-	-	1542	-	-	902	805	1013	896	798	1051
Mov Cap-2 Maneuver	-	-	-	-	-	-	902	805	-	896	798	-
Stage 1	-	-	-	-	-	-	955	847	-	989	872	-
Stage 2	-	-	-	-	_	-	989	872	-	950	841	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			0.3			8.8			0		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	t	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		946	1589			1542	-					
HCM Lane V/C Ratio		0.009		_		0.001	_	_	_			
HCM Control Delay (s)		8.8	7.3	0	-	7.3	0	-	0			
HCM Lane LOS		A	A	A	_	A	A	-	A			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	-			

Intersection												
Int Delay, s/veh	3.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	LDL	4	LDIX	VVDL	4	אטוע	NDL	4	NDI	ODL	4	ODIX
Traffic Vol, veh/h	0	29	5	17	25	5	5	1	20	1	0	0
Future Vol, veh/h	0	29	5	17	25	5	5	1	20	1	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	- Olop	- Otop	None	- Otop	Olop -	None
Storage Length	_	_	-	_	_	-	<u>-</u>	_	-	_	<u>-</u>	-
Veh in Median Storage	# -	0	_	_	0	_	_	0	_	_	0	_
Grade, %	, π -	0	_	_	0	_	_	0	_		0	_
Peak Hour Factor	80	80	80	80	80	80	80	80	80	80	80	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	36	6	21	31	6	6	1	25	1	0	0
		- 00	- 0	<b>L</b> 1	01		- 3	1	20		- 0	
	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	37	0	0	42	0	0	115	118	39	128	118	34
Stage 1	-	-	-	-	-	-	39	39	-	76	76	-
Stage 2	-	-	-	-	-	-	76	79	-	52	42	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1574	-	-	1567	-	-	862	772	1033	845	772	1039
Stage 1	-	-	-	-	-	-	976	862	-	933	832	-
Stage 2	-	-	-	-	-	-	933	829	-	961	860	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1574	-	-	1567	-	-	853	761	1033	815	761	1039
Mov Cap-2 Maneuver	-	-	-	-	-	-	853	761	-	815	761	-
Stage 1	-	-	-	-	-	-	976	862	-	933	820	-
Stage 2	-	-	-	-	-	-	920	817	-	936	860	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			2.7			8.8			9.4		
HCM LOS				۷.۱			Α			3.4 A		
TOW LOO							٨			Λ.		
Minor Long /Maior M		UDL 4	EDI	EDT	EDD	WDI	WOT	MDD	CDL 4			
Minor Lane/Major Mvm	it f	VBLn1	EBL	EBT	EBR	WBL	WBT	WBR				
Capacity (veh/h)		980	1574	-		1567	-	-				
HCM Lane V/C Ratio		0.033	-	-		0.014	-		0.002			
HCM Control Delay (s)		8.8	0	-	-	7.3	0	-	9.4			
HCM Lane LOS		A	A	-	-	A	Α	-	A			
HCM 95th %tile Q(veh)		0.1	0	-	-	0	-	-	0			

Intersection												
Int Delay, s/veh	0.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	LDL	4	LDIN	VVDL	4	VVDIX	INDL	1	NUN	ODL	<u>अ</u>	ODIN
Traffic Vol, veh/h	2	0	5	5	0	4	3	324	13	3	435	0
Future Vol, veh/h	2	0	5	5	0	4	3	324	13	3	435	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	- -	-	None	-	-	None	-	-	None
Storage Length	_	_	-	_	_	-	_	_	-	_	_	-
Veh in Median Storage	e.# -	0	_	_	0	_	_	0	_	_	0	_
Grade, %	-, "	0	_	_	0	_	-	0	_	_	0	_
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	8	12	2	10	2
Mvmt Flow	2	0	6	6	0	4	3	360	14	3	483	0
Major/Minor I	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	864	869	483	865	862	367	483	0	0	374	0	0
Stage 1	489	489	-	373	373	-	-	-	_	-		-
Stage 2	375	380	_	492	489	_	_	_	_	_	_	_
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	_	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	_	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52	-	-	_	-	_	_	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	_	-
Pot Cap-1 Maneuver	274	290	584	274	293	678	1080	-	-	1184	-	0
Stage 1	561	549	-	648	618	-	-	-	-	-	-	0
Stage 2	646	614	-	558	549	-	-	-	-	-	-	0
Platoon blocked, %								-	-		-	
Mov Cap-1 Maneuver	271	288	584	270	291	678	1080	-	-	1184	-	-
Mov Cap-2 Maneuver	271	288	-	270	291	-	-	-	-	-	-	-
Stage 1	559	547	-	645	616	-	-	-	-	-	-	-
Stage 2	639	612	-	551	547	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	13.3			15			0.1			0.1		
HCM LOS	В			С								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT				
Capacity (veh/h)		1080	-	-	439	369	1184	-				
HCM Lane V/C Ratio		0.003	_	_	0.018			_				
HCM Control Delay (s)		8.3	-	_	13.3	15	8	0				
HCM Lane LOS		A	-	-	В	C	A	A				
HCM 95th %tile Q(veh)	)	0	-	-	0.1	0.1	0	-				

Intersection												
Int Delay, s/veh	0.5											
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EDL		EDK	WDL		WDK	NDL		NDK	ODL		SDK
Lane Configurations	2	4	4	0	4	3	1	272	12	7	<b>€</b>	0
Traffic Vol, veh/h Future Vol, veh/h	3	1		8	0	3	1	372 372	12	7 7	343 343	0
Conflicting Peds, #/hr	0	1 0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	Stop -	Siop -	None	Stop -	Stop -	None	-	-	None	-	-	None
Storage Length	_		NOHE	_	-	NONE -		-	INOHE -	_	_	NOHE
Veh in Median Storage	- e.# -	0		_	0			0	_		0	_
Grade, %	-, π	0	<u>-</u>	<u>-</u>	0	<u>-</u>	_	0	<u>-</u>	_	0	_
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	8	12	2	10	2
Mvmt Flow	3	1	4	9	0	3	1	413	13	8	381	0
		•									30 1	
Major/Miner	Minero			Minard			Mais =1			Mais		
	Minor2	005		Minor1	040		Major1	^		Major2		^
Conflicting Flow All	820	825	381	822	819	420	381	0	0	426	0	0
Stage 1	397	397	-	422	422	-	-	-	-	-	-	-
Stage 2	423	428	- 00	400	397	6.00	4.40	-	-	4.40	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52 5.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12 6.12	5.52 5.52	-	6.12 6.12	5.52	<del>-</del>	<del>-</del>	-	-	<del>-</del>	-	<del>-</del>
Critical Hdwy Stg 2	3.518	4.018	3.318	3.518		3.318	2 210	-	-	2.218	-	-
Follow-up Hdwy Pot Cap-1 Maneuver	294	308	666	293	310	633	1177	-	-	1133	-	0
	629	603	000	609	588	033	11//	-	-	1133	-	0
Stage 1 Stage 2	609	585	-	626	603	-	-	-	-	-		0
Platoon blocked, %	009	505	-	020	003	-	-	-	-	-	-	U
Mov Cap-1 Maneuver	290	305	666	288	307	633	1177	-	-	1133	-	_
Mov Cap-1 Maneuver	290	305	-	288	307	- 000	1111		_	1100	_	_
Stage 1	628	598		608	587	-	_	-	-	-	-	-
Stage 2	605	584	_	615	598	_	_	_	_	_	_	_
Olugo Z	505	504		313	550							
Δ				1445			h i m			0.0		
Approach	EB			WB			NB			SB		
HCM Control Delay, s	14			16.1			0			0.2		
HCM LOS	В			С								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT				
Capacity (veh/h)		1177	-	-	408	338	1133	-				
HCM Lane V/C Ratio		0.001	-	-	0.022	0.036	0.007	-				
HCM Control Delay (s)		8.1	-	-	14	16.1	8.2	0				
HCM Lane LOS		Α	-	-	В	С	Α	Α				
HCM 95th %tile Q(veh)	)	0	-	-	0.1	0.1	0	-				

Intersection							
Int Delay, s/veh	2.8						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	YVDL	VVDI\	1\D1	TVDIX	ODL	<u> </u>	
Traffic Vol, veh/h	65	43	394	111	70	492	
Future Vol, veh/h	65	43	394	111	70	492	
Conflicting Peds, #/hr	0.5	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	Stop -	None	-	None	-	None	
Storage Length	0	0	_	590	-	110116	
Veh in Median Storage		-	0	- 590		0	
Grade, %	0	_	0	-	_	0	
Peak Hour Factor	96	96	96	96	96	96	
	38	15	96	22		96 5	
Heavy Vehicles, %					9		
Mvmt Flow	68	45	410	116	73	513	
Major/Minor I	Minor1	N	/lajor1	ı	Major2		
Conflicting Flow All	1069	410	0	0	526	0	
Stage 1	410	-	-	-	-	-	
Stage 2	659	-	_	-	-	-	
Critical Hdwy	6.78	6.35	_	_	4.19	_	
Critical Hdwy Stg 1	5.78	-	_	_	-	_	
Critical Hdwy Stg 2	5.78	_	_	_	_	_	
Follow-up Hdwy	3.842	3,435	_	_	2.281	_	
Pot Cap-1 Maneuver	210	614	_	_	1006	-	
Stage 1	599	-	_	_	-	_	
Stage 2	453	_	_	_	_	_	
Platoon blocked, %	700		_	_		_	
Mov Cap-1 Maneuver	189	614			1006	-	
Mov Cap-1 Maneuver	189	014	_	_	1000		
Stage 1	599	-	<u>-</u>	_		_	
	407		-	-	-		
Stage 2	407	-	<del>-</del>	-	-	-	
Approach	WB		NB		SB		
HCM Control Delay, s	25.1		0		1.1		
HCM LOS	D						
Minor Long/Major Maria	.+	NDT	NDDV	VDI 54V	VDI ~2	CDI	
Minor Lane/Major Mvm	IL	NBT		VBLn1V		SBL	
Capacity (veh/h)		-	-		614	1006	
HCM Lane V/C Ratio		-		0.358		0.072	
HCM Control Delay (s)		-	-	· · · · ·	11.3	8.9	
HCM Lane LOS		-	-	D	В	A	
HCM 95th %tile Q(veh)		-	-	1.5	0.2	0.2	

Intersection							
Int Delay, s/veh	3.5						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
					ODL		
Lane Configurations	<b>\</b>	7	476	7	ΕO	422	
Traffic Vol, veh/h	83	55	476	92	50	433	
Future Vol, veh/h	83	55	476	92	50	433	
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	0	-	590	-	-	
Veh in Median Storage	e, # 0	-	0	-	-	0	
Grade, %	0	-	0	-	-	0	
Peak Hour Factor	96	96	96	96	96	96	
Heavy Vehicles, %	38	15	8	22	9	5	
Mvmt Flow	86	57	496	96	52	451	
WWW.	- 00	OI.	400	30	UL	701	
Major/Minor	Minor1	N	/lajor1	1	Major2		
Conflicting Flow All	1051	496	0	0	592	0	۱
Stage 1	496	-	-	-	-	-	
Stage 2	555	_	_	_	_	_	
Critical Hdwy	6.78	6.35		_	4.19		
Critical Hdwy Stg 1	5.78	0.33			4.13		
, ,			-	-	-	-	
Critical Hdwy Stg 2	5.78	- 125	-	-	- 0.04	-	
Follow-up Hdwy	3.842		-	-	2.281	-	
Pot Cap-1 Maneuver	215	548	-	-	950	-	
Stage 1	544	-	-	-	-	-	
Stage 2	509	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	199	548	-	-	950	-	
Mov Cap-2 Maneuver	199	-	-	-	-	-	
Stage 1	544	-	_	_	-	-	
Stage 2	472	_	_	_	_	_	
Olugo Z	712						
Approach	WB		NB		SB		
HCM Control Delay, s	26.7		0		0.9		
HCM LOS	D				0.0		
1 JOINI LOO	J						
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1V	VBLn2	SBL	
Capacity (veh/h)		-	-	199	548	950	
HCM Lane V/C Ratio		-	_	0.434			
HCM Control Delay (s)		-	_	36.3	12.3	9	
HCM Lane LOS		_	_	E	В	A	
HCM 95th %tile Q(veh	)	_	_	2	0.3	0.2	
HOW JOHN JOHN GUILD WING	1				0.0	0.2	

**Appendix F**ITE Trip Generation Sheets

# Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units On a: Weekday

Setting/Location: General Urban/Suburban

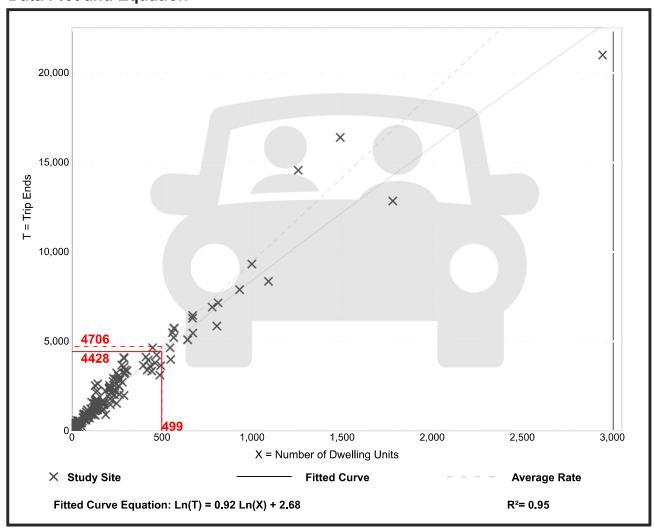
Number of Studies: 174 Avg. Num. of Dwelling Units: 246

Directional Distribution: 50% entering, 50% exiting

### **Vehicle Trip Generation per Dwelling Unit**

Average Rate	Range of Rates	Standard Deviation
9.43	4.45 - 22.61	2.13

### **Data Plot and Equation**



Trip Gen Manual, 11th Edition

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# Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

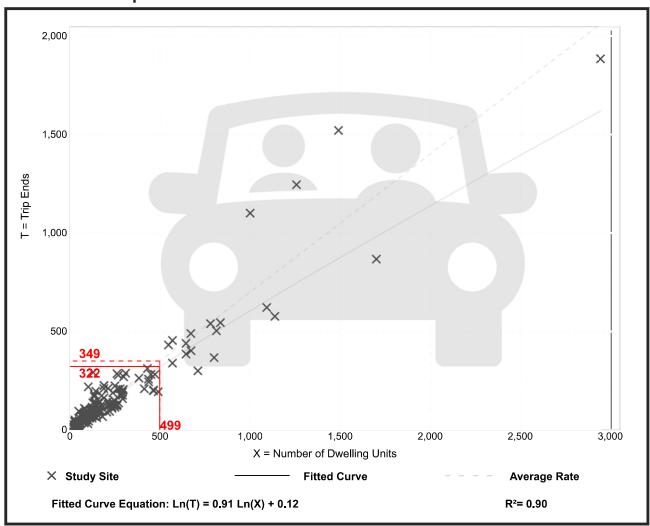
Number of Studies: 192 Avg. Num. of Dwelling Units: 226

Directional Distribution: 25% entering, 75% exiting

### **Vehicle Trip Generation per Dwelling Unit**

Average Rate	Range of Rates	Standard Deviation
0.70	0.27 - 2.27	0.24

#### **Data Plot and Equation**



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# Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

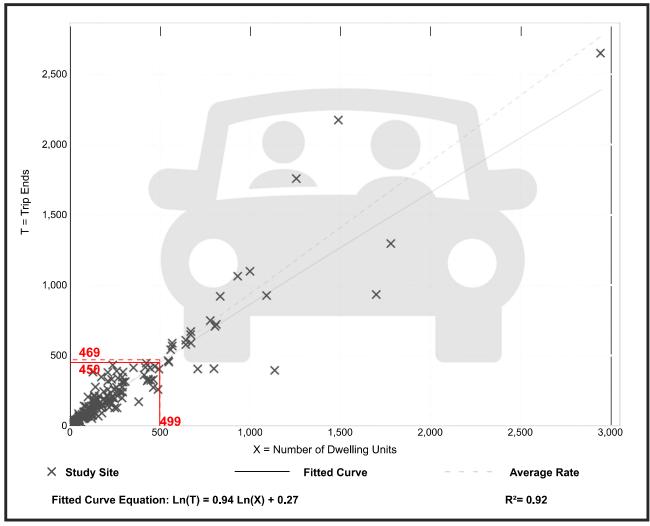
Number of Studies: 208 Avg. Num. of Dwelling Units: 248

Directional Distribution: 63% entering, 37% exiting

### **Vehicle Trip Generation per Dwelling Unit**

Average Rate	Range of Rates	Standard Deviation
0.94	0.35 - 2.98	0.31

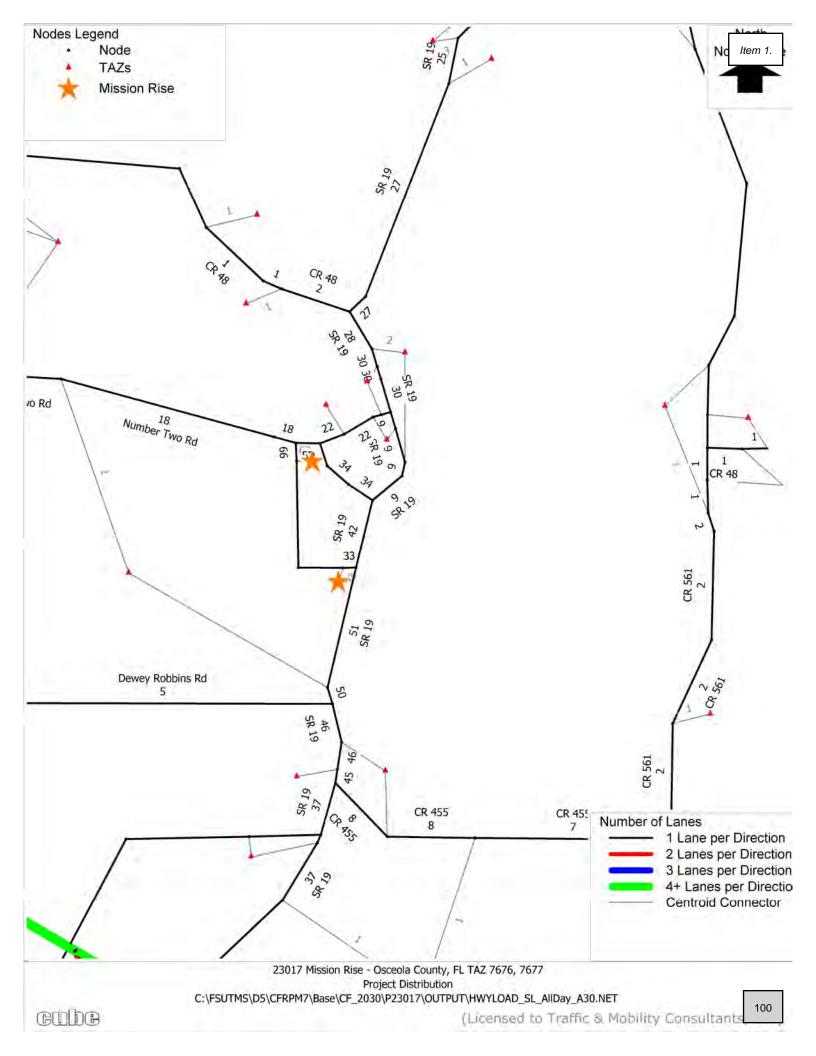
### **Data Plot and Equation**

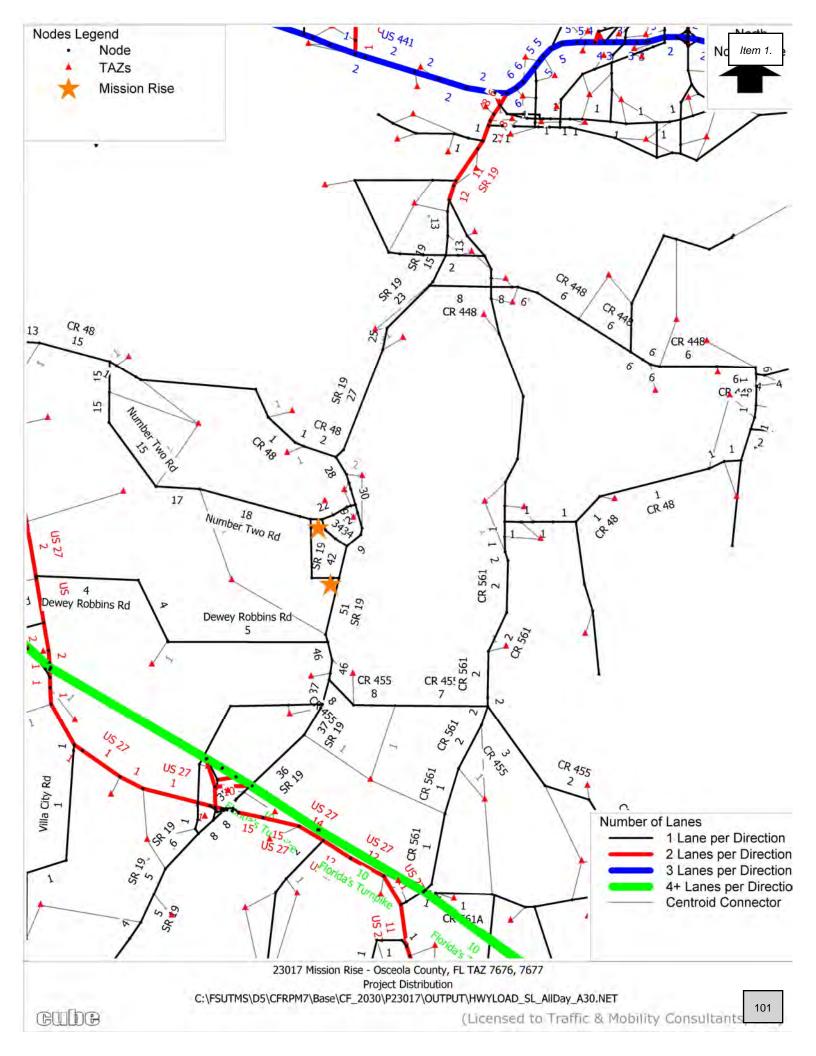


Trip Gen Manual, 11th Edition

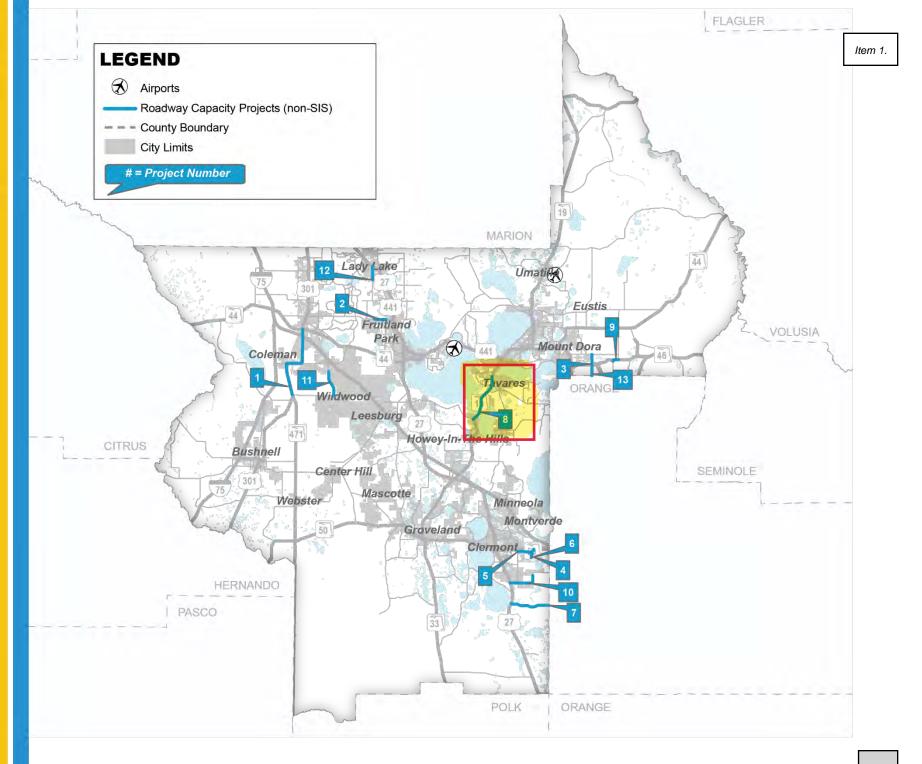
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Appendix G
CFRPM Model Output





**Appendix H** *LSMPO TIP* and *LSMPO LOPP* 



7

8

Project Description: WELLNESS WAY FROM US-27 TO THE LAKE/ORANGE COUNTY LINE

FM# 4487331 Funding Source(s):

Local and State

Work Description: NEW ROAD CONSTRUCTION

LRTP Page: PG. 4-12

Phase	e <	2023		2023		2024		2025		2026		2027		>2027	Amount Fur	nded
PDE	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-
PE	\$	-	\$	-	\$	3,000,000	\$	-	\$	-	\$	-	\$	-	\$	3,000,000
ENV	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-
ROW	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-
LAR	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-
RRU	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-
CST	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-	\$	-
Total	\$	-	\$	-	\$	3,000,000	\$	-	\$	-	\$	-	\$	-	\$	3,000,000
	Responsible Agency: RESPONSIBLE AGENCY NOT AVAILABLE									County: LAKE					oject Cost: \$	3,000,000

Project Description: SR 19 FROM CR 48 TO CR 561

FM#

Funding

State and Federal

2383191

Source(s):

LRTP Page: PG. 4-12

Work Description: ADD LANES & RECONSTRUCT

Phase	<2023		<2023 2023		2024		2025		2026		2027	>2027	Amount Funded		nded
PDE	\$	1,161,015	\$	-	\$	-	\$ -	\$	-	\$	-	\$ -		\$	1,161,015
PE	\$	4,141,718	\$	-	\$	-	\$ -	\$	-	\$	-	\$ -		\$	4,141,718
ENV	\$	492,196	\$	200,000	\$	-	\$ -	\$	-	\$	-	\$ -		\$	692,196
ROW	\$	-	\$	-	\$	-	\$ -	\$	-	\$	-	\$ -		\$	-
LAR	\$	-	\$	-	\$	-	\$ -	\$	-	\$	-	\$ -		\$	-
RRU	\$	-	\$	-	\$	-	\$ -	\$	-	\$	-	\$ -		\$	-
CST	\$	-	\$	-	\$	-	\$ -	\$	-	\$	-	\$ -		\$	-
Total	\$	5,794,929	\$	200,000	\$	-	\$ -	\$	-	\$	-	\$ -		\$	5,994,929
	Responsi	ble Agency:	FDC	T					County:	LAKI		 Tota	l Project Cost:	\$	5,994,929



# **2022 List of Priority Projects**

Lake~Sumter Metropolitan Planning Organization

Adopted June 22, 2022

Table 3 – Roadway Capacity (Non-SIS) Project Priorities

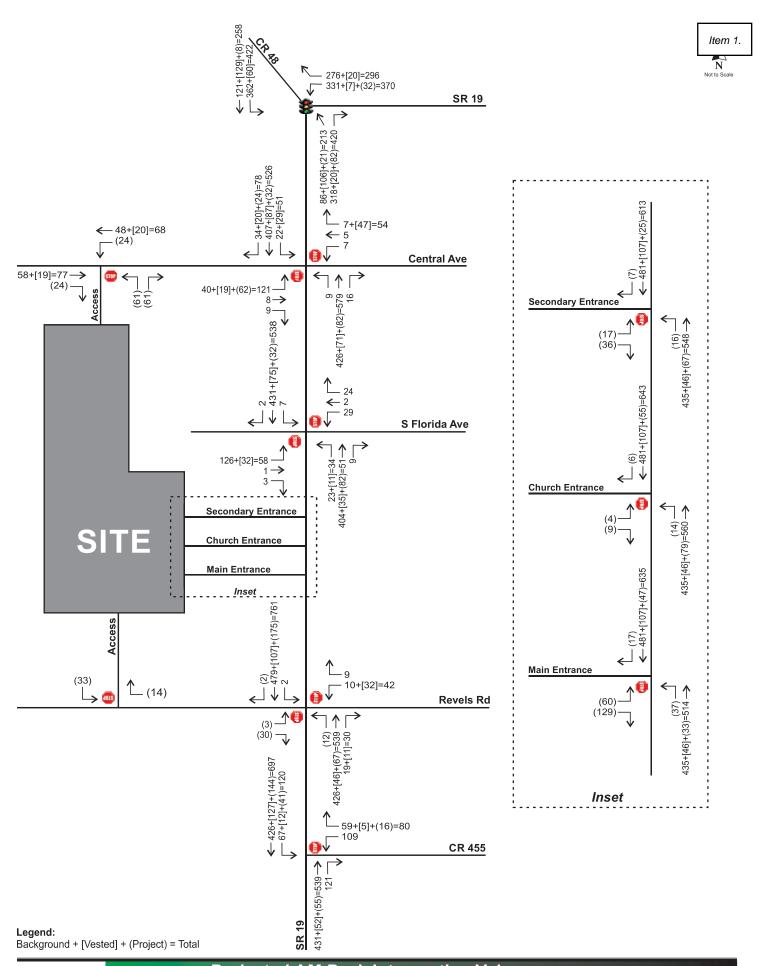
Capacity Rank	Sponsor/ Location	FM#	Project Name	From	То	Description	Performance Measure(s)	Proposed Phase	Proposed Phase FY	Proposed Phase Cost	Programmed Phase(s)	Programmed Phase FY	CMP Congested Corridors 2021 Analysis (for informational purposes)
1	FDOT/ Sumter County	430132-1	SR 35 (US 301)	SR 44	CR 470	Road Widening	System Performance	ROW	2026/27	\$27,000,000	Design	2022/23 2025/26	Extremely Congested (2021)
2	FDOT/ Lake County	409870-1	SR 44 (CR44B)	US 441	SR44	Road Widening	System Performance; Safety	CST	2024/25	\$23,701,500	ROW		Extremely Congested (2021)
3	Sumter County	447931-1	Marsh Bend Trail (CR 501)	Corbin Trail	Central Parkway	Roadway Improvements	System Performance	CST	2023/24	\$1,275,400	CST	2022/23	Operating at Acceptable Level of Service
4	FDOT/ Lake County	238394-3	SR 500 (US 441)	Perkins Street	SR 44	Road Widening	System Performance	CST	2023/24	\$13,794,537			Congested (2026)
5	FDOT/ Lake County	429356-1	SR 500 (US 441)	SR 44	N of SR 46	Road Widening	System Performance	CST	2023/24	\$22,233,040	ROW	2021/22	Not Congested
6	Lake County/ Lady Lake	439665-1	Rolling Acres Road	West Lady Lake Ave.	Griffin Ave	Road Widening	System Performance	Design	2026/27	\$2,000,000	PD&E	2025/26	Extremely Congested (2026)
7	Lake County	441710-1	Round Lake Road	Wolfbranch Rd	North of SR 44	New Roadway/ Alignment	System Performance	CST	2024/25	\$30,000,000	Design		Operating at Acceptable Level of Service
8	Lake County	441779-1	CR 455 (Hartle Rd)	Lost Lake Rd.	Hartwood Marsh Rd.	Roadway Extension/ Widening	System Performance	CST	2024/25	\$19,800,000	ROW	2022/23	New Roadway, Not on CMP Network
9	Lake County	-	CR 455 (Hartle Rd)	Hartwood Marsh Rd	CFX Lake- Orange Connector	Road Extension	System Performance	Design	2023/24	\$3,000,000	PDE		New Roadway, Not on CMP Network

Capacity Rank	Sponsor/ Location	FM #	Project Name	From	То	Description	Performance Measure(s)	Proposed Phase	Proposed Phase FY	Proposed Phase Cost	Programmed Phase(s)	Programmed Phase FY	CMP Congested Corridors 2021 Analysis (for informational purposes)
10	Lake County	-	Citrus Grove Phase II	West of Scrub Jay Lane	Grassy Lake Rd	New Alignment/Wi dening	System Performance	CST	2024/25	\$10,000,000	ROW		New Roadway, Not on CMP Network
11	Lake County	-	Citrus Grove Phase V	Turnpike	Blackstill Lake Dr	New Roadway/Alig nment	System Performance	CST	2024/25	\$5,000,000	Design		New Roadway, Not on CMP Network
12	Lake County	441393-1	CR 437 Realignment	Oak Tree Dr	SR 46	New Alignment/Wi dening	System Performance	CST	2024/25	\$4,000,000	Design		New Roadway, Not on CMP Network
13	Lake County	-	Hartwood Marsh	Regency Hills Dr	Innovation Lane	Road Widening	System Performance	Design	2023/24	\$750,000	PDE		Approaching Congestion
14	Lake County	-	CR 455 Paved Shoulder	CR 561	CR 561A	Paved Shoulder	System Performance	Design	2023/24	\$700,000			Operating at Acceptable Level of Service
15	FDOT/Lak e County	-	CR 470/CR 48	Meggison Road at The Villages	US 27	Road Widening	System Performance	Design	2023/24	\$4,000,000			Congested (2026)
16	Lake County/ Mount Dora	-	Vista Ridge Drive/Wolf Branch Innovation Boulevard	Niles Rd	Round Lake Road	New Roadway	System Performance	Design	2023/24	\$1,000,000	Study		New Roadway, Not on CMP Network
17	Lake County	-	CR 561A	CR 561	CR 455	Realignment	System Performance; Safety	PDE	2023/24	\$750,000	Study		Operating at Acceptable Level of Service
18	FDOT/ Lake County	-	SR 44	Orange Ave	CR 46A	Road Widening	System Performance	PDE	2023/24	\$TBD			Congested (2021)
19	FDOT	-	SR 19	SR 50	CR 455	Road Widening	System Performance	PDE	2023/24	\$TBD			Congested (2021)

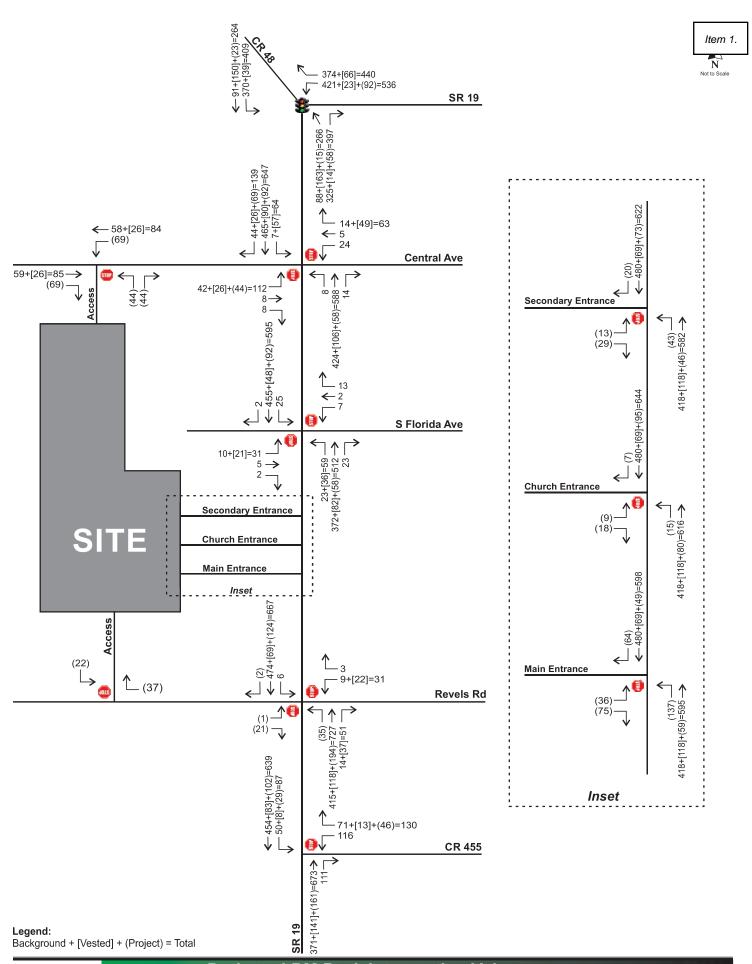
Capacity Rank	Sponsor/ Location	FM #	Project Name	From	То	Description	Performance Measure(s)	Proposed Phase	Proposed Phase FY	Proposed Phase Cost	Programmed Phase(s)	Programmed Phase FY	CMP Congested Corridors 2021 Analysis (for informational purposes)
20	Lake County	-	Woodlea Road	SR 19	End	Road Widening	System Performance	Design Update/ ROW	2023/24	\$3,000,000			Operating at Acceptable Level of Service
21	FDOT/ Lake County	238319-1	SR 19	Howey Bridge	CR 561	Road Widening	System Performance	CST	2023/24	\$35,000,000			Extremely Congested (2021)
22	Lake County	-	Hancock Road	Hartwood Marsh Rd	Wellness Way	New Road	System Performance	CST	2025/26	\$20,000,000			New Roadway, Not on CMP Network
23	Lake County	-	SR 46A	SR 44	SR 46	Road Widening	System Performance	CST	2023/24	\$TBD	Design		Congested (2021)

Top 20 Project

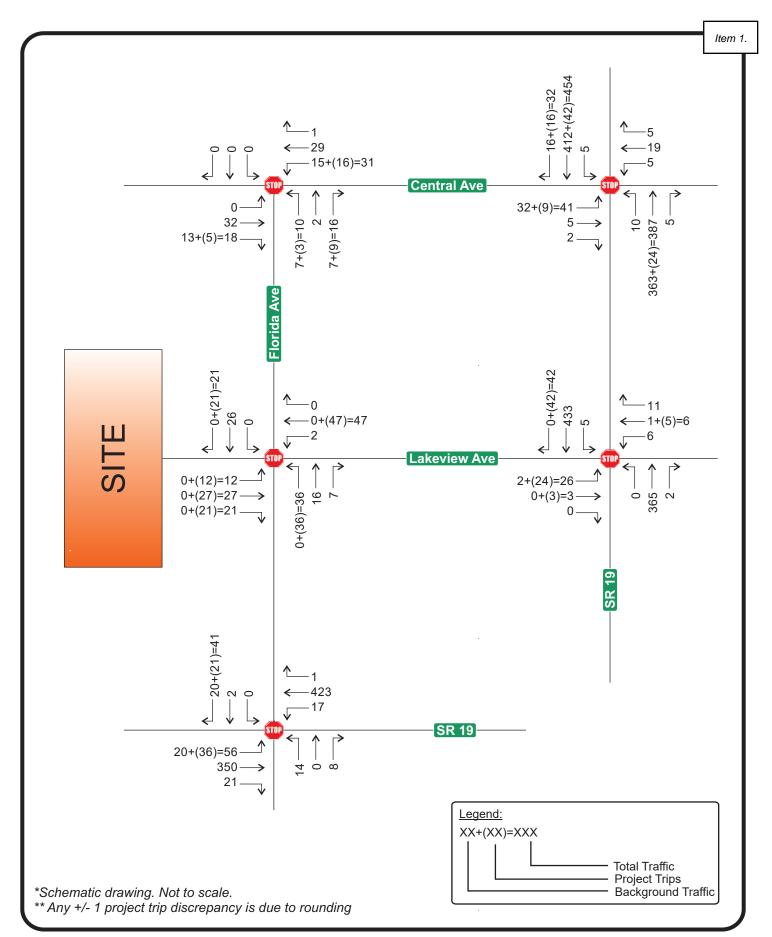
**Appendix I**Vested Trips Data



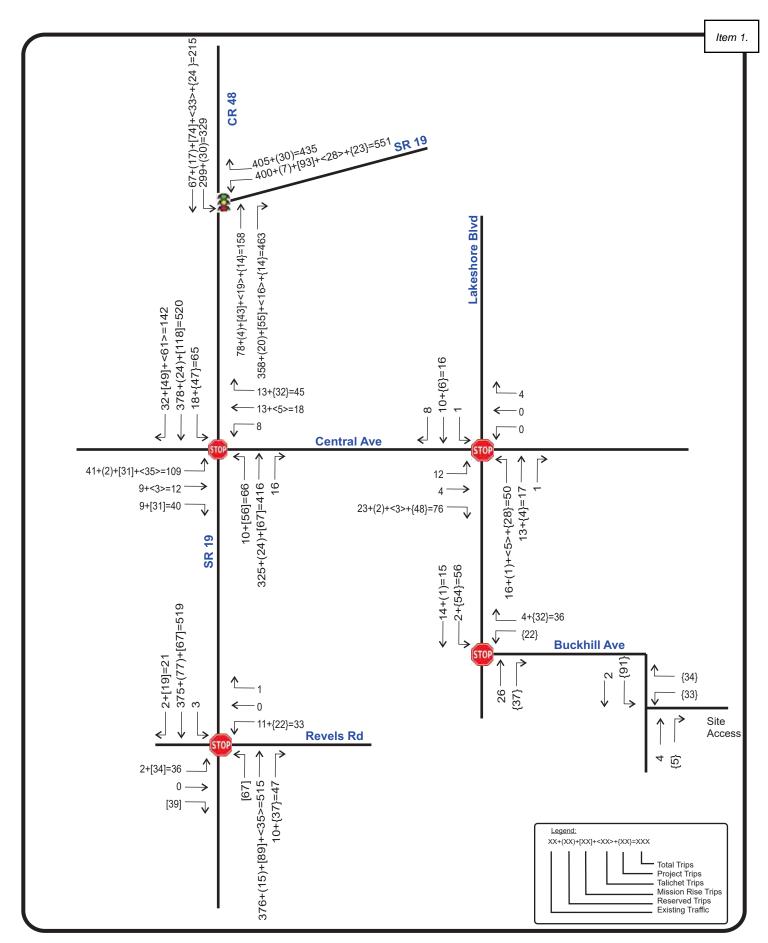






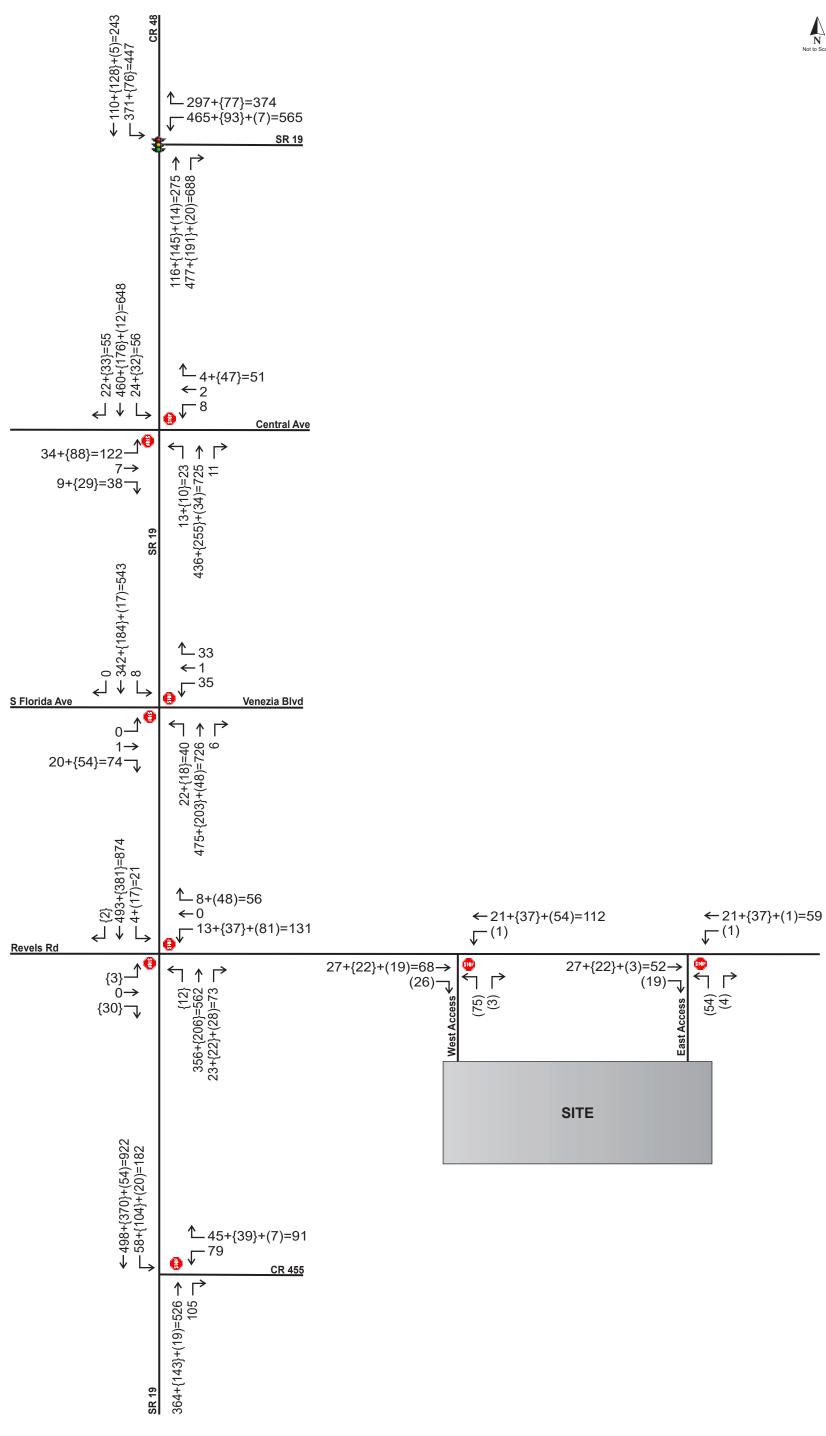






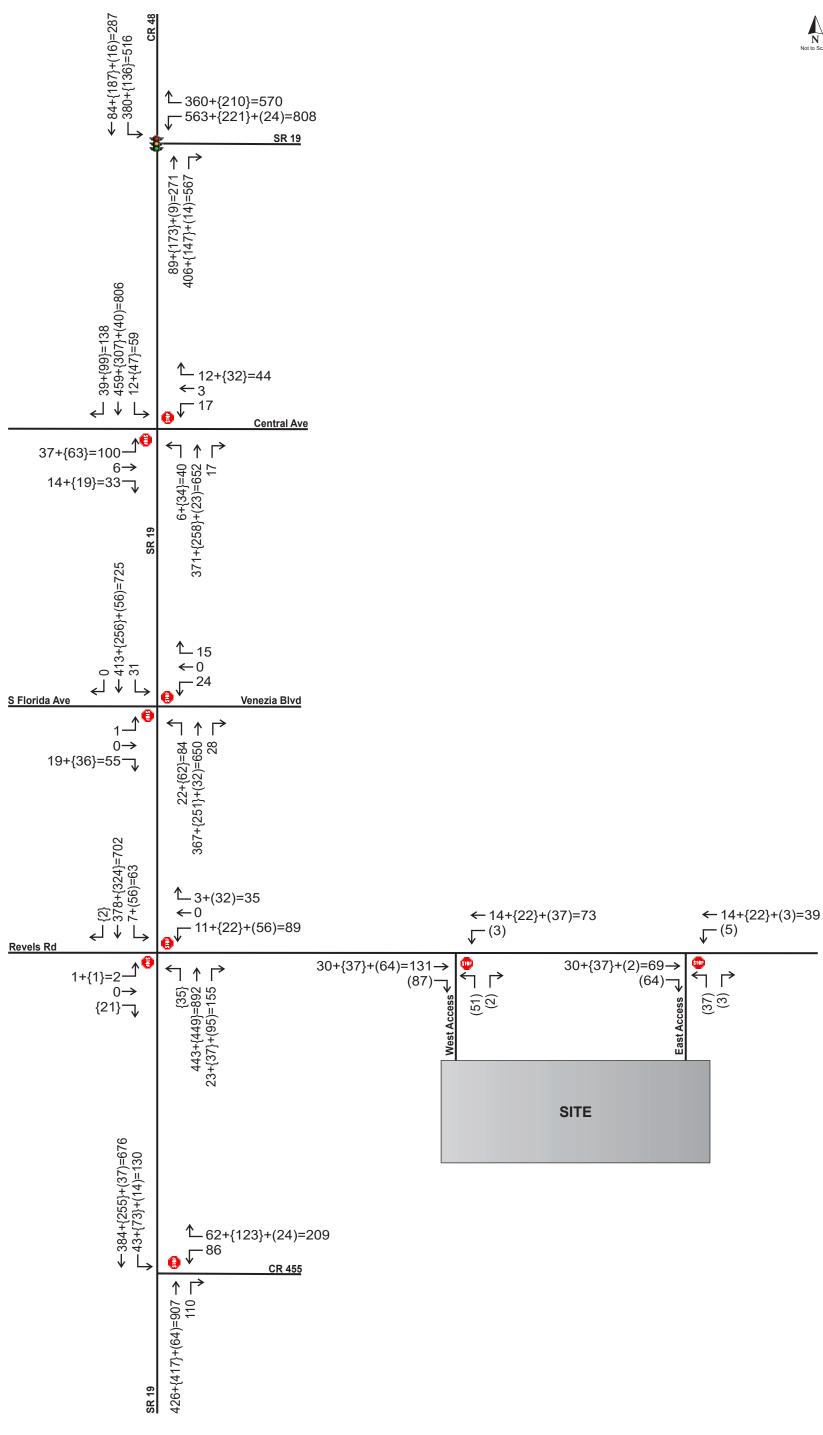






Legend: Background + {Committed} + (Project) = Total





Legend: Background + {Committed} + (Project) = Total Four Seasons Lake Harris
Traffic Impact Analysis Methodology - Revised
Project № 21237
February 8, 2022
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Table 1
Trip Generation Calculations – Phase 1 (2026)

ITE			Da	ily		AM Pe	ak Hour	r		PM Pea	ak Hou	r
Code	Land Use	Size	Rate	Trips	Rate	Total	Enter	Exit	Rate	Total	Enter	Exit
210	Single-Family Detached	184 DU	9.61	1,768	0.71	131	34	97	0.96	177	112	65
215	Single-Family Attached	146 DU	7.27	1,061	0.48	70	22	48	0.57	83	47	36
	Total Trip	Generation (P	hase 1)	2,829		201	56	145		260	159	101

Source: ITE Trip Generation Manual, 11th Edition

ITE equations were used as R2 were greater than 0.75 and with more than 20 studies

Phase 1 of the proposed development is projected to generate 2,829 new daily trips of which 201 trips occur during the AM peak hour, and 260 trips occur during the PM peak hour.

Table 2
Trip Generation Calculations – Phase 1 and Phase 2 (2030)

ITE				Da	ily		AM Pe	ak Hour			PM Pea	ak Hou	
Code	le Land Use Size		Rate	Trips	Rate	Total	Enter	Exit	Rate	Total	Enter	Exit	
210	210 Single-Family Detached 358 DU				3,261	0.66	236	61	175	0.92	329	207	122
215	Single-Family Atta	ched	292 DU	7.45	2,175	0.50	146	45	101	0.59	172	98	74
Tota	I Trip Generation	Buildou	t (Phase 1 + P	hase 2)	5,436		382	106	276		501	305	196

Source: ITE Trip Generation Manual, 11th Edition

ITE equations were used as  $\mathbb{R}^2$  were greater than 0.75 and with more than 20 studies

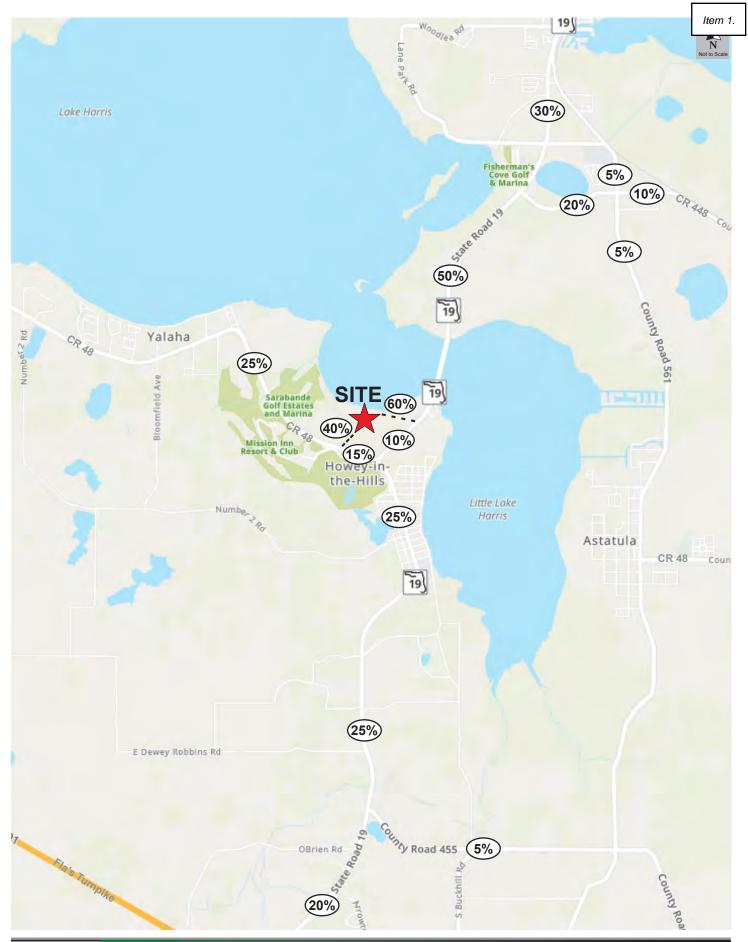
The proposed development at project buildout is projected to generate 5,436 new daily trips of which 382 trips occur during the AM peak hour, and 501 trips occur during the PM peak hour.

#### **Trip Distribution**

A trip distribution pattern was estimated using the *Central Florida Regional Planning Model,* version 7 (CFRPM V7). The model distribution was adjusted based on local knowledge, professional engineering judgement, and the location of the development with respect to the study area attractions and activity centers to reflect prevailing travel patterns in the vicinity of the site and the surrounding transportation network. The raw model plots are provided in the **Attachments**, and the adjusted trip distribution is shown in **Figure 2.** 

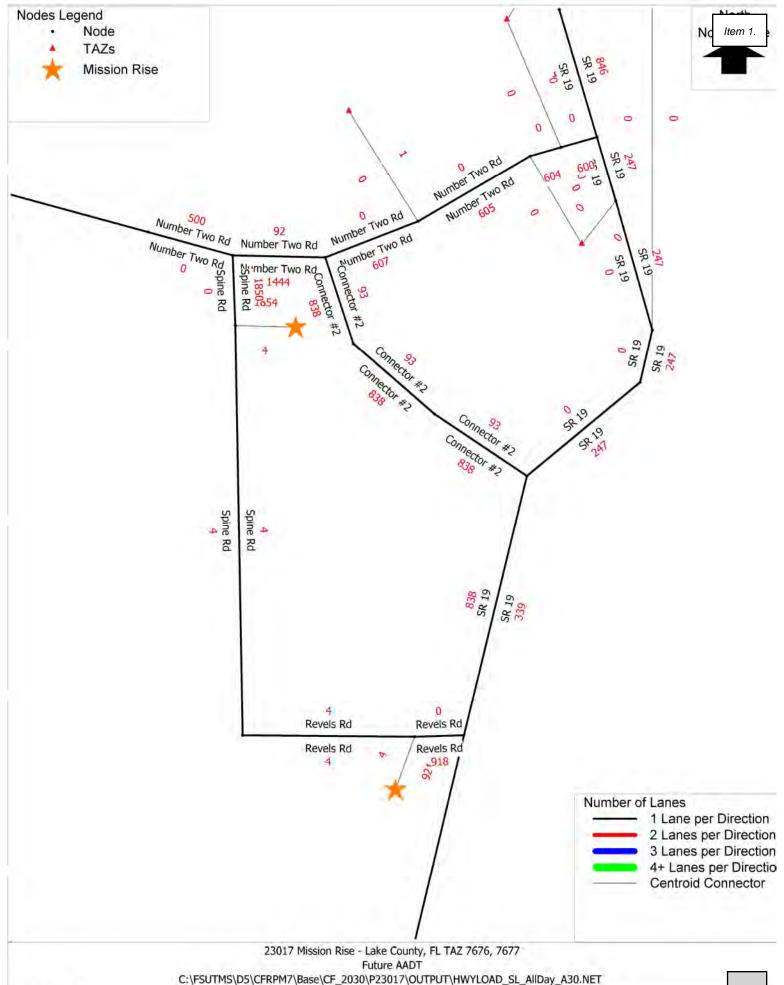
#### Study Area

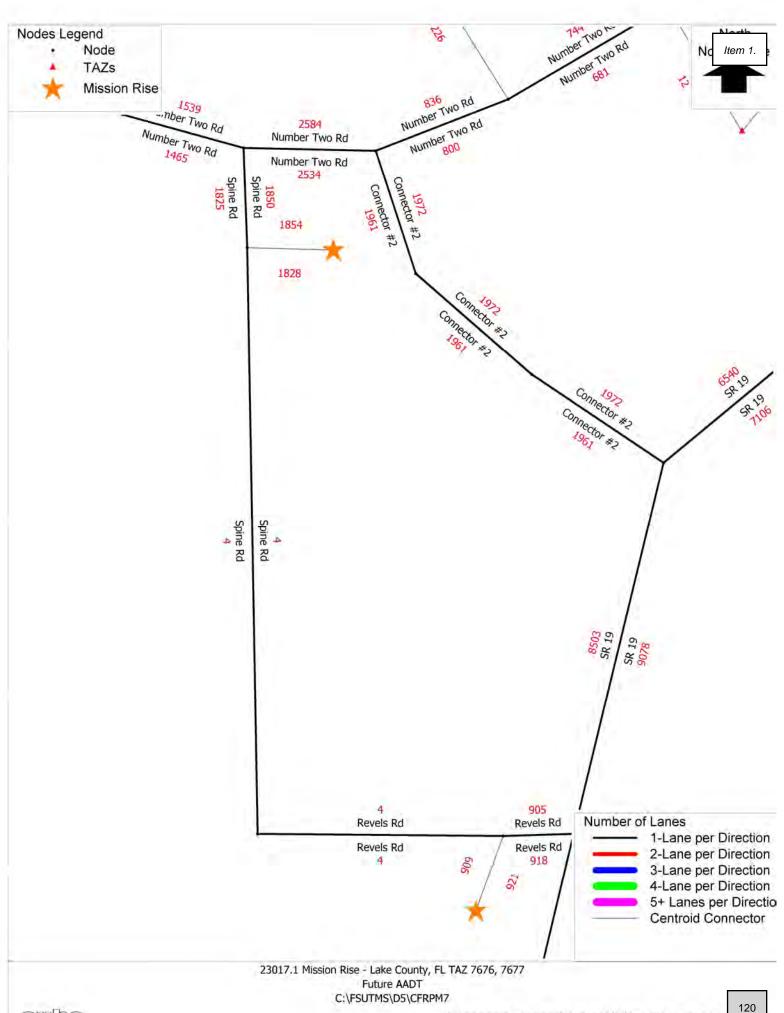
In accordance with the LSMPO requirements for a Tier 2 TIA methodology and the Town of Howey-In-The-Hills Land Development Code, the study area will encompass roadway segments and intersections within a 1-mile radius at minimum. The study will also include segments and intersections within a 4.55-mile radius, (½ the trip length for residential land use), where the project's peak hour trips consume five percent (5%) or more of a roadway's two-way peak hour generalized service volume, based on the adopted LOS and committed number of lanes. The total trip length was obtained from the *Lake County Transportation Impact Fee Schedule Table 9-1* (dated 12/21/2001), included in the **Attachments**. The roadway segments identified by the significance test will be analyzed in the Tier 2 TIA. Excerpts from the *2020 Lake County Congestion Management Plan (CMP) Database* are included in the **Attachments**. The study area significance analysis is summarized in **Table 3**.





Appendix J AADT Model Plot





Cube

**Appendix K**HCM Worksheets - Projected Conditions

## 1: SR 19 & CR 48

	•	•	<b>†</b>	-	-	ţ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>^</b>	7	7	<b>^</b>
Traffic Volume (veh/h)	522	334	455	740	413	180
Future Volume (veh/h)	522	334	455	740	413	180
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1752	1589	1767	1811	1737	1811
Adj Flow Rate, veh/h	538	205	469	0	426	186
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	10	21	9	6	11	6
Cap, veh/h	386	312	695	<u> </u>	502	1139
Arrive On Green	0.23	0.23	0.39	0.00	0.17	0.63
Sat Flow, veh/h	1668	1346	1767	1535	1654	1811
•						
Grp Volume(v), veh/h	538	205	469	0	426	186
Grp Sat Flow(s),veh/h/ln	1668	1346	1767	1535	1654	1811
Q Serve(g_s), s	22.7	13.5	21.5	0.0	14.2	4.2
Cycle Q Clear(g_c), s	22.7	13.5	21.5	0.0	14.2	4.2
Prop In Lane	1.00	1.00		1.00	1.00	4/
Lane Grp Cap(c), veh/h	386	312	695		502	1139
V/C Ratio(X)	1.39	0.66	0.67		0.85	0.16
Avail Cap(c_a), veh/h	386	312	695		535	1139
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	0.00	1.00	1.00
Uniform Delay (d), s/veh	37.7	34.2	24.5	0.0	16.9	7.5
Incr Delay (d2), s/veh	192.0	5.0	5.2	0.0	11.6	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	44.9	8.2	14.6	0.0	10.5	2.8
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	229.7	39.1	29.7	0.0	28.5	7.8
LnGrp LOS	F	D	C	3.0	C	Α
Approach Vol, veh/h	743		469	А		612
Approach Delay, s/veh	177.1		29.7	A		22.2
	1//.1		29.7 C			
Approach LOS	Г		U			С
Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	23.0	45.0		30.0		68.0
Change Period (Y+Rc), s	6.5	6.4		7.3		6.4
Max Green Setting (Gmax), s	18.5	38.6		22.7		38.6
Max Q Clear Time (g_c+l1), s	16.2	23.5		24.7		6.2
Green Ext Time (p_c), s	0.4	2.5		0.0		1.0
* '	0.4	2.0		0.0		1.0
Intersection Summary						
HCM 6th Ctrl Delay			87.2			
HCM 6th LOS			F			
Notes						

Unsignalized Delay for [NBR] is excluded from calculations of the approach delay and intersection delay.

1	١٠	SI	R	1	g	ጴ	C	R	4	R

	•	*	<b>†</b>	-	-	<b>↓</b>
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>^</b>	7	7	<b>^</b>
Traffic Volume (veh/h)	751	483	164	588	451	194
Future Volume (veh/h)	751	483	164	588	451	194
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No	1.00	No	1.00	1.00	No
Adj Sat Flow, veh/h/ln	1752	1589	1767	1811	1737	1811
Adj Flow Rate, veh/h	774	359	169	0	465	200
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
	10	21	9	0.97	11	0.97
Percent Heavy Veh, %				Ö		
Cap, veh/h	380	307	685	0.00	737	1149
Arrive On Green	0.23	0.23	0.39	0.00	0.18	0.63
Sat Flow, veh/h	1668	1346	1767	1535	1654	1811
Grp Volume(v), veh/h	774	359	169	0	465	200
Grp Sat Flow(s),veh/h/ln	1668	1346	1767	1535	1654	1811
Q Serve(g_s), s	22.7	22.7	6.5	0.0	16.0	4.5
Cycle Q Clear(g_c), s	22.7	22.7	6.5	0.0	16.0	4.5
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	380	307	685		737	1149
V/C Ratio(X)	2.04	1.17	0.25		0.63	0.17
Avail Cap(c_a), veh/h	380	307	685		744	1149
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	0.00	1.00	1.00
,	38.4	38.4	20.7	0.00	12.3	7.5
Uniform Delay (d), s/veh						
Incr Delay (d2), s/veh	475.1	105.6	0.9	0.0	1.7	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	92.9	24.6	4.9	0.0	9.5	3.0
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	513.5	144.1	21.5	0.0	14.0	7.8
LnGrp LOS	F	F	С		В	Α
Approach Vol, veh/h	1133		169	Α		665
Approach Delay, s/veh	396.4		21.5			12.1
Approach LOS	F		C			В
Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	24.6	45.0		30.0		69.6
Change Period (Y+Rc), s	6.5	6.4		7.3		6.4
Max Green Setting (Gmax), s	18.5	38.6		22.7		38.6
Max Q Clear Time (g_c+l1), s	18.0	8.5		24.7		6.5
Green Ext Time (p_c), s	0.1	0.9		0.0		1.1
Intersection Summary						
			024.2			
HCM 6th Ctrl Delay			234.3			
HCM 6th LOS			F			
Notes						

Unsignalized Delay for [NBR] is excluded from calculations of the approach delay and intersection delay.

ntersection														
	70.1													
•	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
	LDL		EDN	WDL		WDN	NDL		NDI	ODL		SDN		
_ane Configurations Traffic Vol, veh/h	144	4	12	13	<b>♣</b>	65	14	<b>♣</b> 672	29	37	<b>4</b> 663	49		
· · · · · · · · · · · · · · · · · · ·	144	-	12	13		65	14	672	29	37	663	49		
<u> </u>	0	0	0	0	1 0	00	0	0/2	0	0	003	0		
Conflicting Peds, #/hr								Free	Free		Free			
Sign Control S RT Channelized	Stop -	Stop -	Stop	Stop	Stop -	Stop	Free	riee -	None	Free		Free None		
		-	None	-	-	None	-	-	NOHE -	-	-	None		
Storage Length	-	_			0	-	-	0	-	-	0	-		
/eh in Median Storage, #	- -	0	-	-	0		-	0			0			
Grade, %	97	97	97	97	97	- 07	- 07	97	97	97	97	97		
Peak Hour Factor	12	33	2	2	2	97	97 38	10	2	42	2	11		
Heavy Vehicles, %		33 4	12	13	1		14			38	684			
Mvmt Flow	148	4	12	13	ļ	67	14	693	30	30	004	51		
Asiar/Minor	200			Minera			Mais =1			Ania -O				
•	nor2	4507		Minor1	4547		Major1			Major2	^	^		
	556	1537	710	1530	1547	708	735	0	0	723	0	0		
•	786	786	-	736	736	-	-	-	-	-	-	-		
	770	751	-	794	811	-	- 4.40	-	-	4.50	-	-		
•	7.22	6.83	6.22	7.12	6.52	6.22	4.48	-	-	4.52	-	-		
, ,	5.22	5.83	-	6.12	5.52	-		-	-	-	-	-		
, ,	5.22	5.83	-	6.12	5.52	-	-	-	-	-	-	-		
		4.297	3.318	3.518	4.018	3.318	2.542	-	-	2.578	-	-		
	~ 87	99	434	96	114	435	727	-	-	722	-	-		
	371	362	-	411	425	-	-	-	-	-	-	-		
•	379	376	-	381	393	-	-	-	-	-	-	-		
Platoon blocked, %	00	^7	40.4	.00	400	405	707	-	-	700	-	-		
	- 66	87	434	82	100	435	727	-	-	722	-	-		
	- 66	87	-	82	100	-	-	-	-	-	-	-		
•	359	329	-	398	411	-	-	-	-	-	-	-		
Stage 2	310	364	-	333	358	-	-	-	-	-	-	-		
Approach	EB			WB			NB			SB				
HCM Control Delay, s\$ 72	29.8			26.5			0.2			0.5				
ICM LOS	F			D										
/linor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR					
Capacity (veh/h)		727	-	-	71	248	722							
ICM Lane V/C Ratio		0.02	-	-		0.328	0.053	-	-					
CM Control Delay (s)		10.1	0		729.8	26.5	10.3	0	_					
ICM Lane LOS		В	A	-	F	D	В	A	-					
HCM 95th %tile Q(veh)		0.1	-	-	15.7	1.4	0.2	-	-					
Notes														
-: Volume exceeds capac	\$: De	elav exc	eeds 3	00s	+. Com	nutation	Not De	efined	*· All	maior v	olume ii	n platoon		
. Volumo oxooodo oapac	Jan	ψ. Δ	nay one	.5000 0		+: Computation Not Defined					*: All major volume in platoon			

Intersection													
Int Delay, s/veh	83.2												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
	EDL		EDR	VVDL		WDK	INDL		INDIX	SDL		SDK	
Lane Configurations Traffic Vol, veh/h	108	<b>4</b>	16	20	4	49	19	<b>45</b>	25	66	<b>♣</b> 784	162	
Future Vol, veh/h	108	14	16	20	4	49	19	642	25	66	784	162	
	0	0	0	0	0	0	0	042	0	0	0	0	
Conflicting Peds, #/hr			-										
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	- 4	_	-	-	-	-	-	_	-	-	-	-	
Veh in Median Storage		0	-	-	0	-	-	0	-	-	0	-	
Grade, %	- 07	0	- 07	- 07	0	-	- 07	0	- 07	- 07	0	-	
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97	
Heavy Vehicles, %	12	33	2	2	2	2	38	10	2	42	2	11	
Mvmt Flow	111	14	16	21	4	51	20	662	26	68	808	167	
Major/Minor Minor2 Minor1 Major1 Major2													
		1750			1000			^			^	^	
Conflicting Flow All	1771	1756	892	1758	1826	675	975	0	0	688	0	0	
Stage 1	1028	1028	-	715	715	-	-	-	-	-	-	-	
Stage 2	743	728	-	1043	1111	-	4.40	-	-	4.50	-	-	
Critical Hdwy	7.22	6.83	6.22	7.12	6.52	6.22	4.48	-	-	4.52	-	-	
Critical Hdwy Stg 1	6.22	5.83	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.22	5.83	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	3.608	4.297	3.318	3.518	4.018	3.318	2.542	-	-	2.578	-	-	
Pot Cap-1 Maneuver	~ 61	72	341	66	77	454	582	-	-	746	-	-	
Stage 1	271	275	-	422	434	-	-	-	-	-	-	-	
Stage 2	392	386	-	277	285	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	~ 41	54	341	41	58	454	582	-	-	746	-	-	
Mov Cap-2 Maneuver	~ 41	54	-	41	58	-	-	-	-	-	-	-	
Stage 1	256	218	-	398	410	-	-	-	-	-	-	-	
Stage 2	326	364	-	195	226	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, \$	1096.5			89.7			0.3			0.7			
HCM LOS	F			F									
Minor Lane/Major Mvn	nt	NBL	NBT	NBR	EBLn1V		SBL	SBT	SBR				
Capacity (veh/h)		582	-	-	47	110	746	-	-				
HCM Lane V/C Ratio		0.034	-	-	3.027	0.684	0.091	-	-				
HCM Control Delay (s)		11.4	0	\$	1096.5	89.7	10.3	0	-				
HCM Lane LOS		В	Α	-	F	F	В	Α	-				
HCM 95th %tile Q(veh	)	0.1	-	-	15.4	3.6	0.3	-	-				
Notes													
~: Volume exceeds ca	~: Volume exceeds capacity			eeds 3	00s	+: Com	putation	Not De	efined	*: All	major v	olume ir	n platoon
		,,,,,,	- ,										,

Intersection												
Int Delay, s/veh	2.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol. veh/h	1	68	17	10	31	1	10	0	20	0	0	0
Future Vol, veh/h	1	68	17	10	31	1	10	0	20	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	_	None	_	_	None	_	_	None	_	_	None
Storage Length	-	_	-	_	-	-	-	-	-	-	_	_
Veh in Median Storage	,# -	0	-	-	0	-	_	0	-	-	0	-
Grade, %	-	0	-	_	0	-	-	0	-	-	0	-
Peak Hour Factor	80	80	80	80	80	80	80	80	80	80	80	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	85	21	13	39	1	13	0	25	0	0	0
Major/Minor N	Major1			Major2		ı	Minor1			Minor2		
Conflicting Flow All	40	0	0	106	0	0	164	164	96	176	174	40
Stage 1	40	-	-	100	-	-	98	98	-	66	66	40
Stage 2	_	_		_	_		66	66	_	110	108	_
Critical Hdwy	4.12	_	<u>-</u>	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	7.12	_		4.12	_	_	6.12	5.52	0.22	6.12	5.52	0.22
Critical Hdwy Stg 2	-	-	-	<u>-</u>	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	_		2.218	_	_		4.018		3.518		3.318
Pot Cap-1 Maneuver	1570	-	-	1485		_	801	729	960	786	719	1031
Stage 1	1370	_		1700	_	_	908	814	300	945	840	1001
Stage 2	-	_	<u>-</u>	<u>-</u>	-	_	945	840	-	895	806	-
Platoon blocked, %					_	_	373	0+0		030	000	
Mov Cap-1 Maneuver	1570		-	1485		_	795	722	960	760	712	1031
Mov Cap-2 Maneuver	1010			-	_	_	795	722	300	760	712	1001
Stage 1							907	813		944	832	
Stage 2							936	832	_	871	805	
Olugo Z							500	002		57 1	505	
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			1.8			9.2			0		
HCM LOS	U. I			1.0						A		
I IOIVI LUS							A			A		
Minor Lane/Major Mvm	t I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SRI n1			
Capacity (veh/h)		898	1570	LDI	LDIX	1485	WDI	WDIX.	ODLITT			
HCM Lane V/C Ratio		0.042		-	-	0.008	-					
				-	-		-	-	-			
HCM Lang LOS		9.2	7.3	0	-	7.4	0	-	0			
HCM 05th % tile O(vob)		A	A	Α	-	A	Α	-	Α			
HCM 95th %tile Q(veh)		0.1	0	-		0	-	-	-			

Intersection												
Int Delay, s/veh	3.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	52	11	36	59	6	9	1	33	1	0	0
Future Vol, veh/h	0	52	11	36	59	6	9	1	33	1	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	_	-	None	-	-	None
Storage Length	-	-	-	_	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	_	0	-	_	0	-	-	0	-	-	0	-
Peak Hour Factor	80	80	80	80	80	80	80	80	80	80	80	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	65	14	45	74	8	11	1	41	1	0	0
Major/Minor I	Major1		N	Major2			Minor1			Minor2		
Conflicting Flow All	82	0	0	79	0	0	240	244	72	261	247	78
Stage 1	- 02	-	U	ıσ	-	-	72	72	-	168	168	-
Stage 2	-	-	-	-	-	-	168	172	-	93	79	-
Critical Hdwy	4.12		<u>-</u>	4.12		-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	4.12	-	_	4.12	-	-	6.12	5.52	0.22	6.12	5.52	0.22
Critical Hdwy Stg 2		<u>-</u>	<u>-</u>	<del>-</del>		-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-		3.518		3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1515	-	-	1519	-	-	714	658	990	692	655	983
•	1010	-	-	1019	-	-	938	835	990	834	759	903
Stage 1 Stage 2	-	-	-	-	-		834	756	-	914	829	
Platoon blocked, %	-	-	-	-		-	034	130	-	314	029	-
Mov Cap-1 Maneuver	1515			1519	-	-	697	638	990	646	635	983
		-	-	1013		-	697	638		646	635	
Mov Cap-2 Maneuver	-	-	-	-	-	-	938	835	-	834	735	-
Stage 1	-	-	-	-	-	-	808	733	-	875	829	-
Stage 2	-	_	-	-	-	-	000	133	-	0/0	029	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			2.7			9.3			10.6		
HCM LOS							Α			В		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		899	1515	-	-	1519	_	-	646			
HCM Lane V/C Ratio		0.06	-	_	_	0.03	-		0.002			
HCM Control Delay (s)		9.3	0	-	-	7.4	0	-				
HCM Lane LOS		A	A	_	_	A	A	-	В			
HCM 95th %tile Q(veh)	)	0.2	0	-	_	0.1	-	-	0			
		- ,-				-						

Intersection													
Int Delay, s/veh	128												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		र्स	7		4		*	13			र्स	1	
Traffic Vol, veh/h	41	0	120	124	0	53	44	490	66	21	790	14	
Future Vol, veh/h	41	0	120	124	0	53	44	490	66	21	790	14	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	_	_	None	_	-	None	_	-	None	_	_	None	
Storage Length	_	-	0	_	-	-	430	_	-	-	_	405	
Veh in Median Storage	e.# -	0	_	-	0	-	-	0	-	-	0	-	
Grade, %	-,	0	_	_	0	_	_	0	_	_	0	_	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	2	8	12	2	10	2	
Nymt Flow	46	0	133	138	0	59	49	544	73	23	878	16	
WIVIIICT IOW	40	U	100	100	U	00	70	011	10	20	010	10	
Major/Minor	Minor2			Minor1			Major1		N	/lajor2			
Conflicting Flow All	1632	1639	878	1678	1619	581	894	0	0	617	0	0	
Stage 1	924	924	-	679	679	-	-	-	-	-	-	-	
Stage 2	708	715	_	999	940	_	_	_	_	_	_	_	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	_	_	4.12	_	_	
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	0.22	7.12	_	_	-	_	_	
Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52	_	_	_	_	_	_	_	
Follow-up Hdwy	3.518	4.018	3.318		4.018	3.318	2.218	_	_	2.218	_	_	
Pot Cap-1 Maneuver	81	100	347	~ 75	103	514	759	_	_	963	_	_	
Stage 1	323	348	-	441	451	- 517	100	_	_	-	_	_	
Stage 2	426	434	_	293	342	_	_	_	_	_	_	_	
Platoon blocked, %	720	704	_	233	U+Z	_				_	_	-	
Mov Cap-1 Maneuver	66	89	347	~ 42	92	514	759	-	-	963		_	
Mov Cap-1 Maneuver	66	89	34 <i>1</i>	~ 42	92	514	109	-	-	903	-	-	
Stage 1	302	331	-	412	422	-	-	-	-	-	-	-	
Stage 2	353	406	-	172	326	-	-	-	-	-	-	-	
Slaye 2	303	400	-	1/2	320	-	<del>-</del>	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	51.2		\$ .	1224.7			0.7			0.2			
HCM LOS	F		Ψ	F			0.1			J.L			
10.01 200	ı			'									
Minor Lane/Major Mvn	nt	NBL	NBT	NBR	EBLn1	EBLn2V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		759	-	-	66	347	58	963	-	-			
HCM Lane V/C Ratio		0.064	_	_	0.69		3.391		_	_			
HCM Control Delay (s)		10.1	_	_	137.5		1224.7	8.8	0	_			
HCM Lane LOS		В	-	_	F	C	F	A	A	-			
HCM 95th %tile Q(veh	)	0.2	_	_	3	1.8	20.9	0.1	-	_			
`	1	J.L			3	1.0	20.0	J. 1					
Notes													
~: Volume exceeds ca		¢. D.	Jay ove	eeds 3	nne	+· Com	nutation	Not De	ofined	*· ΔII	maior v	olume ir	n platoon

Intersection													
Int Delay, s/veh	127.1												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		ર્ન	7		4		ሻ	1>			4	7	
Traffic Vol, veh/h	30	1	83	88	0	36	135	744	146	64	602	45	
Future Vol, veh/h	30	1	83	88	0	36	135	744	146	64	602	45	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	_	_	0	_	_	-	430	_	-	_	_	405	
/eh in Median Storage	e.# -	0	-	_	0	_	-	0	_	_	0	-	
Grade, %	-,	0	_	_	0	_	_	0	_	_	0	_	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	2	8	12	2	10	2	
Nymt Flow	33	1	92	98	0	40	150	827	162	71	669	50	
	- 00		- 02	- 50			100	ULI	102		000	- 00	
lajor/Minor	Minor2			Minor1			Major1		ı	Major2			
Conflicting Flow All	2039	2100	669	2091	2069	908	719	0	0	989	0	0	
Stage 1	811	811	- 009	1208	1208	900	7 19	-	-	909	-	-	
Stage 2	1228	1289	-	883	861	_	-	-	-	-	_	-	
ritical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12		-	
ritical Hdwy Stg 1	6.12	5.52		6.12	5.52	0.22	4.12	-	-	4.12	-	-	
	6.12	5.52	-	6.12	5.52	-	-		-		-	-	
ritical Hdwy Stg 2	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-	
ollow-up Hdwy	42	52	458	~ 38	54	334	882	-	-	699	-	-	
ot Cap-1 Maneuver	373	393		224	256	334	002	-	-	099	-	-	
Stage 1	218	234	-	340	372	-	-	-	-	-	-	-	
Stage 2 latoon blocked, %	210	234	-	340	312	-	-	-	-	-	-	-	
	~ 28	36	458	. 22	27	334	882	-	-	699	-	-	
Mov Cap-1 Maneuver	~ 28	36		~ 22 ~ 22	37 37		002	-	-		-	-	
lov Cap-2 Maneuver Stage 1	310	326	-	186	212	-	-	-	-	-		-	
Stage 2	159	194	-	224	308	-	-	-	-	-	-	-	
Stage 2	159	194	<u>-</u>	224	300	-	-	-	-	-	-	-	
unnroach	EB			WB			NB			SB			
Approach			¢										
HCM Control Delay, s			<b>\$</b>	1882.8			1.3			1			
ICM LOS	F			F									
dinant ana/Marianta		NDI	NDT	NDD		EDI 0\	MDL 4	ODI	CDT	CDD			
Minor Lane/Major Mvm	Ιζ	NBL	NBT	NRK		EBLn2V		SBL	SBT	SBR			
Capacity (veh/h)		882	-	-	28	458	30	699	-	-			
ICM Lane V/C Ratio		0.17	-	-		0.201			-	-			
ICM Control Delay (s)		9.9	-	-\$	457.1		1882.8	10.7	0	-			
CM Lane LOS		Α	-	-	F	В	F	В	Α	-			
HCM 95th %tile Q(veh	)	0.6	-	-	4	0.7	16.6	0.3	-	-			
Notes													
: Volume exceeds car	: Volume exceeds capacity			eeds 3	00s	+: Com	putation	Not De	efined	*: All	major v	olume ir	n platoon
			,										,

Intersection								
Int Delay, s/veh	48.7							
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	ሻ	7	1>	7		4		
Traffic Vol, veh/h	78	88	596	133	183	927		
Future Vol, veh/h	78	88	596	133	183	927		
Conflicting Peds, #/hr		00	090	0	0	921		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	Stop -	None	-	None	-	None		
Storage Length	0	0	_	590	-	INOUE		
Veh in Median Storag		-	0	- 590	-	0		
Grade, %	0	-	0	<u>-</u>	-	0		
Peak Hour Factor	96	96	96	96	96	96		
	38	15	8	22	90	5		
Heavy Vehicles, %  Mvmt Flow	81	92	621	139	191	966		
IVIIIL FIOW	01	92	021	139	191	900		
/lajor/Minor	Minor1		Major1		Major2			
Conflicting Flow All	1969	621	0	0	760	0		
Stage 1	621	-	-	-	-	-		
Stage 2	1348	-	-	-	-	-		
ritical Hdwy	6.78	6.35	-	-	4.19	-		
ritical Hdwy Stg 1	5.78	-	-	-	-	-		
ritical Hdwy Stg 2	5.78	-	-	-	-	-		
ollow-up Hdwy	3.842	3.435	-	-	2.281	-		
ot Cap-1 Maneuver	~ 55	465	-	-	821	-		
Stage 1	473	-	-	-	-	-		
Stage 2	203	-	-	-	-	-		
Platoon blocked, %			-	-		-		
Nov Cap-1 Maneuver	~ 27	465	-	-	821	-		
Mov Cap-2 Maneuver		-	-	-	-	-		
Stage 1	473	-	-	-	-	-		
Stage 2	101	-	-	-	-	-		
<u> </u>								
Approach	WB		NB		SB			
HCM Control Delay, s			0		1.8			
TCM CONTROLDERAY, S	ър э/ю./ F		U		1.0			
ICIVI LOS	۲							
Minor Lane/Major Mvi	mt	NBT	NBRV	VBLn1V		SBL	SBT	
Capacity (veh/h)		-	-	27	465	821	-	
ICM Lane V/C Ratio		-		3.009			-	
HCM Control Delay (s	s)	-	\$	1210.8	14.6	10.7	0	
HCM Lane LOS		-	-	F	В	В	Α	
HCM 95th %tile Q(vel	n)	-	-	9.9	0.7	0.9	-	
lotes								
: Volume exceeds ca	anacity	\$· Do	lav exc	eeds 30	00s	+· Comr	outation Not Defined	*: All major volume in platoon
. Volume exceeds Co	μρασιιγ	ψ. De	ay ext	ocus si	003	· . Comp	Julation Not Delineu	. All major volume in piatour

Int Delay, s/veh   68.9   Movement   WBL   WBR   NBT   NBR   SBL   SBT   Traffic Vol, veh/h   100   179   956   110   130   756									
Movement   WBL   WBR   NBT   NBR   SBL   SBT	Intersection								
Lane Configurations Traffic Vol, veh/h 100 179 956 110 130 756 Future Vol, veh/h 100 179 956 110 130 756 Future Vol, veh/h 100 179 956 110 130 756 Fore Free Free Free Free Free Free Free	Int Delay, s/veh	68.9							
Lane Configurations Traffic Vol, veh/h 100 179 956 110 130 756 Future Vol, veh/h 100 179 956 110 130 756 Future Vol, veh/h 100 179 956 110 130 756 Fore Free Free Free Free Free Free Free	Movement	WRI	WRR	NRT	NRR	SRI	SRT		
Traeffic Vol, veh/h 100 179 956 110 130 756 Future Vol, veh/h 100 179 956 110 130 756 Conflicting Peds, #/hr Sign Control Stop Stop Free Free Free Free RT Channelized None None Storage Length 0 0 0 50 0 Reak Hour Factor Reak Ho						ODL			
Future Vol, veh/h Conflicting Peds, #/hr Conflicting Flow All Conflic						130			
Conflicting Peds, #/hr									
Sign Control         Stop RT Channelized         Stop None         Free Proce - None         Free Proce - None         Free RT Channelized         Free Processing RT Channelized         None         Stop RT Channelized									
RT Channelized - None - None   - None   Storage Length   0									
Storage Length									
Veh in Median Storage, # 0							ivone		
Grade, % 0 - 0 - 0 - 0 - 0 Peak Hour Factor 96 96 96 96 96 96 96 96 96 96 96 96 96							-		
Peak Hour Factor 96 96 96 96 96 96 96 Heavy Vehicles, % 38 15 8 22 9 5 5 M/mt Flow 104 186 996 115 135 788 M/mt Flow 105 125 135 788 M/mt Flow Minort Major1 Major2 M/mt Flow All 2054 996 0 0 1111 0 Stage 1 996 Stage 2 1058 Stage 2 1058									
Heavy Vehicles, % 38 15 8 22 9 5 Mvmt Flow 104 186 996 115 135 788  Major/Minor Minor1 Major2  Conflicting Flow All 2054 996 0 0 11111 0 Stage 1 996 Stage 2 1058 Stage 2 1058 Critical Hdwy Stg 1 5.78 Critical Hdwy Stg 2 5.78 Critical Hdwy Stg 2 5.78 Stage 1 307 Stage 1 307 Stage 2 266 Stage 2 266 Stage 2 276 Stage 2 1058 Stage 2 1059 Stage 1 307 Stage 2 1059 Stage 2 1059 Stage 2 1059 Stage 2 1059									
Major/Minor   Minor1   Major1   Major2									
Major/Minor Minor1 Major1 Major2  Conflicting Flow All 2054 996 0 0 11111 0 Stage 1 996 Stage 2 1058 4.19									
Conflicting Flow All 2054 996 0 0 11111 0 Stage 1 996 Stage 2 1058 Stage 2 1058 Stage 2 1058 Stage 2 1058 4.19	Mvmt Flow	104	186	996	115	135	788		
Conflicting Flow All 2054 996 0 0 11111 0 Stage 1 996 Stage 2 1058 Stage 2 1058 Stage 2 1058 Stage 2 1058 4.19									
Conflicting Flow All 2054 996 0 0 11111 0 Stage 1 996 Stage 2 1058 Stage 2 1058 Stage 2 1058 Stage 2 1058 4.19	Major/Minor	Minor1	N	Major1	ľ	Major2			
Stage 1							0		
Stage 2					-	-			
Critical Howy Stg 1 5.78 - 4.19 - Critical Howy Stg 1 5.78 Critical Howy Stg 2 5.78 Critical Howy Stg 2 5.78 Critical Howy Stg 2 5.78 Follow-up Howy 3.842 3.435 2.281 - Pot Cap-1 Maneuver ~48 280 - 603 - Stage 1 307 Stage 2 286 Platoon blocked, % Mov Cap-1 Maneuver ~29 280 - 603 - Mov Cap-2 Maneuver ~29 Stage 1 307 Stage 2 172 Stage 2 172  Mov Cap-2 Maneuver ~29 Stage 2 172  Mapproach WB NB SB HCM Control Delay, s\$ 544.7 HCM LOS F  Minor Lane/Major Mvmt NBT NBRWBLn1WBLn2 SBL SBT  Capacity (veh/h) 29 280 603 - HCM Lane V/C Ratio 3.592 0.666 0.225 - HCM Control Delay (s) - \$1447.7 40.2 12.7 0 HCM Lane LOS - F E B A HCM 95th %tile Q(veh) - 12.5 4.4 0.9 -	•		_	_	_	_	-		
Critical Hdwy Stg 1 5.78				-	-	4.19	-		
Critical Hdwy Stg 2 5.78				_	-		_		
Follow-up Hdwy 3.842 3.435 2.281 - Pot Cap-1 Maneuver				_	-	_	_		
Pot Cap-1 Maneuver				_	_	2 281	_		
Stage 1 307				_			_		
Stage 2				-	_	-	_		
Platoon blocked, %					_				
Mov Cap-1 Maneuver ~ 29 280 - 603 -  Mov Cap-2 Maneuver ~ 29  Stage 1 307  Stage 2 172  Approach WB NB SB  HCM Control Delay, s\$ 544.7 0 1.9  HCM LOS F  Minor Lane/Major Mvmt NBT NBRWBLn1WBLn2 SBL SBT  Capacity (veh/h) - 29 280 603 -  HCM Lane V/C Ratio - 3.592 0.666 0.225 -  HCM Control Delay (s) - \$1447.7 40.2 12.7 0  HCM Lane LOS - F E B A  HCM 95th %tile Q(veh) - 12.5 4.4 0.9 -  Notes		200			_	_			
Mov Cap-2 Maneuver       ~ 29       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -		r ~ 20	280	-	-	603	<u>-</u>		
Stage 1       307       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -				-	_		-		
Stage 2         172         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -			<u>-</u>	-	-	-	_		
Approach WB NB SB  HCM Control Delay, s\$ 544.7 0 1.9  HCM LOS F  Minor Lane/Major Mvmt NBT NBRWBLn1WBLn2 SBL SBT  Capacity (veh/h) - 29 280 603 -  HCM Lane V/C Ratio - 3.592 0.666 0.225 -  HCM Control Delay (s) - \$1447.7 40.2 12.7 0  HCM Lane LOS - F E B A  HCM 95th %tile Q(veh) - 12.5 4.4 0.9 -			-	-	-	-	-		
HCM Control Delay, s\$ 544.7  HCM LOS  Minor Lane/Major Mvmt  NBT  NBRWBLn1WBLn2  SBL  SBT  Capacity (veh/h)  - 29 280 603 -  HCM Lane V/C Ratio - 3.592 0.666 0.225 -  HCM Control Delay (s) - \$1447.7 40.2 12.7 0  HCM Lane LOS - F E B A  HCM 95th %tile Q(veh) - 12.5 4.4 0.9 -  Notes	Slaye 2	1/2	-	-	-	-	-		
HCM Control Delay, s\$ 544.7  HCM LOS  Minor Lane/Major Mvmt  NBT  NBRWBLn1WBLn2  SBL  SBT  Capacity (veh/h)  - 29 280 603 -  HCM Lane V/C Ratio - 3.592 0.666 0.225 -  HCM Control Delay (s) - \$1447.7 40.2 12.7 0  HCM Lane LOS - F E B A  HCM 95th %tile Q(veh) - 12.5 4.4 0.9 -  Notes									
Minor Lane/Major Mvmt	Approach								
Minor Lane/Major Mvmt NBT NBRWBLn1WBLn2 SBL SBT  Capacity (veh/h) 29 280 603 -  HCM Lane V/C Ratio - 3.592 0.666 0.225 -  HCM Control Delay (s) - \$1447.7 40.2 12.7 0  HCM Lane LOS - F E B A  HCM 95th %tile Q(veh) - 12.5 4.4 0.9 -	HCM Control Delay, s	s\$ 544.7		0		1.9			
Capacity (veh/h)       -       -       29       280       603       -         HCM Lane V/C Ratio       -       -       3.592       0.666       0.225       -         HCM Control Delay (s)       -       \$ 1447.7       40.2       12.7       0         HCM Lane LOS       -       -       F       E       B       A         HCM 95th %tile Q(veh)       -       -       12.5       4.4       0.9       -	HCM LOS	F							
Capacity (veh/h)       -       -       29       280       603       -         HCM Lane V/C Ratio       -       -       3.592       0.666       0.225       -         HCM Control Delay (s)       -       \$ 1447.7       40.2       12.7       0         HCM Lane LOS       -       -       F       E       B       A         HCM 95th %tile Q(veh)       -       -       12.5       4.4       0.9       -									
Capacity (veh/h)       -       -       29       280       603       -         HCM Lane V/C Ratio       -       -       3.592       0.666       0.225       -         HCM Control Delay (s)       -       \$ 1447.7       40.2       12.7       0         HCM Lane LOS       -       -       F       E       B       A         HCM 95th %tile Q(veh)       -       -       12.5       4.4       0.9       -	Minor Lane/Major My	ımt	NPT	NIRDV	WRI n1V	VRI n2	SBI	SRT	
HCM Lane V/C Ratio 3.592 0.666 0.225 - HCM Control Delay (s) - \$ 1447.7 40.2 12.7 0 HCM Lane LOS - F E B A HCM 95th %tile Q(veh) - 12.5 4.4 0.9 - Notes		1111		NDIXV					
HCM Control Delay (s) - \$ 1447.7 40.2 12.7 0  HCM Lane LOS - F E B A  HCM 95th %tile Q(veh) - 12.5 4.4 0.9 -  Notes				-					
HCM Lane LOS F E B A HCM 95th %tile Q(veh) 12.5 4.4 0.9 - Notes									
HCM 95th %tile Q(veh) 12.5 4.4 0.9 - Notes		S)		<b>`</b>					
Notes		L	-	-					
	HCM 95th %tile Q(ve	n)	-	-	12.5	4.4	0.9	-	
~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon	Notes								
		apacity	\$: De	lay exc	eeds 30	00s	+: Com	outation Not Defined	*: All major volume in platoon
		,,		,					, ,

## 6: Spine Road & Interconnect Road

Intersection						
Int Delay, s/veh	3.2					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	00	₽	^		4
Traffic Vol, veh/h	0	33	71	0	44	42
Future Vol, veh/h	0	33	71	0	44	42
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storag		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	36	77	0	48	46
Maiay/Mina	Minaria		1-11		Maia = 0	
Major/Minor	Minor1		//ajor1		Major2	
Conflicting Flow All	219	77	0	0	77	0
Stage 1	77	-	-	-	-	-
Stage 2	142	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	769	984	-	-	1522	-
Stage 1	946	-	_	_	_	-
Stage 2	885	_	_	_	_	_
Platoon blocked, %	300		_	<u>-</u>		_
Mov Cap-1 Maneuver	744	984		_	1522	
				-		
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	946	-	-	-	-	-
Stage 2	857	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s			0		3.8	
HCM LOS	Α		- 0		0.0	
TIOWI LOO	Α					
Minor Lane/Major Mvr	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	984	1522	-
HCM Lane V/C Ratio		-	_	0.036		_
HCM Control Delay (s	)	_	-	8.8	7.4	0
HCM Lane LOS	,	_	_	A	Α	A
HCM 95th %tile Q(veh	1)	_	_	0.1	0.1	
HOW JOHN JOHNE Q(VEI	'/	_	_	0.1	0.1	_

Intersection						
Int Delay, s/veh	3.2					
Mayamant	WDI	WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	**	40	<b>♣</b>	0	40	4
Traffic Vol, veh/h	0	48	60	0	42	80
Future Vol, veh/h	0	48	60	0	42	80
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	52	65	0	46	87
N 4 - 1 - 1 / N 41	l'		1.1.1		4 0	
	/linor1		/lajor1		Major2	
Conflicting Flow All	244	65	0	0	65	0
Stage 1	65	-	-	-	-	-
Stage 2	179	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	744	999	-	-	1537	-
Stage 1	958	-	-	-	-	-
Stage 2	852	-	-	-	-	-
Platoon blocked, %			_	_		-
Mov Cap-1 Maneuver	721	999	-	-	1537	-
Mov Cap-2 Maneuver	721	-	_	_	-	_
Stage 1	958	_	_	_	_	_
Stage 2	826	_	_	_	_	_
Olago Z	020					
Approach	WB		NB		SB	
HCM Control Delay, s	8.8		0		2.6	
HCM LOS	Α					
Minor Lang/Major Myss		NDT	NIDDV	VDI p1	CDI	SBT
Minor Lane/Major Mvmt		NBT	NDRV	VBLn1	SBL	201
Capacity (veh/h)		-	-	999	1537	-
HCM Lane V/C Ratio		-	-	0.052	0.03	-
HCM Control Delay (s)		-	-	8.8	7.4	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh)		-	-	0.2	0.1	-

Intersection						
Int Delay, s/veh	5.5					
		EDD	MPL	MOT	ND	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>	7	ሻ	<b>^</b>	Y	
Traffic Vol, veh/h	62	26	46	33	52	78
Future Vol, veh/h	62	26	46	33	52	78
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	420	655	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	67	28	50	36	57	85
		_		_		
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	95	0	203	67
Stage 1	-	-	-	-	67	-
Stage 2	-	-	-	-	136	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	_	_	1499	-	786	997
Stage 1	_	-	_	-	956	-
Stage 2	_	_	_	_	890	_
Platoon blocked, %	_	_		_		
Mov Cap-1 Maneuver	_	_	1499	-	760	997
Mov Cap-2 Maneuver	_	_	-	_	760	-
Stage 1	_	-			956	_
•	-	-		-	861	
Stage 2	-	-	-	-	001	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		4.4		9.8	
HCM LOS					Α	
MC I /NA - ' NA (		UDL . 4	FDT	EDD	MDI	MOT
Minor Lane/Major Mvmt	ſ	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		886	-	-	1499	-
HCM Lane V/C Ratio		0.159	-	-	0.033	-
HCM Control Delay (s)		9.8	-	-	7.5	-
HCM Lane LOS		Α	-	-	Α	-
HCM 95th %tile Q(veh)		0.6	-	-	0.1	-

Intersection						
Int Delay, s/veh	5.1					
		ED.5	14/51	14/57	NE	NES
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>	7	1	<b>↑</b>	A	
Traffic Vol, veh/h	46	59	87	39	41	64
Future Vol, veh/h	46	59	87	39	41	64
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	420	655	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	50	64	95	42	45	70
	lajor1		Major2		Minor1	
Conflicting Flow All	0	0	114	0	282	50
Stage 1	-	-	-	-	50	-
Stage 2	-	-	-	-	232	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1475	-	708	1018
Stage 1	_	-	-	_	972	-
Stage 2	-	-	_	-	807	-
Platoon blocked, %	_	_		_	301	
Mov Cap-1 Maneuver	_	_	1475	_	663	1018
Mov Cap-1 Maneuver	_		-	_	663	-
Stage 1	_	_	-	_	972	
<u> </u>	-	-	-	-	755	
Stage 2	-	-	-	-	100	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		5.3		9.9	
HCM LOS					A	
					- 1	
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		842	-		1475	-
HCM Lane V/C Ratio		0.136	-	-	0.064	-
HCM Control Delay (s)		9.9	-	-	7.6	-
HCM Lane LOS		Α	-	-	Α	-
HCM 95th %tile Q(veh)		0.5	-	-	0.2	-
., .						

Intersection   Int Delay, s/veh   7.6     Movement   WBL   WBR   NBT   NBR   SBL   SBL
Movement         WBL         WBR         NBT         NBR         SBL         SB           Lane Configurations         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★         ★
Traffic Vol, veh/h
Traffic Vol, veh/h         10         108         6         5         142           Future Vol, veh/h         10         108         6         5         142           Conflicting Peds, #/hr         0         0         0         0         0           Sign Control         Stop         Stop         Free
Future Vol, veh/h Conflicting Peds, #/hr O O O O O O O O O O O O O O O O O O O
Conflicting Peds, #/hr         0         0         0         0         0           Sign Control         Stop         Stop         Free
Sign Control         Stop         Stop         Free         Ro           Veh in Median Storage, #         0         -         0         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -
RT Channelized         - None         - None         - None           Storage Length         0         -         -         -           Veh in Median Storage, #         0         -         0         -         -           Grade, %         0         -         0         -         -         -           Peak Hour Factor         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92 </td
Storage Length         0         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -
Veh in Median Storage, #         0         -         0         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -
Grade, %         0         -         0         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -<
Peak Hour Factor         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92         92
Meavy Vehicles, % 2 2 2 2 2 2   Mwmt Flow
Mount Flow         11         117         7         5         154         1           Major/Minor         Minor1         Major1         Major2           Conflicting Flow All         328         10         0         0         12           Stage 1         10         -         -         -         -         -           Stage 2         318         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -
Moment Flow         11         117         7         5         154         1           Major/Minor         Minor1         Major1         Major2           Conflicting Flow All         328         10         0         0         12           Stage 1         10         -         -         -         -           Stage 2         318         -         -         -         -           Critical Hdwy         6.42         6.22         -         4.12         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -
Major/Minor         Minor1         Major1         Major2           Conflicting Flow All         328         10         0         0         12           Stage 1         10         -         -         -         -           Stage 2         318         -         -         -         -           Critical Hdwy         6.42         6.22         -         4.12           Critical Hdwy Stg 1         5.42         -         -         -           Critical Hdwy Stg 2         5.42         -         -         -           Follow-up Hdwy         3.518         3.318         -         2.218           Pot Cap-1 Maneuver         666         1071         -         1607           Stage 1         1013         -         -         -           Stage 2         738         -         -         -           Platoon blocked, %         -         -         -         -           Mov Cap-1 Maneuver         602         1071         -         1607           Mov Cap-2 Maneuver         602         -         -         -           Stage 1         1013         -         -         -           Stage 2
Conflicting Flow All         328         10         0         0         12           Stage 1         10         -         -         -         -           Stage 2         318         -         -         -         -           Critical Hdwy         6.42         6.22         -         -         4.12           Critical Hdwy Stg 1         5.42         -         -         -         -           Critical Hdwy Stg 2         5.42         -         -         -         -           Follow-up Hdwy         3.518         3.318         -         -         2.218           Pot Cap-1 Maneuver         666         1071         -         1607           Stage 1         1013         -         -         -           Stage 2         738         -         -         -           Platoon blocked, %         -         -         -           Mov Cap-1 Maneuver         602         1071         -         1607           Mov Cap-2 Maneuver         602         -         -         -           Stage 1         1013         -         -         -           Stage 2         667         -         -
Conflicting Flow All         328         10         0         0         12           Stage 1         10         -         -         -         -           Stage 2         318         -         -         -         -           Critical Hdwy         6.42         6.22         -         -         4.12           Critical Hdwy Stg 1         5.42         -         -         -         -           Critical Hdwy Stg 2         5.42         -         -         -         -           Follow-up Hdwy         3.518         3.318         -         -         2.218           Pot Cap-1 Maneuver         666         1071         -         1607           Stage 1         1013         -         -         -           Stage 2         738         -         -         -           Platoon blocked, %         -         -         -           Mov Cap-1 Maneuver         602         1071         -         1607           Mov Cap-2 Maneuver         602         -         -         -           Stage 1         1013         -         -         -           Stage 2         667         -         -
Stage 1       10       -       -       -         Stage 2       318       -       -       -         Critical Hdwy       6.42       6.22       -       4.12         Critical Hdwy Stg 1       5.42       -       -       -         Critical Hdwy Stg 2       5.42       -       -       -         Follow-up Hdwy       3.518       3.318       -       -       2.218         Pot Cap-1 Maneuver       666       1071       -       1607         Stage 1       1013       -       -       -         Platoon blocked, %       -       -       -         Mov Cap-1 Maneuver       602       1071       -       1607         Mov Cap-2 Maneuver       602       -       -       -         Stage 1       1013       -       -       -         Stage 2       667       -       -       -
Stage 2       318       -       -       -         Critical Hdwy       6.42       6.22       -       4.12         Critical Hdwy Stg 1       5.42       -       -       -         Critical Hdwy Stg 2       5.42       -       -       -         Follow-up Hdwy       3.518       3.318       -       2.218         Pot Cap-1 Maneuver       666       1071       -       1607         Stage 1       1013       -       -       -         Stage 2       738       -       -       -         Platoon blocked, %       -       -       -         Mov Cap-1 Maneuver       602       1071       -       1607         Mov Cap-2 Maneuver       602       -       -       -         Stage 1       1013       -       -       -         Stage 2       667       -       -       -
Critical Hdwy 6.42 6.22 - 4.12 Critical Hdwy Stg 1 5.42
Critical Hdwy Stg 1 5.42 Critical Hdwy Stg 2 5.42
Critical Hdwy Stg 2 5.42
Critical Hdwy Stg 2 5.42
Follow-up Hdwy 3.518 3.318 - 2.218  Pot Cap-1 Maneuver 666 1071 - 1607  Stage 1 1013  Stage 2 738  Platoon blocked, %  Mov Cap-1 Maneuver 602 1071 - 1607  Mov Cap-2 Maneuver 602  Stage 1 1013  Stage 2 667
Pot Cap-1 Maneuver 666 1071 1607 Stage 1 1013 Stage 2 738 Platoon blocked, % Mov Cap-1 Maneuver 602 1071 - 1607 Mov Cap-2 Maneuver 602 Stage 1 1013 Stage 2 667
Stage 1       1013       -       -       -         Stage 2       738       -       -       -         Platoon blocked, %       -       -       -         Mov Cap-1 Maneuver       602       1071       -       -       1607         Mov Cap-2 Maneuver       602       -       -       -       -         Stage 1       1013       -       -       -       -         Stage 2       667       -       -       -       -
Stage 2       738       -       -       -         Platoon blocked, %       -       -       -         Mov Cap-1 Maneuver       602       1071       -       -       1607         Mov Cap-2 Maneuver       602       -       -       -       -       -         Stage 1       1013       -       -       -       -       -         Stage 2       667       -       -       -       -       -
Platoon blocked, %  Mov Cap-1 Maneuver 602 1071 - 1607  Mov Cap-2 Maneuver 602  Stage 1 1013  Stage 2 667
Mov Cap-1 Maneuver       602       1071       -       - 1607         Mov Cap-2 Maneuver       602       -       -       -       -         Stage 1       1013       -       -       -       -         Stage 2       667       -       -       -       -
Mov Cap-2 Maneuver 602 Stage 1 1013 Stage 2 667
Stage 1 1013 Stage 2 667
Stage 2 667
Approach WR NR SR
Annroach WR NR CR
Approach VVD ND OD
HCM Control Delay, s 9.1 0 7
HCM LOS A
M. I M. M. C. NDT NDDWDL ( CD. CD.
Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SB
Capacity (veh/h) 1005 1607
HCM Lane V/C Ratio 0.128 0.096
HCM Control Delay (s) 9.1 7.5
HCM Lane LOS A A
HCM 95th %tile Q(veh) 0.4 0.3

Intersection						
Int Delay, s/veh	7.8					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	<b>Y</b>	400	<b>∱</b>	40	404	ની
Traffic Vol, veh/h	10	163	9	12	134	5
Future Vol, veh/h	10	163	9	12	134	5
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	11	177	10	13	146	5
Major/Minor	Minor1	N	/lajor1		Major2	
Conflicting Flow All	314	17	0	0	23	0
Stage 1	17	- 17		-	23	-
Stage 2	297	-	-	•	-	-
	6.42	6.22	_	-	4.12	-
Critical Hdwy	5.42	0.22	-		4.12	-
Critical Hdwy Stg 1		-	-	-		
Critical Hdwy Stg 2	5.42	2 240	-	-	2 240	-
Follow-up Hdwy	3.518		-	-	2.218	-
Pot Cap-1 Maneuver	679	1062	-	-	1592	-
Stage 1	1006	-	-	-	-	-
Stage 2	754	-	-	-	-	-
Platoon blocked, %	0.4=	1000	-	-	4500	-
Mov Cap-1 Maneuver	617	1062	-	-	1592	-
Mov Cap-2 Maneuver	617	_	-	-	-	-
Stage 1	1006	-	-	-	-	-
Stage 2	685	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.3		0		7.2	
HCM LOS	9.5 A		U		1.2	
I IOIVI LOG	Α					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	1019	1592	-
HCM Lane V/C Ratio		-	-	0.185	0.091	-
HCM Control Delay (s)		-	-	9.3	7.5	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh	)	-	-	0.7	0.3	-

# 9: Orange Blossom Road & Revels Road

Intersection						
Int Delay, s/veh	7.1					
	EBL	EBT	WDT	WDD	CDI	SBR
Movement Configurations	EBL		WBT	WBR	SBL	SBK
Lane Configurations	7	4	<b>1</b>		12	7
Traffic Vol, veh/h	7	0	0	4	12	7
Future Vol, veh/h	7	0	0	4	12	7
Conflicting Peds, #/hr	0	0	0	0		O Cton
Sign Control RT Channelized	Free	Free	Free	Free	Stop	Stop
	-	None	- -	None	- 0	None -
Storage Length					0	
Veh in Median Storage		0	0	-		-
Grade, %	-	0	0	-	0	92
Peak Hour Factor	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	8	0	0	4	13	8
Major/Minor I	Major1	N	/lajor2		Minor2	
Conflicting Flow All	4	0	-	0	18	2
Stage 1	-	-	-	-	2	-
Stage 2	-	-	-	-	16	-
Critical Hdwy	4.12	-	-	_	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1618	-	-	-	1000	1082
Stage 1	-	-	-	-	1021	-
Stage 2	-	-	-	-	1007	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1618	-	-	-	995	1082
Mov Cap-2 Maneuver	-	-	-	-	995	-
Stage 1	-	-	_	_	1016	-
Stage 2	-	-	_	_	1007	-
- 1 G <b>-</b>						
Annanah	ED		\A/D		OD	
Approach	EB		WB		SB	
HCM Control Delay, s	7.2		0		8.6	
HCM LOS					Α	
Minor Lane/Major Mvm	ıt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		1618	-	-		1025
HCM Lane V/C Ratio		0.005	-	-	-	0.02
HCM Control Delay (s)		7.2	0	-	-	8.6
HCM Lane LOS		A	A	-	_	A
HCM 95th %tile Q(veh)		0	-	-	-	0.1
		•				<b>J.</b> ,

# 9: Orange Blossom Road & Revels Road

Intersection						
Int Delay, s/veh	5.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	4	1>	WDIX	Y	ODIN
Traffic Vol, veh/h	7	0	0	13	8	7
Future Vol, veh/h	7	0	0	13	8	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-		Stop -	None
Storage Length	_	-	_	-	0	NOHE
Veh in Median Storage,	.# -	0	0	_	0	
Grade, %	, # -	0	0	_	0	-
Peak Hour Factor	92	92	92	92	92	92
	2	2	2	2	2	2
Heavy Vehicles, %						
Mvmt Flow	8	0	0	14	9	8
Major/Minor N	Major1	N	Major2	- 1	Minor2	
Conflicting Flow All	14	0		0	23	7
Stage 1	_	_	_	-	7	-
Stage 2	_	_	-	_	16	-
Critical Hdwy	4.12	_	_	_	6.42	6.22
Critical Hdwy Stg 1	-	_	_	_	5.42	-
Critical Hdwy Stg 2	_	_	_	_	5.42	_
	2.218	_	_	_	3.518	3 318
Pot Cap-1 Maneuver	1604	_	_	_	993	1075
Stage 1	-	_	_	_	1016	-
Stage 2	_	_	_	_	1007	_
Platoon blocked, %		_	_	<u>-</u>	1001	
Mov Cap-1 Maneuver	1604		_	_	988	1075
Mov Cap-1 Maneuver	-	_	_	_	988	1075
Stage 1	-	-	-	-	1011	
_	_	_	_	-	1011	
Stage 2	-	-	-	-	1007	-
Approach	EB		WB		SB	
HCM Control Delay, s	7.3		0		8.6	
HCM LOS					Α	
Minor Lane/Major Mvm	. <del>t</del>	EBL	EBT	WBT	WBR :	SRI n1
Capacity (veh/h)		1604	LDI	VVDI	VVDIC	1027
HCM Lane V/C Ratio		0.005	-	-	-	0.016
		7.3	0	-		8.6
		1.0	U	-	-	0.0
HCM Lang LOS						٨
HCM Lane LOS HCM 95th %tile Q(veh)		A 0	A	-	-	A 0

Appendix L
Intersection Volume Projections

Tgen Enter Exit

Α	M Peak				81	241								1.06	2.00%	10	Backg'd + {Vested} + (Project)
Inters	ection=		SR 19	& CR 48													1
Approa	ch Mvmt	Raw	SF	Adjusted	GR	Redirect	Adj Bg'd	The Reserve	Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total Formula
	L	0	1.06	0	1.20		0						0	-		0	0
EB	Т	0	1.06	0	1.20		0						0			0	0
	R	0	1.06	0	1.20		0						0			0	0
	L	326	1.06	346	1.20		415	32	14		36	7	89	23%		18	<b>522</b> 415 + {89} + (18) = 522
WB	Т	0	1.06	0	1.20		0						0			0	0
	R	216	1.06	229	1.20		275				59		59			0	<b>334</b> 275 + {59} = 334
	L	0	1.06	0	1.20		0						0			0	0
NB	Т	298	1.06	316	1.20		379	21	24		12	14	71		2%	5	<b>455</b> 379 + {71} + (5) = 455
	R	429	1.06	455	1.20		546	82	23		14	20	139		23%	55	<b>740</b> 546 + {139} + (55) = 740
	L	261	1.06	277	1.20		332				81		81			0	<b>413</b> 332 + {81} = 413
SB	Т	92	1.06	98	1.20		118	8	14		33	5	60	2%		2	<b>180</b> 118 + {60} + (2) = 180
	R	0	1.06	0	1.20		0						0			0	0

SF

AGR Years

Legend

Inters	section=		SR 1	9 & Cen	tral	Ave														2
Appro	ach Mvmt	Raw	SF	Adjuste	ed	GR	Redirect	Adj Bg'd	The Reserve	Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total	Formula	
	L	33	1.06		35	1.20		42	62		16			78		10%	24	<b>144</b> 42	+ {78} + (24) = 144	
EB	T	3	1.06		3	1.20		4						0			0	<b>4</b> 4		
	R	9	1.06		10	1.20		12						0			0	<b>12</b> 12		
	L	10	1.06	,	11	1.20		13						0			0	<b>13</b> 13		
WB	Т	1	1.06		1	1.20		1						0			0	<b>1</b> 1		
	R	14	1.06		15	1.20		18		47				47			0	<b>65</b> 18	+ {47} = 65	
	L	11	1.06	,	12	1.20		14						0			0	<b>14</b> 14		
NB	Т	356	1.06	37	77	1.20		452	82		42	26	34	184		15%	36	<b>672</b> 452	+ {184} + (36) = 67	'2
	R	23	1.06	2	24	1.20		29						0			0	<b>29</b> 29		
	L	4	1.06		4	1.20		5		32				32			0	<b>37</b> 5 +	{32} = 37	
SB	Т	404	1.06	42	28	1.20		514	32		24	69	12	137	15%		12	<b>663</b> 514	+ {137} + (12) = 66	3
	R	7	1.06		7	1.20		8	24		9			33	10%		8	<b>49</b> 8 +	{33} + (8) = 49	

ntersection=	Cei	ntra	I Ave & S	. Flori	da Ave												
pproach Mvmt R	aw SF		Adjusted	GR	Redirect A	Adj Bg'd	The Reserve	Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total	Formula
L	1 1.0	06	1	1.20		1						0			0	<b>1</b> 1	
В Т	<b>35</b> 1.0	06	37	1.20		44						0		10%	24	<b>68</b> 44 + (24) = 6	8
R	<b>11</b> 1.0	06	12	1.20		14			3			3			0	<b>17</b> 14 + {3} = 17	
L	1 1.0	06	1	1.20		1			9			9			0	<b>10</b> 1 + {9} = 10	
/B T	<b>18</b> 1.0	06	19	1.20		23						0	10%		8	<b>31</b> 23 + (8) = 31	
R	<b>1</b> 1.0	06	1	1.20		1						0			0	<b>1</b> 1	
L	4 1.0	06	4	1.20		5			5			5			0	<b>10</b> 5 + {5} = 10	
3 T	0 1.0	06	0	1.20		0						0			0	0	
R	3 1.0	06	3	1.20		4			16			16			0	<b>20</b> 4 + {16} = 20	
L	0 1.0	06	0	1.20		0						0			0	0	
3 T	0 1.0	06	0	1.20		0						0			0	0	
R	0 1.0	06	0	1.20		0						0			0	0	

Intersection=	SR 19	& Revels	Rd												
Approach Mvmt R	aw SF	Adjusted	GR	Redirect Adj	g'd The Reser	e Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total F	ormula
L	2 1.06	2	1.20	;	3					3		15%	36	41 2 + {3} + (36) =	= 41
EB T	0 1.06	0	1.20	(	)					0			0	0	
R	5 1.06	5	1.20	(	30					30		35%	84	<b>120</b> 6 + {30} + (84)	= 120
L	5 1.06	5	1.20	(	i	37			81	118			0	<b>124</b> 6 + {118} = 12	4
WB T	0 1.06	0	1.20		)					0			0	0	
R	4 1.06	4	1.20		i				48	48			0	<b>53</b> 5 + {48} = 53	
L	3 1.06	3	1.20		12					12	35%		28	44 4 + {12} + (28)	= 44
NB T	306 1.06	324	1.20	3	9 67			26		93	10%		8	490 389 + {93} + (8	3) = 490
R	<b>12</b> 1.06	13	1.20	1	6	22			28	50			0	<b>66</b> 16 + {50} = 66	
L	3 1.06	3	1.20		,				17	17			0	<b>21</b> 4 + {17} = 21	
SB T	<b>410</b> 1.06	435	1.20	5	2 175			69		244		10%	24	<b>790</b> 522 + {244} +	(24) = 790
R	0 1.06	0	1.20		2					2	15%		12	<b>14</b> {2} + (12) = 14	

Inter	section=		SR 1	9 & CR 455	;													4
Appro	ach Mvmt Ra	aw	SF	Adjusted	GR	Redirect Ad	j Bg'd	The Reserve Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total	Formula	
	L	0	1.00	0	1.20		0					0			0	0		
EB	Т	0	1.00	0	1.20		0					0			0	0		
	R	0	1.00	0	1.20		0					0			0	0		
	L	65	1.00	65	1.20		78					0			0	<b>78</b> 78		
WB	Т	0	1.00	0	1.20		0					0			0	0		
	R	43	1.00	43	1.20		52	16		5	7	28	10%		8	88 52 + {28} +	(8) = 88	
	L	0	1.00	0	1.20		0					0			0	0		
NB	Т	394	1.00	394	1.20		473	55		21	19	95	35%		28	<b>596</b> 473 + {95}	+ (28) = 596	
	R	111	1.00	111	1.20		133					0			0	<b>133</b> 133		
	L	70	1.00	70	1.20		84	41		14	20	75		10%	24	183 84 + {75} +	(24) = 183	
SB	Т	492	1.00	492	1.20		590	144		55	54	253		35%	84	927 590 + {253}	+ (84) = 927	
	R	0	1.00	0	1.20		0					0			0	0		

Counts on 1/24/2023

Inters	ection=		Interc	onnect Rd	& Spii	ne Rd (Pro	posed)													<b>6</b>
Approa	ch Mvmt R	aw :	SF	Adjusted	GR	Redirect	Adj Bg'd	The Reserve Wh	nisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total		Formula	
	L						0									0	0			
EB	T						0									0	0			
	R						0									0	0			
	L						0									0	0			
WB	Т						0									0	0			
	R						25							10%		8	33	25 + (8) = 33	3	
	L						0									0	0			
NB	T						20									51	71	20 + (51) = 7	71	
	R						0									0	0			
	L						20								10%	24	44	20 + (24) = 4	14	
SB	T						25									16	41	25 + (16) = 4	11	
	R						0									0	0			

Inters	ection=		Numl	ber 2 Rd &	Spine	Road / No	rth Acce	ss										
Appro	ach Mvmt	Raw	SF	Adjusted	GR	Redirect	Adj Bg'd	The Reserve	Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total	Formula
	L						0									0	0	
EB	Т						59						3			0	<b>62</b> 59 + {3} =	62
	R						15							15%		11	<b>26</b> 15 + (11) =	= 26
	L						30							20%		16	<b>46</b> 30 + (16) =	= 46
WB	Т						28						5			0	<b>33</b> 28 + {5} =	33
	R						0									0	0	
	L						15								15%	37	<b>52</b> 15 + (37) =	= 52
NB	Т						0									0	0	
	R						30								20%	48	<b>78</b> 30 + (48) =	= 78
	L						0									0	0	
SB	Т						0									0	0	
	R						0									0	0	

Inters	ection=		Revel	s Rd & Spi	ne Rd	/ Propose	d											8
Approa	ch Mvmt	Raw	SF	Adjusted	GR	Redirect	Adj Bg'd	The Reserve Whisp.	Hills Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total	Formula	
	L						0								0	0		
EB	Т						0								0	0		
	R						0								0	0		
	L						3							3%	7	<b>10</b> 3 + (7) = 10		
WB	Т						0								0	0		
	R						62						25%		46	108 62 + (46) =	108	
	L						0								0	0		
NB	Т						4						2%		2	<b>6</b> 4 + (2) = 6		
	R						3						3%		2	<b>5</b> $3 + (2) = 5$		
	L						74							25%	68	142 74 + (68) =	142	
SB	Т						4							2%	5	<b>9</b> 4 + (5) = 9		
	R						0								0	0		

Inters	ection=	Re	evels Rd & Ora	ange B	Blossom Rd / South	n Access									9
Approa	ch Mvmt Ra	w SF	Adjusted	GR	Redirect Adj Bg'd	The Reserve Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total	Formula
	L				7								0	<b>7</b> 7	
EB	T				0								0	0	
	R				0								0	0	
	L				0								0	0	
WB	T				0								0	0	
	R				0						5%		4	<b>4</b> (4)	
	L				0								0	0	
NB	T				0								0	0	
	R				0								0	0	
	L				0							5%	12	<b>12</b> (12)	
SB	T				0								0	0	
	R				7								0	<b>7</b> 7	

#### Project No. 23017 Mission Rise

nters	section=	=	SR 1	9 & CR 4	8													
Appro	ach Mvmt	Raw	SF	Adjuste	d	GR	Redirect	Adj Bg'd	The Reserve	Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total Formula
	L	C	1.06	, (	0 1	1.20		0						0			0	0
ΞB	Т	C	1.06	;	0 1	1.20		0						0			0	0
	R	C	1.06	;	0 1	1.20		0						0			0	0
	L	409	1.06	43	4 1	1.20		521	92	23		25	24	164	23%		66	<b>751</b> 521 + {164} + (66) = 751
NΒ	Т	C	1.06	;	0 1	1.20		0						0			0	0
	R	301	1.06	319	9 1	1.20		383				100		100			0	<b>483</b> 383 + {100} = 483
	L	C	1.06	, (	0 1	1.20		0						0			0	0
NΒ	Т	68	1.06	7:	2 1	1.20		86	15	14		37	9	75		2%	3	<b>164</b> 86 + {75} + (3) = 164
	R	333	1.06	35	3 1	1.20		424	58	14		39	14	125		23%	39	<b>588</b> 424 + {125} + (39) = 588
	L	287	1.06	304	4 1	1.20		365				86		86			0	<b>451</b> 365 + {86} = 451
SB	Т	79	1.06	84	4 1	1.20		101	23	24		24	16	87	2%		6	<b>194</b> 101 + {87} + (6) = 194
	R	C	1.06	;	0 1	1.20		0						0			0	0

Intersection=		SR	₹ 19	& Central	Ave													
Approach Mvmt	Raw	SF		Adjusted	GR	Redirect	Adj Bg'd	The Reserve	Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total	Formula
L	3	0 1.	.06	32	1.20		38	44		9			53		10%	17	<b>108</b> 38 +	(53) + (17) = 108
EB T	1	1 1.	.06	12	1.20		14						0			0	<b>14</b> 14	
R	1:	2 1.	.06	13	1.20		16						0			0	<b>16</b> 16	
L	1	6 1.	.06	17	1.20		20						0			0	<b>20</b> 20	
WB T	:	3 1.	.06	3	1.20		4						0			0	<b>4</b> 4	
R	1	3 1.	.06	14	1.20		17		32				32			0	<b>49</b> 17 +	{32} = 49
L	1:	5 1.	.06	16	1.20		19						0			0	<b>19</b> 19	
NB T	34	2 1.	.06	363	1.20		436	58		24	76	23	181		15%	25	<b>642</b> 436 +	· {181} + (25) = 642
R	2	0 1.	.06	21	1.20		25						0			0	<b>25</b> 25	
L	1:	5 1.	.06	16	1.20		19		47				47			0	<b>66</b> 19 +	{47} = 66
SB T	40	8 1.	.06	432	1.20		518	92		42	49	40	223	15%		43	<b>784</b> 518 +	{223} + (43) = 784
R	3	8 1.	.06	40	1.20		48	69		16			85	10%		29	<b>162</b> 48 +	(85) + (29) = 162

Inters	ection=		Centr	al Ave & S	. Flori	da Ave										3
Approa	ch Mvmt R	law	SF	Adjusted	GR	Redirect Adj Bg'd	The Reserve Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total	Formula
	L	0	1.06	0	1.20	0					0			0	0	
EB	T	27	1.06	29	1.20	35					0		10%	17	<b>52</b> 35 + (17) = 5	2
	R	5	1.06	5	1.20	6		5			5			0	<b>11</b> 6 + {5} = 11	
	L	16	1.06	17	1.20	20		16			16			0	<b>36</b> 20 + {16} = 3	6
WB	Т	24	1.06	25	1.20	30					0	10%		29	<b>59</b> 30 + (29) = 59	9
	R	5	1.06	5	1.20	6					0			0	<b>6</b> 6	
	L	5	1.06	5	1.20	6		3			3			0	<b>9</b> 6 + {3} = 9	
NB	Т	1	1.06	1	1.20	1					0			0	<b>1</b> 1	
	R	19	1.06	20	1.20	24		9			9			0	<b>33</b> 24 + {9} = 33	
	L	1	1.06	1	1.20	1					0			0	<b>1</b> 1	
SB	T	0	1.06	0	1.20	0					0			0	0	
	R	0	1.06	0	1.20	0					0			0	0	

Intersection=	SR 19	& Revels	Rd												
Approach Mvmt Ra	aw SF	Adjusted	GR	Redirect A	Adj Bg'd	The Reserve	Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total Formula
L	3 1.06	3	1.20		4	1					1		15%	25	<b>30</b> 4 + {1} + (25) = 30
EB T	<b>1</b> 1.06	1	1.20		1						0			0	<b>1</b> 1
R	4 1.06	4	1.20		5	21					21		35%	57	<b>83</b> 5 + {21} + (57) = 83
L	8 1.06	8	1.20		10		22			56	78			0	<b>88</b> 10 + {78} = 88
WB T	0 1.06	0	1.20		0						0			0	0
R	3 1.06	3	1.20		4					32	32			0	<b>36</b> 4 + {32} = 36
L	<b>1</b> 1.06	1	1.20		1	35					35	35%		99	<b>135</b> 1 + {35} + (99) = 135
NB T	<b>351</b> 1.06	372	1.20		446	194			76		270	10%		28	<b>744</b> 446 + {270} + (28) = 74
R	<b>11</b> 1.06	12	1.20		14		37			95	132			0	<b>146</b> 14 + {132} = 146
L	7 1.06	7	1.20		8					56	56			0	<b>64</b> 8 + {56} = 64
SB T	<b>324</b> 1.06	343	1.20		412	124			49		173		10%	17	<b>602</b> 412 + {173} + (17) = 60
R	0 1.06	0	1.20		0	2					2	15%		43	<b>45</b> {2} + (43) = 45

Inter	section=		SR 19	8 CR 455													
Appro	ach Mvmt Ra	w	SF	Adjusted	GR	Redirect Adj Bg	d The Reserve	Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total	Formula
	L	0	1.00	0	1.20	0						0			0	0	
EB	Т	0	1.00	0	1.20	0						0			0	0	
	R	0	1.00	0	1.20	0						0			0	0	
	L	83	1.00	83	1.20	100						0			0	<b>100</b> 100	
WB	Т	0	1.00	0	1.20	0						0			0	0	
	R	55	1.00	55	1.20	66	46			15	24	85	10%		28	<b>179</b> 66 +	(85) + (28) = 179
	L	0	1.00	0	1.20	0						0			0	0	
NB	Т	476	1.00	476	1.20	571	161			61	64	286	35%		99	<b>956</b> 571 +	- {286} + (99) = 956
	R	92	1.00	92	1.20	110						0			0	<b>110</b> 110	
	L	50	1.00	50	1.20	60	29			10	14	53		10%	17	<b>130</b> 60 +	{53} + (17) = 130
SB	Т	433	1.00	433	1.20	520	102			39	37	178		35%	58	<b>756</b> 520 +	- {178} + (58) = 756
	R	0	1.00	0	1.20	0						0			0	0	

Counts on 1/24/2023

Inters	ection=	I	nterco	nnect Rd	& Spir	ne Rd (Pro	posed)													<b>6</b>
Approa	ch Mvmt Ra	aw S	SF .	Adjusted	GR	Redirect	Adj Bg'd	The Reserve \	Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total	F	ormula	
	L						0									0	0			
EB	T						0									0	0			
	R						0									0	0			
	L						0									0	0			
WB	T						0									0	0			
	R						20							10%		28	48	20 + (28) = 48		
	L						0									0	0			
NB	T						25									36	61	25 + (36) = 61		
	R						0									0	0			
	L						25								10%	17	42	25 + (17) = 42		
SB	T						20									61	81	20 + (61) = 81		
	R						0									0	0			

Inters	section=		Numl	ber 2 Rd & S	Spine	Road / No	rth Acce	ss										
Appro	ach Mvmt	Raw	SF	Adjusted	GR	Redirect	Adj Bg'd	The Reserve	Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total	Formula
	L						0									0	0	
EB	Т						41						5			0	<b>46</b> 41 + {5} = 4	ô
	R						15							15%		44	<b>59</b> 15 + (44) =	59
	L						30							20%		57	<b>87</b> 30 + (57) = 3	37
٧B	Т						36						3			0	<b>39</b> 36 + {3} = 3	9
	R						0									0	0	
	L						15								15%	26	<b>41</b> 15 + (26) = -	41
ΙB	Т						0									0	0	
	R						30								20%	34	<b>64</b> 30 + (34) =	64
	L						0									0	0	
В	Т						0									0	0	
	R						0									0	0	

Intersection= Revels Rd & Sp			ne Rd	/ Propose	ed													
Appro	ach Mvmt	Raw	SF	Adjusted	GR	Redirect	Adj Bg'd	The Reserve	Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total	Formula
	L						0									0	0	
EB	T						0									0	0	
	R						0									0	0	
	L						4								3%	6	10 4 + (	6) = 10
WB	Т						0									0	0	
	R						74							25%		89	163 74 +	(89) = 163
	L						0									0	0	
NB	Т						3							2%		6	<b>9</b> 3 + (	6) = 9
	R						4							3%		8	<b>12</b> 4 + (	8) = 12
	L						62								25%	72	<b>134</b> 62 +	(72) = 134
SB	Т						3								2%	2	<b>5</b> 3 + (2	2) = 5
	R						0									0	0	

Inters	Intersection=		Revels Rd & Orange Blossom Rd / South Access													
Approa	ch Mvmt Ra	w SF	Adjusted	GR	Redirect Adj Bg'd	The Reserve Whisp. Hills	Talichet	Lake Hills	Watermark	Vested	%Proj Ent	%Proj Ext	Project	Total	Formula	
	L				7								0	<b>7</b> 7		
EB	T				0								0	0		
	R				0								0	0		
	L				0								0	0		
WB	T				0								0	0		
	R				0						5%		13	<b>13</b> (13)		
	L				0								0	0		
NB	T				0								0	0		
	R				0								0	0		
	L				0							5%	8	8 (8)		
SB	Т				0								0	0		
	R				7								0	<b>7</b> 7		

Item 1.

**Appendix M**Background Conditions / Buildout Conditions with Mitigation

1: SR 19 & CR 4	8
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	1	*	<b>†</b>	1	-	ļ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b>	7	*	<u> </u>
Traffic Volume (veh/h)	504	334	450	685	413	178
Future Volume (veh/h)	504	334	450	685	413	178
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	-
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No	1.00	No	1.00	1.00	No
Adj Sat Flow, veh/h/ln	1752	1589	1767	1811	1737	1811
	520				426	184
Adj Flow Rate, veh/h		203	464	0 07		
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	10	21	9	6	11	6
Cap, veh/h	386	312	695		506	1139
Arrive On Green	0.23	0.23	0.39	0.00	0.17	0.63
Sat Flow, veh/h	1668	1346	1767	1535	1654	1811
Grp Volume(v), veh/h	520	203	464	0	426	184
Grp Sat Flow(s),veh/h/ln	1668	1346	1767	1535	1654	1811
Q Serve(g_s), s	22.7	13.4	21.2	0.0	14.2	4.1
Cycle Q Clear(g_c), s	22.7	13.4	21.2	0.0	14.2	4.1
Prop In Lane	1.00	1.00	£1.£	1.00	1.00	7.1
Lane Grp Cap(c), veh/h	386	312	695	1.00	506	1139
V/C Ratio(X)	1.35	0.65	0.67		0.84	0.16
. ,						
Avail Cap(c_a), veh/h	386	312	695	1.00	539	1139
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	0.00	1.00	1.00
Uniform Delay (d), s/veh	37.7	34.1	24.4	0.0	16.8	7.5
Incr Delay (d2), s/veh	172.2	4.7	5.0	0.0	11.0	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	41.4	8.1	14.4	0.0	10.4	2.7
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	209.9	38.8	29.5	0.0	27.8	7.8
LnGrp LOS	F	D	С		C	A
Approach Vol, veh/h	723		464	А		610
Approach Delay, s/veh	161.9		29.5	- 1		21.8
Approach LOS	101.9 F		29.5 C			Z 1.0
Appluauli LOO			U			U
Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	23.0	45.0		30.0		68.0
Change Period (Y+Rc), s	6.5	6.4		7.3		6.4
Max Green Setting (Gmax), s	18.5	38.6		22.7		38.6
Max Q Clear Time (g_c+l1), s	16.2	23.2		24.7		6.1
Green Ext Time (p_c), s	0.4	2.5		0.0		1.0
	0.4	2.0		0.0		1.0
Intersection Summary			00.1			
HCM 6th Ctrl Delay			80.1			
HCM 6th LOS			F			
Notes						

## 1: SR 19 & CR 48

Cane Configurations		1	•	<b>†</b>	1	-	<b>↓</b>
Cane Configurations	Movement	WBL	WBR	NBT	NBR	SBL	SBT
Traffic Volume (veh/h) 685 483 161 549 451 188 □ Tuture Volume (veh/h) 685 483 161 549 451 188 □ Tuture Volume (veh/h) 685 483 161 549 451 188 □ Tuture Volume (veh/h) 685 483 161 549 451 188 □ Tuture Volume (veh/h) 685 483 161 549 451 188 □ Tuture Volume (veh/h) 685 483 161 549 451 188 □ Tuture Volume (veh/h) 685 483 161 549 451 188 □ Tuture Volume (veh/h) 100 1.00 1.00 1.00 □ Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 □ Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 □ Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 □ Adj Sat Flow, veh/h/ln 1752 1589 1767 1811 1737 1811 □ Adj Flow Rate, veh/h 706 302 166 0 465 194 □ Parkine Veh, % 10 21 9 6 11 6 □ Parkine Veh, % 10 21 9 6 11 6 □ Parkine On Green 0.23 0.39 0.00 0.18 0.63 □ Sat Flow, veh/h 1668 1346 1767 1535 1654 1811 □ Sign Volume(v), veh/h 706 302 166 0 465 194 □ Sign Sat Flow(s), veh/h/ln 1668 1346 1767 1535 1654 1811 □ Sign Volume(v), veh/h 706 302 166 0 465 194 □ Sign Sat Flow(s), veh/h/ln 1668 1346 1767 1535 1654 1811 □ Sign Volume(v), veh/h 706 302 166 0 465 194 □ Cycle Q Clear(g_c), s 22.7 22.2 6.3 0.0 16.0 4.4 □ Cycle Q Clear(g_c), s 22.7 22.2 6.3 0.0 16.0 4.4 □ Cycle Q Clear(g_c), s 22.7 22.2 6.3 0.0 16.0 4.4 □ Cycle Q Clear(g_c), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap(c_a), veh/h 380 307 685 740 1149 □ Avail Cap							
Future Volume (veh/h) 685 483 161 549 451 188 nitial Q (Qb), veh 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0							
Initial Q (Qb), veh							
Ped-Bike Adj(A_pbT)	. ,						
Parking Bus, Adj  No							•
Work Zone On Approach         No         No         No           Adj Sat Flow, veh/h/In         1752         1589         1767         1811         1737         1811           Adj Flow Rate, veh/h         706         302         166         0         465         194           Peak Hour Factor         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.97         0.9				1 00			1 00
Adj Sat Flow, veh/h/ln  Adj Flow Rate, veh/h  Adj Flow Rate  Adj Flow Rate, veh/h  Adj Flow Rate  Adj Flow Rate, veh/h  Adj Flow Rate  Adj Flow Rate, veh/h  Adj Flow Rate  Adj			1.00		1.00	1.00	
Adj Flow Rate, veh/h Peak Hour Factor  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.97  0.00  0.118  0.63  0.65  1811  0.66  0.465  1811  0.2 Serve(g_s), s. 22.7  22.2 6.3  0.0 0.16.0  4.4  0.663  0.17  0.100  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1.00  1			1580		1811	1737	
Peak Hour Factor 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97	•						
Percent Heavy Veh, % 10 21 9 6 11 6 Cap, veh/h 380 307 685 740 1149 Arrive On Green 0.23 0.23 0.39 0.00 0.18 0.63 Sat Flow, veh/h 1668 1346 1767 1535 1654 1811 Grp Volume(v), veh/h 706 302 166 0 465 194 Grp Sat Flow(s),veh/h/ln 1668 1346 1767 1535 1654 1811 Crp Volume(v), veh/h 706 302 166 0 465 194 Grp Sat Flow(s),veh/h/ln 1668 1346 1767 1535 1654 1811 Crp Volume(v), veh/h 1668 1346 1767 1535 1654 1811 Crp Volume(v), veh/h 1668 1346 1767 1535 1654 1811 Crp Volume(v), veh/h 1668 1346 1767 1535 1654 1811 Crp Volume(v), veh/h 1668 1346 1767 1535 1654 1811 Crp Volume(v), veh/h 1668 1346 1767 1535 1654 1811 Crp Volume(v), veh/h 1668 1346 1767 1535 1654 1811 Crp Volume(v), veh/h 160 1.00 1.00 1.00 1.00 1.00 Crop Volume(v), veh/h 380 307 685 740 1149 Crp Volume(v), veh/h 380 307 685 740 1149 Crp Volume(v), veh/h 380 307 685 747 1149 Crp Volume(v), veh/h 380 307 685 740 1149 Crp Volume(v), veh/h 380 307 685 740 1149 Crp Volume(v), veh/h 380 307 685 740 1149 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.5 2.9 Crp Volume(v), veh/h 79.4 16.3 4.8 0.0 9.0 0.0 0.0 0.0 Crp Volume(v), veh/h 79.4 16.3 4							
Cap, veh/h Arrive On Green O.23 O.23 O.23 O.39 O.00 O.18 O.63 Sat Flow, veh/h 1668 1346 1767 1535 1654 1811 Grp Volume(v), veh/h 706 302 166 O 465 194 Grp Sat Flow(s), veh/h/ln 1668 1346 1767 1535 1654 1811 O.2 Serve(g_s), s 22.7 22.2 6.3 O.0 16.0 4.4 O.2 Clear(g_c), s 22.7 22.2 6.3 O.0 16.0 4.4 O.2 Clear(g_c), veh/h 380 307 685 740 1149 O.2 Clear(g_c), veh/h 380 307 685 740 1149 O.2 Clear(g_c), veh/h 380 307 685 740 1149 O.3 Clear(g_c), veh/h 380 307 O.3 Clear(g_c), veh/h 380 O.3 Clear(g_c), veh/h O.0 Clear(g_c), veh/h O.							
Arrive On Green 0.23 0.23 0.39 0.00 0.18 0.63 Sat Flow, veh/h 1668 1346 1767 1535 1654 1811 Grp Volume(v), veh/h 706 302 166 0 465 194 Grp Sat Flow(s), veh/h/ln 1668 1346 1767 1535 1654 1811 Q Serve(g_s), s 22.7 22.2 6.3 0.0 16.0 4.4 Cycle Q Clear(g_c), s 22.7 22.2 6.3 0.0 16.0 4.4 Cycle Q Clear(g_c), s 22.7 22.2 6.3 0.0 16.0 4.4 Cycle Q Clear(g_c), veh/h 380 307 685 740 1149 (VC Ratio(X) 1.86 0.98 0.24 0.63 0.17 Avail Cap(c_a), veh/h 380 307 685 747 1149 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 Lystream Filter(l) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Jufform Delay (d), s/veh 38.4 38.3 20.6 0.0 12.3 7.4 ncr Delay (d2), s/veh 395.5 46.9 0.8 0.0 1.7 0.3 nitial Q Delay(d3),s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	•				Ö		
Sat Flow, veh/h         1668         1346         1767         1535         1654         1811           Grp Volume(v), veh/h         706         302         166         0         465         194           Grp Sat Flow(s), veh/h/In         1668         1346         1767         1535         1654         1811           Q Serve(g_s), s         22.7         22.2         6.3         0.0         16.0         4.4           Cycle Q Clear(g_c), s         22.7         22.2         6.3         0.0         16.0         4.4           Prop In Lane         1.00         1.00         1.00         1.00         1.00         1.00           Ale Ratic (X)         1.86         0.98         0.24         0.63         0.17           Avail Cap(c_a), veh/h         380         307         685         747         1149           HCM Platoon Ratio         1.00         1.00         1.00         1.00         1.00         1.00         1.00           Jusiform Delay (d), s/veh         38.4         38.3         20.6         0.0         12.3         7.4           nor Delay (d2), s/veh         395.5         46.9         0.8         0.0         1.7         0.3           niti					0.00		
Grp Volume(v), veh/h Grp Sat Flow(s), veh/h/ln Grp Sat Flow(s), veh/h Grp Sat Flow(s), veh/h Grop In Lane Grp Cap(c), s Grop Sat							
Grip Sat Flow(s),veh/h/ln         1668         1346         1767         1535         1654         1811           Q Serve(g_s), s         22.7         22.2         6.3         0.0         16.0         4.4           Cycle Q Clear(g_c), s         22.7         22.2         6.3         0.0         16.0         4.4           Prop In Lane         1.00         1.00         1.00         1.00         1.00         1.00           Lane Grp Cap(c), veh/h         380         307         685         740         1149           V/C Ratio(X)         1.86         0.98         0.24         0.63         0.17           Avail Cap(c_a), veh/h         380         307         685         747         1149           HCM Platoon Ratio         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00         1.00					1535		
Q Serve(g_s), s	Grp Volume(v), veh/h	706	302	166	0	465	194
Q Serve(g_s), s	Grp Sat Flow(s),veh/h/ln	1668	1346	1767	1535	1654	1811
Cycle Q Clear(g_c), s							
Prop In Lane 1.00 1.00 1.00 1.00 1.00  Lane Grp Cap(c), veh/h 380 307 685 740 1149  V/C Ratio(X) 1.86 0.98 0.24 0.63 0.17  Avail Cap(c_a), veh/h 380 307 685 747 1149  HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00  Upstream Filter(I) 1.00 1.00 1.00 0.00 1.00 1.00  Upstream Filter(I) 1.00 1.00 1.00 0.00 1.00 1.00  Upstream Filter(I) 38.4 38.3 20.6 0.0 12.3 7.4  Incr Delay (d2), s/veh 395.5 46.9 0.8 0.0 1.7 0.3  Initial Q Delay(d3),s/veh 0.0 0.0 0.0 0.0 0.0 0.0  Wile BackOfQ(95%),veh/ln 79.4 16.3 4.8 0.0 9.5 2.9  Unsig. Movement Delay, s/veh  LnGrp Delay(d),s/veh 433.9 85.1 21.5 0.0 13.9 7.8  LnGrp LOS F F C B A  Approach Vol, veh/h 1008 166 A 659  Approach LOS F C B A  Phs Duration (G+Y+Rc), s 24.6 45.0 30.0 69.6  Change Period (Y+Rc), s 6.5 6.4 7.3 6.4  Max Green Setting (Gmax), s 18.5 38.6 22.7 38.6  Max Q Clear Time (g_c+I1), s 18.0 8.3 24.7 6.4  Green Ext Time (p_c), s 0.1 0.9 0.0 1.1  Intersection Summary  HCM 6th Ctrl Delay							
Lane Grp Cap(c), veh/h  Avail Cap(c_a), veh/h  Avail Cap(c_a), veh/h  Avail Cap(c_a), veh/h  Avail Cap(c_a), veh/h  Born Delay (d), s/veh  Born Delay (d2), s/veh  Born Delay (d3), s/veh  Born Delay				J.0			
Avail Cap(c_a), veh/h       380       307       685       747       1149         HCM Platoon Ratio       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00       1.00				685	1.00		1149
Avail Cap(c_a), veh/h HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0							
HCM Platoon Ratio							
Digital Content of the Circle of the Circl					1.00		
Jniform Delay (d), s/veh       38.4       38.3       20.6       0.0       12.3       7.4         ncr Delay (d2), s/veh       395.5       46.9       0.8       0.0       1.7       0.3         nitial Q Delay(d3), s/veh       0.0       0.0       0.0       0.0       0.0       0.0         %ile BackOfQ(95%), veh/ln       79.4       16.3       4.8       0.0       9.5       2.9         Jnsig. Movement Delay, s/veh       2.9       2.9       2.9       2.9       2.9         Jnsig. Movement Delay, s/veh       433.9       85.1       21.5       0.0       13.9       7.8         LnGrp LOS       F       F       C       B       A         Approach Vol, veh/h       1008       166       A       659         Approach Delay, s/veh       329.4       21.5       12.1         Approach LOS       F       C       B         Finner - Assigned Phs       1       2       4       6         Phs Duration (G+Y+Rc), s       6.5       6.4       7.3       6.4         Max Green Setting (Gmax), s       18.5       38.6       22.7       38.6         Max Q Clear Time (g_c+l1), s       18.0       8.3       24.7 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>							
ncr Delay (d2), s/veh 395.5 46.9 0.8 0.0 1.7 0.3  nitial Q Delay(d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	,						
Initial Q Delay(d3),s/veh         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Wile BackOfQ(95%),veh/In       79.4       16.3       4.8       0.0       9.5       2.9         Unsig. Movement Delay, s/veh       2.9       2.9       2.9       2.9         Unsig. Movement Delay, s/veh       433.9       85.1       21.5       0.0       13.9       7.8         Unsig. Movement Delay, s/veh       433.9       85.1       21.5       0.0       13.9       7.8         Approach LOS       F       F       C       B       A         Approach LOS       F       C       B       A         Chapproach LOS       F       C       B       B         Timer - Assigned Phs       1       2       4       6         Phs Duration (G+Y+Rc), s       24.6       45.0       30.0       69.6         Change Period (Y+Rc), s       6.5       6.4       7.3       6.4         Max Green Setting (Gmax), s       18.5       38.6       22.7       38.6         Max Q Clear Time (g_c+I1), s       18.0       8.3       24.7       6.4         Green Ext Time (p_c), s       0.1       0.9       0.0       1.1         Intersection Summary       187.5       187.5       187.5       187.5       187.5       187.5 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>							
Unsig. Movement Delay, s/veh  LnGrp Delay(d),s/veh 433.9 85.1 21.5 0.0 13.9 7.8  LnGrp LOS F F C B A  Approach Vol, veh/h 1008 166 A 659  Approach Delay, s/veh 329.4 21.5 12.1  Approach LOS F C B  Timer - Assigned Phs 1 2 4 6  Phs Duration (G+Y+Rc), s 24.6 45.0 30.0 69.6  Change Period (Y+Rc), s 6.5 6.4 7.3 6.4  Max Green Setting (Gmax), s 18.5 38.6 22.7 38.6  Max Q Clear Time (g_c+I1), s 18.0 8.3 24.7 6.4  Green Ext Time (p_c), s 0.1 0.9 0.0 1.1  Intersection Summary  HCM 6th Ctrl Delay 187.5  HCM 6th LOS F							
Approach Vol, veh/h Approach LOS A CLA Approach LOS A C	%ile BackOfQ(95%),veh/ln		16.3	4.8	0.0	9.5	2.9
Approach Vol, veh/h 1008 166 A 659 Approach Delay, s/veh 329.4 21.5 12.1 Approach LOS F C B  Timer - Assigned Phs 1 2 4 6 Phs Duration (G+Y+Rc), s 24.6 45.0 30.0 69.6 Change Period (Y+Rc), s 6.5 6.4 7.3 6.4 Max Green Setting (Gmax), s 18.5 38.6 22.7 38.6 Max Q Clear Time (g_c+l1), s 18.0 8.3 24.7 6.4 Green Ext Time (p_c), s 0.1 0.9 0.0 1.1  Intersection Summary HCM 6th Ctrl Delay 187.5 HCM 6th LOS F	Unsig. Movement Delay, s/veh	1					
Approach Vol, veh/h 1008 166 A 659 Approach Delay, s/veh 329.4 21.5 12.1 Approach LOS F C B  Timer - Assigned Phs 1 2 4 6 Phs Duration (G+Y+Rc), s 24.6 45.0 30.0 69.6 Change Period (Y+Rc), s 6.5 6.4 7.3 6.4 Max Green Setting (Gmax), s 18.5 38.6 22.7 38.6 Max Q Clear Time (g_c+l1), s 18.0 8.3 24.7 6.4 Green Ext Time (p_c), s 0.1 0.9 0.0 1.1  Intersection Summary HCM 6th Ctrl Delay 187.5 HCM 6th LOS F	LnGrp Delay(d),s/veh	433.9	85.1	21.5	0.0	13.9	7.8
Approach Vol, veh/h 1008 166 A 659 Approach Delay, s/veh 329.4 21.5 12.1 Approach LOS F C B  Timer - Assigned Phs 1 2 4 6 Phs Duration (G+Y+Rc), s 24.6 45.0 30.0 69.6 Change Period (Y+Rc), s 6.5 6.4 7.3 6.4 Max Green Setting (Gmax), s 18.5 38.6 22.7 38.6 Max Q Clear Time (g_c+l1), s 18.0 8.3 24.7 6.4 Green Ext Time (p_c), s 0.1 0.9 0.0 1.1  Intersection Summary HCM 6th Ctrl Delay 187.5 HCM 6th LOS F	LnGrp LOS	F	F	С		В	Α
Approach Delay, s/veh 329.4 21.5 12.1 Approach LOS F C B  Timer - Assigned Phs 1 2 4 6  Phs Duration (G+Y+Rc), s 24.6 45.0 30.0 69.6  Change Period (Y+Rc), s 6.5 6.4 7.3 6.4  Max Green Setting (Gmax), s 18.5 38.6 22.7 38.6  Max Q Clear Time (g_c+l1), s 18.0 8.3 24.7 6.4  Green Ext Time (p_c), s 0.1 0.9 0.0 1.1  Intersection Summary  HCM 6th Ctrl Delay 187.5  HCM 6th LOS F					Α		
Approach LOS F C B  Timer - Assigned Phs 1 2 4 6  Phs Duration (G+Y+Rc), s 24.6 45.0 30.0 69.6  Change Period (Y+Rc), s 6.5 6.4 7.3 6.4  Max Green Setting (Gmax), s 18.5 38.6 22.7 38.6  Max Q Clear Time (g_c+I1), s 18.0 8.3 24.7 6.4  Green Ext Time (p_c), s 0.1 0.9 0.0 1.1  Intersection Summary  HCM 6th Ctrl Delay 187.5  HCM 6th LOS F	• •						
Timer - Assigned Phs         1         2         4         6           Phs Duration (G+Y+Rc), s         24.6         45.0         30.0         69.6           Change Period (Y+Rc), s         6.5         6.4         7.3         6.4           Max Green Setting (Gmax), s         18.5         38.6         22.7         38.6           Max Q Clear Time (g_c+l1), s         18.0         8.3         24.7         6.4           Green Ext Time (p_c), s         0.1         0.9         0.0         1.1           Intersection Summary           HCM 6th Ctrl Delay         187.5           HCM 6th LOS         F							
Phs Duration (G+Y+Rc), s       24.6       45.0       30.0       69.6         Change Period (Y+Rc), s       6.5       6.4       7.3       6.4         Max Green Setting (Gmax), s       18.5       38.6       22.7       38.6         Max Q Clear Time (g_c+l1), s       18.0       8.3       24.7       6.4         Green Ext Time (p_c), s       0.1       0.9       0.0       1.1         Intersection Summary         HCM 6th Ctrl Delay       187.5         HCM 6th LOS       F	Appluauti LOS	Г		U			D
Phs Duration (G+Y+Rc), s       24.6       45.0       30.0       69.6         Change Period (Y+Rc), s       6.5       6.4       7.3       6.4         Max Green Setting (Gmax), s       18.5       38.6       22.7       38.6         Max Q Clear Time (g_c+l1), s       18.0       8.3       24.7       6.4         Green Ext Time (p_c), s       0.1       0.9       0.0       1.1         Intersection Summary         HCM 6th Ctrl Delay       187.5         HCM 6th LOS       F	Timer - Assigned Phs	1	2		4		6
Change Period (Y+Rc), s 6.5 6.4 7.3 6.4  Max Green Setting (Gmax), s 18.5 38.6 22.7 38.6  Max Q Clear Time (g_c+l1), s 18.0 8.3 24.7 6.4  Green Ext Time (p_c), s 0.1 0.9 0.0 1.1  Intersection Summary  HCM 6th Ctrl Delay 187.5  HCM 6th LOS F		24.6	45.0		30.0		69.6
Max Green Setting (Gmax), s       18.5       38.6       22.7       38.6         Max Q Clear Time (g_c+l1), s       18.0       8.3       24.7       6.4         Green Ext Time (p_c), s       0.1       0.9       0.0       1.1         Intersection Summary         HCM 6th Ctrl Delay       187.5         HCM 6th LOS       F	,						
Max Q Clear Time (g_c+l1), s       18.0       8.3       24.7       6.4         Green Ext Time (p_c), s       0.1       0.9       0.0       1.1         Intersection Summary         HCM 6th Ctrl Delay       187.5         HCM 6th LOS       F	. ,.						
Green Ext Time (p_c), s         0.1         0.9         0.0         1.1           Intersection Summary         HCM 6th Ctrl Delay         187.5           HCM 6th LOS         F							
Intersection Summary  HCM 6th Ctrl Delay  HCM 6th LOS  F	(0- /-						
HCM 6th Ctrl Delay 187.5 HCM 6th LOS F	`` ′	0.1	0.8		0.0		1.1
HCM 6th LOS F							
	HCM 6th Ctrl Delay			187.5			
Natao	HCM 6th LOS			F			
	Notes						

Intersection														
Int Delay, s/veh	41.1													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations		4			4			4			4			
Traffic Vol, veh/h	120	4	12	13	1	65	14	636	29	37	651	41		
Future Vol, veh/h	120	4	12	13	1	65	14	636	29	37	651	41		
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0		
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free		
RT Channelized	-	-	None	-	-	None	-	-	None	_	_	None		
Storage Length	_	_	-	_	_	-	_	_	-	_	_	-		
Veh in Median Storage	e.# -	0	_	_	0	_	_	0	_	_	0	_		
Grade, %	-	0	_	_	0	_	_	0	_	_	0	_		
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97		
Heavy Vehicles, %	12	33	2	2	2	2	38	10	2	42	2	11		
Mvmt Flow	124	4	12	13	1	67	14	656	30	38	671	42		
WIVIIIL FIOW	124	4	12	13		07	14	050	30	30	0/1	42		
Major/Minor	Minor2			Minor1			Major1		ı	Major2				
Conflicting Flow All	1501	1482	692	1475	1488	671	713	0	0	686	0	0		
Stage 1	768	768	092	699	699	0/1	113	-	-	-	-	-		
Stage 2	733	714	_	776	789	_	_	_	-	_	_	-		
	7.22	6.83	6.22	7.12	6.52	6.22	4.48	-	-	4.52				
Critical Hdwy				6.12		0.22	4.40	-	-		-	-		
Critical Hdwy Stg 1	6.22	5.83	-		5.52	-	-	-	-	-	-	-		
Critical Hdwy Stg 2	6.22	5.83	-	6.12	5.52	2 240	0.540	-	-	- 0 570	-	-		
Follow-up Hdwy	3.608	4.297	3.318		4.018	3.318		-	-	2.578	-	-		
Pot Cap-1 Maneuver	~ 95	107	444	104	124	456	742	-	-	747	-	-		
Stage 1	380	369	-	430	442	-	-	-	-	-	-	-		
Stage 2	397	392	-	390	402	-	-	-	-	-	-	-		
Platoon blocked, %								-	-		-	-		
Mov Cap-1 Maneuver	~ 74	95	444	89	110	456	742	-	-	747	-	-		
Mov Cap-2 Maneuver	~ 74	95	-	89	110	-	-	-	-	-	-	-		
Stage 1	368	338	-	417	428	-	-	-	-	-	-	-		
Stage 2	327	380	-	343	368	-	-	-	-	-	-	-		
Approach	EB			WB			NB			SB				
HCM Control Delay, s\$	472.6			24.5			0.2			0.5				
HCM LOS	F			C			V.=			0.0				
	•													
Minor Lane/Major Mvm	nt	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR					
Capacity (veh/h)		742	-	-	80	265	747	_	_					
HCM Lane V/C Ratio		0.019	_	_	1.753		0.051	_	_					
HCM Control Delay (s)		9.9	0		472.6	24.5	10.1	0	_					
HCM Lane LOS		Α.	A	- Ψ	F	C C	В	A	_					
HCM 95th %tile Q(veh)	)	0.1	-	_	11.9	1.3	0.2		_					
`		J. 1			11.0	1.0	J.L							
Notes	!!	Φ. D.	Jan		20-			NI-CD	.C., .	*. 41		l		
~: Volume exceeds cap	pacity	\$: De	elay exc	eeas 30	JUS	+: Com	putation	Not De	etined	î: All	major v	olume i	n platoon	

Intersection													
	50.4												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	91	14	16	20	4	49	19	617	25	66	741	133	
uture Vol, veh/h	91	14	16	20	4	49	19	617	25	66	741	133	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
/eh in Median Storage, #	<b>#</b> -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97	
leavy Vehicles, %	12	33	2	2	2	2	38	10	2	42	2	11	
/lvmt Flow	94	14	16	21	4	51	20	636	26	68	764	137	
ajor/Minor Mi	nor2			Minor1			Major1		N	/lajor2			
	1686	1671	833	1673	1726	649	901	0	0	662	0	0	
Stage 1	969	969	-	689	689	-	-	-	-	-	-	-	
Stage 2	717	702	_	984	1037	_	_	_	_	_	_	_	
	7.22	6.83	6.22	7.12	6.52	6.22	4.48	_	_	4.52	_	_	
•	6.22	5.83	-	6.12	5.52	-	-	_	_	-	_	_	
	6.22	5.83	_	6.12	5.52	_	_	_	_	_	_	_	
, ,		4.297	3.318	3.518		3.318	2 542	_	_	2.578	_	_	
. ,	~ 70	81	369	76	89	470	624	_	_	764	_	_	
Stage 1	292	294	-	436	446	-	-	_	_	-	_	_	
Stage 2	405	397	_	299	308	_	_	_	_	_	_	_	
latoon blocked, %	. 30			_00	300			_	_		_	_	
	~ 49	63	369	50	69	470	624	-	-	764	-	-	
	~ 49	63	-	50	69	-	-	-	-	-	_	-	
Stage 1	277	240	-	414	423	_	-	-	_	-	-	-	
Stage 2	340	377	_	219	251	_	_	_	_	_	_	_	
0													
Approach	EB			WB			NB			SB			
HCM Control Delay, s\$ 7	01.2			65.2			0.3			0.7			
HCM LOS	F			F									
//inor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR				
Capacity (veh/h)		624	-	-	57	130	764	-	-				
ICM Lane V/C Ratio		0.031	-	-	2.188		0.089	-	-				
ICM Control Delay (s)		11	0		701.2	65.2	10.2	0	-				
ICM Lane LOS		В	A	-	F	F	В	A	-				
HCM 95th %tile Q(veh)		0.1	-	-	12.3	2.9	0.3	-	-				
Notes													
: Volume exceeds capa	city	\$ De	elav exc	eeds 3	00s	+: Com	putation	Not De	efined	*: All	maior v	olume ir	n platoon
. Volumo oxocodo oapa	Jily	ψ. DC	nay one	.5000 0		. 50111	Patation	.100 00	,ou	. 7 WI	najor v		Piatoon

Intersection												
Int Delay, s/veh	2.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		1102	4	77517	1100	4	HOIT	052	4	OBIT
Traffic Vol, veh/h	1	44	17	10	23	1	10	0	20	0	0	0
Future Vol, veh/h	1	44	17	10	23	1	10	0	20	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	_	None	-	_	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	80	80	80	80	80	80	80	80	80	80	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	55	21	13	29	1	13	0	25	0	0	0
Major/Minor N	Major1		ľ	Major2		l	Minor1			Minor2		
Conflicting Flow All	30	0	0	76	0	0	124	124	66	136	134	30
Stage 1	-	-	-	-	-	-	68	68	-	56	56	-
Stage 2	-	-	-	-	-	-	56	56	-	80	78	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1583	-	-	1523	-	-	850	766	998	835	757	1044
Stage 1	-	-	-	-	-	-	942	838	-	956	848	-
Stage 2	-	-	-	-	-	-	956	848	-	929	830	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1583	-	-	1523	-	-	843	758	998	808	749	1044
Mov Cap-2 Maneuver	-	-	-	-	-	-	843	758	-	808	749	-
Stage 1	-	-	-	-	-	-	941	837	-	955	840	-
Stage 2	-	-	-	-	-	-	947	840	-	905	829	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			2.2			9			0		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	it N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		940	1583			1523	-					
HCM Lane V/C Ratio			0.001	_		0.008	_	_	_			
HCM Control Delay (s)		9	7.3	0	-	7.4	0	-	0			
HCM Lane LOS		A	Α	A	-	Α	A	-	A			
HCM 95th %tile Q(veh)		0.1	0	-	-	0	-	-	-			

Intersection												
Int Delay, s/veh	4.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	35	11	36	30	6	9	1	33	1	0	0
Future Vol, veh/h	0	35	11	36	30	6	9	1	33	1	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	80	80	80	80	80	80	80	80	80	80	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	44	14	45	38	8	11	1	41	1	0	0
Major/Minor I	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	46	0	0	58	0	0	183	187	51	204	190	42
Stage 1	-	-	-	-	-	-	51	51	-	132	132	-
Stage 2	_	-	_	<u>-</u>	_	_	132	136	_	72	58	-
Critical Hdwy	4.12	_	_	4.12	_	_	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1		_	_	-	_	_	6.12	5.52	- 0.22	6.12	5.52	-
Critical Hdwy Stg 2	-	_	-	-	_	-	6.12	5.52	_	6.12	5.52	-
Follow-up Hdwy	2.218	_	_	2.218	_	_		4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1562	-	-	1546	-	-	778	708	1017	754	705	1029
Stage 1	-	_	_	-	_	_	962	852	-	871	787	-
Stage 2	-	-	-	-	-	-	871	784	-	938	847	-
Platoon blocked, %		_	_		_	_	3. 1					
Mov Cap-1 Maneuver	1562	_	_	1546	_	_	760	687	1017	706	684	1029
Mov Cap-2 Maneuver	-	-	-	-	-	-	760	687	-	706	684	-
Stage 1	-	-	-	-	-	-	962	852	-	871	763	-
Stage 2	-	-	-	-	-	-	845	760	-	899	847	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			3.7			9.1			10.1		
HCM LOS				3.1			A			В		
							, ,					
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SRI n1			
Capacity (veh/h)	ic I	940	1562	LDI	LDIX	1546	WDI	WDI	706			
HCM Lane V/C Ratio		0.057		-	-	0.029		_	0.002			
		9.1	0	-		7.4	0		10.1			
HCM Control Delay (s) HCM Lane LOS		9.1 A	A	-	-	7.4 A	A	-	10.1 B			
HCM 95th %tile Q(veh)	\	0.2	0	-	-	0.1	- -	_	0			
How som whe wiven		U.Z	U	-	-	U. I		-	U			

Intersection													
Int Delay, s/veh	54.8												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			ĵ.			4		
Traffic Vol, veh/h	5	0	36	124	0	53	16	482	66	21	766	2	
Future Vol, veh/h	5	0	36	124	0	53	16	482	66	21	766	2	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	_	_	-	_	-	-	_	_	-	_	_	-	
Veh in Median Storage	e.# -	0	_	-	0	-	_	0	-	-	0	_	
Grade, %	-,	0	_	_	0	_	_	0	_	_	0	_	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	2	8	12	2	10	2	
Mvmt Flow	6	0	40	138	0	59	18	536	73	23	851	2	
Major/Minor	Minora			Minor1			Maior1			Majora			
	Minor2	4540		Minor1	4500		Major1			Major2	^	^	
Conflicting Flow All	1536	1543	852	1527	1508	573	853	0	0	609	0	0	
Stage 1	898	898	-	609	609	-	-	-	-	-	-	-	
Stage 2	638	645	-	918	899	-	4.40	-	-	4.40	-	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.12	5.52	2 240	6.12	5.52	2 240	2 240	-	-	2 240	-	-	
Follow-up Hdwy	3.518 95	4.018	3.318	3.518 ~ 96	4.018 121	3.318 519	2.218 786	-	-	2.218 970	-	-	
Pot Cap-1 Maneuver	334	358	309	482	485	519	700	-	-	970	-	-	
Stage 1 Stage 2	465	467		326	358	-	-	-	-	-	-	-	
Platoon blocked, %	400	407	-	320	330	-	-	-	-	-	-	-	
Mov Cap-1 Maneuver	79	106	359	~ 80	112	519	786	-	-	970	-	-	
Mov Cap-1 Maneuver	79	106		~ 80	112	319	700	-	-	910	-	-	
Stage 1	322	342	-	465	468	-	-	-	-	-	-	-	
Stage 2	398	451	-	277	342	-	-	-	-	-	-	-	
Staye 2	220	401	-	211	J4Z	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	22.5		\$	478.9			0.3			0.2			
HCM LOS	С			F									
Minor Lane/Major Mvm	nt	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR				
Capacity (veh/h)		786	-	-	251	107	970	-	-				
HCM Lane V/C Ratio		0.023	-	-	0.181			-	-				
HCM Control Delay (s)		9.7	-	-		478.9	8.8	0	-				
HCM Lane LOS		Α	-	_	С	F	Α	A	-				
HCM 95th %tile Q(veh)	)	0.1	-	-	0.6	15.9	0.1	-	-				
Notes													
~: Volume exceeds ca	nacity	¢. D.	lay ova	eeds 30	)ne	r. Com	nutation	Not Do	ofined	*. All .	majory	olumo ir	n platoon
. volume exceeds ca	pacity	φ. De	ay exc	<del>cc</del> u5 3(	JUS .	+. OUIII	putation	NOL DE	siirieu -	. All l	пајог ۷	olullie II	ι μιαιουπ

Intersection													
Int Delay, s/veh	48.7												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			ĵ.			4		
Traffic Vol, veh/h	5	1	26	88	0	36	36	716	146	64	585	2	
Future Vol, veh/h	5	1	26	88	0	36	36	716	146	64	585	2	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	_	-	None	_	_	None	_	_	None	_	_	None	
Storage Length	-	_	-	-	_	-	_	-	-	-	_	_	
Veh in Median Storage	e.# -	0	_	-	0	-	_	0	_	_	0	-	
Grade, %	-	0	-	-	0	-	_	0	_	-	0	_	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	2	8	12	2	10	2	
Mvmt Flow	6	1	29	98	0	40	40	796	162	71	650	2	
		•			J	10	.0	700	102		000	=	
Major/Minor	Minor2		ı	Minor1			Major1		ı	Major2			
	1770	1831	651	1765	1751	877	652	0	0	958	0	0	
Conflicting Flow All Stage 1	793	793		957	957		002			300			
Stage 1 Stage 2	977	1038	-	808	794	-	-	-	-	-	-	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	_	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52		6.12	5.52	0.22	4.12	-	-		-		
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	_	-	
Follow-up Hdwy	65	76	469	~ 65	4.016	348	935	_	-	718	-	-	
Pot Cap-1 Maneuver	382	400		310	336	340	933	-	-	/ 10	-	-	
Stage 1	302	308	-	375	400	-	-	-	-	-	-	-	
Stage 2 Platoon blocked, %	302	300	-	3/3	400	-	-	-	-	-	-	-	
· · · · · · · · · · · · · · · · · · ·	17	EO	460	10	66	240	025	-	-	710	-	-	
Mov Cap-1 Maneuver	47	58 58	469	~ 49 ~ 49	66 66	348	935	-	-	718	-	-	
Mov Cap-2 Maneuver	47		-		304	-	-	-	-	-	-	-	
Stage 1	346	338	-	281		-	-	-	-	-	-	-	
Stage 2	242	279	-	296	338	-	<del>-</del>	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	30		\$	653.3			0.4			1			
HCM LOS	D			F									
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR				
Capacity (veh/h)		935	-	-	179	65	718	-	_				
HCM Lane V/C Ratio		0.043	_		0.199	2.12	0.099	_	<u>-</u>				
HCM Control Delay (s)		9	_	_		653.3	10.6	0	_				
HCM Lane LOS		A	_	_	D	F	В	A	_				
HCM 95th %tile Q(veh	)	0.1	_		0.7	13.1	0.3	-					
•	1	J. 1			0.1	13.1	3.0						
Notes				, .	00					4			
~: Volume exceeds ca	pacity	\$: De	elay exc	eeds 3	00s	+: Com	putation	Not De	etined	*: All	major v	olume ir	n platoon

Intersection								
Int Delay, s/veh	26.6							
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	VVDL	VVDIX		TIDIX	JDL	- उ <u>ष</u> ी		
Traffic Vol, veh/h	78	80	<b>1</b> ≽ 568		159	843		
	78	80	568	133				
Future Vol, veh/h		0		133	159	843		
Conflicting Peds, #/hr			0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	0	0	-	590	-	-		
Veh in Median Storag		-	0	-	-	0		
Grade, %	0	-	0	-	-	0		
Peak Hour Factor	96	96	96	96	96	96		
Heavy Vehicles, %	38	15	8	22	9	5		
Mvmt Flow	81	83	592	139	166	878		
Major/Minor	Minor1	N	Major1	N	/lajor2			
Conflicting Flow All	1802	592	0	0	731	0		
Stage 1	592	-	-	-	-	-		
Stage 2	1210	-	-	-	-	-		
Critical Hdwy	6.78	6.35	-	_	4.19	-		
Critical Hdwy Stg 1	5.78	-	_	_	-	-		
Critical Hdwy Stg 2	5.78	_	_	-	_	-		
Follow-up Hdwy	3.842		_	_	2.281	_		
Pot Cap-1 Maneuver	~ 71	483	_	_	842	_		
Stage 1	489	-	_	_	-	_		
Stage 2	239	_	_		_	_		
Platoon blocked, %	200		_	<u>-</u>		<u>-</u>		
Mov Cap-1 Maneuver	~ 44	483		_	842	_		
Mov Cap-1 Maneuver		405	_		- 042			
Stage 1	489	<u>-</u>	-	-	_	_		
Stage 1	147	-			_			
Staye 2	147	<u>-</u>	_	<u>-</u>	-	-		
Approach	WB		NB		SB			
HCM Control Delay, s	\$ 303.4		0		1.6			
HCM LOS	F							
Minor Lane/Major Mv	mt	NBT	NRRV	VBLn1V	VBI n2	SBL	SBT	
Capacity (veh/h)		-	HUIN	44	483	842	-	
HCM Lane V/C Ratio		-	_	1.847			- -	
HCM Control Delay (s	.)			600.2	14	10.3	0	
HCM Control Delay (s HCM Lane LOS	9)	-	-\$	F				
	۵)	<del>-</del>	-		В	B	A	
HCM 95th %tile Q(vel	I)	-	-	8.3	0.6	0.7	-	
Notes								
~: Volume exceeds ca	apacity	\$: De	lay exc	eeds 30	00s	+: Comp	outation Not Defined	*: All major volume in platoon

-								
Intersection								
Int Delay, s/veh	40.5							
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
					ODL			
Lane Configurations	100	454	<b>♣</b>	110	440	4		
Traffic Vol, veh/h	100	151	857	110	113	698		
Future Vol, veh/h	100	151	857	110	113	698		
Conflicting Peds, #/hr	0	0	0	_ 0	0	_ 0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	0	0	-	590	-	-		
Veh in Median Storag		-	0	-	-	0		
Grade, %	0	-	0	-	-	0		
Peak Hour Factor	96	96	96	96	96	96		
Heavy Vehicles, %	38	15	8	22	9	5		
Mvmt Flow	104	157	893	115	118	727		
Major/Minor	Minor1	N	Major1	N	Major2			
Conflicting Flow All	1856	893	0	0	1008	0		
Stage 1	893	- 093	-	<u>_</u>	1000	-		
Stage 1	963	_	_	_		-		
Critical Hdwy	6.78	6.35	<u>-</u>	<u>-</u>	4.19	-		
Critical Hdwy Stg 1	5.78	0.33	-	-	4.13	-		
Critical Hdwy Stg 2	5.78	-	-	-	-	-		
		3.435	-	-	2.281	-		
Follow-up Hdwy Pot Cap-1 Maneuver	3.842 ~ 65	322	-	-	661	-		
•	347		-	-	100	-		
Stage 1	347	-	-	<del>-</del>	-	-		
Stage 2	320	-	-	-	-	-		
Platoon blocked, %	10	200	-	-	604	-		
Mov Cap-1 Maneuver		322	-	-	661	-		
Mov Cap-2 Maneuver		-	-	-	-	-		
Stage 1	347	-	-	-	-	-		
Stage 2	224	-	-	-	-	-		
Approach	WB		NB		SB			
HCM Control Delay, s			0		1.6			
HCM LOS	F							
Minor Lane/Major Mvr	nt	NBT	NBRV	VBLn1V		SBL	SBT	
Capacity (veh/h)		-	-	46	322	661	-	
HCM Lane V/C Ratio		-		2.264			-	
HCM Control Delay (s	()	-	-\$	768.6	26.4	11.6	0	
HCM Lane LOS		-	-	F	D	В	Α	
HCM 95th %tile Q(veh	1)	-	-	10.9	2.5	0.6	-	
Notes								
	nacity	¢. D-	lov ove	oods 20	)Oc	L. Com-	outation Not Defined	*: All major valume in plates
~: Volume exceeds ca	распу	φ. De	ay exc	eeds 30	JUS	+. Comp	outation Not Defined	*: All major volume in platoon

# HCM 6th Signalized Intersection Summary 1: SR 19 & CR 48

	1	*	<b>†</b>	1	-	ļ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>^</b>	7	*	<b>†</b>
Traffic Volume (veh/h)	522	334	454	740	413	180
Future Volume (veh/h)	522	334	454	740	413	180
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No	1.00	No	1.00	1.00	No
Adj Sat Flow, veh/h/ln	1752	1589	1767	1811	1737	1811
Adj Flow Rate, veh/h	538	205	468	0	426	186
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	10	21	9	6	11	6
Cap, veh/h	548	442	485	U	430	991
Arrive On Green	0.33	0.33	0.27	0.00	0.21	0.55
Sat Flow, veh/h	1668		1767	1535	1654	1811
,		1346				
Grp Volume(v), veh/h	538	205	468	0	426	186
Grp Sat Flow(s),veh/h/ln	1668	1346	1767	1535	1654	1811
Q Serve(g_s), s	35.2	13.3	28.8	0.0	23.1	5.7
Cycle Q Clear(g_c), s	35.2	13.3	28.8	0.0	23.1	5.7
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	548	442	485		430	991
V/C Ratio(X)	0.98	0.46	0.96		0.99	0.19
Avail Cap(c_a), veh/h	548	442	485		430	991
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	0.00	1.00	1.00
Uniform Delay (d), s/veh	36.6	29.3	39.4	0.0	31.2	12.6
Incr Delay (d2), s/veh	33.9	0.8	33.0	0.0	40.8	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	25.7	7.5	23.3	0.0	22.6	4.2
Unsig. Movement Delay, s/veh		1.5	20.0	0.0	22.0	7.2
LnGrp Delay(d),s/veh	70.5	30.0	72.4	0.0	72.0	13.0
	70.5 E	30.0 C	72.4 E	0.0	72.0 E	13.0 B
LnGrp LOS		U		Λ	<u> </u>	
Approach Vol, veh/h	743		468	Α		612
Approach Delay, s/veh	59.4		72.4			54.1
Approach LOS	Е		Е			D
Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	30.0	36.6		43.4		66.6
Change Period (Y+Rc), s	6.5	6.4		7.3		6.4
Max Green Setting (Gmax), s	23.5	30.2		36.1		60.2
Max Q Clear Time (g_c+l1), s	25.1	30.8		37.2		7.7
	0.0					1.1
Green Ext Time (p_c), s	0.0	0.0		0.0		1.1
Intersection Summary						
HCM 6th Ctrl Delay			60.9			
HCM 6th LOS			Ε			
Notes						

1: SR 19 & CR 48
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	•	•	<b>†</b>	~	/	Ţ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>↑</b>	7	7	<b>†</b>
Traffic Volume (veh/h)	751	483	164	587	451	194
Future Volume (veh/h)	751	483	164	587	451	194
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1752	1589	1767	1811	1737	1811
Adj Flow Rate, veh/h	774	359	169	0	465	200
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
	10	21	9	6	11	0.97
Percent Heavy Veh, %				Ö		
Cap, veh/h	777	627	259	0.00	467	743
Arrive On Green	0.47	0.47	0.15	0.00	0.20	0.41
Sat Flow, veh/h	1668	1346	1767	1535	1654	1811
Grp Volume(v), veh/h	774	359	169	0	465	200
Grp Sat Flow(s),veh/h/ln	1668	1346	1767	1535	1654	1811
Q Serve(g_s), s	50.9	21.4	9.9	0.0	22.5	8.1
Cycle Q Clear(g_c), s	50.9	21.4	9.9	0.0	22.5	8.1
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	777	627	259		467	743
V/C Ratio(X)	1.00	0.57	0.65		1.00	0.27
Avail Cap(c_a), veh/h	777	627	259		467	743
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
	1.00	1.00	1.00		1.00	1.00
Upstream Filter(I)				0.00		
Uniform Delay (d), s/veh	29.3	21.4	44.3	0.0	33.2	21.5
Incr Delay (d2), s/veh	31.4	1.3	12.2	0.0	40.4	0.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	33.4	10.6	8.9	0.0	12.2	6.4
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	60.7	22.7	56.5	0.0	73.6	22.4
LnGrp LOS	Е	С	Е		Е	С
Approach Vol, veh/h	1133		169	Α		665
Approach Delay, s/veh	48.7		56.5	• •		58.2
Approach LOS	D		E			E
Approach 200	D		_			_
Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	29.0	22.5		58.5		51.5
Change Period (Y+Rc), s	6.5	6.4		7.3		6.4
Max Green Setting (Gmax), s	22.5	16.1		51.2		45.1
Max Q Clear Time (g_c+I1), s	24.5	11.9		52.9		10.1
Green Ext Time (p_c), s	0.0	0.3		0.0		1.1
	0.0	0.0		0.0		1.1
Intersection Summary						
HCM 6th Ctrl Delay			52.6			
HCM 6th LOS			D			
Notes						

	۶	<b>→</b>	•	•	<b>←</b>	•	4	1	/	/	ţ	4	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Volume (veh/h)	144	4	12	13	1	65	14	672	29	37	663	49	
Future Volume (veh/h)	144	4	12	13	1	65	14	672	29	37	663	49	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approac		No			No			No			No		
Adj Sat Flow, veh/h/ln	1722	1411	1870	1870	1870	1870	1337	1752	1870	1278	1870	1737	
Adj Flow Rate, veh/h	148	4	12	13	1	67	14	693	30	38	684	51	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Percent Heavy Veh, %	12	33	2	2	2	2	38	10	2	42	2	11	
Cap, veh/h	310	10	15	105	29	252	78	1054	45	101	1041	75	
Arrive On Green	0.18	0.18	0.18	0.18	0.18	0.18	0.64	0.64	0.64	0.64	0.64	0.64	
Sat Flow, veh/h	965	56	81	128	159	1375	11	1642	70	43	1623	118	
Grp Volume(v), veh/h	164	0	0	81	0	0	737	0	0	773	0	0	
Grp Sat Flow(s), veh/h/lr	11102	0	0	1663	0	0	1722	0	0	1783	0	0	
Q Serve(g_s), s	5.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Cycle Q Clear(g_c), s	7.2	0.0	0.0	2.2	0.0	0.0	13.5	0.0	0.0	13.2	0.0	0.0	
Prop In Lane	0.90		0.07	0.16		0.83	0.02		0.04	0.05		0.07	
Lane Grp Cap(c), veh/h	335	0	0	386	0	0	1177	0	0	1218	0	0	
V/C Ratio(X)	0.49	0.00	0.00	0.21	0.00	0.00	0.63	0.00	0.00	0.63	0.00	0.00	
Avail Cap(c_a), veh/h	506	0	0	645	0	0	1177	0	0	1218	0	0	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00	
Uniform Delay (d), s/veh	า 19.9	0.0	0.0	18.1	0.0	0.0	5.7	0.0	0.0	5.7	0.0	0.0	
Incr Delay (d2), s/veh	1.1	0.0	0.0	0.3	0.0	0.0	2.5	0.0	0.0	2.5	0.0	0.0	
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(95%),veh	/ln3.2	0.0	0.0	1.4	0.0	0.0	6.1	0.0	0.0	6.4	0.0	0.0	
Unsig. Movement Delay	, s/veh												
LnGrp Delay(d),s/veh	21.0	0.0	0.0	18.3	0.0	0.0	8.2	0.0	0.0	8.2	0.0	0.0	
LnGrp LOS	С	Α	Α	В	Α	Α	Α	Α	Α	Α	Α	Α	
Approach Vol, veh/h		164			81			737			773		
Approach Delay, s/veh		21.0			18.3			8.2			8.2		
Approach LOS		С			В			Α			Α		
Timer - Assigned Phs		2		4		6		8					
Phs Duration (G+Y+Rc)	. S	37.5		13.9		37.5		13.9					
Change Period (Y+Rc),		4.5		4.5		4.5		4.5					
Max Green Setting (Gm		33.0		18.0		33.0		18.0					
Max Q Clear Time (g_c-		15.5		9.2		15.2		4.2					
Green Ext Time (p_c), s	, .	5.0		0.5		5.4		0.3					
Intersection Summary		3.0		3.0		J.,		3.5					
HCM 6th Ctrl Delay			9.9										
HCM 6th LOS			9.9 A										
HOW BUILDS			А										

	۶	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	1	1	ţ	✓	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Volume (veh/h)	108	14	16	20	4	49	19	642	25	66	784	161	
Future Volume (veh/h)	108	14	16	20	4	49	19	642	25	66	784	161	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approac		No			No			No			No		
Adj Sat Flow, veh/h/ln	1722	1411	1870	1870	1870	1870	1337	1752	1870	1278	1870	1737	
Adj Flow Rate, veh/h	111	14	16	21	4	51	20	662	26	68	808	166	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Percent Heavy Veh, %	12	33	2	2	2	2	38	10	2	42	2	11	
Cap, veh/h	338	28	21	191	43	187	124	917	35	154	784	155	
Arrive On Green	0.17	0.17	0.17	0.17	0.17	0.17	0.56	0.56	0.56	0.56	0.56	0.56	
Sat Flow, veh/h	839	169	129	296	258	1130	18	1650	64	64	1410	279	
Grp Volume(v), veh/h	141	0	0	76	0	0	708	0	0	1042	0	0	
Grp Sat Flow(s),veh/h/lr	า1136	0	0	1684	0	0	1731	0	0	1753	0	0	
Q Serve(g_s), s	2.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	8.1	0.0	0.0	
Cycle Q Clear(g_c), s	3.7	0.0	0.0	1.3	0.0	0.0	9.9	0.0	0.0	18.0	0.0	0.0	
Prop In Lane	0.79		0.11	0.28		0.67	0.03		0.04	0.07		0.16	
Lane Grp Cap(c), veh/h	387	0	0	421	0	0	1077	0	0	1094	0	0	
V/C Ratio(X)	0.36	0.00	0.00	0.18	0.00	0.00	0.66	0.00	0.00	0.95	0.00	0.00	
Avail Cap(c_a), veh/h	803	0	0	1020	0	0	1077	0	0	1094	0	0	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00	
Uniform Delay (d), s/vel	า 12.7	0.0	0.0	11.8	0.0	0.0	5.4	0.0	0.0	7.6	0.0	0.0	
Incr Delay (d2), s/veh	0.6	0.0	0.0	0.2	0.0	0.0	1.5	0.0	0.0	17.1	0.0	0.0	
Initial Q Delay(d3),s/veh	n 0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(95%),vel		0.0	0.0	0.7	0.0	0.0	2.7	0.0	0.0	11.8	0.0	0.0	
Unsig. Movement Delay													
LnGrp Delay(d),s/veh	13.3	0.0	0.0	12.0	0.0	0.0	6.8	0.0	0.0	24.7	0.0	0.0	
LnGrp LOS	В	Α	Α	В	Α	Α	Α	Α	Α	С	Α	Α	
Approach Vol, veh/h		141			76			708			1042		
Approach Delay, s/veh		13.3			12.0			6.8			24.7		
Approach LOS		В			В			Α			С		
Timer - Assigned Phs		2		4		6		8			_		
Phs Duration (G+Y+Rc)		22.5		9.9		22.5		9.9					
Change Period (Y+Rc),		4.5		4.5		4.5		4.5					
Max Green Setting (Gm		18.0		18.0		18.0		18.0					
Max Q Clear Time (g_c		11.9		5.7		20.0		3.3					
Green Ext Time (p_c), s		2.5		0.6		0.0		0.3					
. ,	,	2.0		0.0		0.0		0.5					
Intersection Summary													
HCM 6th Ctrl Delay			16.9										
HCM 6th LOS			В										

	۶	<b>→</b>	*	•	•	•	1	<b>†</b>	~	/	Ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7		4		*	₽			र्स	7
Traffic Volume (veh/h)	41	0	120	124	0	53	44	490	66	21	790	14
Future Volume (veh/h)	41	0	120	124	0	53	44	490	66	21	790	14
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1781	1722	1870	1752	1870
Adj Flow Rate, veh/h	46	0	133	138	0	59	49	544	73	23	878	16
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	2	2	2	2	2	2	2	8	12	2	10	2
Cap, veh/h	377	0	210	0	0	210	342	980	131	104	1093	1010
Arrive On Green	0.13	0.00	0.13	0.00	0.00	0.13	0.64	0.64	0.64	0.64	0.64	0.64
Sat Flow, veh/h	1455	0	1585	0	0	1585	622	1538	206	16	1716	1585
Grp Volume(v), veh/h	46	0	133	0	0	59	49	0	617	901	0	16
Grp Sat Flow(s),veh/h/ln	1455	0	1585	0	0	1585	622	0	1744	1731	0	1585
Q Serve(g_s), s	0.0	0.0	3.1	0.0	0.0	1.3	2.5	0.0	7.8	0.0	0.0	0.1
Cycle Q Clear(g_c), s	0.9	0.0	3.1	0.0	0.0	1.3	17.5	0.0	7.8	15.0	0.0	0.1
Prop In Lane	1.00	•	1.00	0.00	•	1.00	1.00	•	0.12	0.03	•	1.00
Lane Grp Cap(c), veh/h	377	0	210	0	0	210	342	0	1111	1197	0	1010
V/C Ratio(X)	0.12	0.00	0.63	0.00	0.00	0.28	0.14	0.00	0.56	0.75	0.00	0.02
Avail Cap(c_a), veh/h	819	0	731	0	0	731	630	0	1921	1984	0	1745
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	15.1	0.0	16.0	0.0	0.0	15.3	11.8	0.0	4.0	5.3	0.0	2.6
Incr Delay (d2), s/veh	0.1	0.0	3.1	0.0	0.0	0.7	0.2	0.0	0.4	1.0	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0 2.0	0.0	0.0	0.0	0.0	0.0	0.0 1.7	0.0 3.3	0.0	0.0
%ile BackOfQ(95%),veh/ln		0.0	2.0	0.0	0.0	0.8	0.5	0.0	1.7	ა.ა	0.0	0.0
Unsig. Movement Delay, s/veh	15.2	0.0	19.2	0.0	0.0	16.0	12.0	0.0	4.4	6.3	0.0	2.6
LnGrp Delay(d),s/veh LnGrp LOS	13.2 B	0.0 A	19.2 B		0.0 A	10.0 B	12.0 B		4.4 A		0.0 A	2.0 A
	D	179	D	A	59	D	D	A 666	A	A	917	A
Approach Vol, veh/h		18.2			16.0			5.0			6.2	
Approach LOS		10.2 B			16.0 B						Α	
Approach LOS					Б			А			А	
Timer - Assigned Phs		2	3	4		6		8				
Phs Duration (G+Y+Rc), s		29.4	0.0	9.7		29.4		9.7				
Change Period (Y+Rc), s		4.5	4.5	4.5		4.5		4.5				
Max Green Setting (Gmax), s		43.0	5.0	18.0		43.0		18.0				
Max Q Clear Time (g_c+I1), s		19.5	0.0	5.1		17.0		3.3				
Green Ext Time (p_c), s		4.8	0.0	0.5		7.9		0.2				
Intersection Summary												
HCM 6th Ctrl Delay			7.3									
HCM 6th LOS			Α									

# HCM 6th Signalized Intersection Summary 4: SR 19 & Revels Rd/Revels Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7		4		7	₽			4	7
Traffic Volume (veh/h)	30	1	83	88	0	36	135	744	146	64	602	45
Future Volume (veh/h)	30	1	83	88	0	36	135	744	146	64	602	45
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1781	1722	1870	1752	1870
Adj Flow Rate, veh/h	33	1	92	98	0	40	150	827	162	71	669	50
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	2	2	2	2	2	2	2	8	12	2	10	2
Cap, veh/h	244	6	140	0	0	140	388	1102	216	126	1006	1207
Arrive On Green	0.09	0.09	0.09	0.00	0.00	0.09	0.76	0.76	0.76	0.76	0.76	0.76
Sat Flow, veh/h	1422	65	1585	0	0	1585	733	1447	283	80	1321	1585
Grp Volume(v), veh/h	34	0	92	0	0	40	150	0	989	740	0	50
Grp Sat Flow(s),veh/h/ln	1486	0	1585	0	0	1585	733	0	1730	1401	0	1585
Q Serve(g_s), s	0.0	0.0	3.4	0.0	0.0	1.4	9.8	0.0	19.1	4.7	0.0	0.5
Cycle Q Clear(g_c), s	1.1	0.0	3.4	0.0	0.0	1.4	33.6	0.0	19.1	24.0	0.0	0.5
Prop In Lane	0.97		1.00	0.00		1.00	1.00		0.16	0.10		1.00
Lane Grp Cap(c), veh/h	249	0	140	0	0	140	388	0	1318	1133	0	1207
V/C Ratio(X)	0.14	0.00	0.66	0.00	0.00	0.29	0.39	0.00	0.75	0.65	0.00	0.04
Avail Cap(c_a), veh/h	542	0	476	0	0	476	600	0	1818	1548	0	1666
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	25.4	0.0	26.5	0.0	0.0	25.6	15.2	0.0	4.0	3.3	0.0	1.8
Incr Delay (d2), s/veh	0.2	0.0	5.2	0.0	0.0	1.1	0.6	0.0	1.2	0.6	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	0.8	0.0	2.5	0.0	0.0	1.0	2.7	0.0	4.3	2.3	0.0	0.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	25.6	0.0	31.6	0.0	0.0	26.7	15.8	0.0	5.1	3.9	0.0	1.8
LnGrp LOS	С	Α	С	Α	Α	С	В	Α	Α	Α	Α	A
Approach Vol, veh/h		126			40			1139			790	
Approach Delay, s/veh		30.0			26.7			6.5			3.8	
Approach LOS		С			С			Α			Α	
Timer - Assigned Phs		2	3	4		6		8				
Phs Duration (G+Y+Rc), s		50.5	0.0	9.8		50.5		9.8				
Change Period (Y+Rc), s		4.5	4.5	4.5		4.5		4.5				
Max Green Setting (Gmax), s		63.0	5.0	18.0		63.0		18.0				
Max Q Clear Time (g_c+l1), s		35.6	0.0	5.4		26.0		3.4				
Green Ext Time (p_c), s		10.8	0.0	0.3		7.2		0.1				
Intersection Summary												
HCM 6th Ctrl Delay			7.3									
HCM 6th LOS			A									

0.T. 20 / I \ (f) 3555 (T)
ent WBL WBR NBT NBR SBL SBT
onfigurations 🌂 🏌 🕴 🗳
Volume (veh/h) 78 88 596 133 183 927
Volume (veh/h) 78 88 596 133 183 927
(Qb), veh 0 0 0 0 0
ke Adj(A_pbT) 1.00 1.00 1.00 1.00
Bus, Adj 1.00 1.00 1.00 1.00 1.00
one On Approach No No No
Flow, veh/h/ln 1337 1678 1781 1574 1767 1826
w Rate, veh/h 81 92 621 139 191 966
our Factor 0.96 0.96 0.96 0.96 0.96
t Heavy Veh, % 38 15 8 22 9 5
sh/h 101 113 1527 1143 214 983
On Green 0.08 0.08 0.86 0.86 0.86 0.86
w, veh/h 1273 1422 1781 1334 216 1146
ume(v), veh/h 81 92 621 139 1157 0
t Flow(s),veh/h/ln1273 1422 1781 1334 1362 0
e(g_s), s 8.9 9.1 10.9 2.4 105.7 0.0
Q Clear(g_c), s 8.9 9.1 10.9 2.4 116.6 0.0
Lane 1.00 1.00 1.00 0.17
rp Cap(c), veh/h 101 113 1527 1143 1197 0
tio(X) 0.80 0.81 0.41 0.12 0.97 0.00
ap(c_a), veh/h 161 180 1540 1153 1208 0
latoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00
am Filter(I) 1.00 1.00 1.00 1.00 0.00
Delay (d), s/veh 64.4 64.4 2.2 1.6 12.5 0.0
lay (d2), s/veh 13.6 13.9 0.2 0.0 18.3 0.0
Delay(d3),s/veh 0.0 0.0 0.0 0.0 0.0 0.0
ckOfQ(95%),veh/lrf5.9 6.7 4.5 0.8 40.9 0.0
Movement Delay, s/veh
Delay(d),s/veh 78.0 78.4 2.4 1.7 30.8 0.0
LOS E E A A C A
ch Vol, veh/h 173 760 1157
ch Delay, s/veh 78.2 2.3 30.8
ch LOS E A C
Assigned Phs 2 6 8
ration (G+Y+Rc), s 126.5 126.5 15.8
e Period (Y+Rc), s 4.5 4.5 4.5
een Setting (Gmax), s 123.0 123.0 18.0
Clear Time (g_c+l1), s 12.9 118.6 11.1
Ext Time (p_c), s 5.3 3.4 0.3
ction Summary
h Ctrl Delay 24.3
th LOS C

	•	•	†	1	1	<b>↓</b>		
Movement WE	BL W	/BR	NBT	NBR	SBL	SBT		
Lane Configurations	4	7	<b>†</b>	7		4		
		179	956	110	130	756		
Future Volume (veh/h) 10	00 ′	179	956	110	130	756		
Initial Q (Qb), veh	0	0	0	0	0	0		
Ped-Bike Adj(A_pbT) 1.0	00 1	1.00		1.00	1.00			
Parking Bus, Adj 1.0	00 1	1.00	1.00	1.00	1.00	1.00		
Work Zone On Approach 1			No			No		
Adj Sat Flow, veh/h/ln 133		678	1781	1574	1767	1826		
		186	996	115	135	788		
Peak Hour Factor 0.9		0.96	0.96	0.96	0.96	0.96		
	38	15	8	22	9	5		
		171	1461	1094	141	755		
Arrive On Green 0.		).12	0.82	0.82	0.82	0.82		
Sat Flow, veh/h 12	73 14	422	1781	1334	138	921		
Grp Volume(v), veh/h	04 ′	186	996	115	923	0		_
Grp Sat Flow(s), veh/h/ln12	73 14	422	1781	1334	1059	0		
Q Serve(g_s), s 11		18.0	34.2	2.5	88.8	0.0		
Cycle Q Clear(g_c), s 11	.7 1	18.0	34.2	2.5	123.0	0.0		
	00 1	1.00		1.00	0.15			
Lane Grp Cap(c), veh/h 1	53 ′	171	1461	1094	896	0		
V/C Ratio(X) 0.0	68 1	1.09	0.68	0.11	1.03	0.00		
Avail Cap(c_a), veh/h 1	53 ′	171	1461	1094	896	0		
HCM Platoon Ratio 1.0	00 1	1.00	1.00	1.00	1.00	1.00		
Upstream Filter(I) 1.0	00 1	1.00	1.00	1.00	1.00	0.00		
Uniform Delay (d), s/veh 63	3.2 6	6.0	5.5	2.7	24.2	0.0		
Incr Delay (d2), s/veh 11		95.0	1.3	0.0	38.1	0.0		
Initial Q Delay(d3),s/veh 0		0.0	0.0	0.0	0.0	0.0		
%ile BackOfQ(95%),veh/ln7		17.1	15.7	1.1	52.3	0.0		
Unsig. Movement Delay, s/								
	.9 16	31.0	6.8	2.7	62.3	0.0		
LnGrp LOS	Е	F	Α	Α	F	Α		
	90		1111			923		
Approach Delay, s/veh 130			6.4			62.3		
Approach LOS	F		Α			E		
		_				_	•	
Timer - Assigned Phs	40	2				6	8	
Phs Duration (G+Y+Rc), s		27.5				127.5	22.5	
Change Period (Y+Rc), s		4.5				4.5	4.5	
Max Green Setting (Gmax)	•	23.0				123.0	18.0	
Max Q Clear Time (g_c+l1)		36.2				125.0	20.0	
Green Ext Time (p_c), s	1	11.8				0.0	0.0	
Intersection Summary								
HCM 6th Ctrl Delay			44.1					
HCM 6th LOS			D					

Appendix N
Lake County Land Development Code (LDC)

#### 2. Turn Lanes

Turn lanes consist of left-turn lanes and right-turn lanes (deceleration lanes). Turn lanes shall be installed on the road which is being accessed at the proposed entrance(s) to the development, as deemed necessary by the County Manager or Designee. The County Manager or Designee may also require turn lanes at adjacent or nearby intersections in lieu of, or in addition to, turn lanes at the development entrances.

Conditions which are to be considered in determining the need for turn lanes include the following:

- a) If the property accessing the road is projected to generate 500 or more vehicle trips per day, or 50 or more vehicle trips in any hour;
- b) If a traffic analysis indicates that turn lanes would be necessary to maintain capacity on fronting roads and/or on adjacent or nearby intersections.
- c) If entrances are proposed at locations where grade, topography, site distance, traffic, or other unusual conditions indicate that turn lanes would be needed for traffic safety. The need for turn lanes to accommodate right turn movements and left turn movements shall be based upon anticipated traffic distribution and projected turning movement volumes among other considerations, including traffic safety.

### C. <u>Traffic Analysis</u>

### 1. Transportation Concurrency Management System

Transportation Concurrency Management System is administered by the Lake-Sumter Metropolitan Planning Organization (LSMPO). All information regarding traffic study could be found on LSPMO website www.lakesumtermpo.com/concurrency/index.aspx

### D. Road Classification

#### 1. Arterial Roads

An arterial road is a route providing service which is relatively continuous and of relatively high traffic volume, long average trip length, high operating speed and of high mobility importance.

Arterial roads are grouped into the following sub-categories:

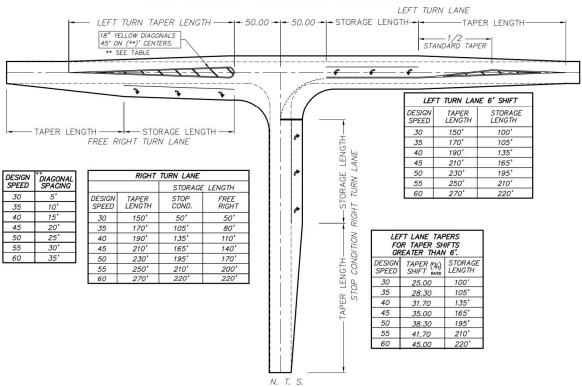
- a) Principal Arterial
- **b)** Minor Arterial

The classification of roads as arterials shall be based upon criteria established by the Florida Department of Transportation utilizing their most recent, adopted functional classification system.

#### 2. Collector Roads

A collector road is a route providing services which is of relatively moderate traffic volume, moderate trip length and moderate operating speed. Collector roads collect and distribute the traffic between local roads and arterial roads and serves as a linkage between land access and mobility needs.

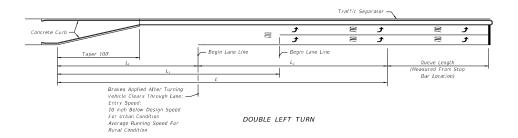
#### LAKE COUNTY STANDARD TURN LANES

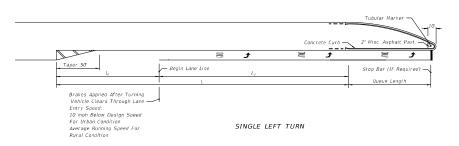


O: \\_CAD STANDARDS\DWG\Turn LanesR1.dwg (02/06/2007)

THIS SHOULD BE USED AS A GUIDE LINE ONLY. ALL DESIGNS SHALL BE SUBMITTED FOR REVIEW. **Appendix O**FDOT Design Manual Exhibit 212-1

## MEDIAN TURN LANES MINIMUM DECELERATION LENGTHS





	MEDIAN TURN LANES													
			URBA	AN CONDIT	IONS	RURAL CONDITIONS								
Design Speed (mph)	Entry Speed (mph)	Clearance Distance L; (ft.)	Brake To Stop Distance L <sub>2</sub> (ft.)	Total Decel. Distance L (ft.)	Clearance Distance L <sub>3</sub> (ft.)	Brake To Stop Distance L <sub>2</sub> (ft.)	Total Decel. Distance L (ft.)	Clearance Distance L <sub>3</sub> (ft.)						
35	25	70	75	145	110	_	_							
40	30	80	75	155	120	_								
45	35	85	100	185	135									
50	40/44	105	135	240	160	185	290	160						
55	48	125	_	_	_	225	350	195						
60	52	145				260	405	230						
65	55	170		_		290	460	270						

NOT TO SCALE

EXHIBIT 212-1 01/01/2022 This instrument prepared by and should be returned to: Thomas J. Wilkes GrayRobinson 301 East Pine Street, Suite 1400 Orlando, Florida 32801

### MISSION RISE PUD DEVELOPMENT AGREEMENT

This MISSION RISE PUD DEVELOPMENT AGREEMENT ("Agreement") is made as of the \_\_\_\_\_\_ day of \_\_\_\_\_\_, 2023 ("Effective Date"), between the Town of Howey in the Hills, Florida, a Florida municipal corporation (the "Town"), and ASF TAP FL I, LLC, a Delaware limited liability company (the "Owner").

#### **RECITALS**

- A. The Owner owns approximately 243 acres of property more particularly described in Attachment A to this Agreement ("the Property").
- B. The Property is within the corporate limits of the Town. The Town has assigned the Property a future-land-use designation of Village Mixed Use and has zoned the Property for PUD Planned Unit Development.
- C. The Owner intends to develop and use the Property as a mixed-use planned development consisting of single-family residential, civic and public uses more specifically set forth herein ("the Project"), to be known as the "Mission Rise PUD."
- D. The Town and Owner enter into this Agreement to set forth the terms and conditions of approval negotiated between them for the development and use of the Property as the Mission Rise PUD.

#### NOW, THEREFORE, the Town and the Owner agree as follows:

- **Section 1.** Land development and uses. Development and use of the Property is subject to the following conditions, requirements, restrictions, and terms:
- (a) **General**. Development of the Project and use of the Property shall be governed by this Agreement, the Town's Comprehensive Plan, the Town's Land Development Code ("LDC") and Code of Ordinances ("Town Code"), and all other applicable state laws and regulations and Town ordinances and rules. Where in conflict, the terms of this Agreement shall supersede and prevail over the LDC and Town Code, but only to the extent of the conflict.

In the Conceptual Land Use Plan for the Project the term "conceptual" means the location of land uses on the site, including areas for residential development, open space, stormwater management, parks, and roads in relation to the site area and other uses on the site. Subsequent plan development may refine the details based on detailed engineering design. "Conceptual" does not mean or contemplate the modification of proposed housing types or the relocation of land uses and roads other than minor adjustments dictated by engineering needs and best practices.

Unless otherwise noted, the definition of terms in this Agreement shall be the same as the definitions set forth in the LDC.

- (b) **Phasing**. The Project will be developed in multiple phases, as shown on the Conceptual Land Use Plan. Each phase must be designed and built to operate independently with all necessary public services and utilities infrastructure, including roads, multimodal trails and master stormwater systems, consistent with Conceptual Land Use Plan. Revisions to the phasing schedule shall be considered as minor amendments to this Agreement, which may be approved by Town Council with no formal amendment to this Agreement required.
  - (c) **Purpose**. The purpose of the Mission Rise PUD is to:
  - 1. Create an attractive and high-quality single-family housing development compatible with the scale and character of existing residential development and land uses in the Town;
  - 2. Develop a residential area that is safe, comfortable and attractive for and to pedestrians;
  - 3. Create a community with direct visual and physical access to open land, with a strong community identity, and with amenities in the form of community open space;
  - 4. Provide a network of open space for future homeowners; and
  - 5. Provide a variety of lot sizes and housing choices for diverse age and income groups and residential preferences.
- (d) **Land uses**. The Conceptual Land Use Plan for the Project is contained in Attachment B and is an integral part of the approval of the Project. Elements in the Concept Plan include single-family detached homes, civic uses, multimodal trails and approximately 69.4 acres of open space.

#### (e) **Development standards.**

#### Setbacks

The setbacks for single family residential lots shall be as shown on the Conceptual Land Use Plan for the Project.

#### **Lot Size**

A range of lot sizes shall be provided in order to create variety and offer opportunity for different income households. Minimum lot size will be 55' x 120'. The Project may consist of up to 499 total single-family residential detached lots of 55' x 120' and 75' x 120'.

#### **Dwelling Size**

The minimum dwelling size for all single-family residences shall be 1,400 square feet of heated/air-conditioned space under roof plus a two-car garage with a minimum of 400 square feet. Maximum dwelling size shall be 4,600 square feet of heated/air-conditioned space under roof.

#### Lot Width

The minimum lot width at building line shall be 55 feet for 55-foot wide lots and 75 feet for 75-foot wide lots with a minimum street frontage of 30 feet.

#### Lot Coverage

Lots shall have a maximum lot coverage of 60% based on the proposed setbacks shown on the Conceptual Land Use Plan for the Project.

#### **Height of Structures**

No residential structure may exceed 35 feet in height

#### **Building Design**

Building design shall be in accordance with the Architectural Requirements of the Town's LDC and will comply specifically with the design requirements of LDC Sections 4.06.02 and 4.06.03.

The following principles seek to promote a high-quality development that will create a sense of place and community through the development of the site.

- Housing styles, shapes and materials shall meet the Towns Land Development Regulations.
- The different housing types shall be integrated architecturally in order to give the development a harmonious appearance.
- The creation of visual richness shall be considered when choosing materials and details. Local characteristics are encouraged.
- Side entrances for garages are encouraged.
- A variety of roof heights, pitches and materials will be encouraged.
- Landscaping shall be incorporated into the overall design as a means of linking the development areas with the open spaces.
- Each exterior wall for a single-family home must be a minimum of two materials and a minimum of two colors. Primary facades must have one base color and a complementary wall material may be used to meet the second color requirement.
- Block face restrictions may be reduced to 300 linear feet. The same house model may not be used more than three times within a single block face. For purposes of this requirement, a different house model is a different floor plan, not the same floor plan flipped in a different direction and not the same floor plan with a different exterior treatment.
- (f) **Wetlands**. Impacts to wetlands, if any, and wetland buffering shall be subject to the Town Land Development Regulations and well as St. Johns River Water Management District regulations.

(g) **Potable water, wastewater, and reclaimed water**. For potable water and wastewater service, well and septic systems are not allowed. The Project must be connected to and served by the Town's potable-water and wastewater systems prior to a certificate of occupancy being issued for a structure in the Project (except temporary construction uses).

Except as may be set forth otherwise in this Agreement, the Owner must install all on-site potable-water, wastewater, and reclaimed-water infrastructure and connect to central water and wastewater systems, and to the Town's reclaimed-water system when available at the Property boundary, all at no cost to the Town. The Owner must pay potable-water, wastewater, and reclaimed-water capital and connection charges, impact fees, and other Town rates, fees, and charges, either applicable currently or in the future.

1. Potable Water. The Town will provide potable water, and may in the future provide reclaimed water, to the Project in accordance with its applicable ordinances, resolutions, operating regulations, policies and procedures. The Town will provide potable water to the Property in sufficient quantities for development of the Project as contemplated herein, subject to the limitations and requirements of permits issued to the Town from time to time by the St. John's River Water Management District in connection with water consumption.

The Owner shall construct, at no expense to the Town, all off-site potable-water-system facilities, lines, pumps, valves, control structures, and appurtenances (other than water-treatment plants) necessary to serve the Project. The construction and route of off-site lines and other structures shall be done according to engineering plans prepared by the Owner and approved by the Town Manager. Potable water shall not be used for irrigation.

2. Wastewater. The Town will provide wastewater-collection and transmission service to the Project, transmitting Project wastewater to the Central Lake Community Development District ("CDD") or another wastewater utility service provider with available capacity to treat and dispose the Project's wastewater ("Wastewater Utility"). The Owner must obtain from the CDD or Wastewater Utility a contract right for the Project to receive treatment and disposal of its wastewater at such provider's treatment and disposal facilities.

The Owner shall construct, at no expense to the Town, all off-site wastewater-system facilities, lines, lift stations, pumps, valves, control structures, and appurtenances (other than wastewater-treatment plants and disposal facilities) necessary to serve the Project. The construction and route of off-site lines, lift stations, pumps, and other structures shall be done according to engineering plans prepared by the Owner and approved by the Town Manager.

3. Town Option to Oversize Water and Wastewater Lines. In conjunction with the review and processing of the preliminary subdivision plans for each phase of the Project, the Town may elect to oversize the off-site lines, pumps, improvements, or other facilities or appurtenances for the Town's water or wastewater system, or for both, necessary to serve such phase. If the Town elects to oversize one or both systems, it must inform the Owner in writing of the specifications for the oversizing(s) prior to or as part of the Town's first round of review comments on the preliminary subdivision plan application. The Town shall reimburse the Owner for the difference in the increase in cost of design, materials and construction to oversize the improvements based on plans and cost estimates provided by the Owner to the Town and approved

by the Town Manager, which approval shall not be unreasonably withheld, conditioned or delayed. The Town shall reimburse the Owners for the difference in the costs within 60 days following (i) completion of the improvements and (ii) receipt by the Town of documentation reasonably demonstrating that the Owner has completed the work and has incurred the costs attributable to the over-sizing, all in keeping with the plans and cost estimate previously approved by the Town Manager.

- 4. Permit-Induced Costs, Restrictions, Requirements, and Risks. Under state and federal laws and regulations, the Town may provide its potable-water and wastewater services to the Property and the Owner and its successors only if the Town first has been issued certain required permits. The Owner acknowledges that the permits are inevitably conditioned with requirements and restrictions that typically impose costs and risks. The Owner further acknowledges that, for the Town to operate its potable-water and wastewater systems in an orderly, dependable, and cost-effective manner, the Town must have the ability legally to spread the costs and risks among customers and property owners benefiting from the services. The Owner acknowledges, therefore, that (i) from time to time the Town may impose rates, fees, and charges and may issue potable-water system and wastewater-system regulations and policies that impose restrictions and requirements on its customers and benefiting property owners, such as the Owner and it successors, and (ii) so long as the Owner or successors are required to pay only their fair share for such rates, fees, and charges, then the imposition of such rates, fees, and charges and the issuance of such system regulations are not prohibited by or otherwise a breach of this Agreement.
- 5. Reclaimed Water. The Owner must install reclaimed water lines as required by the Town's Code of Ordinances, and shall obtain reclaimed-water service for the Project when the Town constructs reclaimed-water lines to the Project's boundaries. Until such time as the Town supplies reclaimed water, the Owner and its successors shall use the reclaimed water lines to irrigate properties within the Project boundaries, but only with stormwater from on-site stormwater-retention ponds or with sources other than potable water as may be approved by the Town and St. John's River Water Management District. Except for installation of reclaimed lines at the time of development as noted above, connection to reclaimed water after the development of the Project may not result in additional costs to the Owner or developer.
  - (h) **Solid Waste**. Solid Waste collection shall be pursuant to Town regulations.
- (i) **Drainage**. The maintenance, repair, and replacement of the drainage system shall be the responsibility of the homeowners association(s).

#### (j) Transportation

#### Street and Sidewalks

The Project must have a connected street system that serves vehicles, pedestrians and bicycles and that connects to recreation facilities and adjacent residential/community areas. There must be ingress and egress points to Revels Road, County Number Two Road and Orange Blossom Road at final buildout of the Project in the approximate location shown on the Conceptual Land Use Plan. The access at County Road Number Two must be a full intersection subject to review and approval by Lake County. Future access connections at

the western and eastern boundaries of the property will also be provided, as shown on the Conceptual Land Use Plan, subject to further coordination with the Town on specific location of interconnections of the street network and the Owner obtaining legal access to the adjacent parcels without imposition of any fees or costs, other than customary fees and costs the Owner incurs in negotiating such access with the owners of adjacent parcels.

Revels Road and the Spine Road must be public, dedicated to and maintained by the Town. Revels Road and the Spine Road must have a minimum 90-foot right-of-way, 2-foot curb and gutter, and a minimum 24-foot-wide pavement with minimum 12-foot travel lanes. All other internal neighborhood roads must have a minimum 50-foot right-of-way, curb and gutter, and a minimum 24-foot-wide pavement with minimum 12-foot travel lanes, which may be reduced to 11-foot travel lanes when adjacent to on-street parking. All alley roads must have a minimum 22-foot right-of-way, curb and gutter, and a minimum 20-foot-wide pavement. Provision must be made in the rights-of-way for underground utilities.

All portions of the development must be accessible by a direct, convenient, attractive, safe, and comfortable system of pedestrian facilities. The development must provide appropriate pedestrian amenities. A multimodal trail with minimum width of twelve feet must be constructed within each phase of the Project consistent with Conceptual Land Use Plan and the Town's bicycle/pedestrian plan. The multimodal trail and all sidewalks within rights-of-way must be dedicated to and maintained by the Town.

#### **Transportation Concurrency and Proportionate Fair Share Mitigation**

The Project must undergo concurrency review. The Owner must complete and submit for review prior to final development order a traffic-impact analysis.

If the results of the traffic-impact analysis require any mitigation for traffic generation, the Town and the Owner will work together and with any other applicable jurisdiction as required by applicable law to address such mitigation requirements through Owner's funding of its proportionate fair share of traffic improvements. Payment of the Owner's fair share must be made in pro-rata amounts upon the issuance of each building permit.

- (k) **Schools**. The Project must apply for concurrency review at Lake County Public Schools. The school district has a specific application process. The Project must be shown to have appropriate school concurrency before building permits are issued.
- (l) **Landscaping Requirements**. All landscaping and buffer requirements shall be in accordance with the LDC and as illustrated on the Conceptual Land Use Plan with the exception of the following:
  - 1. All buffer, street, and canopy trees planted at the Project will be a minimum of a 2" caliper;
  - 2. the Owner shall require homebuilders to plant at least one canopy tree for each single-family lot of at least 3" DBH; and
  - 3. the developer will replace the equivalent of 30% of total tree-inches removed.

All trees planted at the Project shall adhere to the current guidelines established by the Florida Grades and Standards for nursery-grown trees and must be Florida grade #1 or better.

Developer must install street trees along the roadway where common areas abuts the road as required by the LDC.

- (m) **Tree Protection**. Under no circumstances may any tree, regardless of size or species, be removed from any designated wetland or conservation easement. Trees proposed to be maintained on -site must comply with LDC requirements. No construction activity, equipment or material is permitted inside a tree protection barrier.
- (n) **Lighting**. Decorative street lighting (Sanibel fixture, a Duke Energy standard fixture) must be installed (i) at every intersection, (ii) at the end of each cul-de-sac, and (iii) at intervals of 300 feet or as approved otherwise by the Town Manager. Street lighting must be installed by the Owner. All lighting must be directional, shielded lighting designed to minimize light pollution. All lighting must be maintained by the HOA.
  - (o) **Utilities**. All utilities must be underground.
- (p) **Signage**. Entrance signs and informational signage may be located in buffers, setbacks/and or signage easements as approved by the Planning and Zoning Board. The Owner shall present a sign plan for review and approval by the Planning and Zoning Board with the final site plan for each phase of the Project. The Town Council has approved use by the Owner and/or builder(s) of vertical marketing flags, also known as feather banners, with the following stipulations:
  - 1. Feather banners must be placed no less than 200 feet apart.
  - 2. A maximum of 10 feather banners, in total.
  - 3. Feather banners cannot be placed within the right of way.
  - 4. Feather banners cannot be located offsite of PUD property.
  - 5. Feather banners cannot exceed 12 feet in height.
  - 6. Feather banners must be replaced or removed if they become faded, torn, or tattered.
  - 7. Feather banners must be removed when 90% of the homes in the development have received building permit approval.

Billboards and pole signs are prohibited. Unless defined differently in the LDC, a pole sign is a permanent sign supported by at least one upright pole, pylon, or post secured to the ground, with the bottom of the sign face four feet or higher above the finished grade. All additional signage not previously approved must be in compliance with the requirements in the LDC.

- (q) **Maintenance of Common Areas**. Maintenance of all common areas within the Project is the responsibility of the homeowners' association(s) for the affected subdivision.
  - (r) **Prohibited Uses**. No manufactured or modular homes are allowed.
- **Section 2.** Amendments. Any amendments to the Conceptual Land Use Plan that occur after the effective date of this Agreement shall take effect only if and when approved by the Town

Council or Town staff as applicable. Major amendments shall include items such as changes to the location of individual land uses; any increase in the total number of residential units; or relocation of roads and routes for pedestrian and bicycle facilities. Major amendments shall be approved by the Town Council in the manner required by law or otherwise as determined by Town Council, which may include public notice(s) and hearing(s). Minor amendments shall include items such as minor adjustments of roads, trails and pedestrian ways based on more detailed site-specific data; modifications to the phasing schedule; adjustments to utility locations based on more detailed engineering data; or adjustments to parks and open space based on more detailed subdivision design. Minor amendments may be approved by the Town Manager without referral to the Planning and Zoning Board or Town Council. Whether a proposed amendment is major or minor will be determined by the Town Manager. Minor amendments to the Conceptual Land Use Plan shall automatically be incorporated into this Agreement and shall modify or replace the Conceptual Land Use Plan, without the necessity for an amendment to this Agreement.

**Section 3. Notices**. All notices or payments required to be made hereunder shall be made at the following addresses:

To Town: Sean O'Keefe, Town Manager

Town of Howey-in-the-Hills 101 North Palm Avenue

Howey-in-the-Hills, FL 34737

sokeefe@howey.org

With copies to: John Brock, CMC, Town Clerk

Town of Howey-in-the-Hills 101 North Palm Avenue Howey-in-the-Hills, FL 34737

jbrock@howey.org

Thomas J. Wilkes, Town Attorney

Gray Robinson, P.A.

301 East Pine Street, Suite 1400

Orlando, FL 32801

twilkes@gray-robinson.com

To Owner: Jason Humm

1170 Peachtree Street NE, Suite 1150

Atlanta, GA 30309

jhumm@turnstonegroup.com

With copies to: Rhea Lopes, AICP

RVI Planning + Landscape Architecture 10150 Highland Manor Dr, Suite 450

Tampa FL 33610

rlopes@rviplanning.com

Mike Ripley Land Advisors 399 Carolina Ave, Suite 200 Winter Park, Florida 32789 MRipley@landadvisors.com

Jonathan Huels Lowndes 215 North Eola Drive Orlando, Florida 32801 Jonathan.huels@lowndes-law.com

- **Section 4. Severability**. If any provision or portion of this Agreement is declared by a court of competent jurisdiction to be void, unconstitutional, or unenforceable, then all remaining provisions and portions of this Agreement shall remain in full force and effect. To that end, this Agreement is declared to be severable.
- **Section 5. Binding Effect**. This Agreement runs with the land and is binding on and enforceable by and against the parties hereto and all their successors in interest. However, no Lot Owner shall have the obligations imposed on the Owner as the developer of the Project under this Agreement. For that purpose, a "Lot Owner" means an end-user of a lot created within the Property with a completed residential unit constructed thereon, for which a certificate of occupancy has been issued. Each party covenants to each other party that this Agreement is a legal, valid, and binding agreement, enforceable against the party in accordance with its terms.
- Section 6. Negotiated Agreement. The land uses, densities, intensities, and other conditions of approval of the Project have been negotiated and agreed to by the Owner and the Town. The Conceptual Land Use Plan and this Agreement together constitute an agreement between the parties with the knowledge that the Owner's successors in title, the future homeowners, and other landowners within the Property, as well as the Town and its affected property owners and residents, all will rely justifiably on the agreed-to land uses, densities, and intensities authorized hereby for the Property. For that reason, the Owner and the Owner's successors in interest have the contract right to develop the PUD with the uses, densities, and intensities approved by the Town, subject to the restrictions and requirements in the conditions of approval set forth in this Agreement. Neither the Owner (and its successors in interest) nor the Town shall have the right in the future to rezone or downzone the property, or otherwise alter the uses, densities and intensities, or delete, waive or amend any conditions of approval except through an amendment to the Plan negotiated and approved by the Town Council and the owner of the then-subject parcel. This section shall survive the termination and expiration of this Agreement.

#### Section 7. Homeowners' Association(s).

(a) Association Responsibilities. A homeowner's association and/or a property owner's association ("HOA") must be created by the Owner. Membership in the HOA shall be mandatory for all property owners within the Project. The HOA shall be responsible for

maintaining all parks, open-space and buffer areas, streetlights, stormwater-management areas and drainage systems, entrance features, boundary walls and/or fences, access tracts, and landscaped tracts within the Project.

(b) **Requirement for Plat Recording**. Before a plat may be recorded for the Property and the Project, the Owner shall furnish to the Town copies of the pertinent documents for the homeowners' or property owners' association or associations, plus the covenants, conditions and restrictions for the Property, setting forth the requirements and restrictions enumerated in this section 7 and other applicable parts of this Agreement.

#### **Section 8.** Additional Requirements.

- (a) Letter of credit. Construction and dedication to the Town of the public facilities and improvements required under this Agreement for each phase of the Project will be a condition precedent to final plat approval for such phase. In lieu of construction and dedication, however, the Owner may post a letter of credit or performance bond with the Town for 125% of the cost of such improvements not completed at the time of plat, in which event this condition precedent to final plat approval will be deemed satisfied.
- (b) Conveyances to the Town. Property dedicated or otherwise conveyed to the Town under this Agreement must be free and clear of encumbrances unless and to the extent an encumbrance is acceptable to the Town. Encumbrances discovered after the Effective Date of this Agreement must be removed or resolved by the Owner or its successor developer prior to dedication or conveyance of the affected property to the Town.
- (c) Changes in status of land. Until completion of the Project, the Owner or its successor developer of the Project has a continuing duty (i) to disclose promptly to the Town all changes in ownership, encumbrances, and other matters of record affecting the Property and (ii) to resolve all issues, title or otherwise, that may be identified by the Town as a result of such changes. Failure to disclose such changes or to resolve resulting issues may result in delay in issuance of development permits.
- (d) **Developer representations binding**. If at Town Council hearings on the approval of the Project the Owner makes a written or oral promise or representation, and if the promise or representation was relied upon by Town Council in approving the Project or otherwise acted to induce or materially influence Town Council in its vote to approve the Project, the promise or representation is a condition of approval of the Project. The promise or representation is binding on the Owner and its successors and enforceable by the Town against the Owner and its successors as if set forth fully in this Agreement.
- **Section 9.** Governing Law. This Agreement shall be governed by the laws of the State of Florida. Venue for any judicial proceeding pertaining to the Agreement shall be in the Fifth Judicial Circuit of Florida, in Lake County, Florida.

## **Section 10. Effective Date; Termination.**

- (a) **Effective Date**. This Agreement shall take effect upon the Effective Date above, or on the date when it has been executed by both the Town Council and the Owner, whichever is later.
- (b) **Termination**. This Agreement shall remain in effect unless and until terminated under one of the following conditions:
- 1. If as of the second anniversary of the Effective Date of this Agreement an Owner's contract right to treatment and disposal services by the CDD or Wastewater Utility, as required under Section 1(g)1 above, has not taken effect, the Town may terminate this Agreement by vote of its Town Council. The vote must occur no later than (i) the third anniversary of the Effective Date or (ii) the CDD or Wastewater Utility Contract Date, whichever occurs first. The "Contract Date" is the date on which the Owner's contract right to treatment and disposal services by the CDD or Wastewater Utility takes effect.
- 2. If as of the second anniversary of the Contract Date no building permit for a residential unit in the Project has been issued, the Town may terminate this Agreement by vote of its Town Council. The vote must occur no later than (i) the third anniversary of the Contract Date or (ii) the date a building permit is issued, whichever occurs first.
- 3. If as of the fifth anniversary of the Contract Date no building permit for a residential unit in the second phase of the Project has been issued, the Town may terminate this Agreement by vote of its Town Council, but only as it applies to development of the second phase. The vote must occur no later than (i) the sixth anniversary of the Contract Date or (ii) the date a building permit is issued for a residential unit in the second phase, whichever occurs first. Termination of the Agreement for this reason will not act to preclude the Owner or its successor from completing the first phase of the Project.
- 4. If as of the tenth anniversary of the Contract Date no building permit for a residential unit in the third phase of the Project has been issued, the Town may terminate this Agreement by vote of its Town Council, but only as it applies to development of the third phase. The vote must occur no later than (i) the eleventh anniversary of the Contract Date or (ii) the date a building permit is issued for a residential unit in the third phase, whichever occurs first. Termination of the Agreement for this reason will not act to preclude the Owner or its successor from completing the first or second phase of the Project.

Termination of this Agreement, in whole or in part, under this section shall be without prejudice to the Owner or its successor to apply for Town approvals to undertake or continue development of the Property in accordance with the circumstances and land-development regulations then existing in the Town.

- **Section 11. Recording**. This Agreement shall be recorded by the Town, at the Owner's expense, in the Public Records of Lake County, Florida, and shall constitute a covenant running with the land.
- **Section 12. Authority**. This Agreement is entered into by the Town under the home-rule powers granted to it by the Florida constitution (including specifically Article VIII, Section 2(b) thereof), the home-rule powers granted municipalities by statute (including specifically Chapter

166, Florida Statutes), and the Town's Charter. This Agreement does not constitute a "development agreement" under the Florida Local Government Development Agreement Act.

**Section 13. Entire Agreement**. This Agreement constitutes the entire agreement of the parties with respect to the transactions contemplated herein. It supersedes all prior understandings or agreements between the parties relating to the Property and the Project. No amendment to the terms of this Agreement shall be effective unless in writing signed by all parties hereto. Amendments to this Agreement will take effect and will be binding against the Town only if approved by a vote of the Town Council.

**Section 14. Waiver**. The failure of a party hereto to insist upon or enforce any right or privilege granted hereunder shall not constitute or operate as a waiver thereof and nothing shall constitute a waiver of any party's right to insist upon strict compliance with the terms hereof. However, any party may waive in writing the benefit of any provision or condition for its benefit which is contained herein. Waivers of material provisions of either this Agreement or the Town's LDC will be valid and binding against the Town only if approved by a vote of the Town Council.

[ Signature pages follow ]

**IN WITNESS WHEREOF**, the parties are signing this Agreement as of the Effective Date or, if later, the date by which both parties have fully executed this Agreement.

## TOWN OF HOWEY IN THE HILLS, **FLORIDA** By: its Town Council By: \_\_\_\_\_ Hon. Martha McFarlane, Mayor Attest: By: John Brock, CMC, Town Clerk Approved as to form and legality: (for the use and reliance of the Town only) Thomas J. Wilkes, Town Attorney STATE OF FLORIDA COUNTY OF LAKE The foregoing instrument was executed, sworn to and acknowledged before me this \_\_\_\_\_ , 2023, by Martha McFarlane, as Mayor of the Town of Howey in day of the Hills. Signature of Notary (SEAL) Name of Notary Public (Typed, Printed or stamped)

Type of Identification Produced:

Personally Known OR Produced Identification

**IN WITNESS WHEREOF**, the parties have executed this instrument as of the day and year first above written.

Signed, sealed and delivered in the presence of:

"WITNESS	SES"	"OWNER"
Printed Name:		ASF TAP FL I, LLC, a Delaware limited liability company
		By:Printed Name:As its:
Printed Nam	ne:	
STATE OF F COUNTY OI	FLORIDA	_
	_ physical presence or _	ent was executed, sworn to and acknowledged before me online notarization, this day of
2022, by LLC., a Dela	ware limited liability con	, as of <b>ASF TAP FI</b> npany, on its behalf.
(SEAL)		Signature of Notary Public
		Name of Notary Public (Typed, Printed or stamped)
Personally Kı	nown <i>OR</i> Produced	Identification
<i>y</i>		(Type of Identification Produc

### Attachment A To MISSION RISE PUD DEVELOPMENT AGREEMENT

**LEGAL DESCRIPTION** 

# Attachment B To MISSION RISE PUD DEVELOPMENT AGREEMENT

### **CONCEPTUAL LAND USE PLAN**

This instrument prepared by and should be returned to: Thomas J. Wilkes GrayRobinson 301 East Pine Street, Suite 1400 Orlando, Florida 32801

### MISSION RISE PUD DEVELOPMENT AGREEMENT

This MISSION RISE PUD DEVELOPMENT AGREEMENT ("Agreement") is made as of the \_\_\_\_\_\_ day of \_\_\_\_\_\_\_, 2023 ("Effective Date"), between the Town of Howey in the Hills, Florida, a Florida municipal corporation (the "Town"), and ASF TAP FL I, LLC, a Delaware limited liability company (the "Owner").

RECITALS

A The Owner owns approximately 243 acres of property more particularly described in Attachment A to this Agreement ("the Property").

B. The Property is within the corporate limits of the Town. The Town has assigned the Property a future-land-use designation of Village Mixed Use and has zoned the Property for PUD - Planned Unit Development.

C. The Owner intends to develop and use the Property as a mixed-use planned development consisting of single-family residential, civic and public uses more specifically set forth herein ("the Project"), to be known as the "Mission Rise PUD."

### NOW, THEREFORE, the Town and the Owner agree as follows:

**Section 1.** Land development and uses. Development and use of the Property is subject to the following conditions, requirements, restrictions, and terms:

conditions of approval negotiated between them for the development and use of the Property as

The Town and Owner enter into this Agreement to set forth the terms and

(a) **General**. Development of the Project and use of the Property shall be governed by this Agreement, the Town's Comprehensive Plan, the Town's Land Development Code ("LDC") and Code of Ordinances ("Town Code"), and all other applicable state laws and regulations and Town ordinances and rules. Where in conflict, the terms of this Agreement shall supersede and prevail over the LDC and Town Code, but only to the extent of the conflict.

the Mission Rise PUD.

In the Conceptual Land Use Plan for the Project the term "conceptual" means the location of land uses on the site, including areas for residential development, open space, stormwater management, parks, and roads in relation to the site area and other uses on the site. Subsequent plan development may refine the details based on detailed engineering design. "Conceptual" does not mean or contemplate the modification of proposed housing types or the relocation of land uses and roads other than minor adjustments dictated by engineering needs and best practices.

Unless otherwise noted, the definition of terms in this Agreement shall be the same as the definitions set forth in the LDC.

- (b) **Phasing**. The Project will be developed in multiple phases, as shown on the Conceptual Land Use Plan. Each phase must be designed and built to operate independently with all necessary public services and utilities infrastructure, including roads, multimodal trails and master stormwater systems, consistent with Conceptual Land Use Plan. Revisions to the phasing schedule shall be considered as minor amendments to this Agreement, which may be approved by Town Council with no formal amendment to this Agreement required.
  - (c) **Purpose**. The purpose of the Mission Rise PUD is to:
  - 1. Create an attractive and high-quality single-family housing development compatible with the scale and character of existing residential development and land uses in the Town;
  - 2. Develop a residential area that is safe, comfortable and attractive for and to pedestrians;
  - 3. Create a community with direct visual and physical access to open land, with a strong community identity, and with amenities in the form of community open space;
  - 4. Provide a network of open space for future homeowners; and
  - 5. Provide a variety of lot sizes and housing choices for diverse age and income groups and residential preferences.
- (d) **Land uses**. The Conceptual Land Use Plan for the Project is contained in Attachment B and is an integral part of the approval of the Project. Elements in the Concept Plan include single-family detached homes, civic uses, multimodal trails and approximately 65.469.4 acres of open space.

### (e) **Development standards**.

### Setbacks

The setbacks for single family residential lots shall be as shown on the Conceptual Land Use Plan for the Project.

### Lot Size

A range of lot sizes shall be provided in order to create variety and offer opportunity for different income households. Minimum lot size will be 55' x 120'. The Project may

consist of up to  $\frac{592499}{2}$  total single-family residential detached lots of 55' x 120' and 75' x 120'.

### **Dwelling Size**

The minimum dwelling size for all single-family residences shall be 1,400 square feet of heated/air-conditioned space under roof plus a two-car garage with a minimum of 400 square feet. Maximum dwelling size shall be 4,600 square feet of heated/air-conditioned space under roof.

#### Lot Width

The minimum lot width at building line shall be 55 feet <u>for 55-foot wide lots and 75 feet</u> <u>for 75-foot wide lots</u> with a minimum street frontage of 30 feet.

### **Lot Coverage**

Lots shall have a maximum lot coverage of 60% based on the proposed setbacks shown on the Conceptual Land Use Plan for the Project.

### **Height of Structures**

No residential structure may exceed 35 feet in height

### **Building Design**

Building design shall be in accordance with the Architectural Requirements of the Town's LDC and will comply specifically with the design requirements of LDC Sections 4.06.02 and 4.06.03.

The following principles seek to promote a high-quality development that will create a sense of place and community through the development of the site.

- Housing styles, shapes and materials shall meet the Towns Land Development Regulations.
- The different housing types shall be integrated architecturally in order to give the development a harmonious appearance.
- The creation of visual richness shall be considered when choosing materials and details. Local characteristics are encouraged.
- Side entrances for garages are encouraged.
- A variety of roof heights, pitches and materials will be encouraged.
- Landscaping shall be incorporated into the overall design as a means of linking the development areas with the open spaces.
- Each exterior wall for a single-family home must be a minimum of two materials and a minimum of two colors. Primary facades must have one base color and a complementary wall material may be used to meet the second color requirement.
- Block face restrictions may be reduced to 300 linear feet. The same house model
  may not be used more than three times within a single block face. For purposes of
  this requirement, a different house model is a different floor plan, not the same
  floor plan flipped in a different direction and not the same floor plan with a
  different exterior treatment.

- (f) **Wetlands**. Impacts to wetlands, if any, and wetland buffering shall be subject to the <u>Town Land Development Regulations and well as St.</u> Johns River Water Management District regulations.
- (g) **Potable water, wastewater, and reclaimed water**. For potable water and wastewater service, well and septic systems are not allowed. The Project must be connected to and served by the Town's potable-water and wastewater systems prior to a certificate of occupancy being issued for a structure in the Project (except temporary construction uses).

Except as may be set forth otherwise in this Agreement, the Owner must install all on-site potable-water, wastewater, and reclaimed-water infrastructure and connect to central water and wastewater systems, and to the Town's reclaimed-water system when available at the Property boundary, all at no cost to the Town. The Owner must pay potable-water, wastewater, and reclaimed-water capital and connection charges, impact fees, and other Town rates, fees, and charges, either applicable currently or in the future.

1. Potable Water. The Town will provide potable water, and may in the future provide reclaimed water, to the Project in accordance with its applicable ordinances, resolutions, operating regulations, policies and procedures. The Town will provide potable water to the Property in sufficient quantities for development of the Project as contemplated herein, subject to the limitations and requirements of permits issued to the Town from time to time by the St. John's River Water Management District in connection with water consumption.

The Owner shall construct, at no expense to the Town, all off-site potable-water-system facilities, lines, pumps, valves, control structures, and appurtenances (other than water-treatment plants) necessary to serve the Project. The construction and route of off-site lines and other structures shall be done according to engineering plans prepared by the Owner and approved by the Town Manager. Potable water shall not be used for irrigation.

2. Wastewater. The Town will provide wastewater-collection and transmission service to the Project, transmitting Project wastewater to the Central Lake Community Development District ("CDD") or another wastewater utility service provider with available capacity to treat and dispose the Project's wastewater ("Wastewater Utility"). The Owner must obtain from the CDD or Wastewater Utility a contract right for the Project to receive treatment and disposal of its wastewater at such provider's treatment and disposal facilities.

The Owner shall construct, at no expense to the Town, all off-site wastewater-system facilities, lines, lift stations, pumps, valves, control structures, and appurtenances (other than wastewater-treatment plants and disposal facilities) necessary to serve the Project. The construction and route of off-site lines, lift stations, pumps, and other structures shall be done according to engineering plans prepared by the Owner and approved by the Town Manager.

3. Town Option to Oversize Water and Wastewater Lines. Within 270 days of the effective date of the Owner's contract right to receive wastewater-treatment and -disposal service, as referenced above In conjunction with the review and processing of the preliminary subdivision plans for each phase of the Project, the Town may elect to oversize the off-site lines,

pumps, improvements, or other facilities or appurtenances for the Town's water or wastewater system, or for both, necessary to serve such phase. If the Town elects to oversize one or both systems, it must inform the Owners in writing of the specifications for the oversizing(s) within the 270-day periodprior to or as part of the Town's first round of review comments on the preliminary subdivision plan application. The Town shall reimburse the Owner for the difference in the increase in cost of design, materials and construction to oversize the improvements based on plans and cost estimates provided by the Owner to the Town and approved by the Town Manager, which approval shall not be unreasonably withheld, conditioned or delayed. The Town shall reimburse the Owners for the difference in the costs within 60 days following (i) completion of the improvements and (ii) receipt by the Town of documentation reasonably demonstrating that the Owner has completed the work and has incurred the costs attributable to the over-sizing, all in keeping with the plans and cost estimate previously approved by the Town Manager.

- 4. Permit-Induced Costs, Restrictions, Requirements, and Risks. Under state and federal laws and regulations, the Town may provide its potable-water and wastewater services to the Property and the Owner and its successors only if the Town first has been issued certain required permits. The Owner acknowledges that the permits are inevitably conditioned with requirements and restrictions that typically impose costs and risks. The Owner further acknowledges that, for the Town to operate its potable-water and wastewater systems in an orderly, dependable, and cost-effective manner, the Town must have the ability legally to spread the costs and risks among customers and property owners benefiting from the services. The Owner acknowledges, therefore, that (i) from time to time the Town may impose rates, fees, and charges and may issue potable-water system and wastewater-system regulations and policies that impose restrictions and requirements on its customers and benefiting property owners, such as the Owner and it successors, and (ii) so long as the Owner or successors are required to pay only their fair share for such rates, fees, and charges, then the imposition of such rates, fees, and charges and the issuance of such system regulations are not prohibited by or otherwise a breach of this Agreement.
- 5. Reclaimed Water. The Owner must install reclaimed water lines as required by the Town's Code of Ordinances, and shall obtain reclaimed-water service for the Project when the Town constructs reclaimed-water lines to the Project's boundaries. Until such time as the Town supplies reclaimed water, the Owner and its successors shall use the reclaimed water lines to irrigate properties within the Project boundaries, but only with stormwater from on-site stormwater-retention ponds or with sources other than potable water as may be approved by the Town and St. John's River Water Management District. Except for installation of reclaimed lines at the time of development as noted above, connection to reclaimed water after the development of the Project may not result in additional costs to the Owner or developer.
  - (h) **Solid Waste**. Solid Waste collection shall be pursuant to Town regulations.
- (i) **Drainage**. The maintenance, repair, and replacement of the drainage system shall be the responsibility of the homeowners association(s).

### (j) Transportation

#### Street and Sidewalks

The Project must have a connected street system that serves vehicles, pedestrians and bicycles and that connects to recreation facilities and adjacent residential/community areas. There must be ingress and egress points to Revels Road, County Number Two Road and Orange Blossom Road at final buildout of the Project in the approximate location shown on the Conceptual Land Use Plan. The access at County Road Number Two must be a full intersection subject to review and approval by Lake County. Future access connections at the western and eastern boundaries of the property will also be provided, as shown on the Conceptual Land Use Plan, subject to further coordination with the Town on specific location of interconnections of the street network and the Owner obtaining legal access to the adjacent parcels without imposition of any fees or costs, other than customary fees and costs the Owner incurs in negotiating such access with the owners of adjacent parcels.

Revels Road and the Spine Road must be public, dedicated to and maintained by the Town. Revels Road and the Spine Road must have a minimum 90-foot right-of-way, 2-foot curb and gutter, and a minimum 24-foot-wide pavement with minimum 12-foot travel lanes. All other internal neighborhood roads must have a minimum 50-foot right-of-way, curb and gutter, and a minimum 24-foot-wide pavement with minimum 12-foot travel lanes, which may be reduced to 11-foot travel lanes when adjacent to on-street parking. All alley roads must have a minimum 22-foot right-of-way, curb and gutter, and a minimum 20-foot-wide pavement. Provision must be made in the rights-of-way for underground utilities.

The Project must have a connected street system that serves vehicles, pedestrians and bicycles and that connects to recreation facilities and adjacent residential/community areas. Revels Road, the Spine Road and all neighborhood roads within the Project must be public, dedicated to and maintained by the Town. No streets in the Project may be gated or otherwise restricted or obstructed by the Owner, by a homeowners' or property owners' association, or by any other person or entity.

All portions of the development must be accessible by a direct, convenient, attractive, safe, and comfortable system of pedestrian facilities. The development must provide appropriate pedestrian amenities. A multimodal trail with minimum width of twelve feet must be constructed within each phase of the Project consistent with Conceptual Land Use Plan and the Town's bicycle/pedestrian plan. The multimodal trail and all sidewalks within rights-of-way must be dedicated to and maintained by the Town.

### Transportation Concurrency and Proportionate Fair Share Mitigation

The Project must undergo concurrency review. The Owner must complete and submit for review prior to final development order a traffic-impact analysis.

If the results of the traffic-impact analysis require any mitigation for traffic generation, the Town and the Owner will work together and with any other applicable jurisdiction as required by applicable law to address such mitigation requirements through Owner's funding of its proportionate fair share of traffic improvements. Payment of the Owner's fair share must be made in pro-rata amounts upon the issuance of each building permit.

- (k) **Schools**. The Project must apply for concurrency review at Lake County Public Schools. The school district has a specific application process. The Project must be shown to have appropriate school concurrency before building permits are issued.
- (l) **Landscaping Requirements**. All landscaping and buffer requirements shall be in accordance with the LDC and as illustrated on the Conceptual Land Use Plan with the exception of the following:
  - 1. All buffer, street, and canopy trees planted at the Project will be a minimum of a 2" caliper;
  - 2. the Owner shall require homebuilders to plant at least one canopy tree for each single-family lot of at least 3" DBH; and
  - 3. the developer will replace the equivalent of 30% of total tree-inches removed.

All trees planted at the Project shall adhere to the current guidelines established by the Florida Grades and Standards for nursery-grown trees and must be Florida grade #1 or better.

Developer must install street trees along the roadway where common areas abuts the road as required by the LDC.

- (m) **Tree Protection**. Under no circumstances may any tree, regardless of size or species, be removed from any designated wetland or conservation easement. Trees proposed to be maintained on -site must comply with LDC requirements. No construction activity, equipment or material is permitted inside a tree protection barrier.
- (n) **Lighting**. Decorative street lighting (Sanibel fixture, a Duke Energy standard fixture) must be installed (i) at every intersection, (ii) at the end of each cul-de-sac, and (iii) at intervals of 300 feet or as approved otherwise by the Town Manager. Street lighting must be installed by the Owner. All lighting must be directional, shielded lighting designed to minimize light pollution. All lighting must be maintained by the HOA.
  - (o) **Utilities**. All utilities must be underground.
- (p) **Signage**. Entrance signs and informational signage may be located in buffers, setbacks/and or signage easements as approved by the Planning and Zoning Board. The Owner shall present a sign plan for review and approval by the Planning and Zoning Board with the final site plan for each phase of the Project. The Town Council has approved use by the Owner and/or builder(s) of vertical marketing flags, also known as feather banners, with the following stipulations:
  - 1. Feather banners must be placed no less than 200 feet apart.
  - 2. A maximum of 10 feather banners, in total.
  - 3. Feather banners cannot be placed within the right of way.
  - 4. Feather banners cannot be located offsite of PUD property.

- 5. Feather banners cannot exceed 12 feet in height.
- 6. Feather banners must be replaced or removed if they become faded, torn, or tattered.
- 7. Feather banners must be removed when 90% of the homes in the development have received building permit approval.

Billboards and pole signs are prohibited. Unless defined differently in the LDC, a pole sign is a permanent sign supported by at least one upright pole, pylon, or post secured to the ground, with the bottom of the sign face four feet or higher above the finished grade. All additional signage not previously approved must be in compliance with the requirements in the LDC.

- (q) **Maintenance of Common Areas**. Maintenance of all common areas within the Project is the responsibility of the homeowners' association(s) for the affected subdivision.
  - (r) **Prohibited Uses**. No manufactured or modular homes are allowed.

Section 2. Amendments. Any amendments to the Conceptual Land Use Plan that occur after the effective date of this Agreement shall take effect only if and when approved by the Town Council or Town staff as applicable. Major amendments shall include items such as changes to the location of individual land uses; any increase in the total number of residential units; or relocation of roads and routes for pedestrian and bicycle facilities. Major amendments shall be approved by the Town Council in the manner required by law or otherwise as determined by Town Council, which may include public notice(s) and hearing(s). Minor amendments shall include items such as minor adjustments of roads, trails and pedestrian ways based on more detailed site-specific data; modifications to the phasing schedule; adjustments to utility locations based on more detailed engineering data; or adjustments to parks and open space based on more detailed subdivision design. Minor amendments may be approved by the Town Manager without referral to the Planning and Zoning Board or Town Council. Whether a proposed amendment is major or minor will be determined by the Town Manager. Minor amendments to the Conceptual Land Use Plan shall automatically be incorporated into this Agreement and shall modify or replace the Conceptual Land Use Plan in Attachment B to the extent of such amendment to the Conceptual Land Use Plan, without the necessity for an amendment to this Agreement.

**Section 3. Notices**. All notices or payments required to be made hereunder shall be made at the following addresses:

To Town: Sean O'Keefe, Town Manager

Town of Howey-in-the-Hills 101 North Palm Avenue Howey-in-the-Hills, FL 34737

sokeefe@howey.org

With copies to: John Brock, CMC, Town Clerk

Town of Howey-in-the-Hills 101 North Palm Avenue Howey-in-the-Hills, FL 34737

jbrock@howey.org

Thomas J. Wilkes, Town Attorney

Gray Robinson, P.A.

301 East Pine Street, Suite 1400

Orlando, FL 32801

twilkes@gray-robinson.com

To Owner: Jason Humm

1170 Peachtree Street NE, Suite 1150

Atlanta, GA 30309

jhumm@turnstonegroup.com

With copies to: Rhea Lopes, AICP

RVI Planning + Landscape Architecture 10150 Highland Manor Dr, Suite 450

Tampa FL 33610

rlopes@rviplanning.com

Mike Ripley Land Advisors

399 Carolina Ave, Suite 200 Winter Park, Florida 32789 MRipley@landadvisors.com

Jonathan Huels

Lowndes

215 North Eola Drive Orlando, Florida 32801

Jonathan.huels@lowndes-law.com

**Section 4. Severability**. If any provision or portion of this Agreement is declared by a court of competent jurisdiction to be void, unconstitutional, or unenforceable, then all remaining provisions and portions of this Agreement shall remain in full force and effect. To that end, this Agreement is declared to be severable.

**Section 5. Binding Effect**. This Agreement runs with the land and is binding on and enforceable by and against the parties hereto and all their successors in interest. However, no Lot Owner shall have the obligations imposed on the Owner as the developer of the Project under this Agreement. For that purpose, a "Lot Owner" means an end-user of a lot created within the Property with a completed residential unit constructed thereon, for which a certificate of

occupancy has been issued. Each party covenants to each other party that this Agreement is a legal, valid, and binding agreement, enforceable against the party in accordance with its terms.

**Section 6. Negotiated Agreement.** The land uses, densities, intensities, and other conditions of approval of the Project have been negotiated and agreed to by the Owner and the Town. The Conceptual Land Use Plan and this Agreement together constitute an agreement between the parties with the knowledge that the Owner's successors in title, the future homeowners, and other landowners within the Property, as well as the Town and its affected property owners and residents, all will rely justifiably on the agreed-to land uses, densities, and intensities authorized hereby for the Property. For that reason, the Owner and the Owner's successors in interest have the contract right to develop the PUD with the uses, densities, and intensities approved by the Town, subject to the restrictions and requirements in the conditions of approval set forth in this Agreement. Neither the Owner (and its successors in interest) nor the Town shall have the right in the future to rezone or downzone the property, or otherwise alter the uses, densities and intensities, or delete, waive or amend any conditions of approval except through an amendment to the Plan negotiated and approved by the Town Council and the owner of the then-subject parcel. This section shall survive the termination and expiration of this Agreement.

### Section 7. Homeowners' Association(s).

- (a) **Association Responsibilities**. A homeowner's association and/or a property owner's association ("HOA") must be created by the Owner. Membership in the HOA shall be mandatory for all property owners within the Project. The HOA shall be responsible for maintaining all parks, open-space and buffer areas, streetlights, stormwater-management areas and drainage systems, entrance features, boundary walls and/or fences, access tracts, and landscaped tracts within the Project.
- (b) **Requirement for Plat Recording**. Before a plat may be recorded for the Property and the Project, the Owner shall furnish to the Town copies of the pertinent documents for the homeowners' or property owners' association or associations, plus the covenants, conditions and restrictions for the Property, setting forth the requirements and restrictions enumerated in this section 7 and other applicable parts of this Agreement.

### Section 8. Additional Requirements.

- (a) **Letter of credit**. Construction and dedication to the Town of the public facilities and improvements required under this Agreement for each phase of the Project will be a condition precedent to final plat approval for such phase. In lieu of construction and dedication, however, the Owner may post a letter of credit or performance bond with the Town for 125% of the cost of such improvements not completed at the time of plat, in which event this condition precedent to final plat approval will be deemed satisfied.
- (b) **Conveyances to the Town**. Property dedicated or otherwise conveyed to the Town under this Agreement must be free and clear of encumbrances unless and to the extent an encumbrance is acceptable to the Town. Encumbrances discovered after the Effective Date of this Agreement must be removed or resolved by the Owner or its successor developer prior to dedication or conveyance of the affected property to the Town.

- (c) Changes in status of land. Until completion of the Project, the Owner or its successor developer of the Project has a continuing duty (i) to disclose promptly to the Town all changes in ownership, encumbrances, and other matters of record affecting the Property and (ii) to resolve all issues, title or otherwise, that may be identified by the Town as a result of such changes. Failure to disclose such changes or to resolve resulting issues may result in delay in issuance of development permits.
- (d) **Developer representations binding**. If at Town Council hearings on the approval of the Project the Owner makes a written or oral promise or representation, and if the promise or representation was relied upon by Town Council in approving the Project or otherwise acted to induce or materially influence Town Council in its vote to approve the Project, the promise or representation is a condition of approval of the Project. The promise or representation is binding on the Owner and its successors and enforceable by the Town against the Owner and its successors as if set forth fully in this Agreement.
- **Section 9.** Governing Law. This Agreement shall be governed by the laws of the State of Florida. Venue for any judicial proceeding pertaining to the Agreement shall be in the Fifth Judicial Circuit of Florida, in Lake County, Florida.

### Section 10. Effective Date; Termination.

- (a) **Effective Date**. This Agreement shall take effect upon the Effective Date above, or on the date when it has been executed by both the Town Council and the Owner, whichever is later.
- (b) **Termination**. This Agreement shall remain in effect unless and until terminated under one of the following conditions:
- 1. If as of the second anniversary of the Effective Date of this Agreement an Owner's contract right to treatment and disposal services by the CDD or Wastewater Utility, as required under Section 1(g)1 above, has not taken effect, the Town may terminate this Agreement by vote of its Town Council. The vote must occur no later than (i) the third anniversary of the Effective Date or (ii) the CDD or Wastewater Utility Contract Date, whichever occurs first. The "CDD Contract Date" is the date on which the Owner's contract right to treatment and disposal services by the CDD or Wastewater Utility takes effect.
- 2. If as of the second anniversary of the CDD Contract Date no building permit for a residential unit in the Project has been issued, the Town may terminate this Agreement by vote of its Town Council. The vote must occur no later than (i) the third anniversary of the CDD Contract Date or (ii) the date a building permit is issued, whichever occurs first.
- 3. If as of the fifth anniversary of the CDD Contract Date no building permit for a residential unit in the second phase of the Project has been issued, the Town may terminate this Agreement by vote of its Town Council, but only as it applies to development of the second phase. The vote must occur no later than (i) the sixth anniversary of the CDD Contract Date or (ii) the date a building permit is issued for a residential unit in the second phase, whichever

occurs first. Termination of the Agreement for this reason will not act to preclude the Owner or its successor from completing the first phase of the Project.

4. If as of the tenth anniversary of the CDD-Contract Date no building permit for a residential unit in the third phase of the Project has been issued, the Town may terminate this Agreement by vote of its Town Council, but only as it applies to development of the third phase. The vote must occur no later than (i) the eleventh anniversary of the CDD-Contract Date or (ii) the date a building permit is issued for a residential unit in the third phase, whichever occurs first. Termination of the Agreement for this reason will not act to preclude the Owner or its successor from completing the first or second phase of the Project.

Termination of this Agreement, in whole or in part, under this section shall be without prejudice to the Owner or its successor to apply for Town approvals to undertake or continue development of the Property in accordance with the circumstances and land-development regulations then existing in the Town.

**Section 11. Recording**. This Agreement shall be recorded by the Town, at the Owner's expense, in the Public Records of Lake County, Florida, and shall constitute a covenant running with the land.

**Section 12. Authority**. This Agreement is entered into by the Town under the home-rule powers granted to it by the Florida constitution (including specifically Article VIII, Section 2(b) thereof), the home-rule powers granted municipalities by statute (including specifically Chapter 166, Florida Statutes), and the Town's Charter. This Agreement does not constitute a "development agreement" under the Florida Local Government Development Agreement Act.

**Section 13. Entire Agreement.** This Agreement constitutes the entire agreement of the parties with respect to the transactions contemplated herein. It supersedes all prior understandings or agreements between the parties relating to the Property and the Project. No amendment to the terms of this Agreement shall be effective unless in writing signed by all parties hereto. Amendments to this Agreement will take effect and will be binding against the Town only if approved by a vote of the Town Council.

**Section 14. Waiver**. The failure of a party hereto to insist upon or enforce any right or privilege granted hereunder shall not constitute or operate as a waiver thereof and nothing shall constitute a waiver of any party's right to insist upon strict compliance with the terms hereof. However, any party may waive in writing the benefit of any provision or condition for its benefit which is contained herein. Waivers of material provisions of either this Agreement or the Town's LDC will be valid and binding against the Town only if approved by a vote of the Town Council.

[ Signature pages follow ]

**IN WITNESS WHEREOF**, the parties are signing this Agreement as of the Effective Date or, if later, the date by which both parties have fully executed this Agreement.

## TOWN OF HOWEY IN THE HILLS, FLORIDA

	By: its Town Council
	By: Hon. Martha McFarlane, Mayor
Attest:	
By:  John Brock, CMC, Town Clerk  Approved as to form and legality:	
(for the use and reliance of the Town only)	
Thomas J. Wilkes, Town Attorney	
STATE OF FLORIDA COUNTY OF LAKE	
	, sworn to and acknowledged before me this tha McFarlane, as Mayor of the Town of Howe
(SEAL)	Signature of Notary
	Name of Notary Public (Typed, Printed or stamped)
Personally Known OR Produced Identific	ation

Type of Identification Produced:

**IN WITNESS WHEREOF**, the parties have executed this instrument as of the day and year first above written.

Signed, sealed and delivered in the presence of:

"WITNESSES"	"OWNER"
Printed Name:	ASF TAP FL I, LLC, a Delaware limited liability company
	By:
	Printed Name:
	As its:
Printed Name:	
by means of physical prese, 2022, by	nent was executed, sworn to and acknowledged before me ence or online notarization, this day of, as of mited liability company, on its behalf.
(SEAL)	
(SLIL)	Signature of Notary Public
	Name of Notary Public
	(Typed, Printed or stamped)
Personally Known <i>OR</i> Produced	d Identification
-	(Type of Identification Produced)

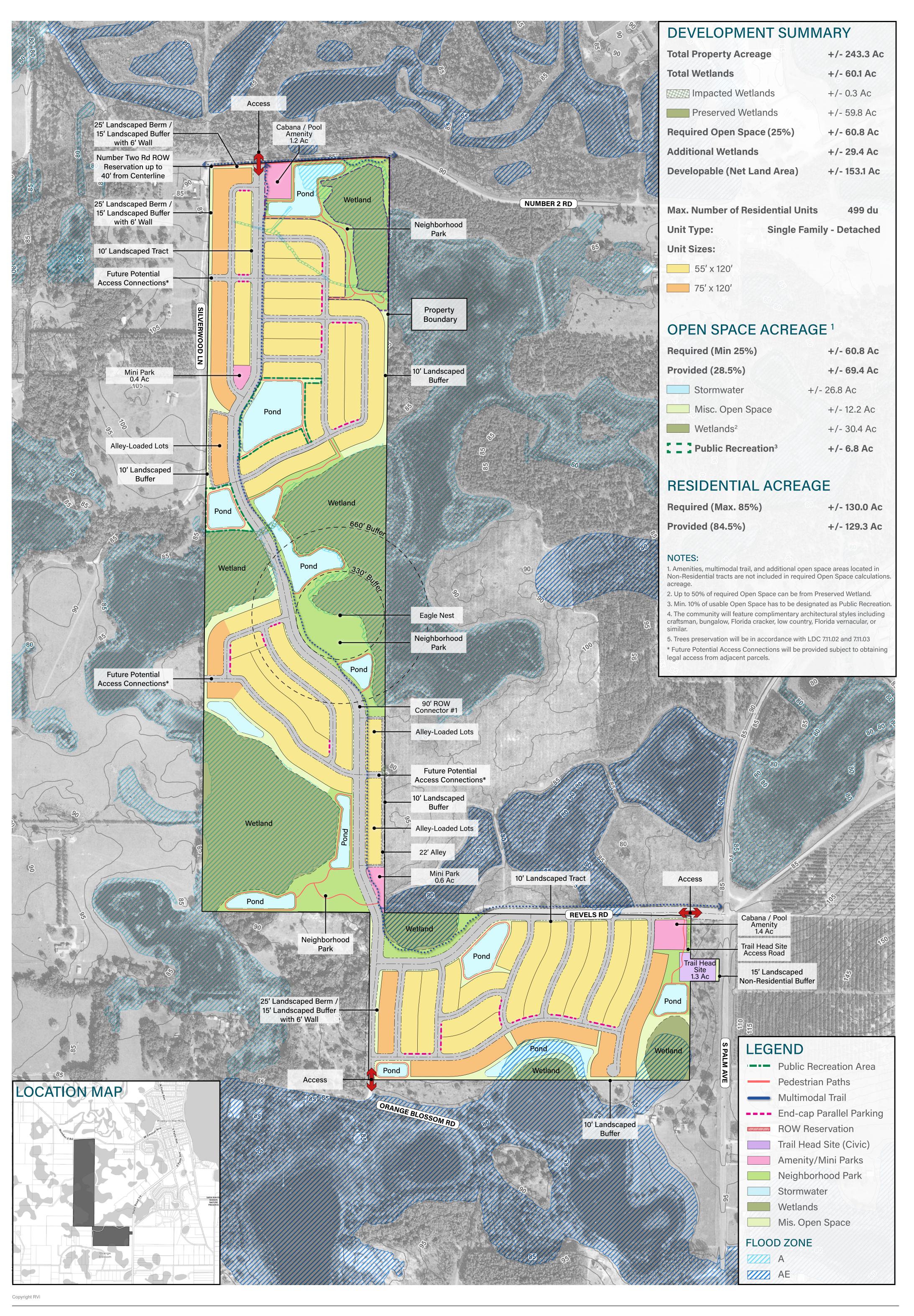
# Attachment A To MISSION RISE PUD DEVELOPMENT AGREEMENT

**LEGAL DESCRIPTION** 

# Attachment B To MISSION RISE PUD DEVELOPMENT AGREEMENT

**CONCEPTUAL LAND USE PLAN** 

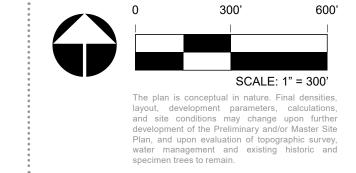
#### **Summary report:** Litera® Change-Pro for Word 10.14.0.46 Document comparison done on 9/28/2023 10:57:55 AM Style name: Lowndes **Intelligent Table Comparison:** Active Original DMS: iw://LOWNDES-DMS.IMANAGE.WORK/Active/12958008/4 **Modified DMS:** iw://LOWNDES-DMS.IMANAGE.WORK/Active/12958008/5 **Changes:** Add 15 17 **Delete** Move From 2 2 Move To 0 **Table Insert** 0 **Table Delete** 0 Table moves to 0 Table moves from Embedded Graphics (Visio, ChemDraw, Images etc.) 0 Embedded Excel 0 0 Format changes **Total Changes:** 36

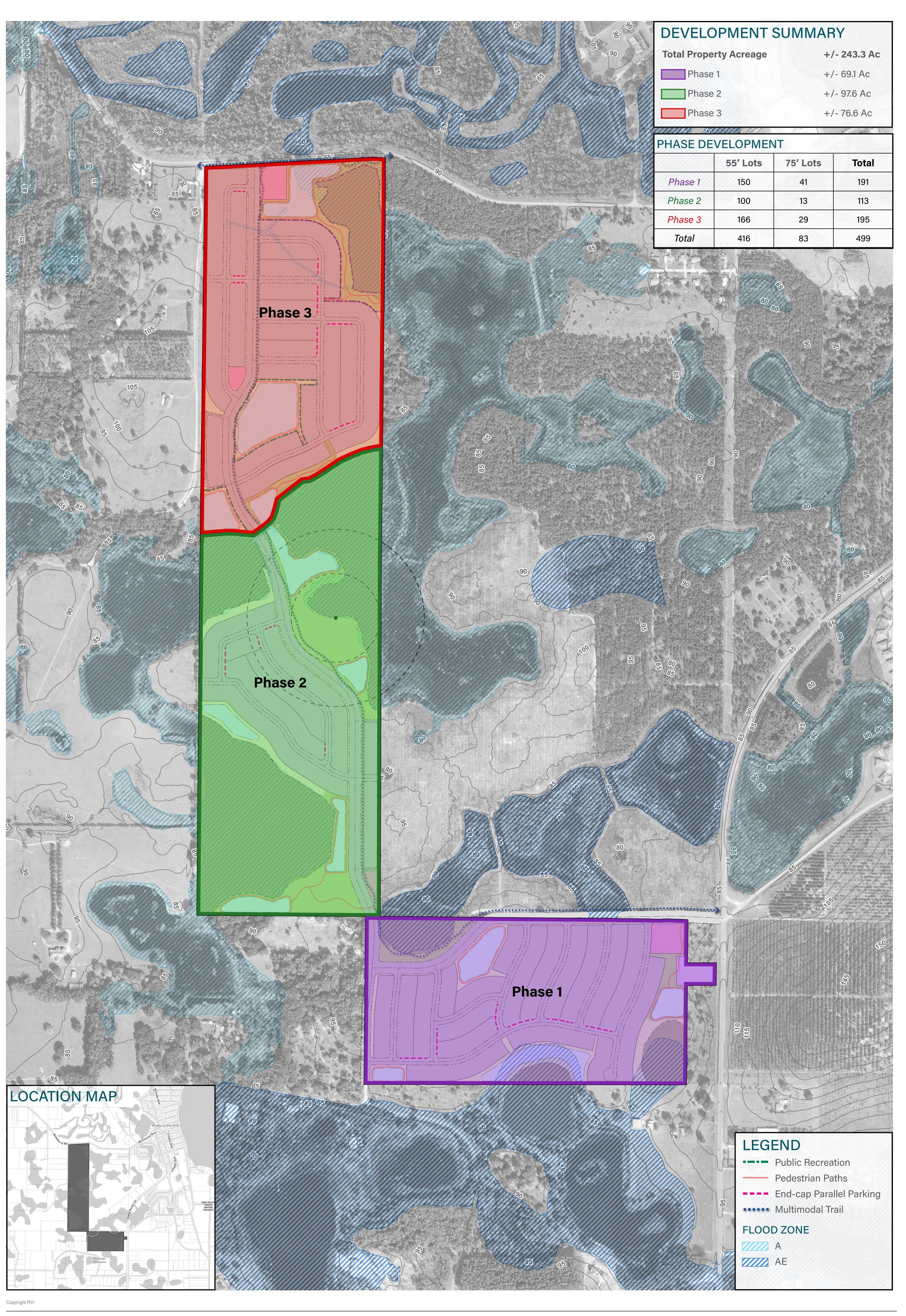




# MISSION RISE • CONCEPTUAL PLAN

- **♀** Town of Howey Hills, FL
- September 22, 2023
- **#** 22003786
- Turnstone Group / ASF TAP FL I LLC.

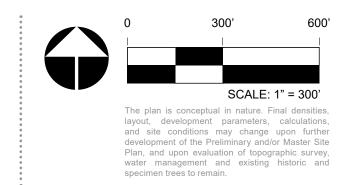


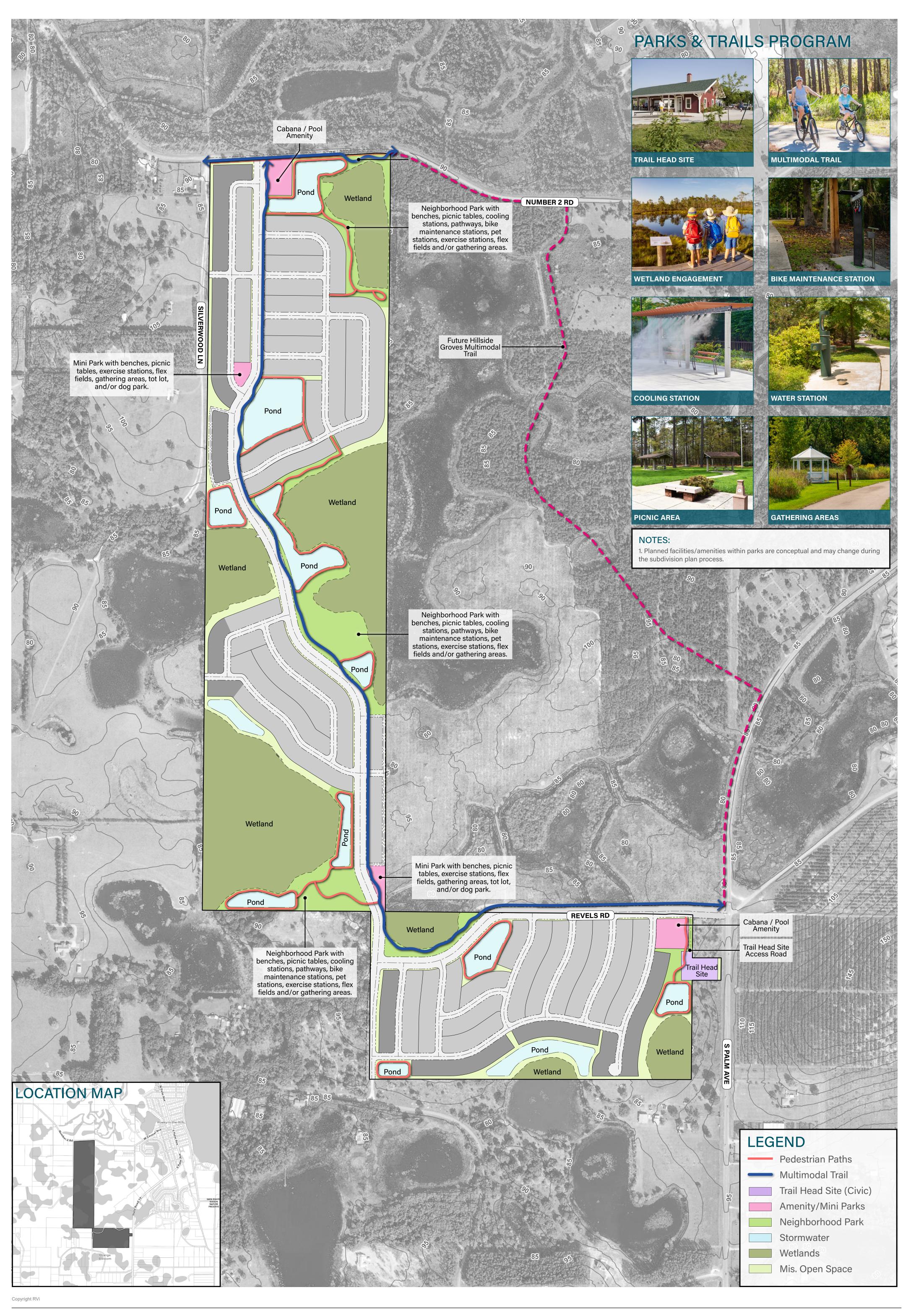




# MISSION RISE • PHASING PLAN

- ▼ Town of Howey Hills, FL
- September 22, 2023
- **#** 22003786
- Turnstone Group / ASF TAP FL I LLC.

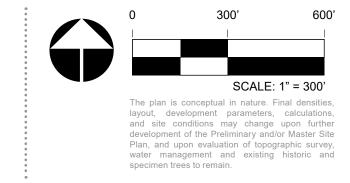


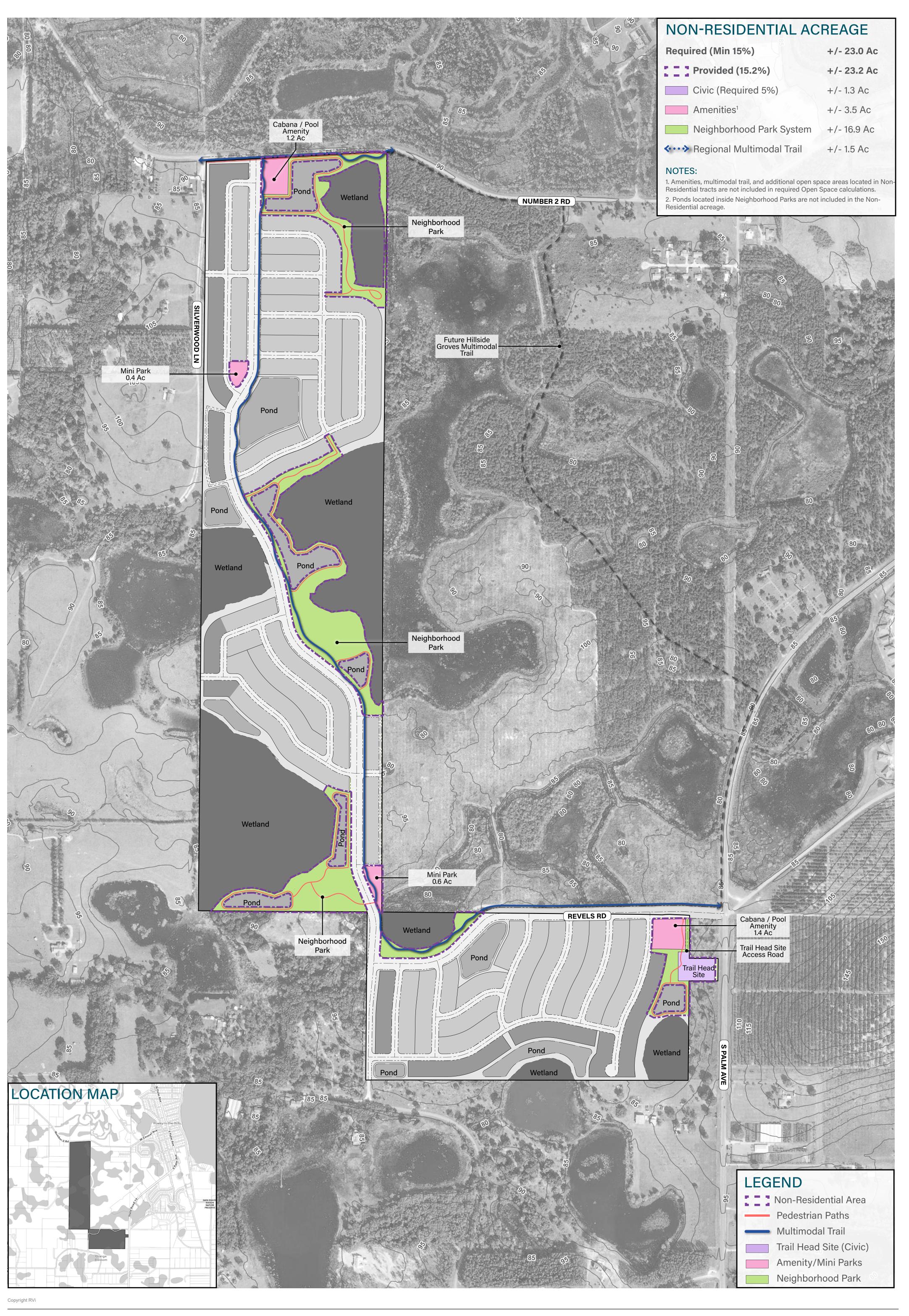




# MISSION RISE • PARKS, TRAILS & OPEN SPACE PLAN

- ▼ Town of Howey Hills, FL
- September 22, 2023
- **#** 22003786
- Turnstone Group / ASF TAP FL I LLC.

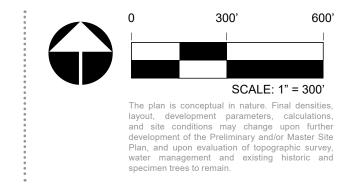






# MISSION RISE • NON-RESIDENTIAL AREAS

- **♦** Town of Howey Hills, FL
- September 22, 2023
- **#** 22003786
- Turnstone Group / ASF TAP FL I LLC.

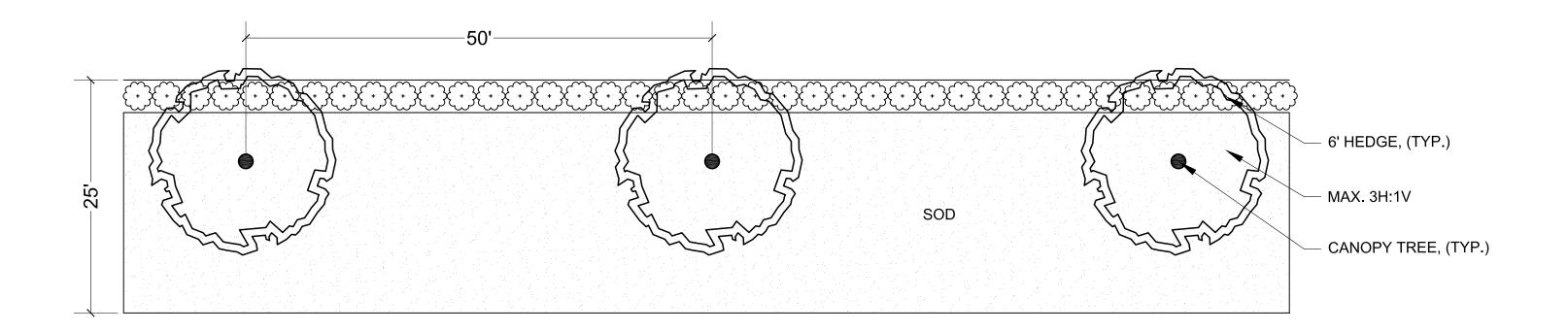


# RESIDENTIAL BUFFERS

# 25' LANDSCAPE BUFFER, TYPICAL

A landscaped berm with a total depth of at least 25 feet and no steeper than 3H:1V. The berm shall be at least three feet (3') in height and the berm together with the landscaping, shall comprise a continuous screen of at least 5 and one half feet (5.5') at time of planting and six feet (6') within one year of planting. Canopy trees shall also be planted every 50 feet along the berm.

For single family subdivisions, these buffers shall be on common property and dedicated to the homeowners' association for ownership and maintenance responsibilities.

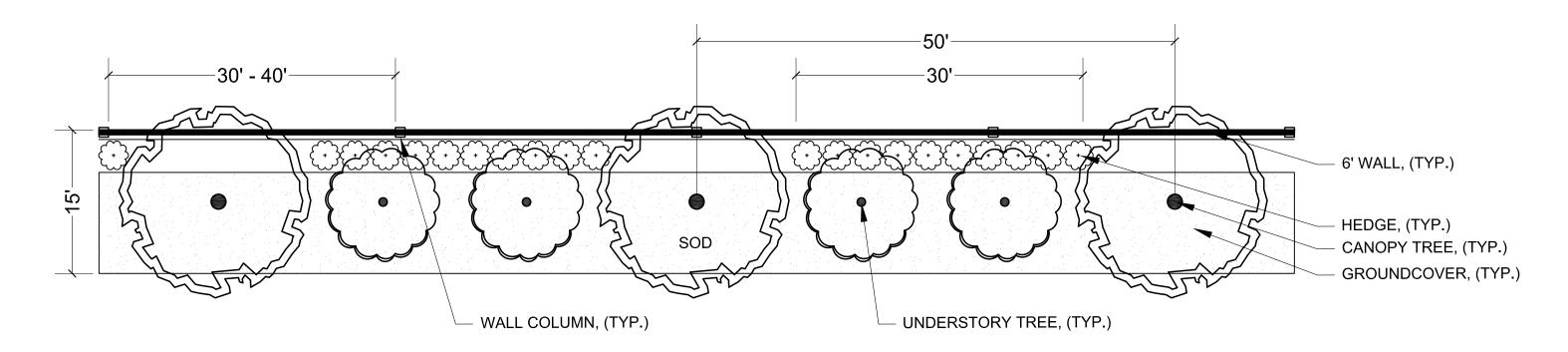


## 15' LANDSCAPE BUFFER, TYPICAL

A landscaped wall buffer with a minimum depth of 15 feet. The wall shall maintain a height of six feet (6') from grade on highest side and all walls shall have a decorative exterior (no exposed block). Acceptable materials for wall faces are brick, stucco or stone or a combination of those materials. Wall columns shall have a maximum spacing of thirty feet (30') on walls up to two hundred feet (200') in length and forty feet (40') on walls more than two hundred feet (200') in length. Wall columns may extend up to two feet (2') above the height of the wall.

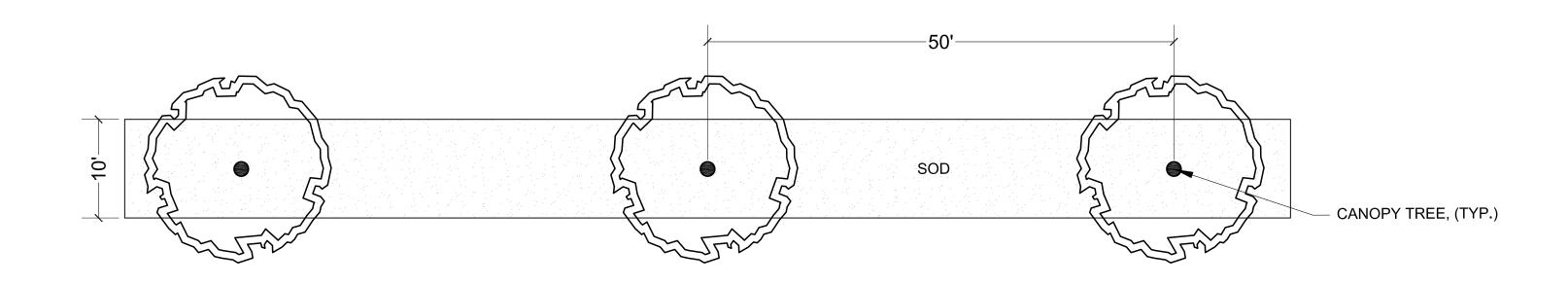
Within each fifty-foot (50') increment along the wall, two (2) canopy trees, two (2) understory trees, and 30 linear feet of shrubs shall be planted. The trees shall not be closer than five feet (5') to a walk or wall. The shrubs shall be at least 30" in height at time of planting.

For single family subdivisions, these buffers shall be on common property and dedicated to the homeowners' association for ownership and maintenance responsibilities.



### 10' LANDSCAPE BUFFER, TYPICAL

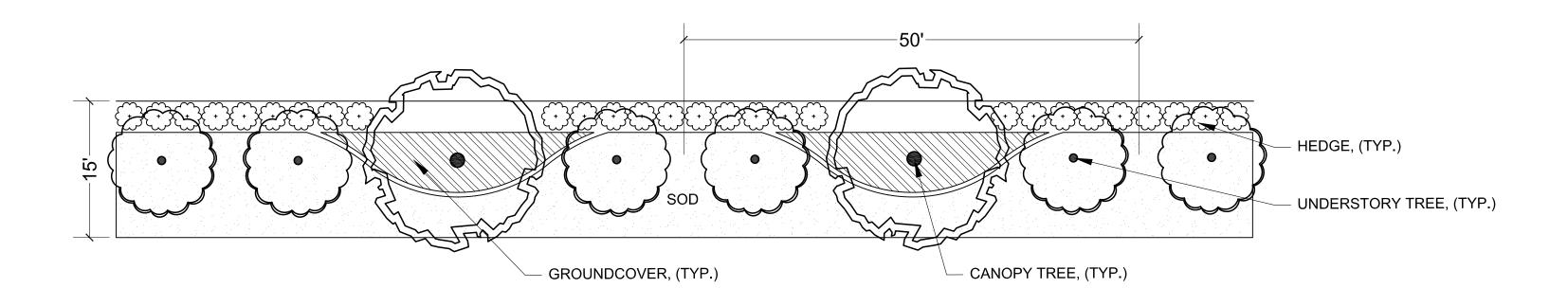
Ten-foot-wide (10') landscaped buffer with trees spaced no more than 50 feet on center.



# **NON-RESIDENTIAL BUFFERS**

# 15' LANDSCAPE BUFFER, TYPICAL

The landscaped buffer shall contain at least one (1) canopy tree, two understory trees and 30 linear feet of shrubs and ground cover for each 50 linear feet of buffer. Canopy tress shall be located no less than five feet (5') and no more than eight feet (8') from sidewalks and other walkways in order to provide shade while minimizing conflicts between tree roots and sidewalks. Similarly, canopy trees shall be used to shade parking areas that adjoin buffers. Understory trees may be planted in groupings and palms may be planted in place of understory trees when clustered in groupings of three or more trees.





Copyright RVi

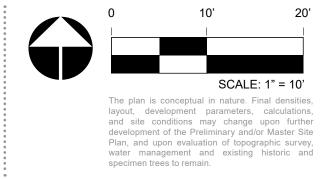
## MISSION RISE • BUFFER TYPICALS

**♀** Town of Howey Hills, FL

September 22, 2023

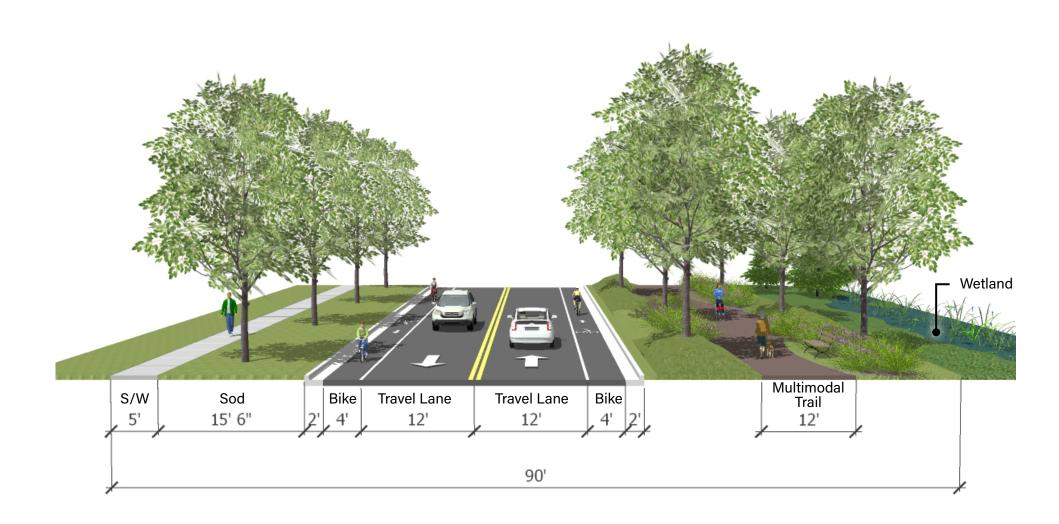
**#** 22003786

Turnstone Group / ASF TAP FL I LLC.



## **SPINE ROAD**

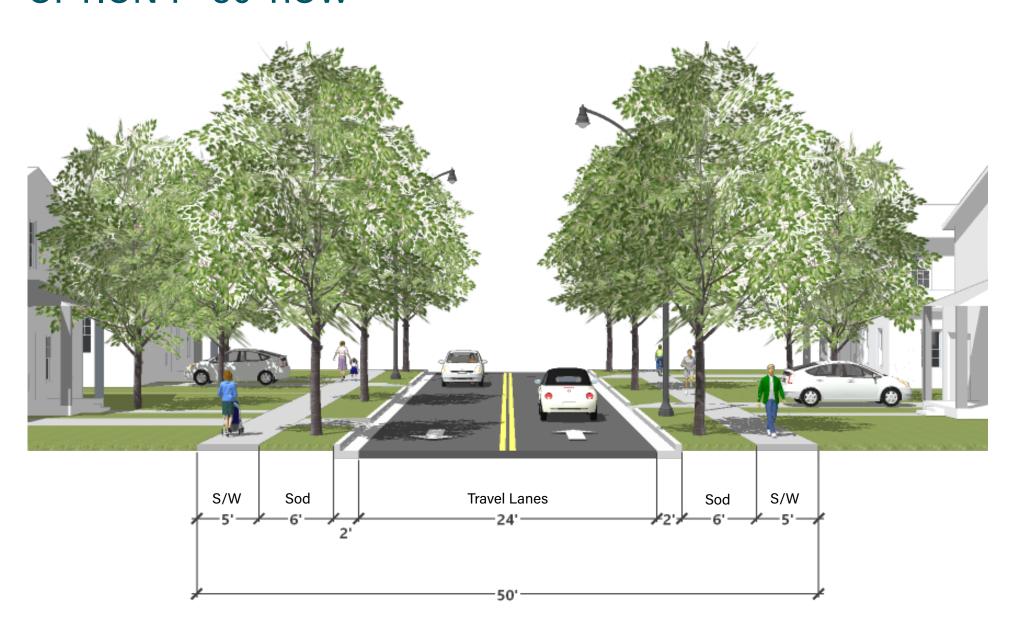
## 90' ROW WITH BIKE LANE & 12' MULTIMODAL TRAIL



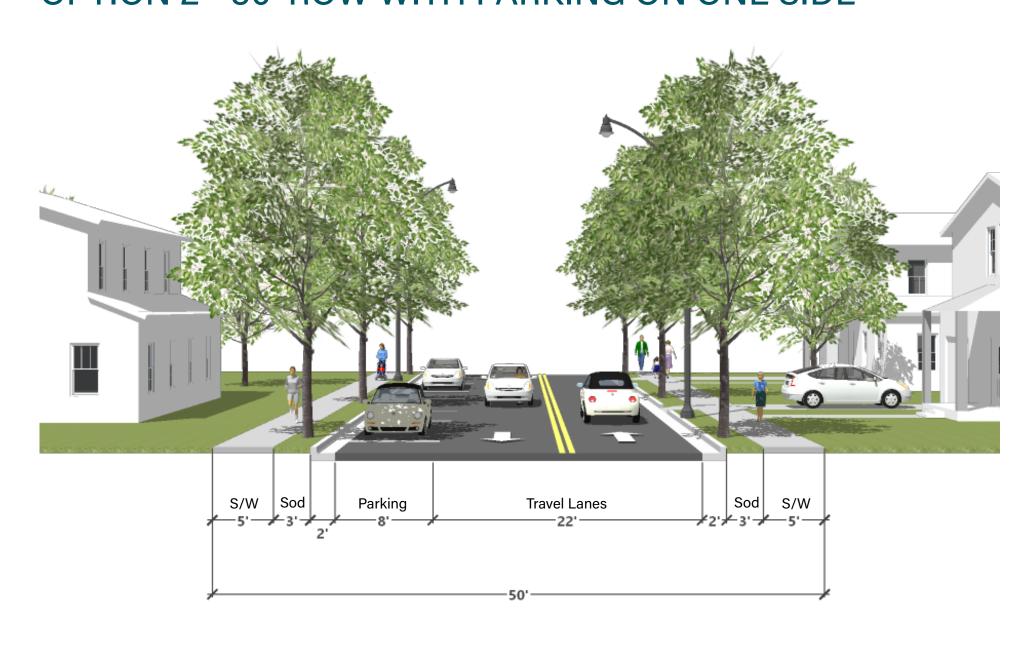
### NOTE:

Multimodal Trail is intended to meander in and out of the proposed ROW. Final location may vary based on grading, utilities & final engineering.

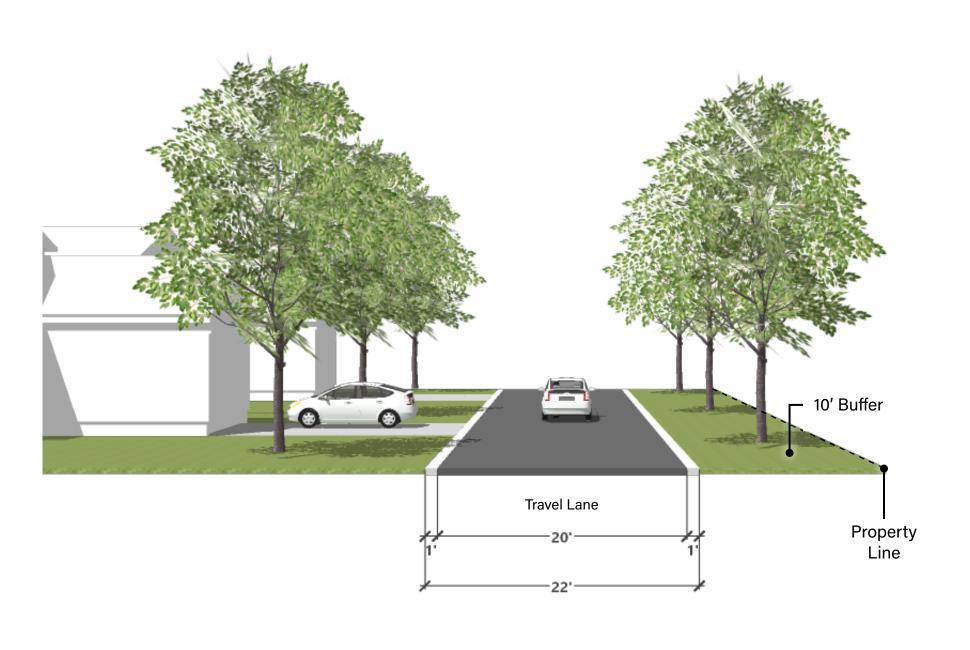
NEIGHBORHOOD ROAD OPTION 1 - 50' ROW



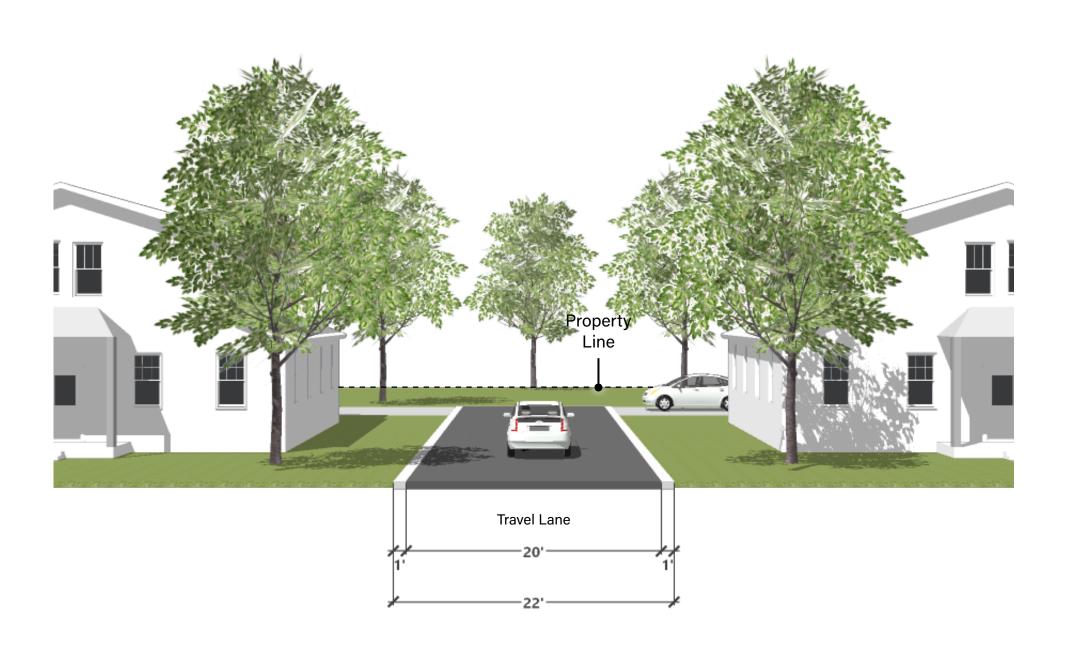
OPTION 2 - 50' ROW WITH PARKING ON ONE SIDE



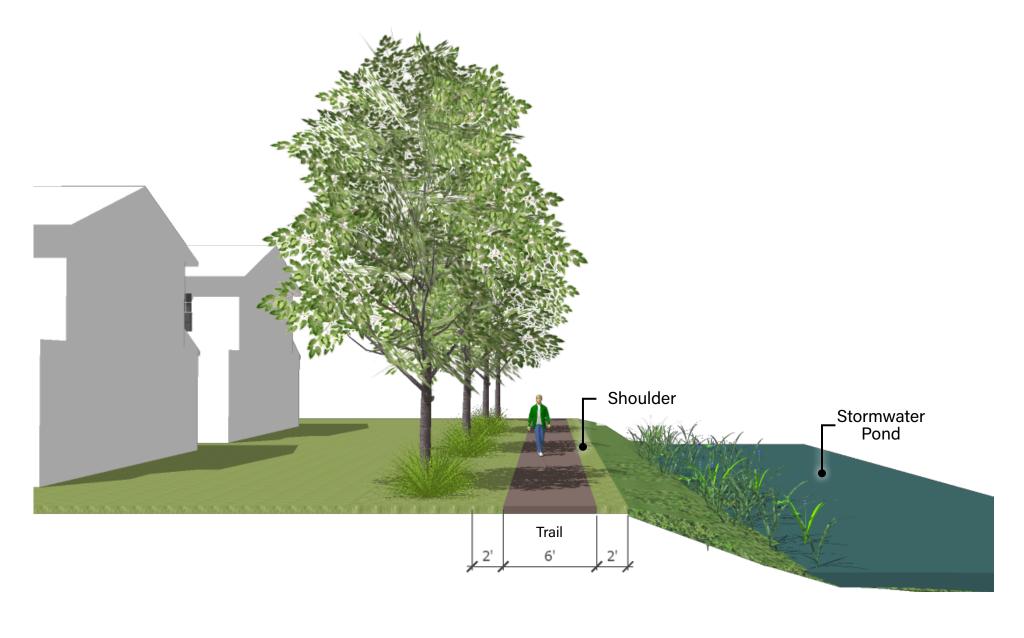
ALLEY ROAD
OPTION 1 - PARALLEL 22' ROW



OPTION 2 - PAIRED 22' ROW



PEDESTRIAN PATH 6' TRAIL



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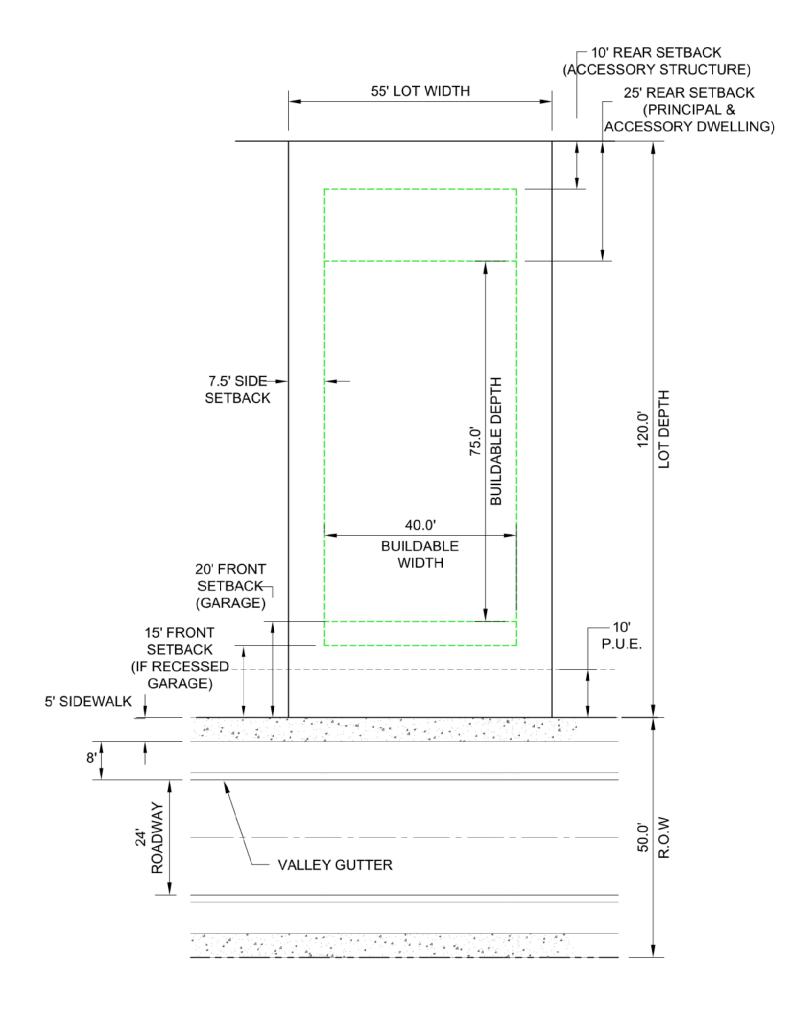
### 111 N Magnolia Ave Suite 1350 Orlando, Florida 32801 Tel: 407.680.0650 www.rviplanning.com

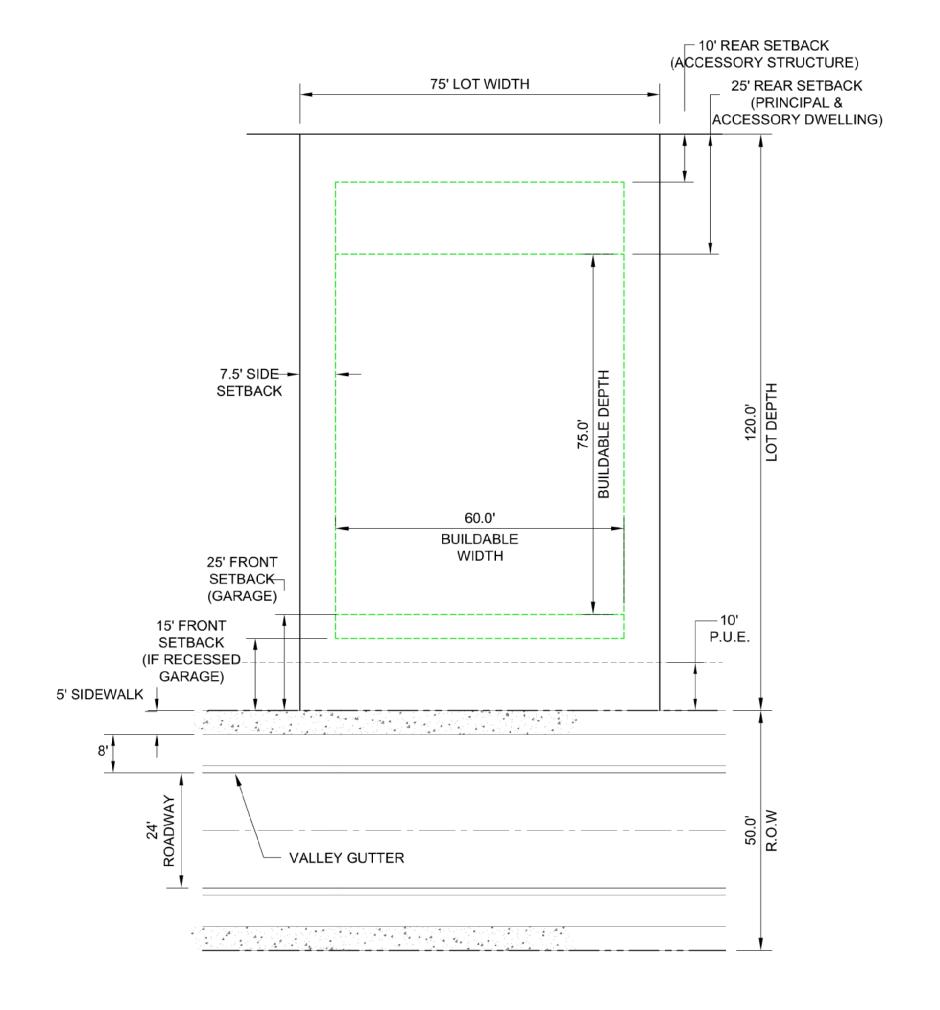
# MISSION RISE • STREET CROSS SECTIONS

- **♀** Town of Howey Hills, FL
- September 22, 2023
- **#** 22003786
- Turnstone Group / ASF TAP FL I LLC.

# 55' LOT FRONT LOAD GARAGE

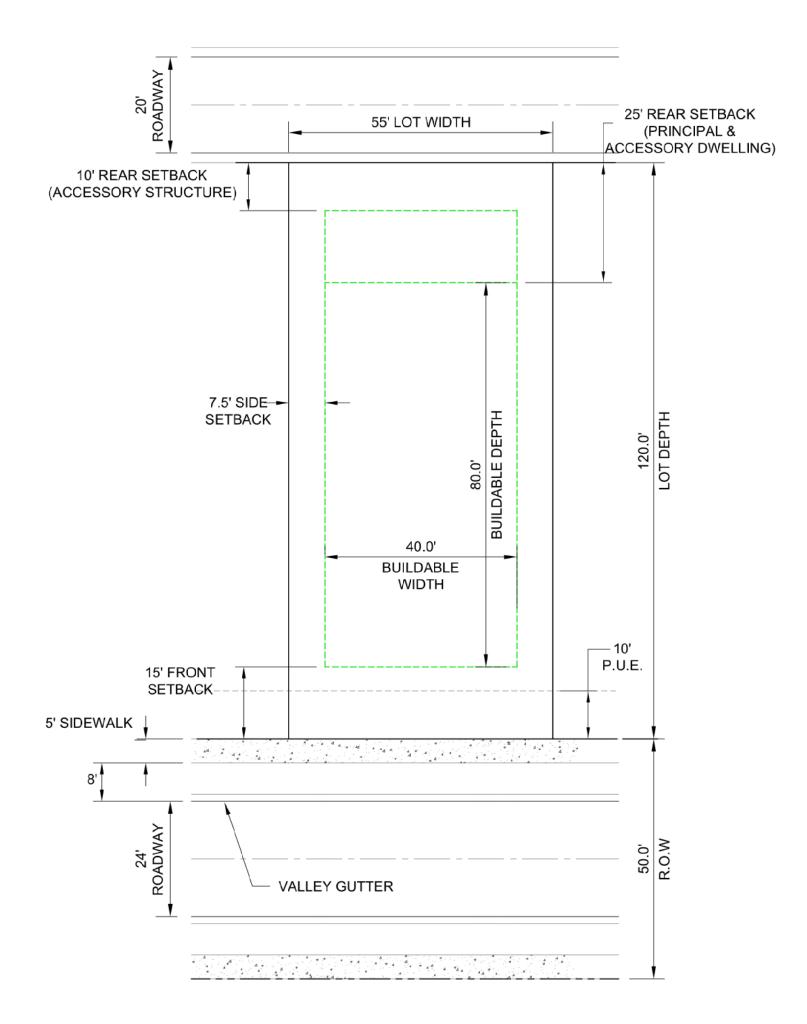
75' LOT FRONT LOAD GARAGE

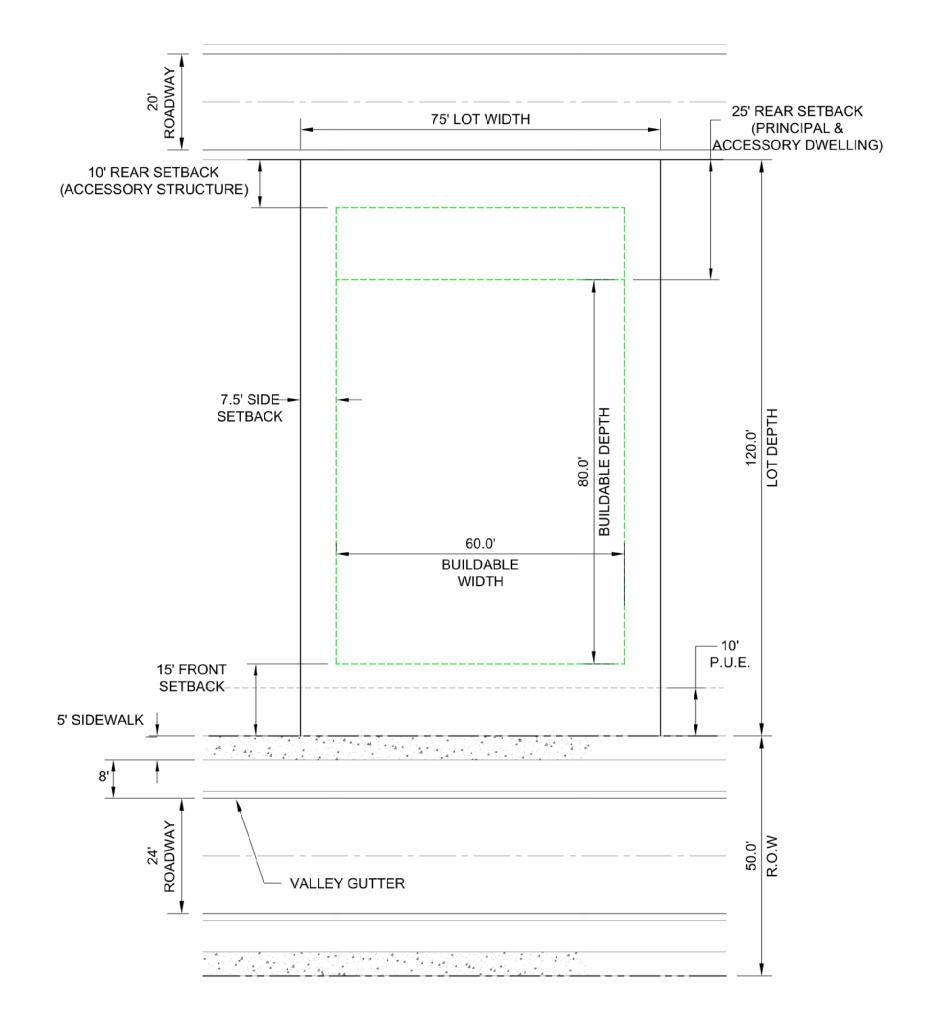




55' LOT REAR LOAD GARAGE

75' LOT REAR LOAD GARAGE











September 22, 2023

# 22003786 • Tamastana

Turnstone Group / ASF TAP FL I LLC.

