

## CITY OF FOREST PARK URBAN REDEVELOPMENT AUTHORITY REGULAR MEETING

Thursday, February 09, 2023 at 5:30 PM Council Chambers

Website: <u>www.forestparkga.gov</u> Phone Number: (404) 363.2454 ECONOMIC DEVELOPMENT

745 Forest Parkway Forest Park, GA 30297

## AGENDA

Kimberly James, Chairwoman Eliot Lawrence, Vice Chairman Debra Patrick, Member Marisol Sconiers, Member Avery Wilson, Member

## **MEETING NOTICE:**

Due to COVID-19, CDC requirements of Masks and Social Distancing will be adhered.

## CALL TO ORDER/WELCOME:

ROLL CALL:

## **APPROVAL OF MINUTES:**

1. Approval of January 26, 2023 Meeting Minutes

### OLD BUSINESS:

2. URA IGA

### NEW BUSINESS:

- 3. Fort Gillem Traffic Study: Kimley-Horn
- 4. Landscaping Contractor
- 5. Anvil Block Road Repair

**EXECUTIVE SESSION:** (Executive Session may be called for issues concerning Personnel, Litigation or Real Estate)

### ADJOURNMENT:

In compliance with the Americans with Disabilities Act, those requiring accommodation for meetings should notify the City Clerk's Office at least 24 hours prior to the meeting at 404-366-1555.



## CITY OF FOREST PARK URBAN REDEVELOPMENT AUTHORITY REGULAR MEETING

Thursday, January 26, 2023 at 6:30 PM Council Chambers

Website: <u>www.forestparkga.gov</u> Phone Number: (404) 363.2454

ECONOMIC DEVELOPMENT 745 Forest Parkway Forest Park, GA 30297

## **MINUTES**

Kimberly James, Chairwoman Eliot Lawrence, Vice Chairman Debra Patrick, Member Marisol Sconiers, Member Avery Wilson, Member

### **MEETING NOTICE:**

Due to COVID-19, CDC requirements of Masks and Social Distancing will be adhered.

### CALL TO ORDER/WELCOME:

Chairwoman James called the Urban Redevelopment Authority Meeting on January 26, 2023, to order at 7:40pm.

### Present:

Kimberly James Eliot Lawrence Debra Patrick Marisol Sconiers Avery Wilson

Also Present: Mike Williams, City Attorney Bruce Abraham, Economic Development Director (Via Zoom) Charise Clay, Economic Development Staff Assistant

### **APPROVAL OF MINUTES:**

1. Approval of October 27, 2022 Meeting Minutes

Marisol Sconiers made a motion to approve the minutes as is. Avery Wilson seconded the motion. Motion approved unanimously.

### 2. Approval of November 17, 2022 Meeting Minutes

Eliot Lawrence made a motion to approve the minutes with the addition of more information on the Pentagon 540 settlement. Avery Wilson seconded the motion. Motion approved unanimously.

### OLD BUSINESS:

3. URA Business Cards

Charise Clay gave an update on the business cards:

 A decision was needed to determine what phone number and email address would be used on the board member business cards. Board members' personal phone numbers and email addresses will not be used on them. Chairwoman James took the discussion before City Council and council was not in favor of members having a city generated email address. Mike Williams, City Attorney, suggested each board member have individual official emails for official city business. Next steps: gather costs for the city to maintain a domain for the board.

Debra Patrick made a motion to move forward with ordering the business cards. Eliot Lawrence seconded the motion. Motion approved unanimously.

### **NEW BUSINESS:**

4. Approval of 2023 Meeting Calendar

Eliot Lawrence made a motion to approve the 2023 meeting calendar. Avery Wilson seconded the motion. Motion approved unanimously.

### 5. Financial Report

Charise Clay, delivered the financial report:

- The 12/30/2022 Truist account statement detailed the URA Bonds account has \$40,238,157.88.
  - Mike Williams informed board members that the funds in this account are the bonds funds used for all city projects financed by the URA on behalf of the city (New fire station at Gillem, New City Hall, Starr Park renovations, etc.). He also noted that about 3 million of those dollars is reserved to pay the Army when final properties are released.
- The 12/30/2022 Truist account statement detailed the URA Sinking account has \$303,506.86.
- The 12/30/2022 Truist account statement detailed the URA Army account has \$4,854,308.26.
- 6. Joint Economic Development Retreat (February 24-26)

Kimberly James, discussed the joint economic development retreat:

- The Chair of the Downtown Development Authority proposed another retreat with all 3 boards (URA, DDA, DA) similar to the one held last year at Stone Mountain Park. All members of the URA agreed that they wanted another joint retreat and would participate on the proposed dates (February 24-26).
  - 7. DRI Work Agreement

Mike Williams, discussed the DRI work agreement:

 Gillem Logistics Center falls under the type of development that needs a DRI (Development of Regional Impact). Forest Park Development Partners, City Staff, as well as the ARC (Atlanta Regional Commission) are working together to get Gillem Logistics' DRI approved. Part of the approval process requires additional work be done at Gillem. The largest project is expected to be the right- turn lane leaving Gillem at Moreland Avenue-going east. This agreement is between the URA and the Properties Owners Association (POA) to collect funds from tenants to fund the cost of the project. The board approved an RFP last year to find a contractor to do the work. Mike Williams recommends that the board approve the work agreement, so the costs are paid by the POA and formally designate Robinson Weeks to manage the project.

Eliot Lawrence made a motion to approve the DRI work agreement. Avery Wilson seconded the motion. Motion approved unanimously.

## 8. Construction Management Agreement

Mike Williams discussed the Construction Management Agreement:

• The agreement formally designates Weeks Robinson Development & Management as the project manager of the DRI project at Gillem. Weeks Robinson will only be managing the project detailed in the scope of work under the DRI work agreement. Once the items in the scope of work are completed, the contract will be over.

Avery Wilson made a motion to approve the construction management agreement. Eliot Lawrence seconded the motion. Motion approved unanimously.

## 9. Gillem POA Officers update

Bruce Abraham gave the POA officer update:

The URA recently reappointed Bruce to serve on the POA at Gillem Logistics Center. The POA originally
was composed of 3 members: Kroger, David Welch of Weeks Robinson Development & Management, and
Bruce Abraham as a representative of the URA. The new board is now composed of 5 members: Bruce
Abraham as a representative of the URA, Weeks Robinson Development & Management, Kroger, TA
Investments, and Blue Star Studios. The POA elected Bruce Abraham as the President, Rich Goldberg of
Blue Star Studios as Vice President, and Robinson Weeks will continue to act as Secretary/Treasurer of
the board. Bruce proposes a meeting between the URA and POA to formally establish a relationship
between the two entities.

## 10. Gillem road repairs

Bruce Abraham discussed the Gillem road repairs:

An engineering firm drove through Gillem and estimated the road repairs to total around \$200,000.00. Any project over \$100,000 has to go through the public bidding process. Falcon Design, the city's engineer, was asked to assist in preparing the bidding documents. Falcon Design prepared a Preliminary Opinion of Probable Cost document with the Anvil Block Road Roadway Repairs, estimating \$530,083.01 as the total. Bruce recommended moving forward with the public bidding process for the Anvil Block Road Repairs. The URA asked Bruce to see if the cost of the road repairs can be shared by the POA as well.

**Other Discussion:** The Board has asked Bruce to consider hiring a local, small business as a landscaping contractor to clean up areas around Gillem instead of going with a larger, commercial company.

**EXECUTIVE SESSION:** (Executive Session may be called for issues concerning Personnel, Litigation or Real Estate)

## ADJOURNMENT:

Debra Patrick made a motion to adjourn the meeting at 8:26pm. Marisol Sconiers seconded the motion. Motion approved unanimously.

In compliance with the Americans with Disabilities Act, those requiring accommodation for meetings should notify the City Clerk's Office at least 24 hours prior to the meeting at 404-366-1555.

## MEMORANDUM

То:	Bruce Abraham Urban Redevelopment Authority of Forest Park
From:	Rob Ross, P.E. Kimley-Horn and Associates, Inc.
Date:	December 14, 2022
Subject:	Transportation Services – Fort Gillem

### **Project Overview**

The Urban Redevelopment Authority of Forest Park has retained Kimley-Horn and Associates, Inc. to provide professional engineering services related to the former Fort Gillem site.

The roadway geometry and potential changes to the intersection of Anvil Block Road at 1<sup>st</sup> Street were analyzed. A guard house for Fort Gillem previously operated as a security checkpoint at the center of this intersection. Preliminary concept sketches and an opinion of probable construction costs for three (3) different alternatives for the intersection of Anvil Block Road at 1<sup>st</sup> Street is included in the memorandum. Exhibit A and B in the attachments illustrate the project location and the intersection study network.

The development site completed the Development of Regional Impact (DRI) review process in 2020 with the DRI Traffic Study (*Gillem Logistics Center DRI #3073*) completed by Kimley-Horn. Since the submission of *DRI #3073*, portions of the site plan have been partially built out while other portions have been updated with a new development program.

Because of these updates, this memorandum summarizes a comparison of current 2022 build-out and master planned conditions to the former DRI projections. Site circulation/traffic evaluation based on the updated development program is also documented for current 2022 and projected future conditions. The traffic evaluation consisted of volume development and a level-of-service analysis of three (3) study intersections along Anvil Block Road in the vicinity of Moreland Avenue (SR 42). The studied intersections are those most impacted by proposed development program changes since the DRI.

### **Task 1: Existing Conditions and DRI Projections**

The *Gillem Logistics Center DRI* #3073 entitled the development to approximately 10.78 million SF. As of May 2022, when the turning movement counts were collected, approximately 6.94 million SF of development were already complete, and an additional 1.11 million SF were under construction. The buildings under construction included Building 700, Building 1200, the proposed movie studios (newly proposed for the site), and the conversion of the former Forest Park Fire Station into restaurant/retail space. The movie studios replace Buildings A, B, and C, from the *Gillem Logistics Center DRI* #3073 site plan, which were originally master planned to include commercial land uses. **Table 1** outlines the Development Program Updates from the 2020 *Gillem Logistics Center DRI* #3073 to the current 2022 site plan.

It should be noted that the current 2022 development includes approximately 8 million SF, with approximately 2.73 million SF remaining that is entitled and not yet accounted for with programmed

development projects. For the analysis of future full build-out for the site under the updated 2022 development program, the remaining 2.73 million SF were assumed to be future warehousing. Exhibit H and I in the attachments include the *Gillem Logistics Center DRI #3073* site plan and the proposed movie studio site plan, respectively.

Table 1: Development Program Updates									
2020* 2022									
Existing	3.25 million SF	6.94 million SF							
Under Construction	3.18 million SF	1.11 million SF							
Future Proposed Buildings (Based on master plan)	4.35 million SF*	-							
Subtotal	10.78 million SF	8.04 million SF							
DRI Entitled Total	10,779,161 SF								
Remaining Entitled	-	2.73 million SF							

\*2020 DRI total considered under construction or previously entitled

The turning movement count collection dates and peak hours for each intersection are listed below in **Table 2**. The peak hour traffic counts were used to perform the analysis presented in this report.

	Table 2: Intersection Peak Hours											
	Intersection	Date Collected	AM Peak Hour	PM Peak Hour								
1.	Anvil Block Road at 1 <sup>st</sup> Street (unsignalized)	5/18/2022	7:45 AM – 8:45 AM	4:00 PM – 5:00 PM								
2.	Anvil Block Road at Cub Drive (unsignalized)	5/18/2022	8:00 AM – 9:00 AM	4:00 PM – 5:00 PM								
3.	Anvil Block Road at Moreland Avenue (SR 42) (signalized)	5/18/2022	7:30 AM – 8:30 AM	4:30 PM – 5:30 PM								

## Task 2: Site Circulation/Traffic Evaluation

#### **Traffic Volume Development**

Future projected background (non-project) traffic is defined as the expected traffic on the roadway network in the future year(s) absent the continued build-out of the *Fort Gillem* development. The Existing 2022 peak hour traffic volumes were increased 1.3% per year for ten (10) years to account for the expected background growth in traffic through the projected build-out year of the project 2032.

Project traffic used in this analysis is defined as the vehicle trips expected to be generated by the proposed full build-out of the development that has not yet been completed, including the portions currently under construction and the 3.28 million SF of entitled, but not yet programmed development assumed to be future warehousing on the site. Gross trips associated with the proposed development were estimated using the *Institute of Transportation Engineers' (ITE) Trip Generation Manual, 11<sup>th</sup> Edition.* The trip generation estimates for the proposed movie studio were provided by the planned building user.

Based on trip generation and the anticipated trip distribution, new project trips were assigned to the study roadway network based on former DRI assignment of traffic with updates to account for updated land use and site access considerations in the current master plan. Exhibit C in the attachments contains a summary of the anticipated gross trip generation for the proposed development upon full build-out (2032).

#### **Intersection Analysis**

Level-of-service (LOS) is used to describe the operating characteristics of a road segment or intersection in relation to its capacity. LOS is defined as a qualitative measure that describes operational conditions and motorists' perceptions within a traffic stream. The Highway Capacity Manual defines six levels-of-service, LOS A through LOS F, with A being the best and F being the worst. LOS analyses were conducted at all intersections within the study network using *Synchro 11*.

LOS for signalized intersections is reported for the overall intersection. One or more movements at an intersection may experience a low LOS while the overall intersection may operate acceptably.

LOS for unsignalized intersections with stop control on the minor street only is reported for the side street approaches and the major street left-turn movements. Low LOS for side street approaches is not uncommon, as vehicles may experience delays in turning onto a major roadway. **Table 3** outlines the LOS results for the study network.

	Table 3: Level-of-Service Summary           LOS (Delay in Seconds)									
	Internection	Approach/	20 Existing C	22 Conditions	2032 Build Conditions					
	intersection	Movement	AM Peak	PM Peak	AM Peak	PM Peak				
		NB	A (9.8)	B (11.6)	B (10.9)	C (15.5)				
1.	Anvil Block Road at 1 <sup>st</sup> Street	SB	A (12.1)	B (12.5)	D (27.9)	C (19.4)				
	(Unsignalized)	EBL	A (7.9)	A (7.9)	A (8.7)	A (8.5)				
		WBL	A (7.7)	A (9.3)	A (8.0)	B (10.8)				
		NB			B (11.7)	C (21.4)				
2.	Anvil Block Road at Cub Drive/ Future	SB	B (14.3)	C (16.0)	F (69.0)	E (44.8)				
	Site Driveway (Unsignalized)	EBL	A (8.8)	A (8.6)	B (10.4)	A (9.4)				
		WBL			A (8.2)	B (10.4)				
3.	Anvil Block Road at Moreland Avenue (Signalized)	Overall	C (34.3)	D (39.7)	D (44.6)	D (54.8)				

**Intersection 1:** The analyses indicate that under the Existing 2022 Conditions, the northbound and southbound approaches and the eastbound and westbound left-turning movements all operate at a LOS B or better during both the AM and PM peak hours. Under the 2032 Build Conditions, the

northbound and southbound approaches and the eastbound and westbound left-turning movements are all projected to operate at a LOS D or better during both the AM and PM peak hours.

**Intersection 2:** The analyses indicate that under the Existing 2022 Conditions, the southbound approach and the eastbound left-turning movements operate at a LOS C or better during both the AM and PM peak hours. Under the 2032 Build Conditions, the northbound approach and the eastbound and westbound left-turning movements are all projected to operate at a LOS C or better during both the AM and PM peak hours. The southbound approach is projected top operate at LOS F and LOS E during the AM and PM peak hours, respectively. It should be noted that a low levels-of-service for the side street approaches are not uncommon, as vehicles may experience a delay turning onto a major roadway.

**Intersection 3:** The analyses indicate that under the Existing 2022 Conditions, the intersection currently operates at an overall LOS C and LOS D during the AM and PM peak hours, respectively. Under the 2032 Build Conditions, the intersection is projected to operate at an overall LOS D E during the AM and PM peak hours.

In conclusion, the study intersections have sufficient capacity to handle the traffic under Existing 2022 Conditions and projected 2032 Build Conditions. The traffic does not require mitigation.

### ANVIL BLOCK ROAD AT 1<sup>ST</sup> STREET (INTERSECTION 1) EVALUATION

#### **Preliminary Signal Warrant Analysis**

A traffic signal warrant analysis was performed based on the criteria contained in the *Manual on Uniform Traffic Control Devices (MUTCD), 2009 Edition* published by the Federal Highway Administration (FHWA).

According to the MUTCD, the investigation of the need for a traffic control signal shall include an analysis of the applicable factors contained in the following traffic signal warrants and other factors related to existing operation and safety at the study location:

- Warrant 1, Eight-Hour Vehicular Volume
- Warrant 2, Four-Hour Vehicular Volume
- Warrant 3, Peak Hour
- Warrant 4, Pedestrian Volume
- Warrant 5, School Crossing
- Warrant 6, Coordinated Signal System
- Warrant 7, Crash Experience
- Warrant 8, Roadway Network
- Warrant 9, Intersection Near a Grade Crossing

This traffic signal warrant analysis evaluated existing traffic conditions to determine if they satisfy the minimum vehicular volume warrants established by the MUTCD. Warrants 1, 2, and 3 are the vehicular volume warrants most applicable to this study and are based on mainline traffic volumes, side street traffic volumes, and number of travel lanes.

**Warrant 1** (Eight Hour Vehicular Volume) Condition 1A is intended for application at locations where a large volume of intersecting traffic is the principal reason to consider installing a traffic signal. Warrant 1 Condition 1B is intended for application where Condition 1A is not satisfied and where the traffic

volume on a major street is so heavy that traffic on the intersecting minor street suffers excessive delay or conflict in entering or crossing the major street.

**Warrant 2** (Four Hour Vehicular Volume) is intended at locations where the volume of intersecting traffic is the principal reason to consider installing a traffic signal.

**Warrant 3** (Peak Hour) is intended at locations where traffic conditions are such that for a minimum of 1 hour of an average day, the minor street traffic suffers undue delay when entering or crossing the major street.

The results of the traffic signal warrant analysis (Warrants 1-3) for the intersection of Anvil Block Road at 1<sup>st</sup> Street are shown in **Table 4**.

For this traffic signal warrant analysis, the Projected Build traffic conditions assume the following roadway geometry:

- The westbound approach along Anvil Block Road has one (1) left-turn lane and two (2) through lanes.
- The eastbound approach along Anvil Block Road has one (1) shared right-turn/through lane, one (1) through lane, and one (1) left-turn lane.
- The northbound approach along 1<sup>st</sup> Street has one (1) left-turn lane and one (1) shared through/right-turn lane.
- The southbound approach along the warehouse driveway has one (1) shared left/through/right-turn lane.

Note: For all scenarios, traffic signal warrant analysis results were reported <u>with</u> minor street right-turn volume. The posted speed limit along Anvil Block Road is 40 MPH; thus, warrant threshold volume reductions associated with higher speeds were <u>not</u> applied for this traffic signal warrant analysis.

Table 4: Traffic Signal Volume Warrant Analysis Summary										
Warrant	Projected Build									
vvarrant	Hours Met/Needed	Result								
1A*	0 / 8	Not Warranted								
1B*	0 / 8	Not Warranted								
1C**	3 / 8	Not Warranted								
2*	0 / 4	Not Warranted								
3*	0 / 1	Not Warranted								

\*100% Thresholds \*\*80% Thresholds

The intersection of Anvil Block Road at 1<sup>st</sup> Street does not satisfy any warrants under projected Build 2032 conditions.

#### Additional Intersection Considerations

Sight Distance Measurements

Kimley-Horn conducted field observations at the intersection of 1<sup>st</sup> Street and Anvil Block Road on to document potential restrictions to sight distance at both study intersections. The GDOT *Regulations for* 

*Driveway and Encroachment Control, Revision 5.3 (November 2021)* was referenced to determine the minimum sight distances recommended based on roadway speeds and vehicle maneuvers. **Table 5** outlines the required minimum and observed sight distances per study intersection, based on vehicle speeds of 40 mph.

Table 5: Sight Distance - Anvil Block Road at 1 <sup>st</sup> Street										
Location	Stopping Sig Left-Turn M	ght Distance for aneuver (feet)	Stopping Sight Distance for Right-Turn Maneuver (feet)							
	Required	Observed	Required	Observed						
Northbound Stop Bars along 1 <sup>st</sup> Street	475'	205'	530'	750'						
Southbound Stop Bars along 1 <sup>st</sup> Street	475'	900'	530'	210'						

As outlined in **Table 2**, the northbound right-turn from 1<sup>st</sup> Street onto Anvil Block Road has sufficient sight distance, but the <u>northbound left-turn does not meet sight distance recommendations</u>. The southbound left-turn from 1<sup>st</sup> Street onto Anvil Block Road has sufficient sight distance, but the <u>southbound right-turn does not meet minimum sight distance recommendations</u>. Traffic turning onto Anvil Block Road cannot see approaching traffic from the east. The former Forest Park Fire Station and the sharp horizontal curve of the eastern leg of the intersection along Anvil Block Road obstruct the view for the two maneuvers with insufficient sight distances.

Exhibit E contains a photo log from the site visit, illustrating the sight distances for each movement.

Crash Data, Driver Expectancy, and Safety Concerns

Crash report data were obtained for a five-year period from January 1, 2017 to December 31, 2021 from the Georgia Department of Transportation Georgia Numetric crash database. The crash history is summarized in **Table 6**.

	Table 6: Summary of Crashes												
	Total	laiua	Estality		Crash	Туре							
Year	Crashes	Crashes	Crashes	Angle Crashes	Sideswipe Crashes	Not a Collision with a Motor Vehicle							
2017	0	0	0	0	0	0							
2018	1	0	0	0	0	1							
2019	1	0	0	0	0	1							
2020	1	0	0	1	0	0							
2021	2	0	0	1	1	0							
Total	5	0	0	2	1	2							

As shown in **Table 6**, a total of five (5) crashes were reported at the intersection of 1<sup>st</sup> Street and Anvil Block Road, resulting in no injuries or no fatalities. The crash types included two (2) angle crashes, one (1) sideswipe crashes, and two (2) collisions not with a motor vehicle.

The MUTCD states that a traffic signal may be warranted if five (5) or more crashes have occurred in the vicinity of the intersection within a twelve-month period. The crashes must involve personal injury or property damage and be of a crash type that could be corrected by the installation of a traffic signal.

Although this intersection does not meet the MUTCD criterion for installing a traffic signal based on crash history, it should be noted that the irregular intersection geometry and stop control measures do not align with driver expectancy and likely cause confusion for drivers. Reduced driver expectancy can hinder a driver's readiness to respond in a predictable and successful manner. The two-stage crossing is not standard for a two-way stop control intersection. In addition, the westbound left-turning movement along Anvil Block Road has a stop bar and stop sign, but the through and right-turn movements along Anvil Block Road operate at free-flow. During field observations at the intersection, multiple vehicles traveling through Anvil Block Road in the westbound direction were observed stopping, even though they were not required to do so.

It is also notable that the intersection of Anvil Block Road at 1<sup>st</sup> Street likely experienced significant changes in development intensity and traffic starting in 2020 and 2021 with the opening of the warehouse and warehouse/distribution center, respectively, along the southwest frontage of 1<sup>st</sup> Street. Traffic at the intersection will likely continue to grow and change with the proposed movie studio development and warehouse programmed along the southeast frontage of 1<sup>st</sup> Street, along with the proposed conversion of the former Forest Park Fire Station into restaurant space.

Another safety and operational concern is the queuing that takes place within the median. Vehicles making a southbound left-turn at the median were observed overflowing the median storage length. This created queueing along Anvil Block Road, which blocked the northbound left-turning vehicles within the median.

Finally, the size of the median does not accommodate heavy vehicles. The median length is approximately 60 feet, which would not accommodate a semitrailer (WB-55 or larger), causing left-turn crossing maneuvers to block the cross-traffic along Anvil Block Road. In addition, the median is not wide enough to accommodate the turning radius of a semi-trailer if vehicle is traveling the opposite direction within the median. This was also observed during the field visit to the intersection. The proposed movie studio is anticipated to generate additional heavy vehicle traffic utilizing this intersection, which will increase the frequency of these hazards.

#### **Design Considerations**

The following design alternatives for the intersection of Anvil Block Road at 1<sup>st</sup> Street were reviewed for consideration:

#### Alternative 1: Traffic Signal, Minimal Roadway Impact

Installing a traffic signal at the intersection of Anvil Block Road at 1<sup>st</sup> Street, adjusting the median, and removing the guard house would have minimal impact to the roadway geometry. A signal would improve safety and operations due to the unconventional configuration of the intersection, but the intersection does not meet MUTCD signal warrants. As noted above, the horizontal curve of Anvil Block Road limits

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sight distance for the northbound left-turn and southbound right-turn maneuvers. The installation of a traffic signal would limit conflicts between the northbound left-turn and southbound right-turn maneuvers. However, installing a signal at the intersection while maintaining the existing geometry with a wide median does not align with driver expectancy, which may reduce safety at the intersection.

#### Alternative 2: Traffic Signal Realignment, Larger Roadway Impact

Removing the guardhouse from the median of Anvil Block Road and reconstructing the roadway to reduce the width of the medians is an additional consideration for improving the operations and safety of the intersection of Anvil Block Road at 1<sup>st</sup> Street. This would remove the two-stage crossing, which vehicles experience today at the intersection. In addition, pulling the intersection to the south would improve sight-distance for the southbound right-turn and northbound left-turn maneuvers.

However, does not meet MUTCD signal warrants and reconstructing this section of Anvil Block Road would be expensive and may be cost prohibitive.

#### Alternative 3: Roundabout, Larger Roadway Impact

Constructing a roundabout at the intersection of Anvil Block Road at 1<sup>st</sup> Street is another option to improve the safety and operations of the intersection. Due to the high volume of heavy vehicles that are anticipated to travel through this intersection, a roundabout with a large radius would be needed to accommodate the heavy vehicles.

The roundabout size would likely require the southern driveway serving the Amazon facility along 1<sup>st</sup> Street to be moved or removed. This would likely alter the operations of the cross-docked warehouse, which may not be favorable.

## Task 3: Preliminary Concept Sketches and Opinion of Probable Construction Costs

**Table 7** provides cost estimates for each of the alternatives. **Figure 1, Figure 2,** and **Figure 3**, provide the preliminary concept sketches of *Alternative 1, Alternative 2*, and *Alternative 3*, respectively. Exhibit E provides full-size concepts, and Exhibit F provides cost estimate details.

Table 7: Cost Estimates	3
Alternative	Estimated Total Cost
Alternative 1: Traffic Signal	\$2,160,000
Alternative 2: Traffic Signal with Realignment	\$2,540,000
Alternative 3: Roundabout	\$2,880,000



Figure 1: Alternative 1 - Traffic Signal

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Figure 2: Alternative 2 - Traffic Signal with Realignment



Figure 3: Alternative 3 - Roundabout

## ADDITIONAL CONSIDERATION – RESTRICT ACCESS AT INTERSECTION

Because a traffic signal is not warranted based on MUTCD criteria *Alternative 1: Traffic Signal* is not recommended. *Alternative 2: Traffic Signal with Realignment* and *Alternative 3: Roundabout* require significant roadway construction and may be cost prohibitive.

Because these alternatives present some challenges, the following option should also be considered:

- Close the northern driveway that serves the Package Price Distribution Center/Kuhne & Nagel
- Restrict northbound left-turns from 1<sup>st</sup> Street onto Anvil Block Road
- Remove the guard house to create a reduced conflict u-turn (RCUT) intersection configuration

This alternative is less expensive and presents a functional way to serve traffic and improve safety.

**Figure 4** illustrates how traffic would be rerouted with the closure of the northern leg and with the northbound left-turn restriction.

Closing the northern leg of the intersection would simplify operations of the intersection. It would also improve the safety of the intersection by eliminating the sight distance hazard experienced by the southbound right-turn maneuver. To close the northern leg of the intersection, The Urban Redevelopment Authority of Forest Park would have to coordinate with the owners of the warehouse that is served by the driveway. The intersections labeled as A, B, and C on **Figure 4**, are projected to operate at an acceptable LOS during the AM and PM peak under Build 2032 conditions with the rerouted traffic.

Restricting northbound left-turns from 1<sup>st</sup> Street onto Anvil Block would also improve the safety of the intersection by eliminating the sight distance hazard experienced by the northbound left-turn maneuver. To restrict the northbound left-turn, The Urban Redevelopment Authority of Forest Park would have to coordinate with the owners of the warehouses that are served by 1<sup>st</sup> Street.

To accommodate the RCUT configuration, the guard house should be removed.

**Figure 5** provides an example of an RCUT at a four-leg intersection. The proposed configuration at the intersection of Anvil Block Road at 1<sup>st</sup> Street is a three-leg RCUT.

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Figure 4: Traffic Rerouting

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Figure 5: RCUT Intersection Configuration Example (Joel Cowan Pkwy at Sandy Creek Rd)

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# Kimley » Horn

### Summary

The former Fort Gillem site completed the DRI review process in 2020 (*Gillem Logistics Center DRI* #3073). Since the submission of *DRI* #3073, portions of the site plan have been partially built out while other portions have been updated with a new development program. This memorandum summarized a comparison of current 2022 build-out and master planned conditions to the former DRI projections. The traffic evaluation consisted of volume development and a level-of-service analysis of three (3) study intersections along Anvil Block Road in the vicinity of Moreland Avenue (SR 42). The studied intersections are those associated with the areas most impacted by proposed development program changes since the DRI. Each intersection is projected to operate at an acceptable LOS under 2032 Build Conditions.

The roadway geometry and potential changes to the intersection of Anvil Block Road at 1<sup>st</sup> Street were analyzed. A guard house for Fort Gillem previously operated as a security checkpoint at the center of this intersection. Preliminary concept sketches and an opinion of probable construction costs for three (3) different alternatives for the intersection of Anvil Block Road at 1<sup>st</sup> Street is included in the memorandum. The following alternatives were analyzed: *Alternative 1: Traffic Signal, Alternative 2: Realignment,* and *Alternative 3: Roundabout*. A traffic signal is not warranted based on MUTCD criteria. The realignment and roundabout are reasonable options, they would require more significant and expensive roadway changes.

An additional option that should be considered is closing the northern driveway that serves the Package Price Distribution Center/Kuhne & Nagel, removing the guard house, and constructing an RCUT to restrict the northbound left-turn from 1<sup>st</sup> Street to Anvil Block Road. This option would simplify operations and improve safety at the intersection.

If you have any questions or comments on the preliminary evaluation of event traffic, please do not hesitate to contact me at 404-201-6146, or rob.ross@kimley-horn.com.

Sincerely,

KIMLEY-HORN AND ASSOCIATES, INC.

I. A. Rom

Rob Ross, P.E. Project Engineer

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#### Attachments:

- Exhibit A: Project Location Map
- Exhibit B: Site Aerial
- Exhibit C: Trip Generation
- Exhibit D: Synchro Reports
- Exhibit E: Preliminary Concept Sketches
- Exhibit F: Cost Estimates
- Exhibit G: Site Visit Photo Log
- Exhibit H: Gillem Logistics Center DRI #3073 Site Plan
- Exhibit I: Movie Studio Site Plan





BUILD	Trip Generation Analysis (11th Editi Fort Gillem City of Forest Park, GA	on ITE)						
Land Use	AN	I Peak H	our	PN	I Peak H	our		
		Trips	Total	In	Out	Total	In	Out
Proposed Site Traffic								
150 Warehousing	3,280,000 s.f.	5,221	417	321	96	420	117	303
932 High-Turnover (Sit-Down) Restaurant	10,000 s.f.	1,072	96	53	43	91	55	36
N/A Movie Studio		1,200	300	270	30	150	15	135
		-					-	
New Trips		5,592	693	551	143	456	121	336

\kimley-horn.com\se\_amt2\amt\_tpto\014621000 fort gillem ura traffic\analysis\[2022-08-04\_fort gillem\_trip gen.xlsm]trip generation (aml)

Item #3.

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#### Intersection

Int Delay, s/veh

Mayamant	EDI	ГРТ						NDT		ODI	ODT	CDD
wovement	EBL	EBT	ERK	WBL	WBI	WBR	INBL	INRI	NBK	SBL	SBI	SBR
Lane Configurations		412			412			4			4	
Traffic Vol, veh/h	4	107	22	51	261	0	8	0	15	1	0	2
Future Vol, veh/h	4	107	22	51	261	0	8	0	15	1	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	2	-	-	2	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	2	35	23	6	17	2	2	2	33	100	2	100
Mvmt Flow	4	120	25	57	293	0	9	0	17	1	0	2

Major/Minor	Major1		ľ	Major2		l	Vinor1		Ν	/linor2			
Conflicting Flow All	293	0	0	145	0	0	402	548	73	475	560	147	
Stage 1	-	-	-	-	-	-	141	141	-	407	407	-	
Stage 2	-	-	-	-	-	-	261	407	-	68	153	-	
Critical Hdwy	4.14	-	-	4.22	-	-	7.54	6.54	7.56	9.5	6.54	8.9	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	8.5	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	8.5	5.54	-	
Follow-up Hdwy	2.22	-	-	2.26	-	-	3.52	4.02	3.63	4.5	4.02	4.3	
Pot Cap-1 Maneuver	1265	-	-	1406	-	-	533	442	882	303	436	635	
Stage 1	-	-	-	-	-	-	847	779	-	390	596	-	
Stage 2	-	-	-	-	-	-	721	596	-	711	770	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1265	-	-	1406	-	-	511	419	882	286	414	635	
Mov Cap-2 Maneuver	-	-	-	-	-	-	622	520	-	363	516	-	
Stage 1	-	-	-	-	-	-	844	777	-	389	567	-	
Stage 2	-	-	-	-	-	-	684	567	-	695	768	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0.2			1.3			9.8			12.1			
HCM LOS							А			В			
							MOT		<b>D</b> I 4				

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	
Capacity (veh/h)	770	1265	-	-	1406	-	-	508	
HCM Lane V/C Ratio	0.034	0.004	-	-	0.041	-	-	0.007	
HCM Control Delay (s)	9.8	7.9	0	-	7.7	0.1	-	12.1	
HCM Lane LOS	А	А	А	-	А	А	-	В	
HCM 95th %tile Q(veh)	0.1	0	-	-	0.1	-	-	0	

#### Intersection

Int Delay s/veh

Int Delay, s/veh	1.4							
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations	5	<b>^</b>	<b>†</b> ‡		5	1		
Traffic Vol, veh/h	13	126	285	52	37	8		
Future Vol, veh/h	13	126	285	52	37	8		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Stop	Stop		
RT Channelized	-	None	-	None	-	None		
Storage Length	150	-	-	-	0	0		
Veh in Median Storage	, # -	0	0	-	0	-		
Grade, %	-	0	0	-	0	-		
Peak Hour Factor	89	89	89	89	89	89		
Heavy Vehicles, %	38	35	15	17	65	75		
Mvmt Flow	15	142	320	58	42	9		

Major/Minor	Major1	Ν	/lajor2	١	Minor2				
Conflicting Flow All	378	0	-	0	450	189			
Stage 1	-	-	-	-	349	-			
Stage 2	-	-	-	-	101	-			
Critical Hdwy	4.86	-	-	-	8.1	8.4			
Critical Hdwy Stg 1	-	-	-	-	7.1	-			
Critical Hdwy Stg 2	-	-	-	-	7.1	-			
Follow-up Hdwy	2.58	-	-	-	4.15	4.05			
Pot Cap-1 Maneuver	956	-	-	-	404	635			
Stage 1	-	-	-	-	529	-			
Stage 2	-	-	-	-	753	-			
Platoon blocked, %		-	-	-					
Mov Cap-1 Maneuver	956	-	-	-	398	635			
Mov Cap-2 Maneuver	· -	-	-	-	398	-			
Stage 1	-	-	-	-	521	-			
Stage 2	-	-	-	-	753	-			
Approach	FB		WB		SB				
HCM Control Delay	0.8		0		14.3				
HCM LOS	0.0		U		. <del>.</del> .0 R				
					5				
Minor Lane/Major Mvi	mt	EBL	EBT	WBT	WBR S	SBLn1	SBLn2		
Capacity (veh/h)		956	-	-	-	398	635		
HCM Lane V/C Ratio		0.015	-	-	-	0.104	0.014		
HCM Control Delay (s	3)	8.8	-	-	-	15.1	10.8		
HCM Lane LOS		Α	-	-	-	С	В		

HCM 95th %tile Q(veh)

0

0.3

0

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## HCM 6th Signalized Intersection Summary 3: Moreland Ave & Anvil Block Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	2	<b>†</b> 1 <sub>2</sub>		2	<b>†</b> 1 <sub>2</sub>		2	<b>^</b>	*	2	<b>^</b>	1
Traffic Volume (veh/h)	40	78	35	142	213	292	41	428	139	208	259	76
Future Volume (veh/h)	40	78	35	142	213	292	41	428	139	208	259	76
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1307	1278	1604	1663	1722	1796	1678	1767	1559	1678	1693	1693
Adj Flow Rate, veh/h	44	87	15	158	237	176	46	476	44	231	288	-7
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	40	42	20	16	12	7	15	9	23	15	14	14
Cap, veh/h	127	237	40	289	313	223	612	1749	689	530	1821	812
Arrive On Green	0.04	0.11	0.11	0.10	0.17	0.17	0.03	0.52	0.52	0.08	0.57	0.00
Sat Flow, veh/h	1245	2078	350	1584	1824	1300	1598	3357	1321	1598	3216	1434
Grp Volume(v), veh/h	44	50	52	158	212	201	46	476	44	231	288	-7
Grp Sat Flow(s),veh/h/ln	1245	1214	1215	1584	1636	1488	1598	1678	1321	1598	1608	1434
Q Serve(g_s), s	4.4	5.3	5.6	12.0	17.2	18.2	1.9	11.1	2.3	9.1	6.0	0.0
Cycle Q Clear(g_c), s	4.4	5.3	5.6	12.0	17.2	18.2	1.9	11.1	2.3	9.1	6.0	0.0
Prop In Lane	1.00		0.29	1.00		0.87	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	127	138	139	289	281	255	612	1749	689	530	1821	812
V/C Ratio(X)	0.35	0.36	0.38	0.55	0.75	0.79	0.08	0.27	0.06	0.44	0.16	-0.01
Avail Cap(c_a), veh/h	198	326	326	289	439	400	714	1749	689	560	1821	812
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	52.6	57.3	57.4	47.0	55.2	55.6	14.5	18.7	16.6	12.9	14.5	0.0
Incr Delay (d2), s/veh	3.5	3.4	3.6	3.8	8.4	11.0	0.0	0.4	0.2	0.6	0.2	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.5	1.7	1.8	5.0	7.6	7.5	0.7	4.3	0.7	3.2	2.2	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	56.1	60.6	61.0	50.9	63.6	66.6	14.6	19.1	16.8	13.5	14.7	0.0
LnGrp LOS	E	E	E	D	E	E	В	В	В	В	В	<u> </u>
Approach Vol, veh/h		146			571			566			512	
Approach Delay, s/veh		59.4			61.1			18.5			14.3	
Approach LOS		E			E			В			В	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	11.1	85.6	20.0	23.4	17.4	79.3	12.0	31.4				
Change Period (Y+Rc), s	6.9	6.3	6.6	7.4	6.1	6.3	* 6.7	7.4				
Max Green Setting (Gmax), s	13.1	48.7	13.4	37.6	13.9	48.7	* 13	37.6				
Max Q Clear Time (g_c+I1), s	3.9	8.0	14.0	7.6	11.1	13.1	6.4	20.2				
Green Ext Time (p_c), s	0.0	3.6	0.0	1.0	0.2	6.6	0.1	3.9				
Intersection Summary												
HCM 6th Ctrl Delay			34.2									
HCM 6th LOS			С									

#### Notes

User approved pedestrian interval to be less than phase max green.

\* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

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#### Intersection

Int Delay, s/veh

Movement EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL	SBT	ST SBF
Lane Configurations	4	\$⇒
Traffic Vol, veh/h 1 438 14 27 257 0 10 0 43 8	0	0 3
Future Vol, veh/h 1 438 14 27 257 0 10 0 43 8	0	0 3
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0	0	0 (
Sign Control Free Free Free Free Free Stop Stop Stop	Stop	op Stop
RT Channelized None None None -	-	- None
Storage Length	-	-
Veh in Median Storage, # - 0 0 2	2	2
Grade, % - 0 0 0	0	0
Peak Hour Factor 80 80 80 80 80 80 80 80 80 80 80	80	30 80
Heavy Vehicles, % 2 13 21 22 25 2 20 2 7 25	2	2 33
Mvmt Flow 1 548 18 34 321 0 13 0 54 10	0	0 4

Major/Minor	Major1		Μ	lajor2		Ν	linor1		Ν	/linor2			
Conflicting Flow All	321	0	0	566	0	0	788	948	283	665	957	161	
Stage 1	-	-	-	-	-	-	559	559	-	389	389	-	
Stage 2	-	-	-	-	-	-	229	389	-	276	568	-	
Critical Hdwy	4.14	-	-	4.54	-	-	7.9	6.54	7.04	8	6.54	7.56	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.9	5.54	-	7	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.9	5.54	-	7	5.54	-	
Follow-up Hdwy	2.22	-	-	2.42	-	-	3.7	4.02	3.37	3.75	4.02	3.63	
Pot Cap-1 Maneuver	1236	-	-	876	-	-	252	259	699	304	256	766	
Stage 1	-	-	-	-	-	-	438	509	-	548	607	-	
Stage 2	-	-	-	-	-	-	704	607	-	646	505	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1236	-	-	876	-	-	242	247	699	270	244	766	
Mov Cap-2 Maneuver	-	-	-	-	-	-	392	418	-	436	400	-	
Stage 1	-	-	-	-	-	-	438	508	-	547	578	-	
Stage 2	-	-	-	-	-	-	668	578	-	596	504	-	
Approach	FB			WB			NB			SB			
HCM Control Delay	0			11			11.6			12.5			
HCM LOS	0			1.1			R			12.J R			
							0			D			

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR \$	SBLn1
Capacity (veh/h)	609	1236	-	-	876	-	-	494
HCM Lane V/C Ratio	0.109	0.001	-	-	0.039	-	-	0.028
HCM Control Delay (s)	11.6	7.9	0	-	9.3	0.2	-	12.5
HCM Lane LOS	В	Α	А	-	А	А	-	В
HCM 95th %tile Q(veh)	0.4	0	-	-	0.1	-	-	0.1

#### Intersection

Int Delay, s/veh	0.7						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	1	<b>^</b>	<b>≜</b> ↑₽		5	1	
Traffic Vol, veh/h	7	471	265	18	30	3	
Future Vol, veh/h	7	471	265	18	30	3	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	150	-	-	-	0	0	
Veh in Median Storage	e, # -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	77	77	77	77	77	77	
Heavy Vehicles, %	28	14	21	56	17	67	
Mvmt Flow	9	612	344	23	39	4	

Major/Minor	Major1	Ma	ajor2	Ν	linor2		
Conflicting Flow All	367	0	-	0	680	184	
Stage 1	-	-	-	-	356	-	
Stage 2	-	-	-	-	324	-	
Critical Hdwy	4.66	-	-	-	7.14	8.24	
Critical Hdwy Stg 1	-	-	-	-	6.14	-	
Critical Hdwy Stg 2	-	-	-	-	6.14	-	
Follow-up Hdwy	2.48	-	-	-	3.67	3.97	
Pot Cap-1 Maneuver	1022	-	-	-	353	658	
Stage 1	-	-	-	-	637	-	
Stage 2	-	-	-	-	663	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1022	-	-	-	350	658	
Mov Cap-2 Maneuver	-	-	-	-	350	-	
Stage 1	-	-	-	-	631	-	
Stage 2	-	-	-	-	663	-	
Annroach	FR		WR		SR		
HCM Control Dolov o			0		16		
HCM LOS	0.1		0				
					U		

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1	SBLn2	
Capacity (veh/h)	1022	-	-	- 350	658	
HCM Lane V/C Ratio	0.009	-	-	- 0.111	0.006	
HCM Control Delay (s)	8.6	-	-	- 16.6	10.5	
HCM Lane LOS	А	-	-	- C	В	
HCM 95th %tile Q(veh)	0	-	-	- 0.4	0	

## HCM 6th Signalized Intersection Summary 3: Moreland Ave & Anvil Block Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	<b>†</b> î»		7	<b>†</b> 1>		5	**	*	7	<b>^</b>	7
Traffic Volume (veh/h)	106	304	44	155	179	179	36	291	157	431	614	84
Future Volume (veh/h)	106	304	44	155	179	179	36	291	157	431	614	84
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1633	1752	1767	1722	1559	1530	1648	1752	1752	1856	1841	1544
Adj Flow Rate, veh/h	115	330	39	168	195	80	39	316	74	468	667	20
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	18	10	9	12	23	25	17	10	10	3	4	24
Cap, veh/h	214	415	49	211	303	120	407	1647	735	689	2037	762
Arrive On Green	0.08	0.14	0.14	0.08	0.15	0.15	0.03	0.49	0.49	0.12	0.58	0.58
Sat Flow, veh/h	1555	3001	352	1640	2072	820	1570	3328	1485	1/6/	3497	1309
Grp Volume(v), veh/h	115	182	187	168	137	138	39	316	74	468	667	20
Grp Sat Flow(s),veh/h/ln	1555	1664	1688	1640	1481	1411	1570	1664	1485	1767	1749	1309
Q Serve(g_s), s	10.0	16.9	17.2	13.4	14.0	14.8	2.0	8.5	4.2	18.9	15.7	1.0
Cycle Q Clear(g_c), s	10.0	16.9	17.2	13.4	14.0	14.8	2.0	8.5	4.2	18.9	15.7	1.0
Prop In Lane	1.00		0.21	1.00	• • •	0.58	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	214	230	233	211	216	206	407	1647	735	689	2037	762
V/C Ratio(X)	0.54	0.79	0.80	0.80	0.63	0.67	0.10	0.19	0.10	0.68	0.33	0.03
Avail Cap(c_a), veh/h	226	339	344	211	302	288	545	1647	/35	689	2037	762
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	54.1	66.7	66.8	56.0	64.3	64.6	18.9	22.6	21.5	16.9	17.2	14.2
Incr Delay (d2), s/veh	4.5	13.0	13.8	21.1	6.4	1.1	0.0	0.3	0.3	2.7	0.4	0.1
Initial Q Delay(d3),s/ven	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%IIE BackOfQ(50%),ven/In	4.2	8.0	8.2	7.0	5.0	5.7	0.7	3.4	1.5	8.8	0.3	0.3
Unsig. Movement Delay, s/ven	F0 C	70 7	00.0	77 4	70.7	70.0	10.0	00.0	04.0	40.0	477	44.0
LnGrp Delay(d),s/ven	58.6	/9./	80.6	//.1 	/0./	12.3	18.9	22.8	21.8	19.6	17.7 D	14.Z
	E	E 40.4	F	E	E	E	В	400	U	В	B	В
Approach Vol, ven/n		484			443			429			1155	
Approach Delay, s/ven		/5.0			/3.0			22.3			18.4	
Approach LUS		E			E			U			В	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	11.0	99.5	20.0	29.5	25.0	85.5	18.7	30.8				
Change Period (Y+Rc), s	6.9	6.3	6.6	7.4	6.1	6.3	* 6.7	7.4				
Max Green Setting (Gmax), s	18.1	68.7	13.4	32.6	18.9	68.7	* 13	32.6				
Max Q Clear Time (g_c+I1), s	4.0	17.7	15.4	19.2	20.9	10.5	12.0	16.8				
Green Ext Time (p_c), s	0.0	10.1	0.0	2.9	0.0	4.8	0.1	2.3				
Intersection Summary												
HCM 6th Ctrl Delay			39.7									
HCM 6th LOS			D									

#### Notes

User approved pedestrian interval to be less than phase max green.

\* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Kimley-Horn Page 3 2.5

#### Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4 P			412			4			4	
Traffic Vol, veh/h	5	205	31	67	487	53	10	0	19	44	0	2
Future Vol, veh/h	5	205	31	67	487	53	10	0	19	44	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	2	-	-	2	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	2	35	23	6	17	2	2	2	33	100	2	100
Mvmt Flow	6	230	35	75	547	60	11	0	21	49	0	2

Major/Minor	Major1		N	lajor2		Ν	1inor1		Ν	/linor2			
Conflicting Flow All	607	0	0	265	0	0	684	1017	133	854	1004	304	
Stage 1	-	-	-	-	-	-	260	260	-	727	727	-	
Stage 2	-	-	-	-	-	-	424	757	-	127	277	-	
Critical Hdwy	4.14	-	-	4.22	-	-	7.54	6.54	7.56	9.5	6.54	8.9	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	8.5	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	8.5	5.54	-	
Follow-up Hdwy	2.22	-	-	2.26	-	-	3.52	4.02	3.63	4.5	4.02	4.3	
Pot Cap-1 Maneuver	967	-	-	1267	-	-	335	236	801	137	240	471	
Stage 1	-	-	-	-	-	-	722	692	-	219	427	-	
Stage 2	-	-	-	-	-	-	578	414	-	641	680	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	967	-	-	1267	-	-	309	213	801	124	217	471	
Mov Cap-2 Maneuver	-	-	-	-	-	-	462	341	-	203	349	-	
Stage 1	-	-	-	-	-	-	717	687	-	217	389	-	
Stage 2	-	-	-	-	-	-	523	377	-	620	675	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0.2			1.1			10.9			27.9			

Minor Lane/Maior Mymt	NBI n1	FBI	FBT	FBR	WBI	WBT	WBR	SBI n1
Canacity (veh/h)	630	967			1267			208
HCM Lane V/C Patio	0.051	0.006	-	-	0.050		-	0.248
HCM Control Doloy (a)	10.0	0.000	-	-	0.059	0.2	-	0.240
HCM Long LOC	10.9	0.7	0	-	0	0.3	-	27.9
HOM Lane LUS	В	A	A	-	A	A	-	D
HCM 95th %tile Q(veh)	0.2	0	-	-	0.2	-	-	0.9

В

D

HCM LOS

4.3

#### Intersection

Int Delay, s/veh

-												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	<b>1</b>		1	<b>≜</b> ‡}			÷.	1		÷.	1
Traffic Vol, veh/h	15	251	26	114	574	59	2	0	9	42	0	9
Future Vol, veh/h	15	251	26	114	574	59	2	0	9	42	0	9
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	150	-	-	100	-	-	-	-	0	-	-	0
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	38	35	2	2	15	17	2	2	2	65	2	75
Mvmt Flow	17	282	29	128	645	66	2	0	10	47	0	10

Major/Minor N	/lajor1		I	Major2		1	Minor1		I	Minor2			
Conflicting Flow All	711	0	0	311	0	0	910	1298	156	1109	1279	356	
Stage 1	-	-	-	-	-	-	331	331	-	934	934	-	
Stage 2	-	-	-	-	-	-	579	967	-	175	345	-	
Critical Hdwy	4.86	-	-	4.14	-	-	7.54	6.54	6.94	8.8	6.54	8.4	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	7.8	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	7.8	5.54	-	
Follow-up Hdwy	2.58	-	-	2.22	-	-	3.52	4.02	3.32	4.15	4.02	4.05	
Pot Cap-1 Maneuver	682	-	-	1246	-	-	230	160	862	102	165	470	
Stage 1	-	-	-	-	-	-	656	644	-	187	343	-	
Stage 2	-	-	-	-	-	-	468	331	-	656	635	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	682	-	-	1246	-	-	204	140	862	91	144	470	
Mov Cap-2 Maneuver	-	-	-	-	-	-	204	140	-	91	144	-	
Stage 1	-	-	-	-	-	-	640	628	-	182	308	-	
Stage 2	-	-	-	-	-	-	411	297	-	632	619	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0.5			1.3			11.7			69			
HCM LOS							В			F			
Minor Lane/Maior Mymt	ł	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2		
Capacity (veh/h)	-	204	862	682	-	-	1246	-	-		470		
HCM Lane V/C Ratio		0.011	0.012	0.025	-	-	0.103	-	-	0.519	0.022		
HCM Control Delay (s)		22.8	9.2	10.4	-	-	8.2	-	-	81	12.8		

-

-

С

0

А

0

В

0.1

-

-

А

0.3

-

-

-

-

F

2.3

В

0.1

HCM Lane LOS

HCM 95th %tile Q(veh)

## HCM 6th Signalized Intersection Summary 3: Moreland Ave & Anvil Block Rd

	۶	<b>→</b>	7	4	+	•	1	Ť	1	1	ŧ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	<b>†</b> 1>		٦	<b>†</b> ‡		٦	<b>††</b>	1	ሻ	<b>††</b>	1
Traffic Volume (veh/h)	74	155	63	182	432	332	101	494	149	237	376	206
Future Volume (veh/h)	74	155	63	182	432	332	101	494	149	237	376	206
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1307	1278	1604	1663	1722	1796	1678	1767	1559	1678	1693	1693
Adj Flow Rate, veh/h	82	172	33	202	480	271	112	549	35	263	418	73
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	40	42	20	16	12	7	15	9	23	15	14	14
Cap, veh/h	143	464	8/	365	525	295	418	1305	514	411	13/2	612
Arrive On Green	0.06	0.23	0.23	0.10	0.26	0.26	0.06	0.39	0.39	0.10	0.43	0.43
Sat Flow, veh/h	1245	2039	383	1584	2020	1134	1598	3357	1321	1598	3216	1434
Grp Volume(v), veh/h	82	101	104	202	388	363	112	549	35	263	418	73
Grp Sat Flow(s),veh/h/ln	1245	1214	1209	1584	1636	1518	1598	1678	1321	1598	1608	1434
Q Serve(g_s), s	7.0	9.8	10.2	13.4	32.3	32.5	5.9	16.7	2.3	13.9	12.0	4.3
Cycle Q Clear(g_c), s	7.0	9.8	10.2	13.4	32.3	32.5	5.9	16.7	2.3	13.9	12.0	4.3
Prop In Lane	1.00	070	0.32	1.00	405	0.75	1.00	4005	1.00	1.00	4070	1.00
Lane Grp Cap(c), veh/h	143	2/6	275	365	425	395	418	1305	514	411	1372	612
	0.57	0.37	0.38	0.55	0.91	0.92	0.27	0.42	0.07	0.64	0.30	0.12
Avail Cap(c_a), ven/n	183	320	325	305	439	408	4/8	1305	514	411	1372	1.00
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filler(I)	1.00	1.00	1.00	1.00	1.00 50.2	1.00 50.4	1.00	1.00	1.00	1.00	1.00	1.00
Iner Delay (d2), s/veh	41.1	40.0	40.7	37.5	00.0 04.1	00.4 26.4	23.4	31.3	20.9	23.0	20.4	24.2
Initial O Delay (d2), s/veh	7.5	1.7	1.0	0.0	24.1	20.4	0.1	1.0	0.5	0.0	0.0	0.4
%ile BackOfO(50%) veh/lp	2.4	0.0	0.0	5.6	15.7	1/ 0	2.0	6.8	0.0	5.4	0.0	1.5
Unsig Movement Delay s/veh	2.4	J.1	J.Z	5.0	13.7	14.3	2.2	0.0	0.0	J.4	4.0	1.5
InGrn Delay(d) s/veh	48.6	473	47 5	40.6	74 4	76.8	23.5	32.2	27 1	26.3	27 0	24.6
LIGIP Delay(u), siven	-0.0 D	ч <i>г</i> .5 П	чт.5 П	-0.0 D	/ <del>.</del> .+	70.0 F	20.0 C	52.2 C	27.1 C	20.5	21.0 C	24.0
Annroach Vol. veh/h		287	D		053		0	<u> </u>	0	0	754	
Approach Delay, s/yeb		<u>7</u> 77			68.1			30.6			26.5	
Approach LOS		י. יד ח			00.1 F			0.00 C			20.5 C	
					-			U			Ŭ	_
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	14.7	66.0	20.0	39.3	20.0	60.7	15.5	43.8				
Change Period (Y+Rc), s	6.9	6.3	6.6	7.4	6.1	6.3	* 6.7	7.4				
Max Green Setting (Gmax), s	13.1	48.7	13.4	37.6	13.9	48.7	* 13	37.6				
Max Q Clear Time (g_c+l1), s	7.9	14.0	15.4	12.2	15.9	18.7	9.0	34.5				
Green Ext Time (p_c), s	0.1	5.9	0.0	2.1	0.0	7.2	0.1	1.9				
Intersection Summary												
HCM 6th Ctrl Delay			44.6									
HCM 6th LOS			D									

#### Notes

User approved pedestrian interval to be less than phase max green.

\* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Kimley-Horn Page 3 2

Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		đ þ			đ þ			4			4	
Traffic Vol, veh/h	1	675	18	34	374	55	17	0	58	45	0	3
Future Vol, veh/h	1	675	18	34	374	55	17	0	58	45	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	4 -	0	-	-	0	-	-	2	-	-	2	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	80	80	80	80	80	80	80	80	80	80	80
Heavy Vehicles, %	2	13	21	22	25	2	20	2	7	25	2	33
Mvmt Flow	1	844	23	43	468	69	21	0	73	56	0	4

Major/Minor	Major1		N	lajor2		Ν	/linor1		N	/linor2			
Conflicting Flow All	537	0	0	867	0	0	1178	1481	434	1013	1458	269	
Stage 1	-	-	-	-	-	-	858	858	-	589	589	-	
Stage 2	-	-	-	-	-	-	320	623	-	424	869	-	
Critical Hdwy	4.14	-	-	4.54	-	-	7.9	6.54	7.04	8	6.54	7.56	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.9	5.54	-	7	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.9	5.54	-	7	5.54	-	
Follow-up Hdwy	2.22	-	-	2.42	-	-	3.7	4.02	3.37	3.75	4.02	3.63	
Pot Cap-1 Maneuver	1027	-	-	658	-	-	127	124	556	164	128	644	
Stage 1	-	-	-	-	-	-	283	372	-	409	494	-	
Stage 2	-	-	-	-	-	-	618	476	-	521	367	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1027	-	-	658	-	-	117	112	556	132	116	644	
Mov Cap-2 Maneuver	-	-	-	-	-	-	253	286	-	300	269	-	
Stage 1	-	-	-	-	-	-	282	371	-	408	448	-	
Stage 2	-	-	-	-	-	-	557	431	-	452	366	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0			1.1			15.5			19.4			
HCM LOS							С			С			

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1
Capacity (veh/h)	437	1027	-	-	658	-	-	310
HCM Lane V/C Ratio	0.215	0.001	-	-	0.065	-	-	0.194
HCM Control Delay (s)	15.5	8.5	0	-	10.8	0.4	-	19.4
HCM Lane LOS	С	Α	А	-	В	А	-	С
HCM 95th %tile Q(veh)	0.8	0	-	-	0.2	-	-	0.7

2.2

#### Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	<b>≜</b> †₽		1	<b>†</b> ]			÷.	1		÷.	1
Traffic Vol, veh/h	8	786	2	7	428	20	14	0	39	34	0	3
Future Vol, veh/h	8	786	2	7	428	20	14	0	39	34	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	150	-	-	100	-	-	-	-	0	-	-	0
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	77	77	77	77	77	77	77	77	77	77	77	77
Heavy Vehicles, %	28	14	2	2	21	56	2	2	2	17	2	67
Mvmt Flow	10	1021	3	9	556	26	18	0	51	44	0	4

Major/Minor	Major1		Major2		ľ	Minor1		M	Minor2			
Conflicting Flow All	582	0 (	) 1024	0	0	1339	1643	512	1118	1631	291	
Stage 1	-			-	-	1043	1043	-	587	587	-	
Stage 2	-	· _		-	-	296	600	-	531	1044	-	
Critical Hdwy	4.66	-	- 4.14	-	-	7.54	6.54	6.94	7.84	6.54	8.24	
Critical Hdwy Stg 1	-	-		-	-	6.54	5.54	-	6.84	5.54	-	
Critical Hdwy Stg 2	-			-	-	6.54	5.54	-	6.84	5.54	-	
Follow-up Hdwy	2.48	-	- 2.22	-	-	3.52	4.02	3.32	3.67	4.02	3.97	
Pot Cap-1 Maneuver	830		- 674	-	-	111	99	507	144	101	545	
Stage 1	-	· _ ·		-	-	245	305	-	427	495	-	
Stage 2	-	· _ ·		-	-	688	488	-	463	304	-	
Platoon blocked, %		-	-	-	-							
Mov Cap-1 Maneuver	830		- 674	-	-	108	97	507	127	98	545	
Mov Cap-2 Maneuver	-	-		-	-	108	97	-	127	98	-	
Stage 1	-	· _ ·		-	-	242	301	-	422	489	-	
Stage 2	-	· _		-	-	674	482	-	412	300	-	
Annroach	FB		WB			NR			SB			
HCM Control Delay s	0 1		0.2			21.4			44.8			
HCM LOS	0.1		0.2			21.4 C			E			
						U						
Minor Lane/Major Mvn	nt	NBLn1 NBLn2	2 EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1	SBLn2		

	THE LITT	(DENE			LDIX			TIBIC OBEI	OBENE	
Capacity (veh/h)	108	507	830	-	-	674	-	- 12	7 545	
HCM Lane V/C Ratio	0.168	0.1	0.013	-	-	0.013	-	- 0.348	3 0.007	
HCM Control Delay (s)	45	12.9	9.4	-	-	10.4	-	- 47.	7 11.7	
HCM Lane LOS	Е	В	А	-	-	В	-	- 6	E B	
HCM 95th %tile Q(veh)	0.6	0.3	0	-	-	0	-	- 1.4	4 0	

## HCM 6th Signalized Intersection Summary 3: Moreland Ave & Anvil Block Rd

Fort Gill	ltem #3.
Build 2032 Conditions - PM Peak H	lour

	٠	+	*	4	+	•	1	1	1	*	ţ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	<b>†</b> 1>		٦	<b>†</b> Ъ		ሻ	**	1	ሻ	<b> </b>	1
Traffic Volume (veh/h)	204	510	92	177	282	204	70	363	187	490	703	122
Future Volume (veh/h)	204	510	92	177	282	204	70	363	187	490	703	122
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1633	1752	1767	1722	1559	1530	1648	1752	1752	1856	1841	1544
Adj Flow Rate, veh/h	222	554	84	192	307	95	76	395	74	533	764	5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	18	10	9	12	23	25	17	10	10	3	4	24
Cap, veh/h	268	620	94	223	479	146	314	1312	585	562	1677	628
Arrive On Green	0.10	0.21	0.21	0.10	0.21	0.21	0.04	0.39	0.39	0.13	0.48	0.48
Sat Flow, veh/h	1555	2899	438	1640	2238	680	1570	3328	1485	1/6/	3497	1309
Grp Volume(v), veh/h	222	317	321	192	201	201	76	395	74	533	764	5
Grp Sat Flow(s),veh/h/ln	1555	1664	1673	1640	1481	1437	1570	1664	1485	1767	1749	1309
Q Serve(g_s), s	15.3	29.6	29.8	14.6	19.8	20.4	4.6	13.1	5.1	20.9	23.3	0.3
Cycle Q Clear(g_c), s	15.3	29.6	29.8	14.6	19.8	20.4	4.6	13.1	5.1	20.9	23.3	0.3
Prop In Lane	1.00		0.26	1.00	<b>•</b> / <b>-</b>	0.47	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	268	356	358	223	317	307	314	1312	585	562	1677	628
V/C Ratio(X)	0.83	0.89	0.90	0.86	0.64	0.65	0.24	0.30	0.13	0.95	0.46	0.01
Avail Cap(c_a), veh/h	268	381	383	223	339	329	429	1312	585	562	16//	628
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/ven	49.6	61.1	61.1	46.5	57.2	57.4	27.2	33.3	30.9	33.9	27.7	21.8
Incr Delay (d2), s/ven	20.9	23.1	23.7	29.1	5.4	6.2	0.1	0.6	0.4	25.5	0.9	0.0
Nitial Q Delay(03),S/Ven	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Mile BackOlQ(50%), Ven/in	ა.0	14.7	14.9	1.1	1.0	1.0	1.7	5.5	1.9	13.9	9.0	0.1
LnCrn Delay(d) s/veh	70 5	0/1	010	75.6	62.6	62.6	07/	22.0	21 /	50 /	<u> </u>	01.0
	70.5	04.1 E	04.0 E	75.0	02.0 E	03.0 E	21.4	33.9	01.4 C	59.4 E	20.0	21.0
Approach Vol. yoh/h	E	060	Г	<u> </u>	E	E	U	E 4 E	0	E	1202	0
Approach Vol, ven/n		000			594 67.1			245 20.7			1302	
Approach LOS		00.9 E			07.1 E			32.1			41.Z	
Approach LOS		Г			E			U			U	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	13.3	83.0	22.0	41.6	27.0	69.4	22.0	41.6				
Change Period (Y+Rc), s	6.9	6.3	6.6	7.4	6.1	6.3	* 6.7	7.4				
Max Green Setting (Gmax), s	18.1	62.7	15.4	36.6	20.9	60.7	* 15	36.6				
Max Q Clear Time (g_c+I1), s	6.6	25.3	16.6	31.8	22.9	15.1	17.3	22.4				
Green Ext Time (p_c), s	0.1	11.0	0.0	2.4	0.0	5.9	0.0	3.3				
Intersection Summary												
HCM 6th Ctrl Delay			54.8									
HCM 6th LOS			D									

#### Notes

User approved pedestrian interval to be less than phase max green.

\* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Kimley-Horn Page 3



![](_page_36_Picture_0.jpeg)

![](_page_37_Picture_0.jpeg)

ESTIMATED TOTAL COST	\$ 2,330,000	\$ 2,090,000	\$ 2,400,000
ТОТАL	2,324,455	2,087,865	2,393,209
PE (15%)	303,189.76 \$	272,330.24 \$	312,157.67 \$
ntingency 20% <sup>1</sup>	2,021,265.06 \$	1,815,534.90 \$	2,081,051.10 \$
Raw Cost	1,684,387.55 \$	1,512,945.75	1,734,209.25
NOTES	*	<del>ب</del>	<del>ب</del>
PRJ_DESC			
TENGTH_MI	0.3600	0.2000	0.2500
PRJ_NAME	Roundabout-Anvil Block Rd @ 1st St	Signal	Signal + Realignment

11720 Amber park Drive Suite 600 Alpharetta, GA 30009 Urban Redevelopment Authority of Forest Park

**Photograph Sheet** 

![](_page_39_Picture_5.jpeg)

![](_page_39_Picture_6.jpeg)

![](_page_39_Picture_7.jpeg)

11720 Amber park Drive Suite 600 Alpharetta, GA 30009 Urban Redevelopment Authority of Forest Park

**Photograph Sheet** 

Site Name: Fort Gillem

KHA Job No.:	0146210	00	1	tem #3.
KHA Rep.:	AML			
Date:	Septemb	er 19, 20	22	
Page:	2	of	6	

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![](_page_40_Picture_6.jpeg)

![](_page_40_Picture_7.jpeg)

Looking northbound right from 1st Street (median)

11720 Amber park Drive Suite 600 Alpharetta, GA 30009 Urban Redevelopment Authority of Forest Park

**Photograph Sheet** 

Site	Name:	Fort Gillem
0.00		rore oniciti

		140m #2
KHA Job No.:	014621000	nem #3.
KHA Rep.:	AML	
Date:	September 19, 2	022
Page:	3 of	6

![](_page_41_Picture_6.jpeg)

![](_page_41_Picture_7.jpeg)

11720 Amber park Drive Suite 600 Alpharetta, GA 30009

Photo No. 7

Urban Redevelopment Authority of Forest Park

**Photograph Sheet** 

![](_page_42_Picture_4.jpeg)

Comments:

Looking southbound left from 1st Street

![](_page_42_Picture_7.jpeg)

Item #3.

KHA Rep.:	AML
Date:	September 19, 2022

KHA Job No.: 014621000

11720 Amber park Drive Suite 600 Alpharetta, GA 30009 Urban Redevelopment Authority of Forest Park

**Photograph Sheet** 

KHA Job No.:	0146210	000		ltem #3.
KHA Rep.:	AML			
Date:	Septem	ber 19,	2022	2
Page:	5	of		6

Site Name: Fort Gillem

![](_page_43_Picture_6.jpeg)

![](_page_43_Picture_7.jpeg)

11720 Amber park Drive Suite 600 Alpharetta, GA 30009 Urban Redevelopment Authority of Forest Park

**Photograph Sheet** 

Site	Name:	Fort Gillem

KHA Job No.:	0146210	00	1	tem #3.
KHA Rep.:	AML			
Date:	Septemb	er 19, 20	22	
Page:	6	of	6	

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Photo No. 11
Comments: Guard house
Photo No. 12

Comments:

Looking DIRECTION from STREET/DRIVEWAY NAME (Intersection #X)

![](_page_45_Picture_0.jpeg)

![](_page_45_Picture_1.jpeg)

![](_page_46_Picture_0.jpeg)

![](_page_46_Picture_1.jpeg)

![](_page_47_Figure_0.jpeg)

![](_page_47_Picture_1.jpeg)

![](_page_48_Figure_0.jpeg)

![](_page_48_Picture_1.jpeg)

![](_page_49_Figure_0.jpeg)

![](_page_49_Picture_1.jpeg)

![](_page_50_Figure_0.jpeg)

![](_page_50_Picture_1.jpeg)

![](_page_51_Figure_0.jpeg)

#### ZONING DATA

PHASE 1	
ZONING	GZ (GILLEM ZONING
USE	FILM STUDIO
SITE AREA MIN PROPOSED	1 AC 17.67 AC
LOT COVERAGE MAX PROPOSED	50% 13%
OPEN SPACE MINIMUM PROPOSED	15% <15%
SETBACKS* FRONT SIDE REAR	20' 10' 25'
BUILDING HEIGHT MAX	60' < 60'

\*ONCE FUTURE PHASE PARCELS ARE OWNED BY STUDIO OR RELATED ENTITY(IES), SETBACKS WILL NOT EXIST AT COMMON PROPERTY BOUNDARIES.

#### PARKING SUMMARY

PARKING SPACES PH1 HQ LEASE AREA TOTAL	184 115 299
ACCESSIBLE SPACES	STD/VAN
REQ'D	7/2
PROVIDED	7/2

#### PAVING LEGEND

PROPOSED ASPHALT	
PROPOSED CONCRETE	
PROPOSED SIDEWALK	

]

0 50 100 200 SCALE IN FEET

![](_page_51_Figure_9.jpeg)

Item #3.