

### PLANNING & ZONING COMMISSION FEBRUARY 2024

February 06, 2024 at 6:00 PM 0110 Whispering Pines Circle, Blue River, CO

### **AGENDA**

The public is welcome to attend the meeting either in person or via Zoom.

The Zoom link is available on the Town website:

https://townofblueriver.colorado.gov/planning-zoning

Please note that seating at Town Hall is limited.

- I. CALL TO ORDER, ROLL CALL
- II. APPROVAL OF MINUTES
  - A. Minutes from October 3, 2023
- III. PROJECT APPROVAL
  - B. Selection of Chair & Vice Chair

Sec. 2-6-60. Organization. The Commission shall select its own Chairperson and a Vice Chairperson from among its members. The Chairperson or, in his or her absence, the Vice Chairperson shall be the presiding officer of all Commission meetings. In the absence of both the Chairperson and the Vice Chairperson from a meeting, the members present shall appoint a member to serve as acting Chairperson at the meeting.

- C. 0037 Rivershore-New Construction
- **D.** Building Official Code Update Report for recommendation to Board of Trustees
- IV. ADJOURN

**NEXT MEETING -**



# BLUE RIVER PLANNING & ZONING COMMISSION OCTOBER 2023

October 03, 2023 at 6:00 PM 0110 Whispering Pines Circle, Blue River, CO

### **MINUTES**

The public is welcome to attend the meeting either in person or via Zoom.

The Zoom link is available on the Town website:

https://townofblueriver.colorado.gov/planning-zoning

Please note that seating at Town Hall is limited.

### I. CALL TO ORDER, ROLL CALL

Chair Johnson called the meeting to order at 6:00 p.m.

**PRESENT** 

Bevan Hardy

Tim Johnson

Gordon Manin

Ben Stuckey

**Troy Watts** 

Noah Hopkins-Board Liaison

Excused

Travis Beck

Doug O'Brien

Also present: Town Manager Michelle Eddy; Building Official Kyle Parag.

### II. APPROVAL OF MINUTES

Section II. ItemA.

Motion made by Watts, Seconded by Stuckey to approve the minutes of September 2025

Voting Yea: Hardy, Johnson, Stuckey, Watts, Hopkins

A. Minutes from September 5, 2023

### III. PROJECT APPROVAL

### B. New Construction-0066 Conifer

Manager Eddy presented the project. She noted this is the third submittal for the site. She noted the questions concerning the elevations were updated as well as the exterior lighting. She noted plans that show a utility/mechanical room for the garage have not been received. The Building Official did not recommend approval until the outstanding questions were addressed. Motion made by Stuckey, Seconded by Hardy conditioned on garage meeting square footage requirements as amended. Motion carried.

Voting Yea: Hardy, Johnson, Stuckey, Watts

Voting Abstaining: Manin

### C. Other Business

Manager Eddy referred to the Staff memo provided. She noted that discussions will be made at the Trustees concerning removing the road base requirement and the 150% deposit requirement. It is asked to remove the road base requirement to prevent interference with the road maintenance. As the Town does not allow temporary CO's so the deposit is not necessary. It is being recommended to ask that natural grass seeds be spread.

Discussion of requirements as they exists and current CO requirements. Discussion that the current requirements meet the needs for building and decision to not make any changes beyond the recommendations noted.

### D. Proposed Ordinance Review

Manager Eddy explained the proposed camping ordinance to the Commission for recommendation to the Trustees.

Discussion of the proposed ordinance. It was asked to consider allowing the residents use temporary RV living on vacant lot. There was discussion for further clarification of the code and definition between 16A-22-10 and 16A-22-20 and allowance for a resident in RV in a driveway.

The proposed ordinance is not recommended as presented.

### IV. ADJOURN

Voting Yea: Hardy, Johnson, Manin, Stuckey, Watts

### **NEXT MEETING -**

November 7, 2023

Respectfully submitted:

Michelle Eddy, MMC

Town Clerk

TO: Michelle Eddy, CMC/CPM - Town Manager/Clerk

FROM: Kyle Parag, Plan Reviewer - CAA

DATE: January 19, 2024

RE: Planning/Zoning/Architectural Guidelines review – 0037 Rivershore

Below please find staff's analysis that outlines the review with the Town's Zoning regulations and adopted Architectural Design Guidelines for the structure proposed

### **Zoning Regulation analysis -**

Proposal: A new single-family residence with an attached garage. The proposed 3

story, 3 bedroom, 4 bath home, includes 3,581 s.f. of living space and an attached 1605 s.f., 3 vehicle garage for a combined 4,646 square feet.

Zoning

R

district:

Lot Size:

~ 42,207 sq. ft.

80,000 sq. ft. Required- Existing Non-Conforming

Lot Width: ~ 347'

100 ft. Required - Complies

Setbacks: Proposed principal residence complies with required setbacks based upon

submitted docs. 15' is required in the Rivershore subdivision.

Height: Complies with required height limitations. The height at the highest roof

ridge is proposed at 34'-8"

Garage Stds: The proposed garage is ~1065 sq. ft. and complies with the standards for

structures less than 5,000 sq. ft. in habitable size.

A maximum of 1200 sqft is permitted.

Parking Stds:

Parking requirements will be met through the proposed garage.

### **Architectural Design Guideline analysis -**

Please note the following key to the interpretation of the analysis table:

V	Element is in substantial compliance with the design guidelines	
•	Lientent is in substantial compliance with the design guidelines	
N	Does not comply with the design guidelines	
PC	Subject to Planning Commission Specific approval	
	Requires additional information from applicant	
N/A	Not Applicable to the application	

STANDARD NOTES/REMARKS		SUBSTANTIAL COMPLIANCE
DEVELOPMENT STANDARD		
Article 3: Easements	Survey is provided indicating an easement along the back of the property. No indicated easements are of concern.	Y
Article 4: Buildable Area/setbacks	The proposed home sits well within the irregular shaped buildable area.	Υ
	Article 5 Building Design Standards	
Article 5-20 Building Height	The measured height at all locations is less than 35' and complies	Y
Article 5-60 Foundation	Foundation details are not clear, the elevations do not depict any exposed foundational elements.	Y
Article 5-70 Roofs	Roof design proposed is traditional gabled, with numerous items of visual interest. Slopes vary from 3:12 to 10:12 and indicate general compliance with section 5-70	Υ
Article 5-80 Garages	A 3 car attached garage is proposed with a total sqft of 1,064 sqft, and comply with the sizing requirements. The garage is subordinate to the structure and sized accordingly. Scaled garage size indicates only 881 sqft.	Υ

Article 5-90 Window and Door design	Windows are proportional to the structure and do not comprise of unusual shapes. Front door is substation and show general compliance.	Y
Article 5-100 Balconies and railings	of vertical wood elements. Shows general compliance.	
Article 5-110 Chimney and Roof Penetrations	A substantial chimney is indicate. Materials indicated are stone, consistent with the remining materials of the home.	Υ
	Article 6 Building Materials and Colors	
Article 6-20 Materials	Materials consist of traditional woods and stone veneer. Material are consistent with requirements	У
Article 6-30 Colors	Color board is provided on page 2 of the plan set, colors are natural, consisting of stained woods and stones. Roof will be a neutral brown.	Υ
	Article 7 Accessory Improvements	
Article 7-(20-40, 110) Berms, Garages,	No accessory structures are indicated.	
sheds and Gazebos		N/A
Article 7-50 Driveways	Driveway is proposed at 12' wide and opening up to a large automobile yard. Slopes are between 0% and 5%. The driveway re-enters the setback area for the parking area and opens to an excessive width.	PC
Article 7-60 Parking Areas	Parking is met through the attached garage.	Υ
Article 7-100 Decks	Only a small balcony is provided, shows general compliance.	Υ
Article 7-120 Hot Tubs	Spa is not indicated	Υ
Article 7-140 Fences	No fencing is indicated	Υ

Article 7-150 Retaining walls				
	Article 8 Signs			
Article 8 Signs	None indicated	Y		
	Article 9 Lighting			
Article 9 Lighting	Details of the exterior lights are not provided. Unable to determine compliance with regulations.			
Article 13 Environmental Regulations				
Article 13-20 Wetlands	None indicated.	Υ		



South:22 ft

East:120 ft West:75 ft

New Addition/Res. Sq.Ft.:

Total Square footage:4646

Garage Sq.Ft.:1065

### **Building Permit Application**

Email to: <u>info@townofblueriver.org</u> Questions? Call (970) 547-0545 ext. 1

Lot Number: 2	Subdivision: Ri	vershore
Blue River Physical Add	ress: 0037 Rivershore Dr. Blue R	iver CO 80424
Homeowner Information:		
Name:		
Mailing Address: 24237 E Glas	sgow Circle Aurora CO 80016	
8		
Email:		
Contractor Information		
Company Name: N/A - Owner	Build	
Contact Name: Steve Lassa		
Mailing Address: 24237 E Glas	sgow Cir, Aurora CO 80016	
Phone: 970-389-7500		
Email: lassa110@comcast.net		
Contractor Registration #:		
**Please note a Town of Blue River	Business License is required for all busin	nesses to conduct business in the Town of
Blue River including contractors, such	b-contractors and architects. **	
Description of Project:		
New Single Family Residential	Construction	
Distance to Property Line	Type of Heat: Radiant in floor	Construction Type: Frame
North:15 ft	Roof: Asphalt Shingle	Building Height:34 ft 11 inches

SEPARATE PERMITS ARE REQUIRED FOR ELECTRICAL, PLUMBING, HEATING, VENTILIATION WORK, & FIREPLACES. THIS PERMIT BECOMES NULL AND VOID IF CONSTRUCTION AUTHORIZED IS NOT COMMENCED WITHIN \_\_\_\_ OR IF CONSTRUCTION IS SUSPENDED OR ABANDONED FOR A PERIOR OF \_\_\_ AT ANY TIME AFTER WORK IS COMMENCED.

Exterior Walls:Stone and Wood Siding

Interior Walls:Drywall

Basement Fin. Sq.Ft.:866

Main Level Sq.Ft.:2715

2<sup>nd</sup> Level Sq.Ft.:

3<sup>rd</sup> Level Sq.Ft.:

No. Stories: 2

Septic or Sewer:

Total # Bedrooms: 4

Total # Bathrooms: 4.5

Septic

I HEREBY CERTIFY THAT I HAVE READ AND EXAMINDED THIS APPLICATION AND KNOW THE SAME TO BE TRUE AND CORRECT. I AGREE TO COMPLY WITH ALL TOWN ORDINANCES AND STATE LAWS REGARDING BUILDING CONSTRUCTION AND TO BUILD ACCORDING TO THE APPROVED PLANS. THE GRANT OF A PERMIT DOES NOT PRESUMED TO GIVE AUTHORITY TO VIOLATE OR CANCEL THE PROVISIONS OF ANY OTHER STATE OR LOCAL LAW REGULATING CONSTRUCTION OR THE PERFORMANCE OF CONSTRUCTION.

Signature of Owner or Contractor: Steve Lassa	Date: 1/15/2024
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### **Submittal Requirements**

\*\*ALL Submittals Must be Electronic\*\*
Emailed to: info@townofblueriver.org

### Planning & Zoning Review Submittal Requirements

\*\*Please indicate via check box item included as well as page number in submitted packet.

Completed √	Item	Description	Page #	
	Site Plan	Scale: 1" = 10'; May appear on a single		
	0100 1 1011	sight plan. IF on a separate page, please		
		indicate the page.		
		Property Boundaries	A1.1	
		Building Envelope with setbacks	A1.1	
		Proposed Buildings	A1.1	
		Structures (existing & proposed)	A1.1	
		Driveway & Grades	A1.1	
		A wetlands delineation & Stream		
		crossing structures where applicable.	n/a	
		Topographic survey, prepared and	attachm	
		stamped by a licensed surveyor,	ent.	
		indicating site contours at 2' intervals,		
		easements, and significant natural		
		features such as rock outcroppings,		
		drainages and mature tree stands.		
		Transformer & vault location (if	n/a	
		installed by owner or existing)	11/a	
		Well location; septic if applicable	Attachmen	
		Snow storage areas and calculations	CM	
		Major site improvements	n/a	
		Existing & proposed grading & drainage	A1.1	
	Landscaping Plan	*May be included in the site plan**	L.1	
		Landscaping must indicate tree removal for defensible space	L.1	
		requirement; any trees 6" or more		
		primarily noting the removal of any		
		ponderosa pines or large trees. Clear		
		cutting of a site is not allowed.		
		Indicate the percentage of trees	15%	
		removed and revegetation to be	1070	
		conducted.		
		Upon completion of the construction		
		project, all land must be raked and		

•		,
	reseeded with native seed prior to issuance of CO. in cases of completion during snow coverage and/or winter, CO may be issued with	L.1
	conditions for completions within 60 days of the last snow and a deposit.	
	Any major structures (retaining walls; fences; landscaping rocks) must be indicated in detail on plans in conformance with the design regulations.	n/a
	Indicating building walls, floors and roof relative to the site, including existing and proposed grades, retaining wall and proposed site improvements.	A2.3
Floor Plans	Scale 1/8" = 1'	
	Indicate the general layout of all rooms, approximate size, and total square footage of enclosed space for each floor level.	A2.1-A2. 2
Exterior Elevations	Scale same as floor plans	
	Detail to indicate the architectural character of the residence, fenestration and existing and proposed grades. Elevations must include a description of exterior materials and colors.	Info and Info 2
Roof Plan	Scale same as floor plans	
	Indicate the proposed roof pitch, overhang lengths, flue locations, roofing materials and elevations of major ridge lines and all eave lines.	A3.1-A3. 2
Materials Sheet	Display materials to be used. Color renderings are suggested as well. In cases of additions, if matching the existing structure, photos of current home.	Info and Info 2

### After Approval and BEFORE Permit is Issued:

### **ELECTRONIC COPY** Stamped set.

• All of the above mentioned plus items below in one plan set.

Completed √	Item	Page #
	Soils report if applicable	attachment
	Electrical, plumbing and mechanical plans.	
	Construction Management Plan. Please refer to the Town Code and Architectural Guidelines for all requirements.	СМ
	Stamped structural plan	S 1 - S9
	Current Summit County Septic System Permit (including system plot plan), or evidence of full payment of tap fees to Upper Blue Sanitary District.	Attachmen t.
	Current Colorado Well Permit or evidence of full payment of tap fees to Timber Creek Water District	n/a
	Colorado Department of Transportation Hwy Access Permit	n/a
	Designation of General Contractor, except for bona fide homeowner contractor	n/a
	For Manufactured Homes the following additional information is required	n/a
	State of Colorado Division of Housing Approved Plans	
	State of Colorado Division of Housing Registered Installer Certificate	

### Blue River Plan Submittal Requirements for Residential Plan Review

- ❖ When designing the structure, refer to the Blue River Municipal Town Code, Chapter 16 for zoning information and allowable uses/construction. The Building Code information is available under Chapter 18. <a href="https://townofblueriver.colorado.gov">https://townofblueriver.colorado.gov</a>.
- Building Codes Adopted:
  - o International Residential Code 2018
  - o The Electrical Code is the current code adopted by the State of Colorado: 2020

Note: Applicable codes are required to be notated on plans.

- ❖ Snow loads:
  - Roofs shall be designed in accordance with accepted engineering practice based upon a ground snow load of 100 psf.
  - o Balconies/decks-125 psf.
  - o No reductions for duration.
- Frost line depth:
  - o Foundation footing minimum depth below grade-40 inches.
  - O Uncovered deck piers may be set at 24 inches.
- \* Roof underlayment 100% Ice & Water shield.
- \* Roof may be metal; 30-year minimum architectural grade, composition fiberglass (dark brown, dark gray, dark green, weathered wood or black only); or class-A #1 cedar shakes.
- ❖ Wind speed: 90 mph, exposure "B". Seismic design category: "B".
- Propane gas alarm/shutoff system required.
- ❖ Wood burning stoves: Required to meet Colorado Dept. of Health, Regulation No. 4.
- ❖ The building height limit in the Town is 35 feet. Refer to the Architectural Guidelines for additional information.
- ❖ Locally re-settable GFCI breakers are required in bathrooms.
- ❖ Compliance with the International Energy Conservation Code is required.
- Any application that would create an accessory apartment must meet zoning regulations and will not be processed without prior approval of the Town Board of Trustees.
- Note that Hwy 9 access permits may require 3-4 months and well permits 5-6 weeks.
- ❖ Planning & Zoning Commission approvals become void if the building permit is not issued within eighteen (18) months.
- ❖ Building permits become void if construction is discontinued for more than 180 days.

In order for your permit application to be reviewed and processed properly, the following construction information must be provided. **Note:** "Preliminary" and/or plans shown as "Not for Construction" or similar are unacceptable. *Hardcopy submittals will not be accepted.* 

Note: Items below are not all inclusive of the requirements. Please review the Building Application Packet, design guidelines, building and land use codes for complete information.

### Soils Report

Must be sealed and signed by a licensed Colorado Engineer.

 Provide an engineer's soil investigation report indicating type of soil and recommended foundation design. include any required shoring.

### Improvement Survey Plat

- Provide an Improvement Survey Plat (ISP) following Colorado Revised Statutes for new principal structures, substantial expansions (25% or more) to principal structures and new accessory dwelling units (ADU's).
- Provide a permanent reference to spot elevation (benchmark) that will not be disturbed during construction.
- Provide existing spot elevations at property corners and at midpoints of the side property lines
- Must be stamped and signed by a Professional Land Surveyor (PLS) licensed by the state of Colorado.

### Site Plan

- Provide site plan that shows dimensions reflecting the distances to property lines
- Indicate all public or private easements
- Show location of all proposed and existing structures with dimensions
- Prove type of construction for all structures on site
- Provide landscaping plan.
- Show permanent reference spot elevation (benchmark), existing spot elevations at property corners and at midpoints of the side property lines.
- Indicate roof drainage on site plan with arrows showing the direction of the gutter downspouts. Roof drainage shall flow towards the road and away from all structures.

### Structural Plans

Plans must be sealed and signed by a Colorado Structural Engineer or Architect

 Indicate size, location and method of reinforcement for all proposed footings, column pads, piers, caissons, grad beams, foundation walls, decks, guardrails, guardrail posts. Specify location of reinforcing steel and anchor bolts.

- Provide complete and clearly dimensioned floor framing plan for each level and roof framing plan which indicates the materials, types, sizes and location of all structural elements.
- Provide complete structural design criteria including but not limited to required design loads, material specifications and structural construction requirements.
- Provide complete structural calculations for each structure.

### **Architectural Plans**

- Provide complete and dimensioned floor layout at each level which identifies the use of each room.
- Provide Complete and dimensioned roof plan and indicate all roof slopes.
- Provide complete and dimensioned reflected ceiling plan.
- Provide exterior elevations for each side of the building which contains an overall building
  height and floor-to-floor heights and indicate location, size and types of all doors and glazed
  openings including hazardous glazing and fall protection locations.
- Provide a bulk plane diagram on front and rear exterior elevations relative to the base plane elevation. The base plan for the bulk plane is establishing by taking the average of the existing grades of the midpoints of the two side property lines.
- Provide building and wall sections which clearly identify the required type and location of all
  materials for construction of beams, columns, floors, walls, ceilings, roofs.
- Provide stair geometry. Include rise and run, handrail and guardrail heights.
- Provide one major section through the exterior wall from footings to the highest part of the roof (min. scale 1/4"=1")
- Provide square foot area breakdown per floor level.

### **Electrical Plans**

Provide electrical plans showing the location and capacity of the service equipment and electrical panels, the location of all smoke detectors, carbons monoxide detectors, electrical receptacles, switches, and lighting fixtures.

### **Mechanical Plans**

- Provide mechanical plans and indicate the location of all heating, ventilating and air conditioning equipment. Show the location of the condensing unit. Detail the equipment access and working clearances.
- Show dryer exhaust termination location and clearances, environmental exhaust termination locations and clearances.
- Provide Manual J and Manual D calculations. Must be legible. No exceptions.
- Provide all fireplace specifications, rated separation details, direct vent termination details
  when applicable, hearth extensions when required, chimney clearances, shutoff and control
  access.

### **Plumbing Plans**

- Provide plumbing plans and indicate the location of all plumbing fixtures and appliances (Isometric may be required per the discretion of the plans examiner.)
- Provide the supply line size and main discharge size. Note the water supply inlet location.
- Indicate whether appliances are gas-operated, electric, or otherwise. List types of material to be used for all water supply, drainage and vent piping. Provide fixture max flow rates and insulation values.
- Gas load calculations and piping diagram is required.

### **Energy Conservation Plans**

Provide verification that the project meets the requirements of the IECC, or provide a simulated energy performance analysis such as RES-check. Provide all required information per 2012 IECC R103.2.

### Resubmittal Requirements

- Provide a written response addressing each correction.
- Provide revision clouds for each correction made.
- Provide updated information in the revision section of the title block.
- Provide complete plan packs per discipline requiring corrections. Example: If you are resubmitting for Civil corrections, provide a complete revised plan pack.







LASSA RESIDENCE
LOT 2. RIVERSHORE SUBDIVISION
TOWN OF BLUE RIVER. SUMMIT COUNTY. COLORADO
TITLE:

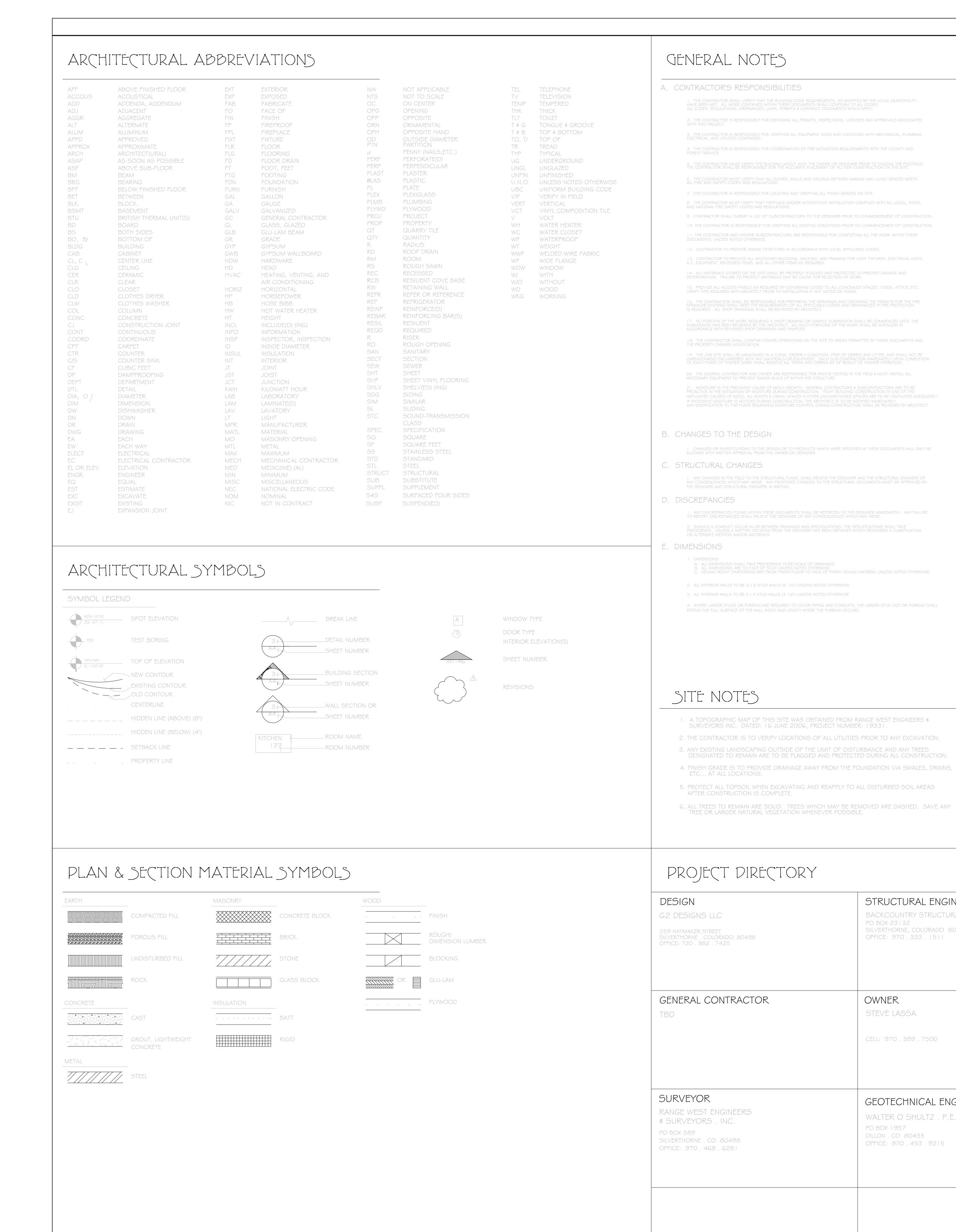
COVER SHEET

ISSUE:
PERMIT

O7/11/2023

Project # 2309

C.S.



# SHEET INDEX

CS COVER SHEET

## INFO 2 INFORMATION SHEET 2

CONSTRUCTION MANAGEMENT PLAN

LOWER FLOOR PLAN MAIN FLOOR PLAN

EXTERIOR ELEVATIONS

EXTERIOR ELEVATIONS

GENERAL NOTES

TYPICAL DETAILS LOWER LEVEL/FOUNDATION PLAN

MAIN LEVEL FRAMING PLAN ROOF FRAMING PLAN

59

# BUILDING AREA CALCULATIONS

	FINISHED	UNFINISHED	TOTAL
GARAGE	0	1,065	1,065
LOWER FLOOR	678	188	866
MAIN FLOOR	2,715	0	2,715
TOTAL:	3,393	1,253	4,646

# SITE CALCULATIONS

SITE AREA: .969 ACRES OR 42,207 SQUARE FEET FOOTPRINT WITH DECKS, PATIOS & OVERHANGS: 4,456 SQUARE FEET

PAVED DRIVEWAY AREA: 2,928 SQUARE FEET SNOWSTACK AREA: 732 SQUARE FEET (25%-MIN.)

# VICINITY MAP

STRUCTURAL ENGINEER

SILVERTHORNE, COLORADO 80498

OFFICE: 970.333.1511

PO BOX 23132

OWNER

STEVE LASSA

CELL: 970 . 389 . 7500

GEOTECHNICAL ENGINEER

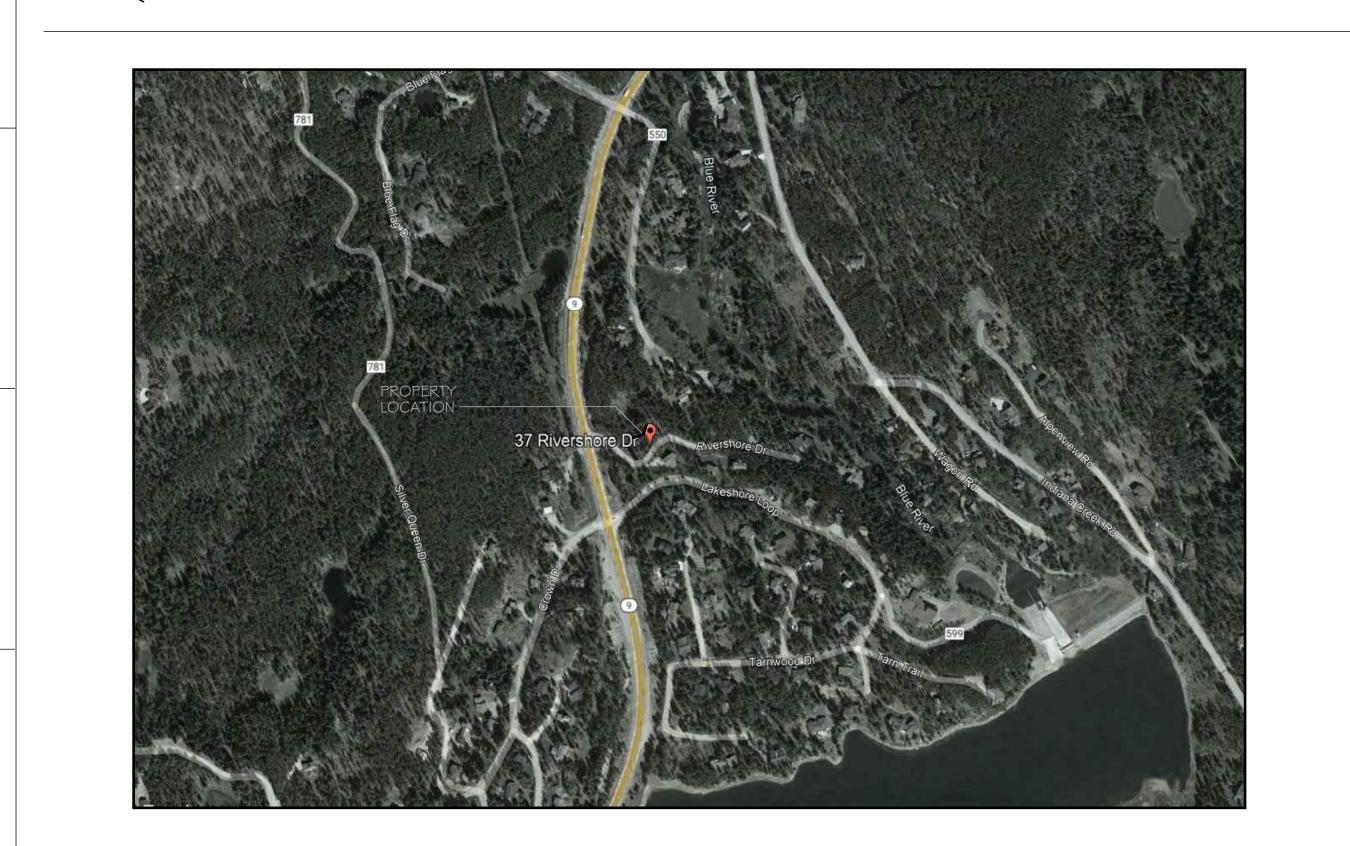
WALTER O SHULTZ . P.E.

OFFICE: 970.453.9215

PO BOX 1957

DILLON . CO 80435

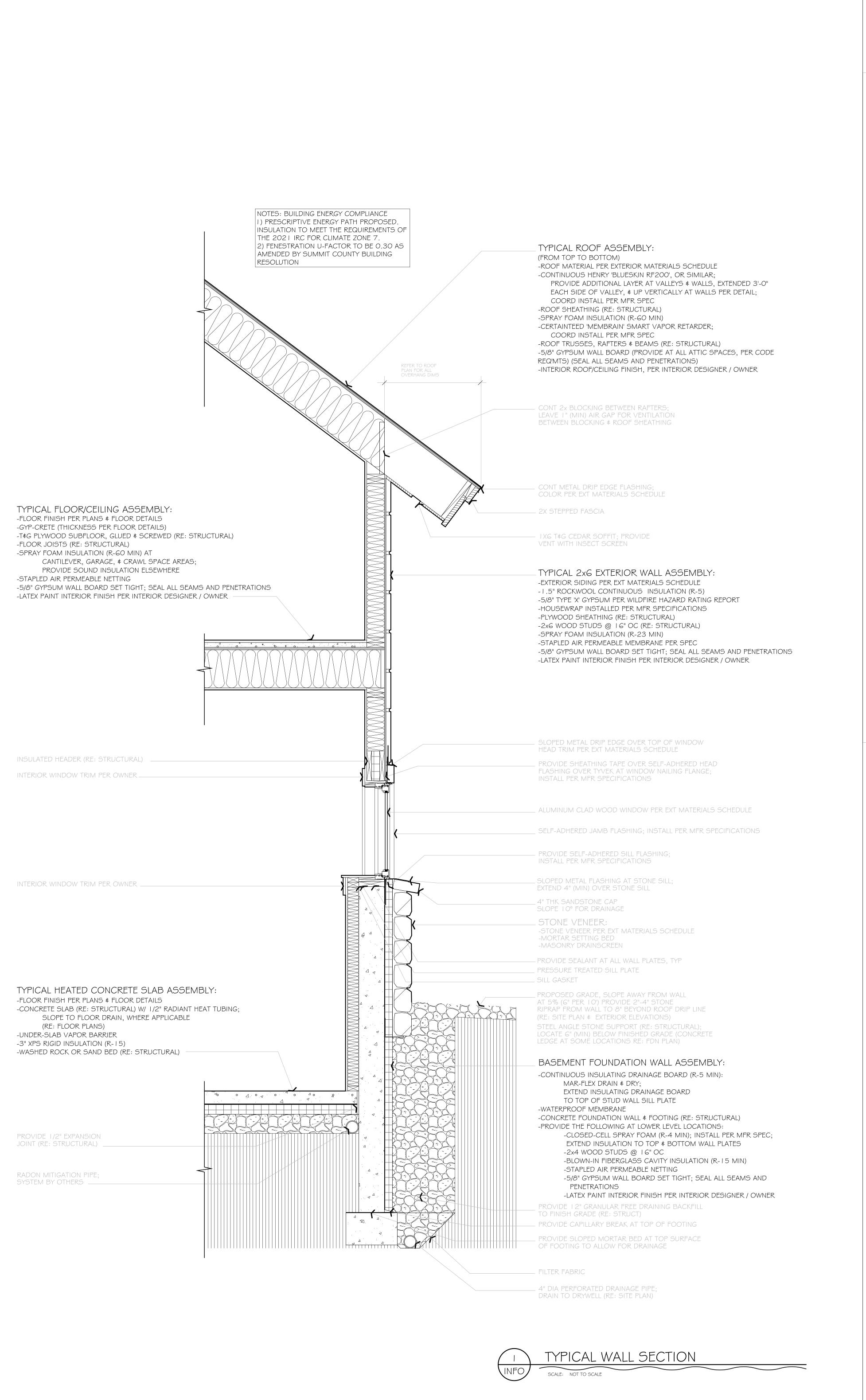
BACKCOUNTRY STRUCTURAL ENGINEERING

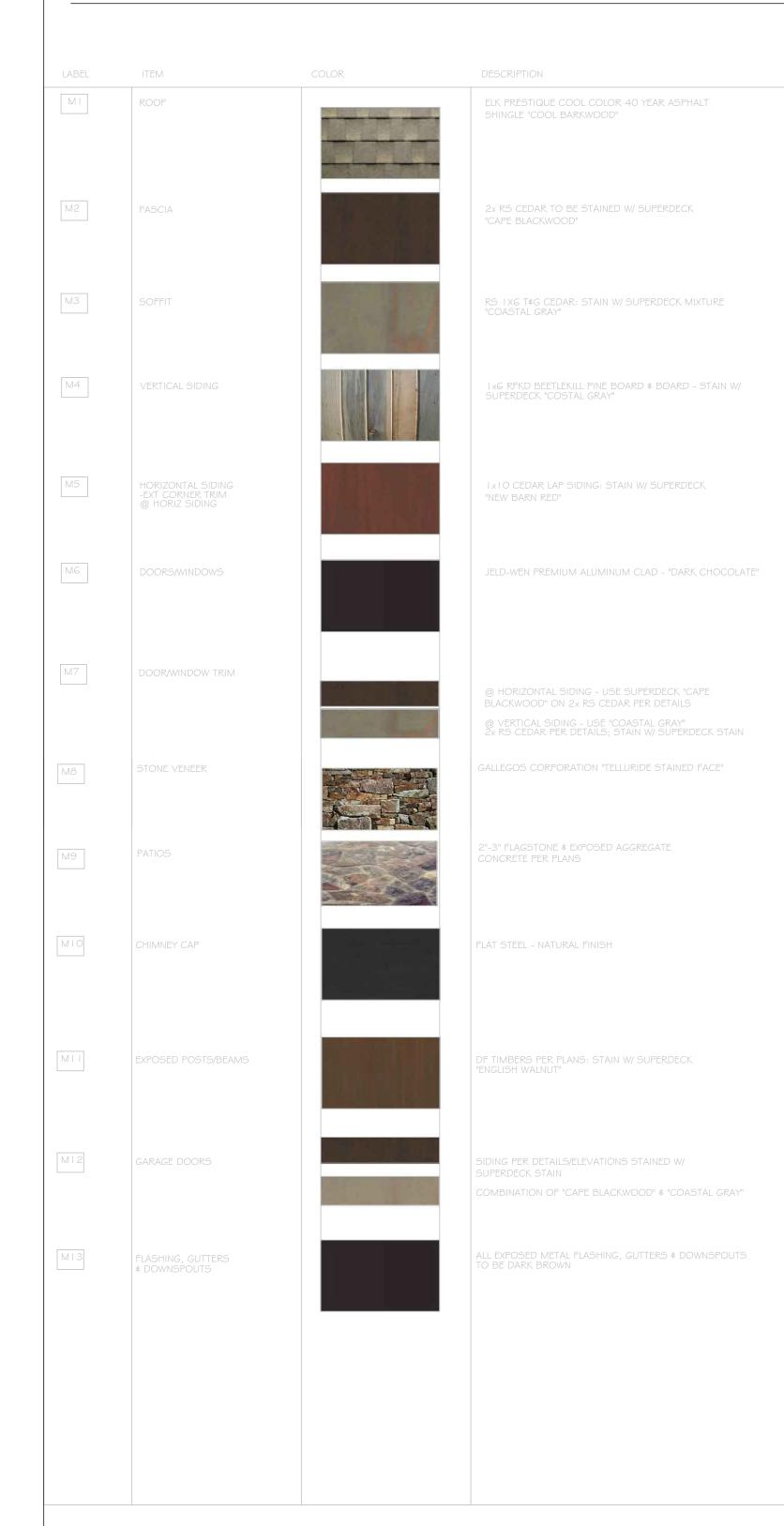




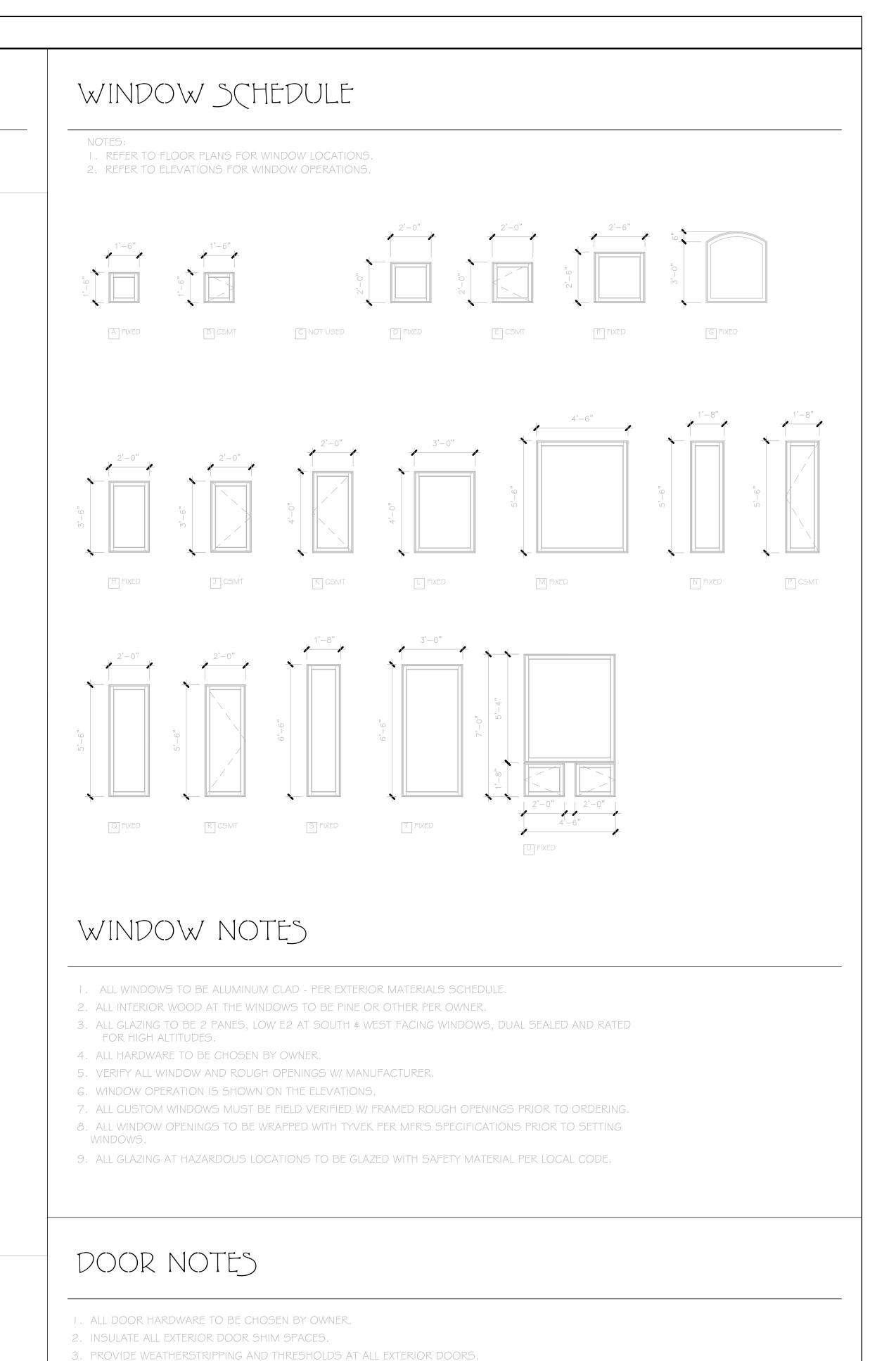
G2 Designs LLC Silverthorne . Colorado 720-982-7425 g2designsllc@outlook.com

Project #





EXTERIOR MATERIALS SCHEDULE





4. ALL DOOR OPENINGS TO BE WRAPPED WITH TYVEK PRIOR TO SETTING DOOR JAMBS.

I. ALL MANUFACTURERS' INSTALLATION SPECIFICATIONS AND INSTRUCTIONS ARE TO BE FOLLOWED.

5. ALL EXTERIOR DOORS (EXCEPT FRONT DOOR) TO BE ALUMINUM CLAD PER EXTERIOR MATERIALS SCHED.

- 2. TYPICAL WALL BASE TRIM: PER OWNER.
- 3. TYPICAL INTERIOR DOOR CASING: PER OWNER. 4. TYPICAL INTERIOR WINDOW CASING/TRIM: PER OWNER.
- 5. TYPICAL WALLS AND CEILINGS TO RECEIVE 1/2" GYP BOARD, UNLESS NOTED OTHERWISE. ALL GYP BOARD TO RECEIVE TAPE, THREE COATS OF MUD LIGHT HAND TROWELED TEXTURE, PRIMER, AND TWO COATS OF PAINT UNLESS NOTED OTHERWISE. SAMPLES OF TEXTURE TO BE SUPPLIED ON SITE \$ SELECTED BY OWNER AND DESIGNER AT THAT TIME. ROUND CORNER BEAD TO BE USED THROUGHOUT.
- 7. PROVIDE 1/2" CEMENTITIOUS "TILE BACKER" UNDERLAYMENT AT ALL CERAMIC TILE AND WITHIN
- 2'-0" OF ANY WATER SOURCE.
- 8. ALL PLUMBING FIXTURES PER INTERIOR SPECIFICATIONS.
- 9. ALL VANITY CABINETS TO BE PER OWNER, 36" HIGH, WIDTH PER FLOOR PLANS. 10. ALL FLOORING PER OWNER.
- II. ALL INTERIOR WALLS TO HAVE R-II BATT INSULATION FOR SOUNDPROOFING.

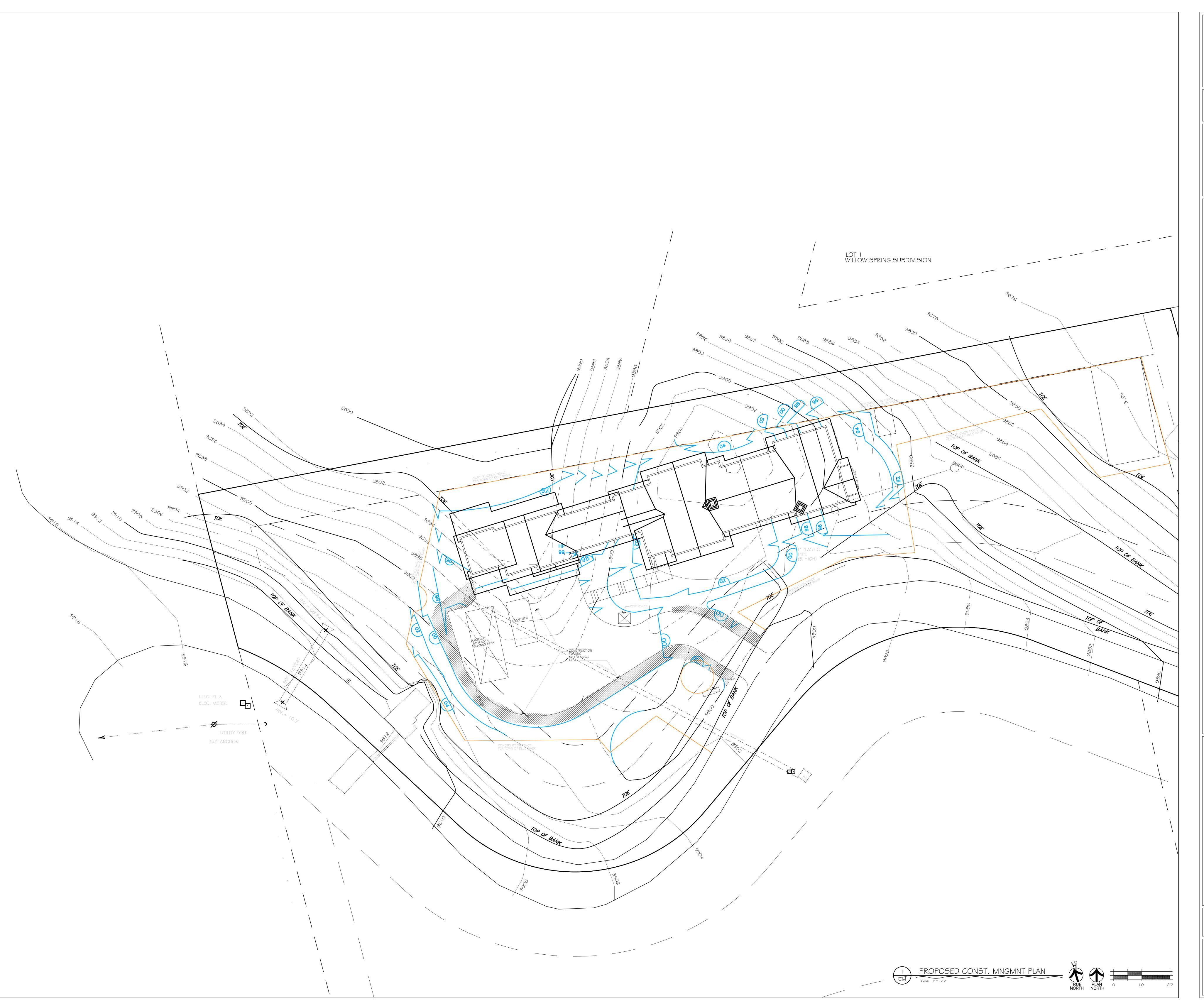
# MECHANICAL/PLUMBING NOTES

- I. HEATING SYSTEM: IN-FLOOR RADIANT HEAT.
- 2. HEATING SYSTEM AT GARAGE: IN-SLAB RADIANT HEAT.
- 3. IN-FLOOR RADIANT HEAT PIPING TO COME WITH A 20 YEAR WARRANTY, MINIMUM TO BE APPROVED
- 4. BOILER TO BE SIZED BY MECHANICAL COMPANY IN BID AND APPROVED BY GC.
- 5. MECHANICAL CONTRACTOR TO PROVIDE VENT PIPES PER TOWN STANDARDS FOR ALL VENTS INCLUDING, BUT NOT LIMITED TO: KITCHEN HOODS, DRYER VENTS, BATH VENTS, UTILITY VENTS, ECT...
- THE VENTS ARE TO BE SIZED PER THE MANUFACTURER'S SPECIFICATIONS. 6. ALL PLUMBING TO BE INSTALLED PER THE APPLICABLE CODE.
- 7. WET PLUMBING DROPS FROM SECOND TO FIRST FLOOR TO BE ABS. ALL OTHER DRAIN VENT PIPES
- 8. MECHANICAL CONTRACTOR TO PROVIDE GAS CHIMNEY FOR BOILER.
- 9. ALL WATER PIPE TO BE COPPER AND SIZED TO ALLOW AN ADEQUATE SUPPLY TO EACH FIXTURE. IO. HOT WATER RECIRCULATING SYSTEM AND PUMP TO BE INSTALLED PER MANUFACTURER'S
- I I. WATER HEATER TO BE SIZED TO ALLOW TWO TUBS TO BE USED SIMULTANEOUSLY.
- I 2. EXTERIOR FROST-FREE HOSE BIBBS TO BE PLACED PER FLOOR PLANS. 13. INSTALL BALL VALVE SHUT-OFF AT MECHANICAL ROOM.
- 14. INSTALL BALL VALVE AT HOT AND COLD WATER AT WATER HEATER.
- 15. PLUMBER TO SUPPLY ROOF JACKS AND GAS CHIMNEY FOR HOT WATER HEATER AS REQUIRED. 16.3/4" COPPER PIPE TO EACH SHOWER FIXTURE, TYPICAL (PER CONTRACTOR).

G2 Designs LLC Silverthorne . Colorado 720-982-7425 g2designsllc@outlook.com

ISSUE:	DATE:
PERMIT	07/11/2023
PERIVITI	——————————————————————————————————————

Project #





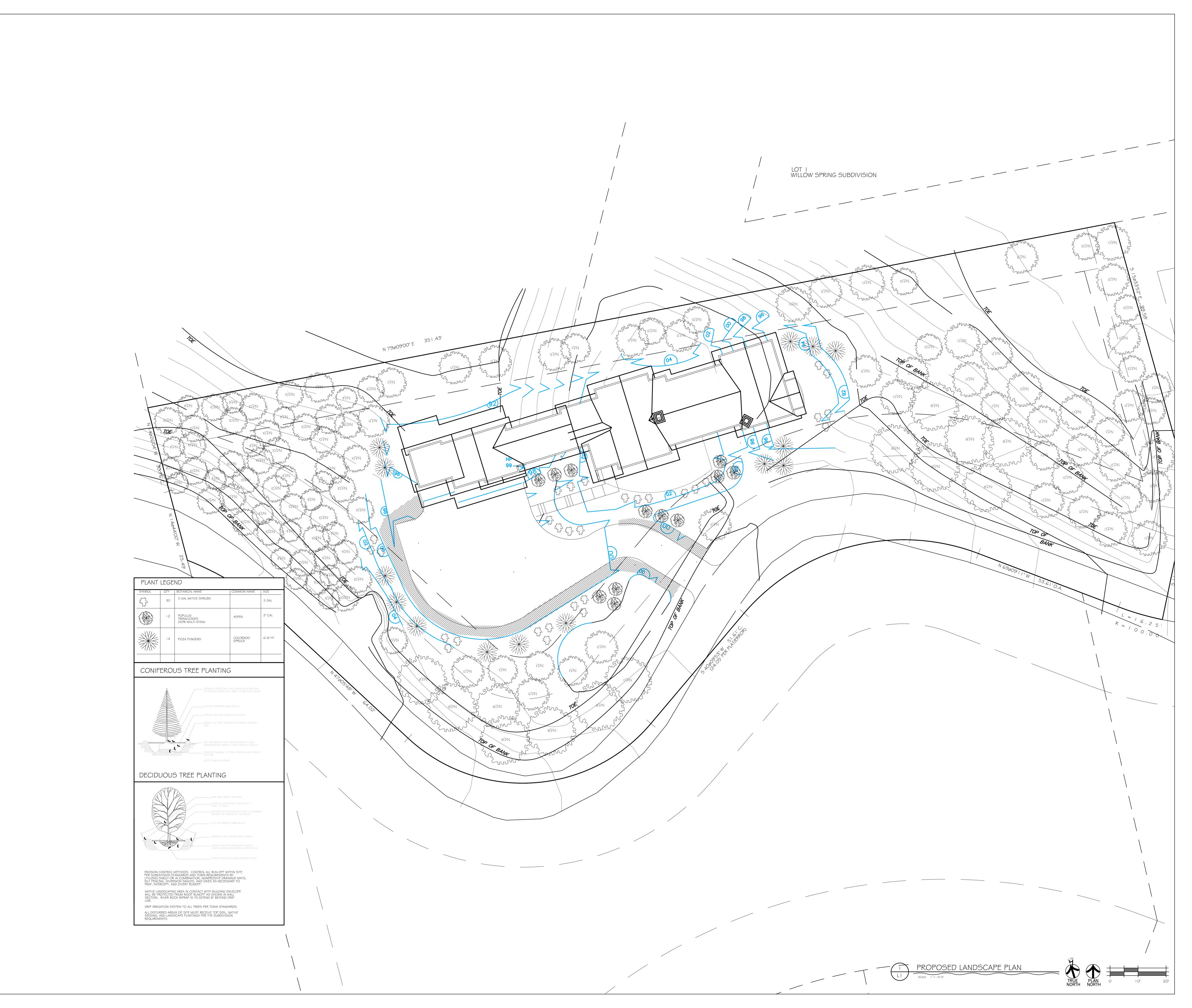
LASSA RESIDENCE
LOT2. RIVERSHORE SUBDIVISION
TOWN OF BLUE RIVER. SUMMIT COUNTY. COLORADO

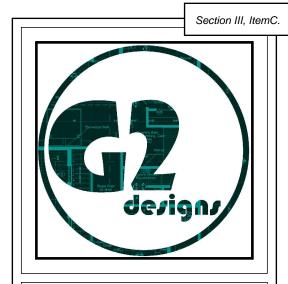
THE:

PROPOSED LANDSCAPE PLAN

Project # 2309

CM

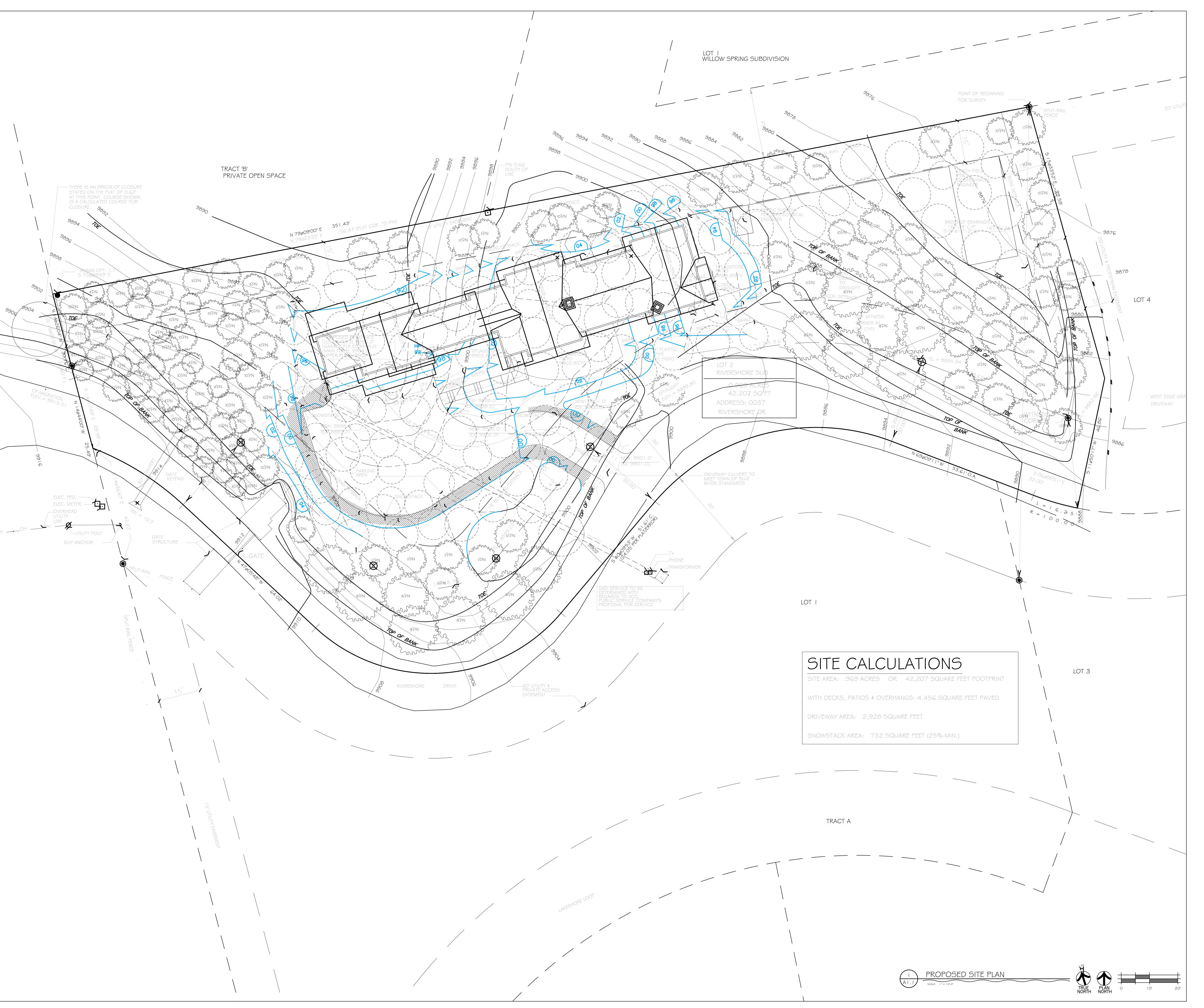




LANDSCAPE PLAN

Project # 2309

**L1** 

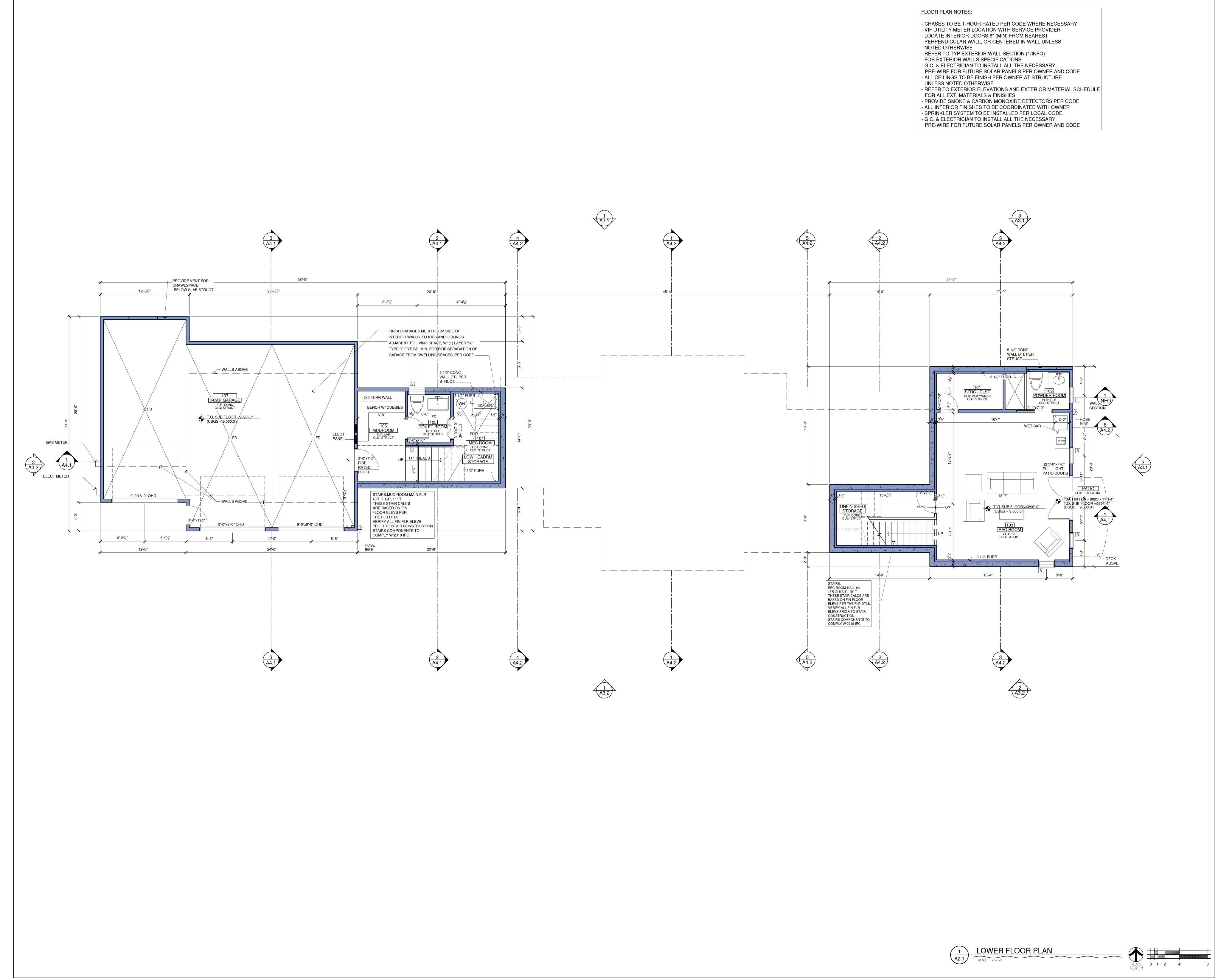




POSED SITE PLAN

Project # 2309

A1.1



Section III, ItemC.

G2 Designs LLC Silverthorne . Colorado 720-982-7425 g2designsllc@outlook.com

SA RESIDENCE
RSHORE SUBDIVISION
LUE RIVER. SUMMIT COUNTY. COLORADO

VER FLOOR PLAN

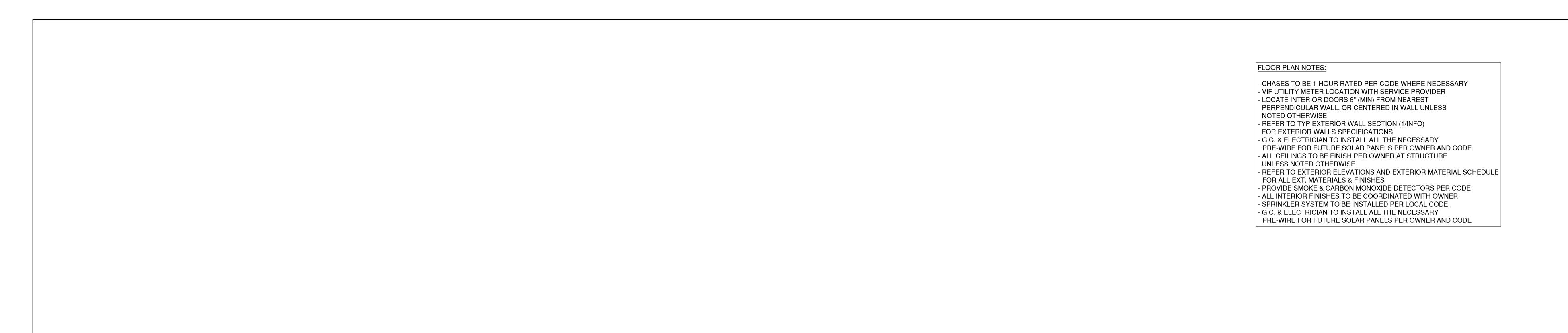
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O7/11/

Project # 2309

**A2.1** 



136'-0"

10'-0"

6'-0"

STAIRS: REC ROOM-HALL #3 13R @ 7 3/8", 10" T THESE STAIR CALCS ARE

BASED ON FIN FLOOR ELEVS PER THE FLR DTLS. VERIFY ALL FIN FLR ELEVS PRIOR TO STAIR CONSTRUCTION. STAIRS COMPONENTS TO COMPLY W/2018 IRC 11'-2"

5'-10"

11'-5" STAIRS: HALL #3-LANDING TO PRIMARY 13R 6 3/8", 11" T
THESE STAIR CALCS ARE
BASED ON FIN FLOOR

ELEVS PER THE FLR DTLS.
VERIFY ALL FIN FLR
ELEVS PRIOR TO STAIR
CONSTRUCTION.

STAIRS COMPONENTS TO COMPLY W/2018 IRC

5'-11"

10'-0"

GAS | | | FIREPLACE<sub>+</sub> | —

— SEE ELEVATION 1/A6 FOR

LOCATION OF WDWS ABV

16'-3½"

6'-6"

3'-8½"

(2) 2'-6"x7'-6" FULL LIGHT PATIO DRS

14'-0"

11'-6"

SHELVING

6" 4'-6"

4'-6" 2'-0" 2'-6" 1

9'-0"

STAIRS: MAIN FLR-LANDING UPPER FLR 7R 7 1/16", 11" T THESE STAIR CALCS ARE BASED ON FIN FLOOR

ELEVS PRIOR TO STAIR CONSTRUCTION.

STAIRS COMPONENTS TO COMPLY W/2018 IRC

BENEATH SUBFLOOR)

STAIRS: MAIN FLR-MUDROOM 10R @ 7 1/14", 11" T THESE STAIR CALCS ARE

BASED ON FIN FLOOR ELEVS PER THE FLR DTLS.

STAIRS COMPONENTS TO COMPLY W/2018 IRC

VERIFY ALL FIN FLR ELEVS PRIOR TO STAIR CONSTRUCTION.

12'-5½"

<sup>∖</sup>— WٰALLS

121 BEDROOM #4 FLR: CPT CLNG: STRUCT

DOBSCURED DOBSCURED

12'-0"

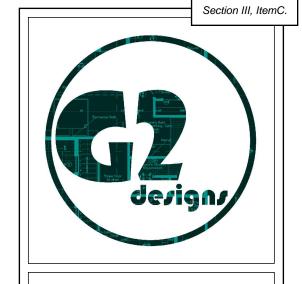
3'-0" 3'-0" 3'-0" 3'-0" 3'-0" 3'-6" 3'-6" 3'-6"

11'-0"

BELOW —

6'-4"

BI-FOLD DOOR



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LASSA RESIDE
LOT 2. RIVERSHORE SUBDIVISION
TOWN OF BLUE RIVER. SUMMIT COUNTY. C
TITLE:

MAIN FLOOR P

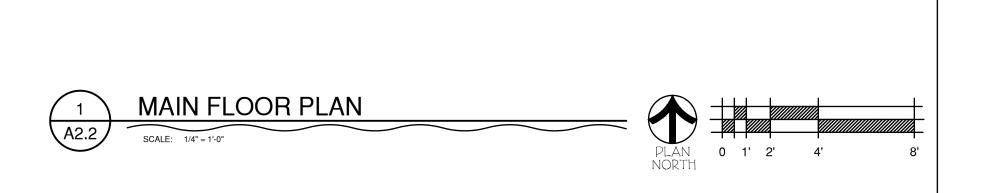
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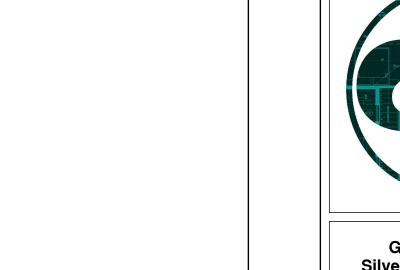
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Project # 2309

**A2.2** 





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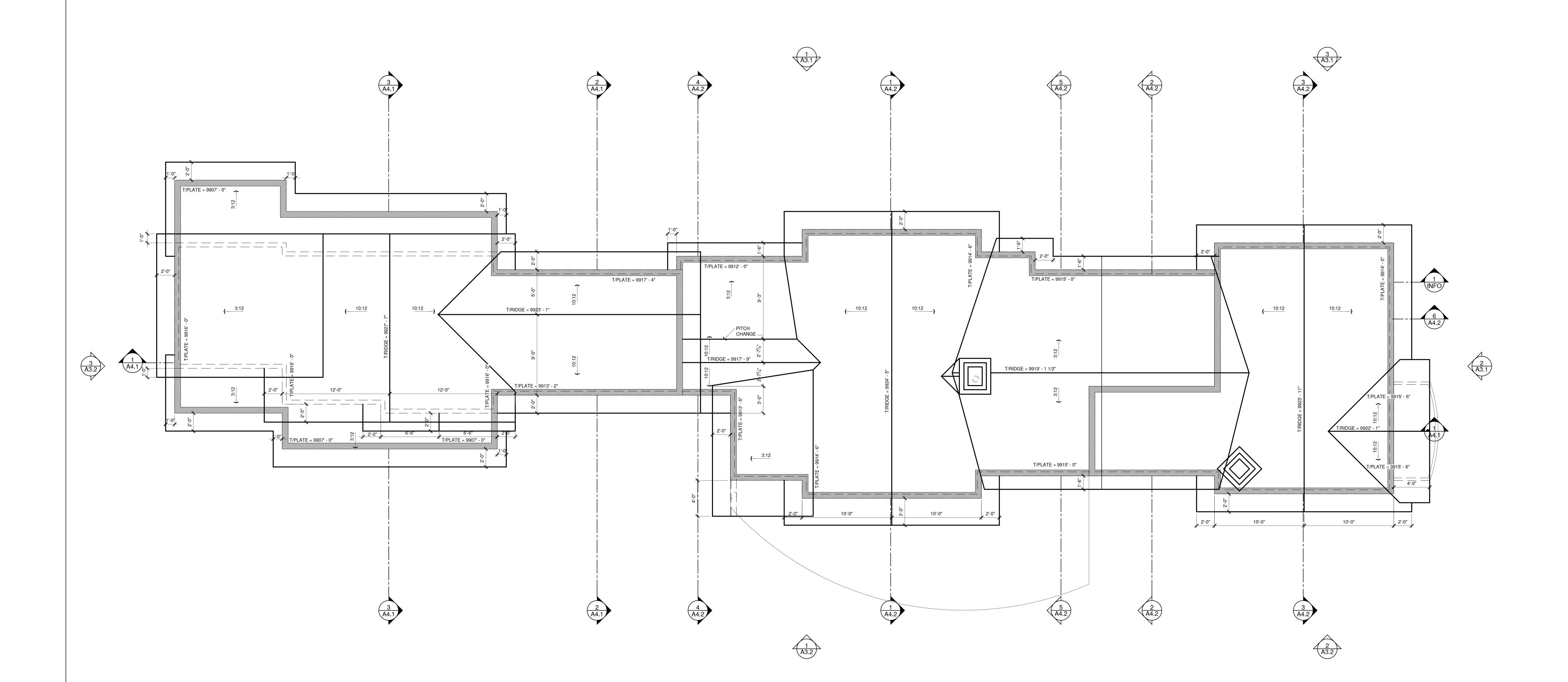
Project # 2309

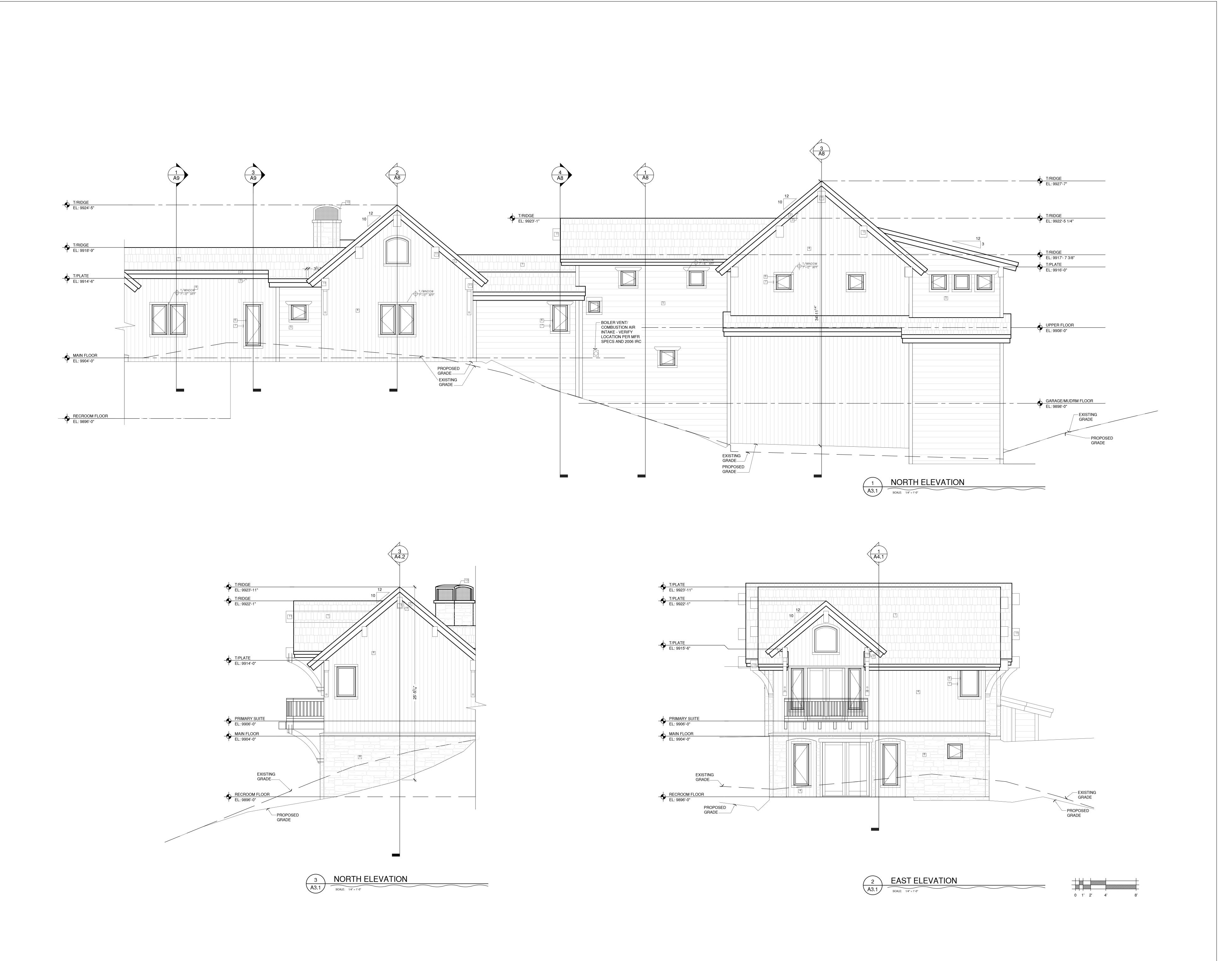
ROOF PLAN NOTES: - G.C. TO INSTALL SNOW GUARDS, GUTTERS, DOWNSPOUTS AND HEAT TAPE ABOVE ALL DECKS, PATIO, WALKWAYS, AND DRIVE WAYS. SNOW GUARD DESIGN AND LAYOUT FOR ROOF TO BE ENGINEERED BY TRA SNOW & SUN INC. OR SIMILAR (SNOW GUARDS SHOWN ON ROOF PLAN ONLY FOR PRELIMINARY BIDDING PURPOSE) - INSTALL SOFFIT MOUNTED RECEPTACLES PER OWNER AND LOCAL CODE - REFER TO EXTERIOR ELEVATIONS AND EXTERIOR MATERIAL SCHEDULE FOR ALL EXT. MATERIALS & FINISHES - EXTEND DOWNSPOUTS ENDS 4' MIN FROM FOUNDATION

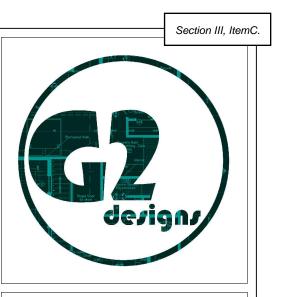
- G.C. & ELECTRICIAN TO INSTALL ALL THE NECESSARY

PRE-WIRE FOR FUTURE SOLAR PANELS PER OWNER AND CODE

WALLS OR AS REQUIRED FOR DRAINAGE

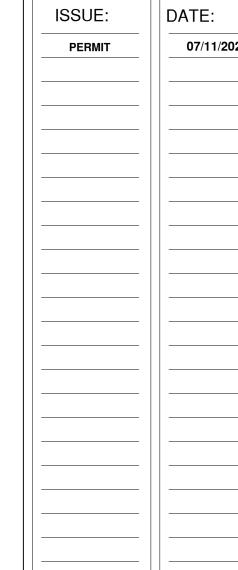






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TITLE:

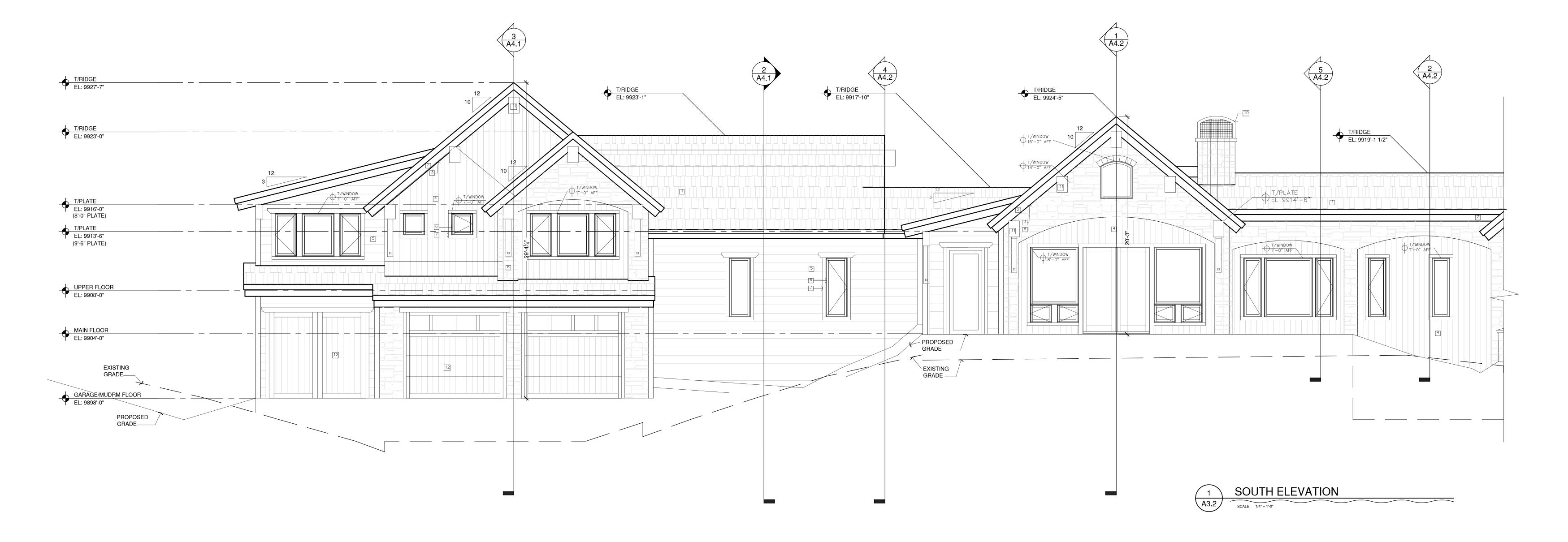
EXTERIOR ELEVATIONS

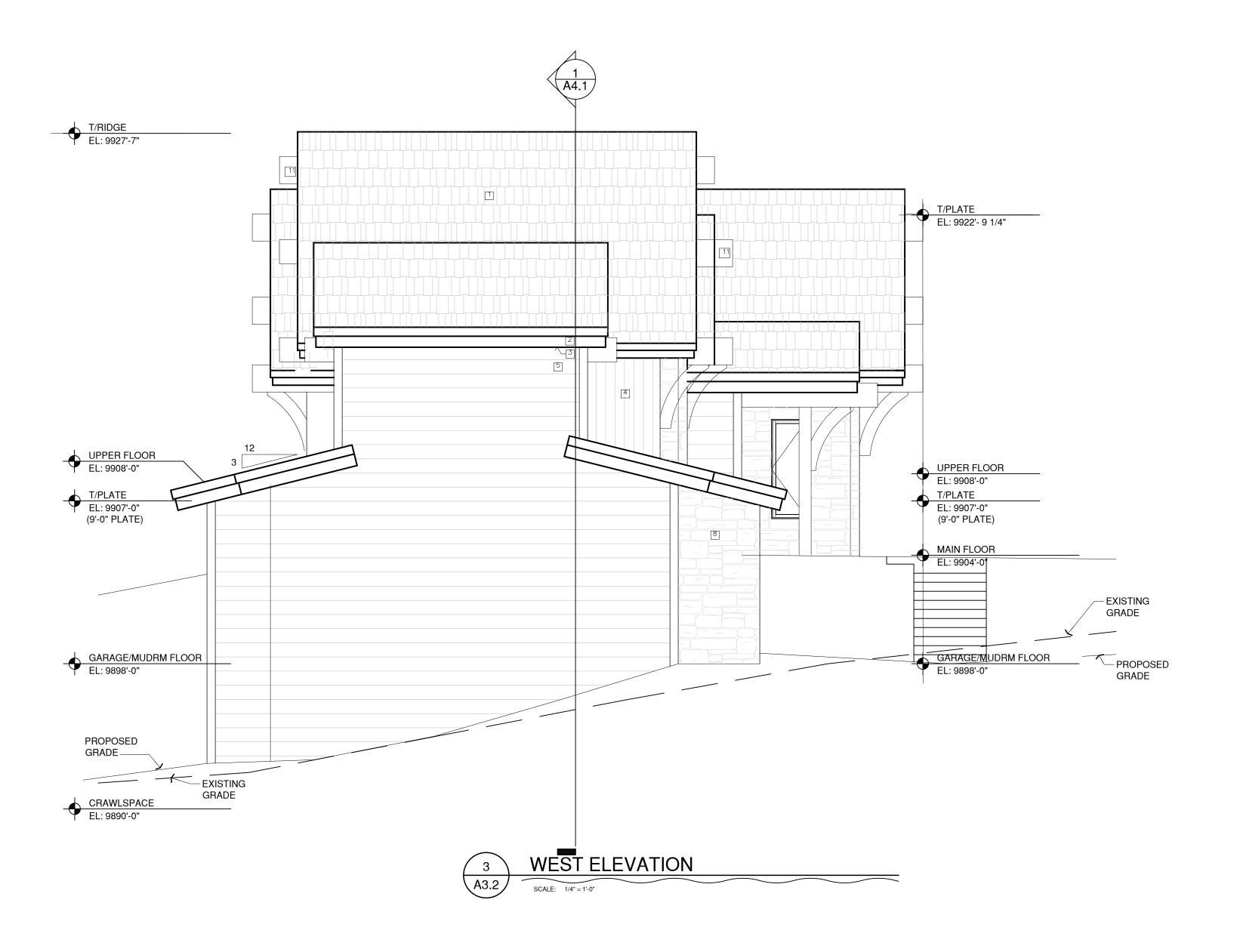


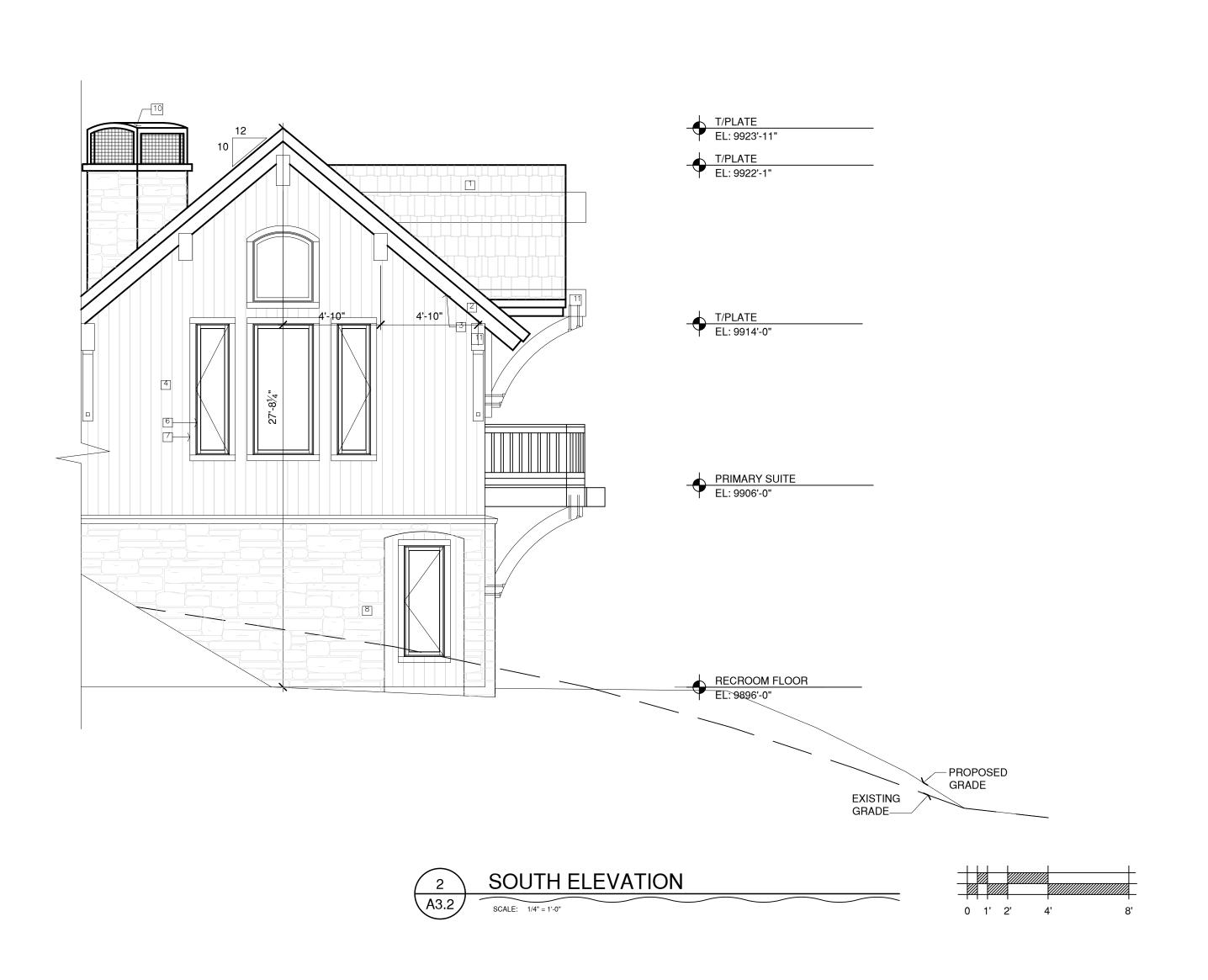
Project # 2309

**A3.1** 









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TOWN OF BLUE RIVER. SUMMIT COUNTY. COLORADO

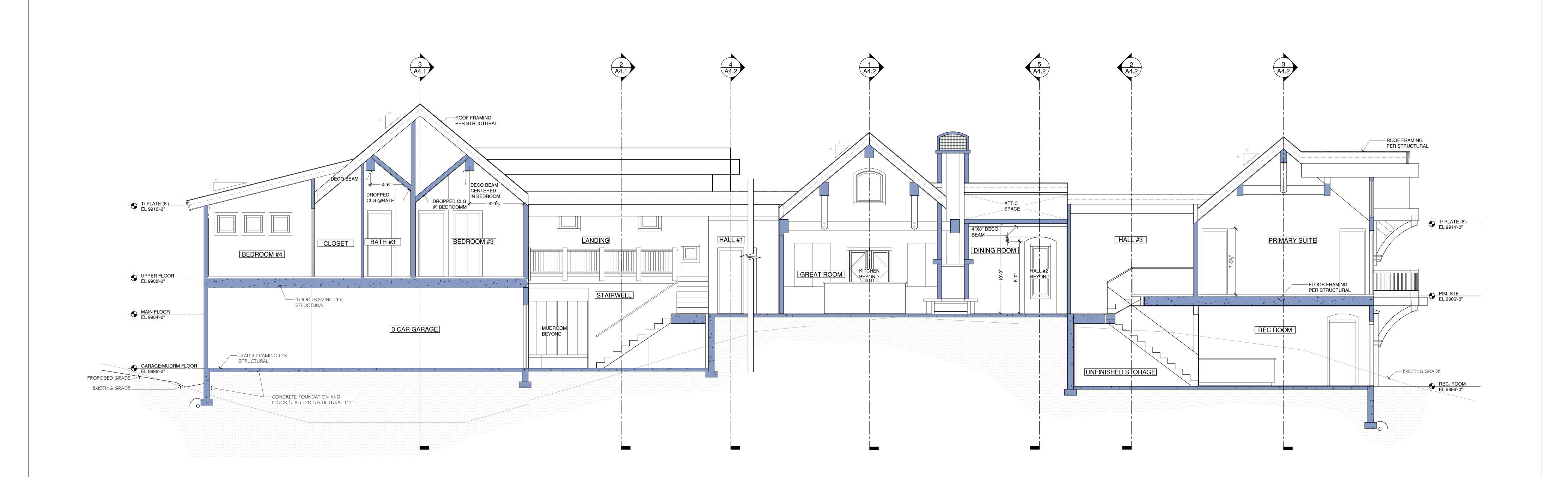
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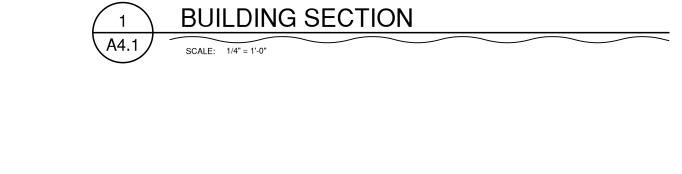
EXTERIOR ELEVATIONS

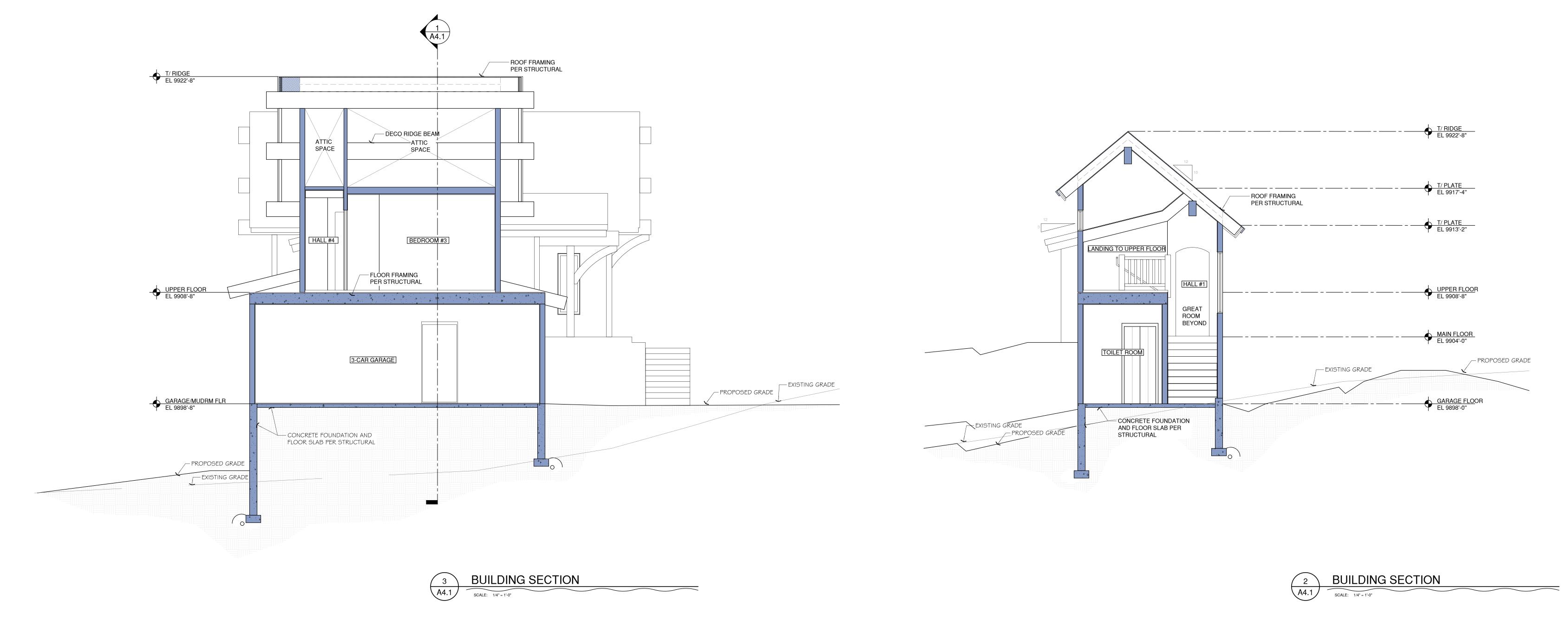
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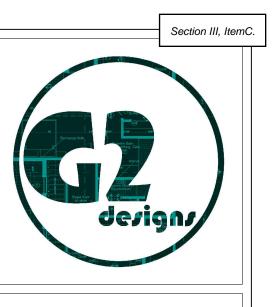
Project # 2309

**A3.2** 









LASSA RESIDENCE
LOT 2. RIVERSHORE SUBDIVISION
TOWN OF BLUE RIVER. SUMMIT COUNTY. COLORADO
TITLE:

BUILDING SECTIONS

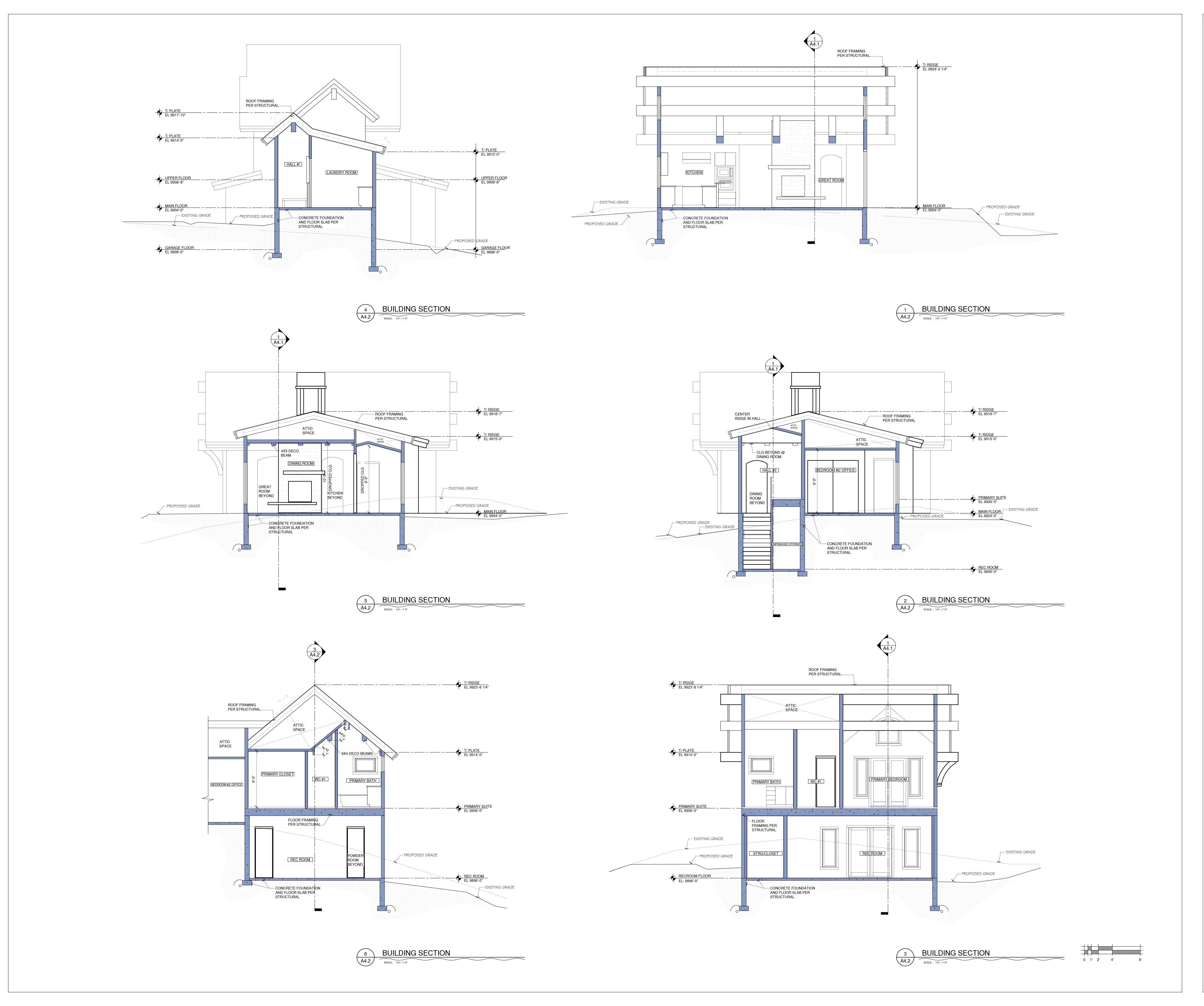
07/11/2023

PERMIT

Project # 2309

0 1' 2' 4' 8'

**A4.1** 





RESIDENCE SUBDIVISION : COLORADO

NG SECTIONS

ISSUE:
PERMIT

DATE:
07/11/2023

Project # 2309

**A4.2** 

1.1. Wide Flange and WT sections — ASTM A992 or ASTM 572 Grade 50, 1.2. Other rolled shapes, M, S, HP, C, MC, and angles — ASTM A36, Fy=36

1.3. Pipe – ASTM A53, Fy=35 ksi 1.4. Square and Rectangular HSS – ASTM A500, Fy=46 ksi 1.5. Round HSS – ASTM A500, Fy=42 ksi 1.6. Plate – ASTM A36, Fy=36 ksi

1.7. Anchor rods - ASTM F1554 Grade 36, Fy=36 ksi 2. All structural steel shall be fabricated and erected per the current edition of AISC Steel Construction Manual. 3. Connections: 3.1. Engineer of Record has designed all connections. If a connection design

is inadvertently omitted from contract documents the contractor shall request connection design from the Structural Engineer. 3.2. Bolted Connections:

3.2.1. Minumum bolt diameter:  $\sqrt[3]{4}$   $\phi$  A325 unless noted otherwise 3.2.2. Snug tight bolts unless noted otherwise 3.3. Welded Connections: 3.3.1. Welding Qualifications: Welding shall be done only by welding operators currently qualified according to AWS D1.1.

3.3.3. 3/16" continuous fillet welds unless noted otherwise

3.3.2. E70XX series electrodes

4. Shop Cleaning and Painting 4.1. Coordinate all shop painting of structural steel with Architect's painting requirements as specified on the architectural drawings and specifications. Primer paint shall be compatible with architectural finish paint. Clean structural steel scheduled to receive architectural finish paint in accordance with SP-6 "Commercial Blast Cleaning".

4.2. Members that are exposed to earth or weather in the finished structure shall be hot—dipped galvanized unless noted otherwise. Galvanizing shall not contaminate or otherwise impede the welding process. 5.1. Submit Shop Drawings including complete details and schedules for fabrication and shop assembly of members, and details, schedules, procedures and diagrams showing the sequence of erection.

5.1.1. Include details of cuts, connections, camber, holes and other pertinent data. Indicate welds by standard AWS symbols, show size. length and type of each weld. 5.1.2. Provide setting drawings, templates and directions for the installation of anchor bolts and other anchorages to be installed

under other Sections of Work.

1. All framing and details not specifically specified shall comply with the prescriptive (non-engineered) requirements of the International Residential 2. Products: Unless noted other wise on the drawings, all wood framing shall have the following minimum properties and be at a moisture content of

2.1. Studs: Hem-Fir Stud Grade or better **9** 16" o.c. 2.2. Light Framing (4x of less): 2.2.1. Hem-Fir No. 1 (HF No. 1) Flexural Stress Fb = 975 psi Compressive Srtess Fc = 1350 psi Horizontal Shear Stress Fv = 150 psi Modulus of Elasticity E = 1,500,000 psi2.2.2. Hem—Fir No. 2 (HF No. 2) Flexural Stress Fb = 850 psiCompressive Stress Fc = 1300 psiHorizontal Shear Stress Fv = 150 psi Modulus of Elasticity E = 1,300,000 psi

2.3. Heavy Timbers (5x5 or larger): 2.3.1. Douglas Fir—Larch No. 1 (DFL No. 1) Beams (d>b+2") Columns (d≤b+2") Fb = 1350 psiFb = 1200 psi

Fc = 925 psi Fc = 1000 psiFv = 170 psi Fv = 170 psi E = 1,600,000 psiE = 1,600,000 psi2.3.2. Douglas Fir-Larch No. 2 (DFL No. 2) Beams (d>b+2") Columns (d≤b+2") Fb = 875 psi Fb = 750 psi Fc = 600 psi Fc = 700 psi

Fv = 170 psi Fv = 170 psi E = 1,600,000 psiE = 1,600,000 psi2.4. See architectural drawings for appearance grading of members. 2.5. Sills: All sill plates shall be pressure treated Hem Fir or Southern Pine. 2.6. Glulams per ANSI A190.1: 2.6.1. Designation 24F-V4: Flexural Stress Top Fbx+ = 2400 psiFlexural Stress Bottom Fbx- = 1850 psi Horizontal Shear Stress Fvx = 265 psiCompressive Stress Fc = 1650 psiFt = 1100 psi Tension Stress Modulus of Elasticity Ex = 1,800,000 psiEaxial = 1,700,000 psi2.7. Engineered Wood: 2.7.1. Laminated Strand Lumber –  $1\frac{1}{2}$  Wide (LSL – 1.3E Studs):

Flexural Stress Fb = 1700 psi Compressive Parallel to Grain Fc|| = 1400 psi Compressive Prep to Grain  $Fc \perp = 680 \text{ psi}$ Horizontal Shear Stress Fv = 400 psi Modulus of Elasticity E = 1,300,000 psi2.7.2. Laminated Strand Lumber  $-1\frac{1}{4}$ " (LSL -1.55E Beams): Flexural Stress Fb = 2325 psiCompressive Parallel to Grain Fc|| = 2050 ps

Compressive Prep to Grain Fc⊥ = 800 psi Horizontal Shear Stress Fv = 310 psi Modulus of Elasticity E = 1,550,000 ps2.7.3. Laminated Veneer Lumber  $-1\frac{3}{4}$ " (LVL -1.9E): Flexural Stress Compressive Parallel to Grain Fc|| = 2510 psi Compressive Prep to Grain  $Fc \perp = 750 \text{ psi}$ Horizontal Shear Stress Fv = 285 psi

Modulus of Elasticity E = 1,900,000 psi 2.8. Wood I-Joist: Boise Cascade "BCI" engineered wood I-Joists 2.8.1. Substitution of equal product is acceptable upon Submittal of equal by contractor and approval by structural engineer. 2.9. Structural Panels (Plywood or OSB):

2.9.1. Sheathing for roofs and walls shall conform to APA PS-1 standards. Lay panel with long dimension perpendicular to joists with short edges staggered. All panels shall be exposure I, U.N.O. on plan. 2.9.2. Panel grades and thickness: APA Span Rating (min) 19/32

23/32 Floors(STRUD-I T&G) 24 oc Single Floor 15/32 Shear Walls 15/32 Exterior Walls 3. Connectors 3.1. All blots, metal connectors, hangers, anchors, and fasteners in contact with preservative treated wood shall be hot dipped galvanized or

3.2. Anchor Bolts: 3.2.1. Provide  $\frac{1}{2}$  embedded bolts © 32" OC (max), with 7" minimum embed, tops of walls for attaching sill plates, except provide anchor bolts **9** 16" OC under shear walls. As a minimum, provide 2 bolts, each within 12" of the ends of each piece of sill plate. 3.2.2. Provide 1/4"x3"x3" plate washers at all shear wall anchor bolt

connections to sill plates. 3.3.1. Minimum nailing shall comply with Table 2304.9.1 of the IBC unless more stringent requirements are shown on these drawings. 3.3.2. All nails are to be common nails. Where power nails are used, the

shall be equivalent in diameter to the common nails indicated. 3.4. Bolts: 3.4.1. All bolts shall conform to ASTM 307. 3.4.2. Holes for bolts shall be  $\frac{1}{16}$  versize. 3.4.3. Retighten all bolts prior to closing in.

3.5. Lag Screws:

4.1. Floor and Roof Sheathing:

3.5.1. Lag screws shall penetrate the main member a minimum of 8 times the shaft diameter. 3.5.2. Lead holes for lag screws shall be 60% to 70% of lag shank diameter in compliance with AITC criteria.

4.1.1. Floors: Glue and nail 8d @ 6" OC edges and 8d @ 12" OC field. unless noted otherwise on plans 4.1.2. Roofs: Nail 8d @ 6" OC edges and 8d @ 12" OC field, unless noted otherwise on plans 4.2. Wall Sheathing:

4.2.1. Walls not designated as Shear Walls, nail 8d **©** 6" OC edges and 8d

12" OC field 4.3. Shear Walls: 4.2.3. Where shear walls are noted on the plans the sheathing is used as part of the lateral load resisting system. Typical details for wood

panel shear walls in the drawings and the following requirements 4.2.3.1. The shear wall extends between consecutive "king" studs at

adjacent wall openings. 4.2.3.2. All panel edges within the extent of the shear wall shall be blocked with flat 2x4 blocking. 4.2.3.3. Where shear walls bear on joists or beams, provide 5-16d

nails through sill plate to joist below at each stud space within the width of the shear wall. 4.2.3.4. Where floor and roof diaphragms abut shear walls, provide a minimum of 8d **@** 3" o.c. nailing to blocking or rim joist and Simpson A35 connection at 32" o.c. between blocking or rim

ioist and shear wall top plate. 4.2.3.5. Provide hold-downs to foundation walls on all boundary 4.3. General Framing Tolerances:

4.3.1. Framing members which will be covered by finishes such as wallboard, plaster, or ceramic tile set in a mortar setting bed, shall be within the following limits: 4.3.1.1. Layout of walls and partitions:  $\frac{1}{4}$  from intended position;

4.3.1.2. Plates and runners: 1/4 inch in 8 feet from a straight line; 4.3.1.3. Studs:  $\frac{1}{4}$  inch in 8 feet out of plumb, not cumulative; 4.3.1.4. Face of framing members: 1/4 inch in 8 feet from a true plane. 4.3.2. Framing members which will be covered by ceramic tile set in dry—set mortar, latex—portland cement mortar, or organic adhesive

shall be within the following limits: 4.3.2.1. Layout of walls and partitions: 1/4 inch from intended position; 4.3.2.2. Plates and runners:  $\frac{1}{8}$  inch in 8 feet from a straight line; 4.3.2.3. Studs: 1/8 inch in 8 feet out of plumb, not cumulative; Face of framing members:  $\frac{1}{8}$  in 8 feet from a true plane

1. See Material section of these General Notes for required shop drawings. 2. Manufacturers Data: Submit two (2) copies of manufacturer's specifications and installation instructions for each product specified. 3. Shop Drawings: Submit four (4) prints or one (1) electronic copy of each shop drawing. Shop drawings shall be reviewed by Contractor prior to submission and shall bear the Contractors approval stamp. Allow 14 calendar days in the Structural Engineers office for review of shop

drawings. MISCELLANEOUS NOTES 1. The Contractor is solely responsible for all safety regulations, programs and precautions related to all work on this project. 2. The Contractor is solely responsible for the protection of persons and

property either on or adjacent to the project and shall protect it against

injury, damage, or loss.

3. Means and methods of construction and erection of structural materials are solely the Contractor's responsibility. 4. Do not place equipment when shipping or operating weight exceeds weight indicated on structural drawings..

5. Fireproofing of structural elements is not shown on the structural drawings. Refer to the specifications and architectural drawings for fire rating

7. No structural modifications, alterations, or repairs shall be made without prior review by Structural Engineer. Submit details and calculations prepared by a professional engineer registered in state where project is located and employed by contractor.

6. Do not scale these drawings, use the dimensions shown.

**Quality Control** 1. The Contractor is responsible for quality control, including workmanship and materials furnished by his subcontractors and suppliers. 2. Inspection or testing by the Owner does not relieve the Contractor of his responsibility to perform the work in accordance with the Contract

3. Workmanship: The Contractor is responsible and shall bear the cost of correcting work which does not conform to the specified requirements. 4. Correct deficient work by means acceptable to the Architect. The cost of extra work incurred by the Architect to approve corrective work shall be borne by the Contractor.

Structural Special Inspection, Testing, and Observation 1. Special Inspection beyond normal city and county inspection is not required per IBC 1704.



		GENERAL LEGEND	
1 A101	ELEVATION VIEW	X	INDICATES ADDENDUM NUMBER
A101	SECTION CUT		INDICATES STEPS AND SLOPES IN DECKS & SLABS
1 A101	DETAIL CALL OUT	◆ XXX'-YY" T/SLAB	ELEVATION CALLOUT Name = T.O. (OR B.O.) OBJECT Elevation = OBJECT ELEVATION

# STRUCTURAL ABBREVIATIONS

<u>ABBREVIATION</u>	<u>DEFINITION</u>	<u>ABBREVIATION</u>	<u>DEFINITION</u>
ALT	alternate	KLF	kips per lineal foot
ARCH	architectural	LBS	pounds
ВОТ	bottom	LLH	long leg horizontal
BLDG	building	LLV	long leg vertical
3 <b>M</b>	beam	LSL	laminated strand lumber
BTWN	between	LVL	laminated veneer lumber
CIP	cast-in-place	MAX	maximum
CLR	clear	MECH	mechanical
CMU	concrete masonry unit	MIN	minimum
COL	column	(N)	new construction
CONC	concrete	OC	on center
CONT	continuous	ОН	opposite hand
DIA, φ	diameter	PAF	powder actuated fastener
DIM	dimension	PERP	perpendicular
DTL	detail	PJP	partial joint penetration
DWGS	drawings	PL, - [2	plate
DWL	dowel	PLF	pounds per lineal foot
EA .	each	REINF	reinforcement
F	each face	REQD	required
ELEV	elevation	SOG	slab on grade
EW .	each way	SCHED	schedule
EXIST, (E)	existing construction	SIP	structural insulating panel
EXP	expansion	STFNR	stiffener
-DN	foundation	STL	steel
FLR	floor	THK	thickness
-TG	footing	TRAN	transverse
GLB	glulam	TYP	typical
HORIZ	horizontal	UNO	unless noted otherwise
ICF	insulated concrete form	VERT	vertical
KIP	1000 lbs	<i>WWF</i>	welded wire fabric

Engineering for the High Country BACKCOUNTRY STRUCTURAL ENGINEERING P.O. BOX 23132 SILVERTHORNE . COLORADO . 80498 PH: 970.333.1511 WEB SITE: WWW.BCSTRUCTURAL.COM THE STRUCTURAL ENGINEERS SEAL ON THIS DRAWING INDICATES THAT THE INFORMATION SHOWN AND THE CALCULATIONS PERTAINING TO THAT INFORMATION HAVE BEEN PREPARED BY QUALIFIED PEOPLE UNDER THE DIRECTION OF THE ENGINEER-OF-RECORD. THE SEAL DOES NOT IMPLY RESPONSIBILITY FOR INFORMATION PREPARED BY OTHERS NOR FOR ANY INFORMATION NOT SHOWN ON THIS DRAWING AND SUCH RESPONSIBILITY IS SPECIFICALLY DISCLAIMED. ON PHASED PROJECTS, DRAWINGS THAT ARE ISSUED BUT NOT SEALED SHALL BE

THESE DRAWINGS ARE TO BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS ON THE PROJECT TO CLEARLY DEFINE ALL OF THE REQUIREMENTS FOR THE CONSTRUCTION. WHERE CONFLICTS OCCUR CONTACT ARCHITECT FOR CLARIFICATION.

CONSIDERED TO BE PRELIMINARY IN NATURE AND

ARE ISSUED FOR INFORMATION ONLY.



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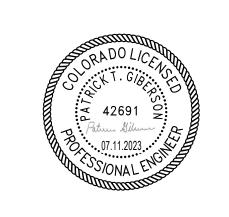
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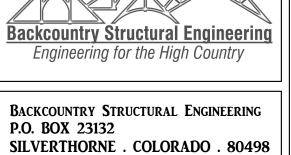
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**DATE** 

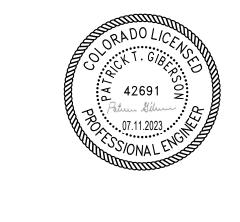
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CONTACT ARCHITECT FOR CLARIFICATION.



OBTUSE ANGLE CORNERS

SAWCUT OR TOOLED CONTROL JOINT, CAULK PER ARCH 

1. SAWCUT CONTROL JOINT WITHIN 8 HOURS OF PLACEMENT 2. PROVIDE CONTROL JOINTS AT 15 FT MAX OC EACH WAY UNO 3. RE: ARCH FOR CONTROL JOINT LAYOUT

DOWELS & CORNER BARS MATCH HORZ REINF EXPECT USE #5 DOWELS

TYPICAL IBC MINIMUM NAILING REQUIREMENTS

COMMON NAILS

3-8d

2-8d

2-8d

3-8d

2-16d

2-16d

4-8d OR

16d@24" OC

16d@16" OC

2-10d

3-8d

4-8d

3-16d

3-16d

2-8d

3-8d

3-8d

16d@24" OC

20d@32" OC

16d @ EA BEARING

2-20d

STANDARD 180° HOOK

8d@6" OC

16d@16" OC

16d@16" OC

CONNECTION

2. BRIDGING TO JOIST

7. TOP PLATE TO STUD

8. STUD TO SOLE PLATE

10. DOUBLE TOP PLATES

12. BLOCKING BETWEEN STUDS

15. CEILING JOIST TO TOP PLATE

16. CONTINUOUS HEADER TO STUD

7. CEILING JOISTS, LAPS OVER PARTITIONS

19. 1" DIAGONAL BRACE TO EACH STUD AND PLATE

21. WIDER THAN 1"x8" SHEATHING TO EACH BEARING

**INTERSECTIONS** 

CLASS B LAP SPLICE TYP

RIGHT ANGLE CORNERS

TYP WALL CORNER REINF - SNGL LINE

18. CEILING JOISTS TO PARALLEL RAFTERS

20. 1"x8" SHEATHING TO EACH BEARING

22. BUILT-UP CORNER STUDS

25. WOOD STRUCTURAL PANELS

 $4 \frac{\text{TYP MIN NAILING REQMNTS}}{3/4" = 1'-0"}$ 

24. 2" PLANKS

STANDARD HOOK -

23. BUILT-UP GIRDER AND BEAMS

14. CONTINUOUS HEADER, TWO PIECES

13. RIM JOIST TO TOP PLATE

9. DOUBLE STUDS

. JOIST OR RAFTERS AT ALL BEARINGS

3. 1"x6" SUBFLOOR OR LESS TO EA JOIST

5. 2" SUBFLOOR TO JOIST OR GIRDER

6. SOLE PLATE TO JOIST OR BLOCKING

4. WIDER THAN 1"x6" SUBFLOOR TO EA JOIST

11. BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE 3-8d

LOCATION

TOENAIL

FACE NAIL

FACE NAIL

FACE NAIL

END NAIL

END NAIL

FACE NAIL

FACE NAIL

TOE NAIL

TOE NAIL

TOE NAIL

TOE NAIL

TOE NAIL

FACE NAIL

FACE NAIL

FACE NAIL

FACE NAIL

FACE NAIL

FACE NAIL

AT EA SPLICE

FACE NAIL

END OF WALL

CLASS B LAP SPLICE TYP

FACE NAIL T&B STAGGER

OPPOSITE SIDES FACE NAIL AT ENDS AND

SEE GENERAL NOTES AND TYPICAL DETAILS

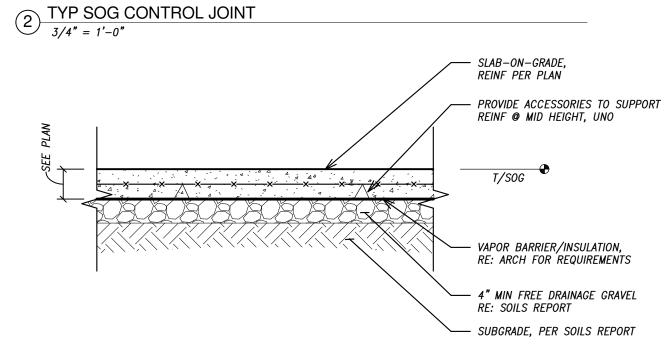
ALONG EA EDGE

8-16d EA SIDE OF SPLICE LAP SPLICE FACE NAIL

TOE NAIL EA END

BLIND AND FACE

1 TYP SOG 3/4" = 1'-0"



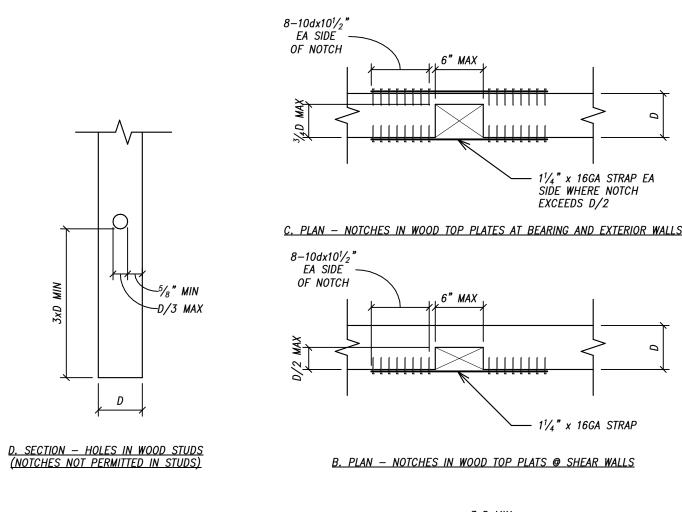
2 PIECES 1¾," LVL 3 PIECES 1¾," LVL OR OR 4 PIECES 13/4" LVL 2 PIECES  $1\frac{1}{2}$ " 2x 3 PIECES  $1\frac{1}{2}$ " 2x 4 PIECES 11/2" 2x ALL MULTIPLE WOOD MEMBERS MUST BE FASTENED TOGETHER TO ACT AS A SINGLE UNIT.

LAMINATIONS SHALL BE CONTINUOUSLY GLUED WITH EXTERIOR GLUE AND NAILED WITH TWO ROWS OF 16d @ 12" O.C. OR BOLTED WITH TWO ROWS OF  $\frac{1}{2}$ "  $\phi$  A307 @ 12" O.C. (SEE DETAIL ABOVE).

LAMINATIONS SHALL BE DRY (LESS THAN 16% MOISTURE CONTENT) WHEN GLUED. DO NOT SPLICE

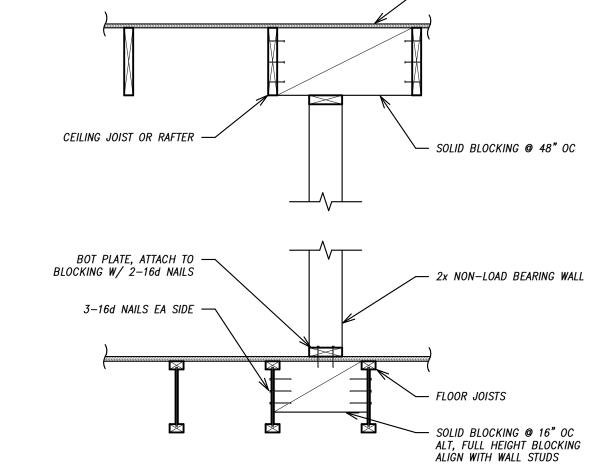
8 TYPICAL BUILT-UP BEAM

1 1/2" = 1'-0"



3xD MIN A. SECTION — NOTCHES AND HOLES IN WOOD JOISTS AND RAFTERS

1. CONTACT STRUCTURAL ENGINEER FOR HOLES/NOTCHES EXCEEDING ABOVE LIMITATIONS. 2. FOR ENGINEERED WOOD PRODUCTS, SEE MANUFACTURES RECOMMENDATIONS FOR NOTCHES AND HOLES. WOOD 05 - TYP REQMNTS FOR HOLES & 7 NOTCHES IN WOOD MEM. 1'' = 1'-0''



6 TYP NON-BEARING PARTITION 3/4" = 1'-0"

✓ BUILT-UP 2x COL — TYP (3) PART COL 

1 1/2" 1 1/2" 1 1/2" <u>(2) PART COL:</u> STAGGERED 10d <u>(3) PART COL:</u> STAGGERED 16d <u>(4+) PART COL:</u> 4TH PART – NAILS @ 4" OC ALT SIDES STAGGERED 16d NAILS @ 4" OC NAILS @ 4" OC ALT SIDES

 $5 \frac{\text{TYP}}{3/4" = 1'-0"}$ 

	SHEAR WALL LENGTH, SEE PLAN	
HOLD DOWN, SEE TYP —— WOOD SHEAR WALL		
BOUNDARY DTL		
	SIMPSON CLIP AT SHEAR	
	WALLS, SEE SCHED	
	T/DBL TOP R	
FLAT BLOCKING AT ALL —	DBL 2x TOP R, TYP	
HORZ, SHEATHING EDGES		
WINDOW OPENING —	SHEATHING, SEE SCHED	
SHEATHING EDGE —— NAILING, SEE SCHED		
	SHEATHING FIELD NAILING	
	PT SILL PLATE, SEE SCHED FOR SIZE	
	T/FDN WALL	
HOLD DOWN, SEE TYP —— WOOD SHEAR WALL	ANCHOR BOLT, SEE GENE NOTES FOR SIZE SPACING WASHER PLATE REQRMNTS	G AND
BOUNDARY DTL		

		SHE	AR WALL SCHED	ULE		
SHEAR WALL TYPE	SHEATHING	PANEL EDGE FRAMING	SILL PLATE	PANEL EDGE NAILING	PANEL FIELD NAILING	RIM JOIST OR BLOCKING CONN TO TOP PLATE
A 425 PLF	15/32" APA ONE-SIDED	SINGLE 2x	SINGLE PT 2x	10d @ 4" OC (0.148"x3")	10d @ 12" OC (0.148"x3")	SIMPSON A35 @ 16" OC
B 550 PLF	15/32" APA ONE-SIDED	DBL 2x OR SINGLE 3X	SINGLE PT 2x	10d @ 3" 0C (0.148"x3")	10d @ 12" OC (0.148"x3")	SIMPSON A35 @ 8" OC
C 1110 PLF	15/32" APA TWO-SIDED	DBL 2x OR SINGLE 3X	SINGLE PT 2x	10d @ 3" OC (0.148"x3")	10d @ 12" OC (0.148"x3")	SIMPSON A35 @ 8" OC, @ RIM SIMPSON LTP5 @ 12" OC, @ BLKNG

TYP WOOD SHEAR WALLS SCHED

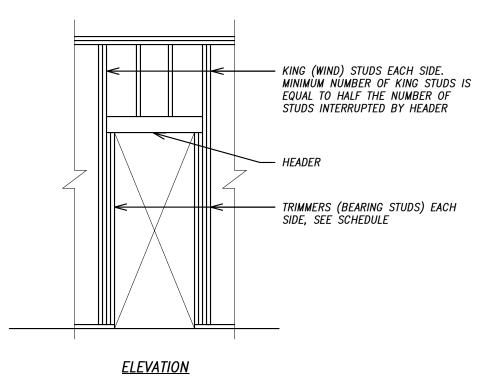
1/2" = 1'-0"

SPAN	DIMENSIONED LUMBER	I.V.L. ALTERNATES	NO. OF BEARING STUDS AT EACH
	HEMP-FIR		END
3'-0"	(2) 2x10 or (3) 2x8	(2) 1 <sup>3</sup> / <sub>4</sub> "x5 <sup>1</sup> / <sub>2</sub> " or (3)1 <sup>3</sup> / <sub>4</sub> "x5 <sup>1</sup> / <sub>2</sub> "	2
4'-0"	(2) 2x12 or (3) 2x12	(2) $1\frac{3}{4}$ "x7 $\frac{1}{4}$ " or (3) $1\frac{3}{4}$ "x5 $\frac{1}{2}$ "	2
5'-0 <b>"</b>	N/A (3) 2x12	(2) 1 <sup>3</sup> / <sub>4</sub> "x7 <sup>1</sup> / <sub>4</sub> " or (3) 1 <sup>3</sup> / <sub>4</sub> "x7 <sup>1</sup> / <sub>4</sub> "	2
6'-0"	N/A N/A	(2) $1\frac{3}{4}$ "x9 $\frac{1}{2}$ " or (3) $1\frac{3}{4}$ "x7 $\frac{1}{4}$ "	2
7'-0"	N/A N/A	(2) $1\frac{3}{4}$ "x11 $\frac{7}{8}$ " or (3) $1\frac{3}{4}$ "x9 $\frac{1}{2}$ "	3
8'-0"	N/A N/A	(2) $1\frac{3}{4}$ "x11 $\frac{7}{8}$ " or (3) $1\frac{3}{4}$ "x9 $\frac{1}{2}$ "	3
9'-0"	N/A N/A	(2) $1\frac{3}{4}$ " x 14" or (3) $1\frac{3}{4}$ "x11 $\frac{7}{8}$ "	3
10-0"	N/A N/A	(2) $1\frac{3}{4}$ "x16" or (3) $1\frac{3}{4}$ "x11 $\frac{7}{8}$ "	3

	-	•				
HEADERS IN	I LOAD	<b>BEARING</b>	AND	<b>EXTERIOR</b>	WALLS	<u>S</u>

SPAN	DIMENSIONED LUMBER	IVI ALTERNATES	NO. OF BEARING STUDS AT EACH
	HEMP-FIR		END
3'-0"	(2) 2x4 or (3) 2x4	(2)1-3/4"x5-1/2"or(3)1-3/4"x5-1/2"	1
4'-0"	(2) 2x6 or (3) 2x4	(2)1-3/4"x5-1/2"or(3)1-3/4"x5-1/2"	1
5'-0"	(2) 2x6 or (3) 2x6	(2)1-3/4"x5-1/2"or(3)1-3/4"x5-1/2"	1
6'-0"	(2) 2x8 or (3) 2x6	(2)1-3/4"x5-1/2"or(3)1-3/4"x5-1/2"	1
7'-0"	(2) 2x10 or (3) 2x8	(2)1-3/4"x7-1/4"or(3)1-3/4"x5-1/2"	1
8'-0"	(2) 2x10 or (3) 2x8	(2)1-3/4"x7-1/4"or(3)1-3/4"x7-1/4"	1
9'-0"	(2) 2x12 or (3) 2x10	(2)1-3/4"x7-1/4"or(3)1-3/4"x7-1/4"	1
10-0"	N/A (3) 2x12	(2)1-3/4"x9-1/4"or(3)1-3/4"x7-1/4"	1

RECOMMENDED HEADERS IN NON-LOAD BEARING WALLS



NOTES:
1. THIS SCHEDULE APPLIES TO HEADERS WHICH ARE NOT EXPLICITLY CALLED OUT ON PLAN WITH SPANS OF 10'-0" OR LESS

2. HEADERS IN LOAD BEARING WALLS DESIGNED FOR 1500 PLF DEAD + LIVE LOAD.

3. HEADERS IN NON-LOAD BEARING WALLS DESIGNED FOR 400 PLF DEAD + LIVE LOAD.

4. DIMENSIONED LUMBERS TO BE No. 2 HEMP-FIR. 5. LVL = LAMINATED VENEER LUMBER 5. DEFLECTION CRITERIA IS L/360 7. HEADERS SUPPORTING POINT LOADS FROM BEAMS OR COLUMNS SHOULD NOT BE SIZED FROM THIS TABLE. NOTIFY STRUCTURAL ENGINEER.

 $\frac{5}{8}$ " $\phi$  EPOXIED ANCHOR BOLT  $\frac{7}{8}$  % EMBEDDED ANCHOR BOLT  $\frac{7}{8}$ " $\phi$  EPOXIED ANCHOR BOLT 1/4"x2"x2" P2 EPOXIED ANCHOR BOLTS SHALL BE SECURED WITH HILTI HY150 INJECTION ADHESIVE OR EQUIVALENT CONDITION AT FOUNDATION -JOISTS BEAR ON TOP OF WALL

TYP WOOD WALLS- BOUNDARY DTL

3/4" = 1'-0"

STEEL BEAM

3/16" STFNR № EA SIDE

(2)  $\frac{1}{2}$   $\mathring{\phi}$  A307 BOLTS, — COUNTER SINK AS REQD

BUILT-UP 2x COL

ROOF SHEATHING -EDGE NAILING

EDGE NAILING -

FLOOR SHEATHING EDGE NAILING

EDGE NAILING EA — SIDE OF JOINT

SPLICE SHEATHING @ -TOP OF TOP PLATE

EDGE NAILING ES OF —

JOINT, STAGGER

SHEATHING JOINT

EDGE NAILING AT — PT SILL PLATE

SEE GEN NOTES -

TYPE A AND B SHEAR WALLS

SHOP WELD THREADED STUD
TO MATCH HOLD-DOWN WITH

A.B. REQUIREMENTS

CONDITION AT INTERMEDIATE

HOLD DOWN SCHEDULE

SCREWS

6-SDS 1/4"x21/2"

20-SDS 1/4"x21/2"

BNDRY

EMBEDMENT

12"

12"

14"

STUDS

SIMPSON HOLD-

DOWN

HDU2

HDU8

ANCHOR BOLT EMBEDMENT

FLOOR - SHEAR WALL ABOVE STEEL BEAM

FOR ANCHOR BOLT

NOTE: SOME INFORMATION NOT SHOWN FOR CLARITY

SIMPSON CLIP

NAILING, SEE TYP

SIMPSON CLIP

BLOCKING

HOLD DOWN PER

--- BOUNDARY STUD

HOLD-DOWN SCHEDULE

1/4"x3"x71/2" KNIFE 12 -ROTATE KNIFE 12 W/ COL PLIES

TYP STEEL BEAM TO WOOD COL CONN

1" = 1'-0"

- DBL 2x STUD STITCH NAILED (OR

STAGGERED

SIDE OF JOINT,

SIDE OF JOINT,

STAGGERED

TYP WOOD SHEAR WALL DTLS

3/4" = 1'-0"

- BOUNDARY STUD

— HOLD DOWN PER HOLD-DOWN SCHEDULE

— 1/4"x3"x3" 12 WASHER

— COUNTERSINK NUT & WASHER AT BOTTOM

SHEAR

WALL TYPE

**ANCHOR** 

DIAM.

ANCHOR BOLT TYPE AND

DIAMETER

 $\frac{5}{8}$ " $\phi$  EMBEDDED ANCHOR BOLT

OF LVL AS REQD

STAGGERED

TYPE B SHEAR WALLS

TYPE A SHEAR WALLS

SECTION A-A

- 4|- -- -- -- --. || • • •

CONDITION AT INTERMEDIATE

FLOOR - SHEAR WALL ABOVE

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OPTIONAL THREADED -

COUPLER

<u>LVL BEAM</u>

— BOUNDARY STUD

- HOLD DOWN PER

HOLD-DOWN SCHEDULE

(2) ½"φ A307 BOLTS,

NOTE: SOME INFORMATION NOT SHOWN FOR CLARITY

COUNTĒR SINK AS REQD

1/4"x3"x71/2" KNIFE 1? -ROTATE KNIFE 1? W/ COL PLIES

9 TYP WOOD HEADER SCHED

31

### <u>PLAN LEGEND</u>

	CONCRETE CONSTRUCTION	Г
3 4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	CIP CONCRETE WALL ABOVE	
4 - 4 4	CIP CONCRETE WALL ABOVE W/ WINDOW	
4,4 A	CIP CONCRETE WALL ABOVE W/ DOOR	
	CIP/PC WALL BELOW	
	PC CONCRETE COLUMN ABOVE	
	CIP/PC CONCRETE COLUMN BELOW	

	PLAN KEYS
<b>97'−0"</b>	FOOTING NOTATION  F6 = FTG TYPE PER SCHEDULE  97'-0" = T/ FOOTING ELEVATION
s ——— s	FOOTING / FOUNDATION STEP
T=+XXXK	TRANSFER LOAD (K)
	BEAM POCKET
H1	H1 = JOIST HANGER TYPE SEE JOIST HANGER SCHEDULE

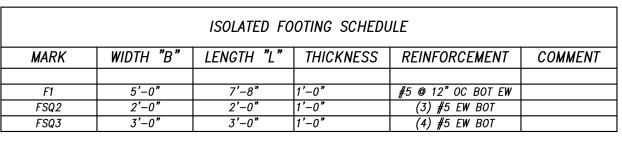
	STEEL CONSTRUC
_	- BEAM / GIRDER
$\circ \Box \mathbf{I}$	COLUMN ABOVE
	COLUMN BELOW

	FRAME WALL ABOVE
	FRAME WALL ABOVE W/ WINDOW
	FRAME WALL ABOVE W/ DOOR
	FRAME WALL BELOW
	TRUSS
	BEAM / GIRDER
	JOIST
MM 🖂 🗏	COLUMN ABOVE
	COLUMN BELOW

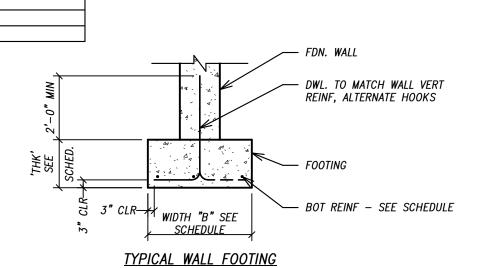
12'-0"

24'-0"

	SEE JOIST HANGER SCHEDULE
	LATERAL ELEME
	LATERAL ELEMENT ABOVE
	LATERAL ELEMENT BELOW
	LATERAL ELEMENT ABOVE & BELOW
A X'-YY"	WOOD SHEAR WALL DESIGNATION AND MINIMUM REQUIRED LENGTH SEE SHEAR WALL SCHEDULE
HD	HOLD DOWN FOR WOOD SHEAR WALL SEE HOLD DOWN SCHEDULE
	A X'-YY"



	CONTINUC	OUS FOOTING S	SCHEDULE	
MARK	WIDTH "B"	THICKNESS	REINFORCEM ENT	COMMENT
FC16	1'-4"	10"	(2) #5 CONT.	
FC48	4'-0"	10"	#5 @ 12" OC TOP EW	



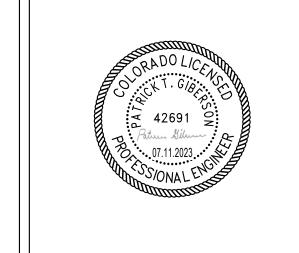
FOOTING NOTES:

1. FOOTINGS SHALL BEAR ON PROOF ROLLED NATIVE SOIL OR COMPACTED FILL AS SPECIFIED IN THE SOILS REPORT.
2. ALL BEARING MATERIAL SHALL BE INSPECTED BY THE GEOTECHNICAL ENGINEER PRIOR TO CONCRETE PLACEMENT. THE GEOTECHNICAL ENGINEER SHALL BE THE SOLE JUDGE AS TO THE SUITABILITY OF THE BEARING MATERIAL.
3. FOOTINGS DESIGNED FOR ALLOWABLE BEARING PRESSURE OF 4000 PSF.
4. CENTER CONTINUOUS FOOTING UNDER WALLS U.N.O. COLUMN FOOTINGS ARE CENTERED UNDER COLUMNS, U.N.O.
5. BEARING ELEVATIONS ARE SUBJECT TO ADJUSTMENT AS REQUIRED BY SUITABILITY OF BEARING MATERIAL.
6. DOWELS TO MATCH VERTICAL WALL AND PILASTER REINF. U.N.O. EXTEND DWLS 24" MIN. ABOVE FTG. U.N.O.
7. SEE COLUMN SCHEDULE AND WALL DETAILS FOR FOOTING DOWELS.
8. SEE GENERAL NOTES FOR ADDITIONAL INFORMATION.

POOTING SCHEDULE

1/2" = 1'-0"

12'-0"



Engineering for the High Country

BACKCOUNTRY STRUCTURAL ENGINEERING

SILVERTHORNE . COLORADO . 80498

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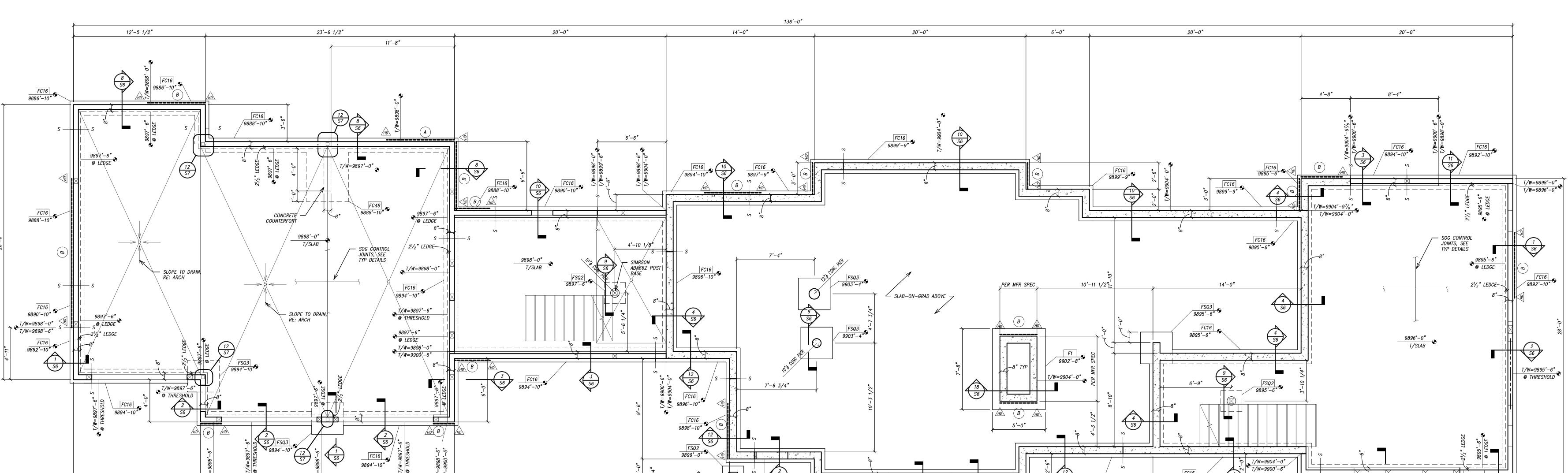
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CONSTRUCTION. WHERE CONFLICTS OCCUR
CONTACT ARCHITECT FOR CLARIFICATION.

P.O. BOX 23132

PH: 970.333.1511





1. TYPICAL FOUNDATION IS CONCRETE WALLS ON SPREAD FOOTINGS.
2. LOWER LEVEL FLOOR IS 4" SOG OVER 4" FREE DRAINAGE GRANULAR BASE MATERIAL. REINF WITH 6x6xW1.4xW1.4 WWF CHAIRED AT MID HEIGHT OF SLAB OR #3 @ 16" OC EA WAY CHAIRED AT MID HEIGHT OF SLAB FOR INCREASED CRACK CONTROL.
3. TYPICAL WOOD STUD WALL IS 2x6 HF STUDS @ 16" OC
4. TYPICAL BUILT—UP COLUMN IS (3) PART 2x6 HF N₀. 2, SHOWN AS THUS ☑ UNO.
5. SEE PLANS FOR FOOTING, WALL, AND SOG ELEVATIONS. GC TO VERIFY FOOTING AND WALL ELEVATIONS WITH GRADE REQUIREMENTS. CONTACT THE STRUCTURAL ENGINEER WITH ANY ELEVATION CHANGES PRIOR TO CONSTRUCTION.
6. SEE S1 FOR GENERAL NOTES AND LEGEND
7. SEE S2 FOR TYPICAL DETAILS

8. SEE PLAN FOR FOUNDATION SCHEDULE

# LASSA RESIDENCE

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TOWN OF THE COMPANY O

DATE
6 JUN 2023
11 JUL 2023

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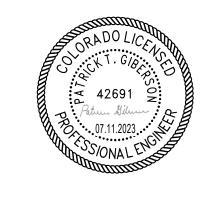
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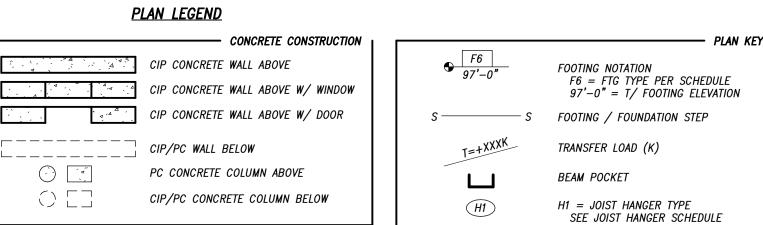
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<b>ISSUE</b>	DATE
STRUC REVIEW	6 JUN 2023
PERMIT	11 JUL 2023

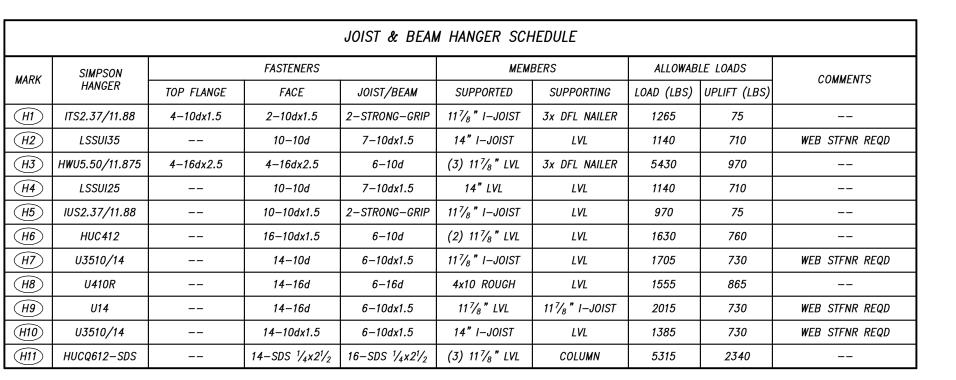
PROJECT # A0727



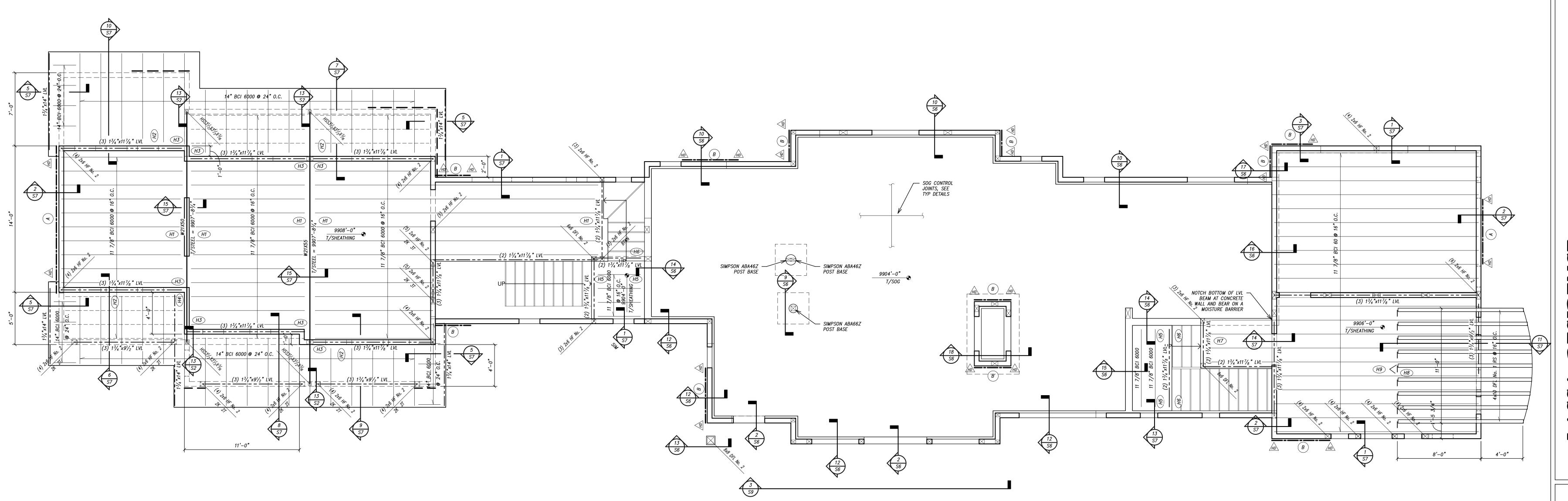
		l L	
	STEEL CONSTRUCTION	- 	
	BEAM / GIRDER		
$\circ \Box \mathbf{I}$	COLUMN ABOVE		_
	COLUMN BELOW		_

	LATERAL ELEMEN
	LATERAL ELEMENT ABOVE
	LATERAL ELEMENT BELOW
	LATERAL ELEMENT ABOVE & BELOW
A X'-YY"	WOOD SHEAR WALL DESIGNATION AND MINIMUM REQUIRED LENGTH SEE SHEAR WALL SCHEDULE
HD	HOLD DOWN FOR WOOD SHEAR WALL SEE HOLD DOWN SCHEDULE

		LATERAL ELEMENT BELOW
		LATERAL ELEMENT ABOVE & BELOW
) 	A X'-YY"	WOOD SHEAR WALL DESIGNATION AND MINIMUM REQUIRED LENGTH SEE SHEAR WALL SCHEDULE
	HD	HOLD DOWN FOR WOOD SHEAR WALL SEE HOLD DOWN SCHEDULE



2 HANGER SCHEDULE
12" = 1'-0"



MAIN LEVEL

1/4" = 1'-0" MAIN LEVEL FRAMING PLAN NOTES: 1. TYPICAL FLOOR FRAMING IS 1½" GYPCRETE TOPPING OVER ¾" SHEATHING SUPPORTED BY WOOD I—JOIST FRAMING, UNO
2. GREAT ROOM FLOOR IS 4" SOG OVER RIGID INSULATION AND 4" FREE DRAINAGE GRANULAR BASE MATERIAL. REINF WITH 6x6xW1.4xW1.4 WWF CHAIRED AT MID HEIGHT OF SLAB OR #3 © 16" OC EA WAY CHAIRED AT MID HEIGHT OF SLAB FOR INCREASED CRACK CONTROL.
3. SEE PLAN FOR T/SHEATHING AND T/SOG ELEVATIONS 4. TYPICAL WOOD STUD WALL IS 2x6 HF STUDS @ 16" OC
5. TYPICAL BUILT-UP COLUMN IS (3) PART 2x6 HF No. 2, SHOWN AS THUS ☑ UNO.
6. SEE THE BCI BUILDERS GUIDE FOR ALLOWABLE JOIST AND LVL BEAM PENETRATIONS. FOR ALL OTHER BEAM PENETRATIONS NOT SHOWN ON PLAN CONTACT THE STRUCTURAL ENGINEER

FOR GUIDANCE.
7. SEE S1 FOR GENERAL NOTES AND LEGEND
8. SEE S2 FOR TYPICAL DETAILS AND HEADER SCHEDULE
9. SEE PLAN FOR HANGER SCHEDULE

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P.O. BOX 23132

PH: 970.333.1511

### PLAN LEGEND

-	3 III EEVELIUS	
	CONCRETE CONSTRUCTION	Γ
* A * A * A * A * A * A * A * A * A * A	CIP CONCRETE WALL ABOVE	١
4 , 4 , 4	CIP CONCRETE WALL ABOVE W/ WINDOW	١
7,44	CIP CONCRETE WALL ABOVE W/ DOOR	
	CIP/PC WALL BELOW	
() (*)	PC CONCRETE COLUMN ABOVE	١
$\circ$ $\Box$	CIP/PC CONCRETE COLUMN BELOW	

	PLAN KEYS
<b>●</b>	FOOTING NOTATION F6 = FTG TYPE PER SCHEDULE 97'-0" = T/ FOOTING ELEVATION
ss	FOOTING / FOUNDATION STEP
T=+XXXK	TRANSFER LOAD (K)
ُ ا	BEAM POCKET
(H1)	H1 = JOIST HANGER TYPE SEE JOIST HANGER SCHEDULE

	STEEL CONSTRUCTION
BEAM / GIRDER	
COLUMN ABOVE	
COLUMN BELOW	
	COLUMN ABOVE

	LATERAL ELEMEN
	LATERAL ELEMENT ABOVE
	LATERAL ELEMENT BELOW
	LATERAL ELEMENT ABOVE & BELOW
A X'-YY"	WOOD SHEAR WALL DESIGNATION AND MINIMUM REQUIRED LENGTH SEE SHEAR WALL SCHEDULE
HD	HOLD DOWN FOR WOOD SHEAR WALL SEE HOLD DOWN SCHEDULE

FRAME WALL ABOVE
FRAME WALL ABOVE W/ WINDOW
FRAME WALL ABOVE W/ DOOR
FRAME WALL BELOW
 TRUSS
 BEAM / GIRDER
 JOIST
COLUMN ABOVE

SIMPSON			SIMPSON		FASTENERS		MEM	BERS	ALLOWAB	LE LOADS	0014451170
MARK	RK HANGER	TOP FLANGE	FACE	JOIST/BEAM	SUPPORTED	SUPPORTING	LOAD (LBS)	UPLIFT (LBS)	COMMENTS		
H1)	ITS2.37/11.88	4-10dx1.5	2-10dx1.5	2-STRONG-GRIP	11 7/8 " I-JOIST	3x DFL NAILER	1265	75			
(H2)	LSSUI35		10-10d	7–10dx1.5	14" I–J0IST	LVL	1140	710	WEB STFNR REQL		
(H3)	HWU5.50/11.875	4-16dx2.5	4-16dx2.5	6-10d	(3) 11 <sup>7</sup> / <sub>8</sub> " LVL	3x DFL NAILER	5430	970			
<u>H4</u>	LSSUI25		10-10d	7–10dx1.5	14" LVL	LVL	1140	710			
(H5)	IUS2.37/11.88		10-10dx1.5	2-STRONG-GRIP	11 ½ " I–JOIST	LVL	970	75			
(H6)	HUC412		16-10dx1.5	6-10d	(2) 11 <sup>7</sup> / <sub>8</sub> " LVL	LVL	1630	760			
(H7)	U3510/14		14-10d	6-10dx1.5	11 <sup>7</sup> / <sub>8</sub> " I–JOIST	LVL	1705	730	WEB STFNR REQL		
(H8)	U410R		14-16d	6-16d	4x10 ROUGH	LVL	1555	865			
<u>H9</u>	U14		14-16d	6-10dx1.5	11 ½ " LVL	11 <sup>7</sup> / <sub>8</sub> " I–JOIST	2015	730	WEB STFNR REQL		
(H10)	U3510/14		14-10dx1.5	6-10dx1.5	14" I–J0IST	LVL	1385	730	WEB STFNR REQL		
(H11)	HUCQ612-SDS		14-SDS 1/4×21/2	16-SDS 1/4x21/2	(3) 11 <sup>7</sup> / <sub>8</sub> " LVL	COLUMN	5315	2340			

<u>FRAMING</u>	<u>MAX SPAN</u>	
<u>MEMBER</u>	SPACING = 16" O.C.	SPACING = 24" O.C.
2x4 HF-No.2	4'-0"	3'-0"
2x6 HF-No.2	6'-0"	5'-0"
2x8 HF-No.2	8'-0"	6'-6"
2x10 HF-No.2	10'-0"	8'-0"
2x12 HF-No.2	12'-0"	9'-0"
NOTEC		

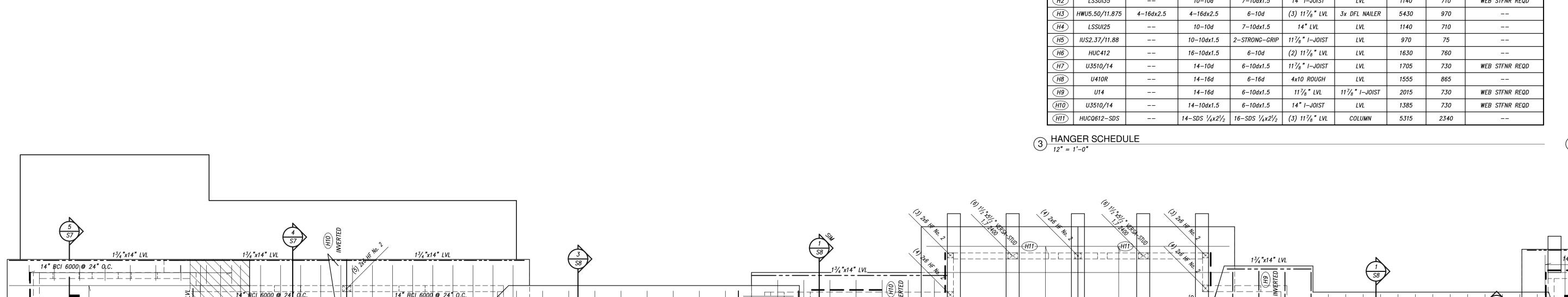
NOTES:

1. OVER FRAMING IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR.

2. THE SCHEDULE ABOVE IS INTENDED TO AID THE GENERAL CONTRACTOR IN SELECTION OF FRAMING MEMBERS FOR VARIOUS SIZE AND SPACING. OVER FRAME SPANS SHOULD BE LIMITED TO THOSE ABOVE.

3. FRAMING MEMBERS DESIGNED FOR 15 PSF DL + 1.43\*Pf (FLAT ROOF SNOW). OVER FRAMING USUALLY OCCURS AT INTERSECTIONS OF ROOF PLANES. THE 43% INCREASE ACCOUNTS FOR DRIFTING SNOW USUALLY SEEN IN THESE ARFAS. SEEN IN THESE AREAS. 4. DEFLECTION CRITERIA USED = L/180.

2 TYP OVER FRAMING SCHED



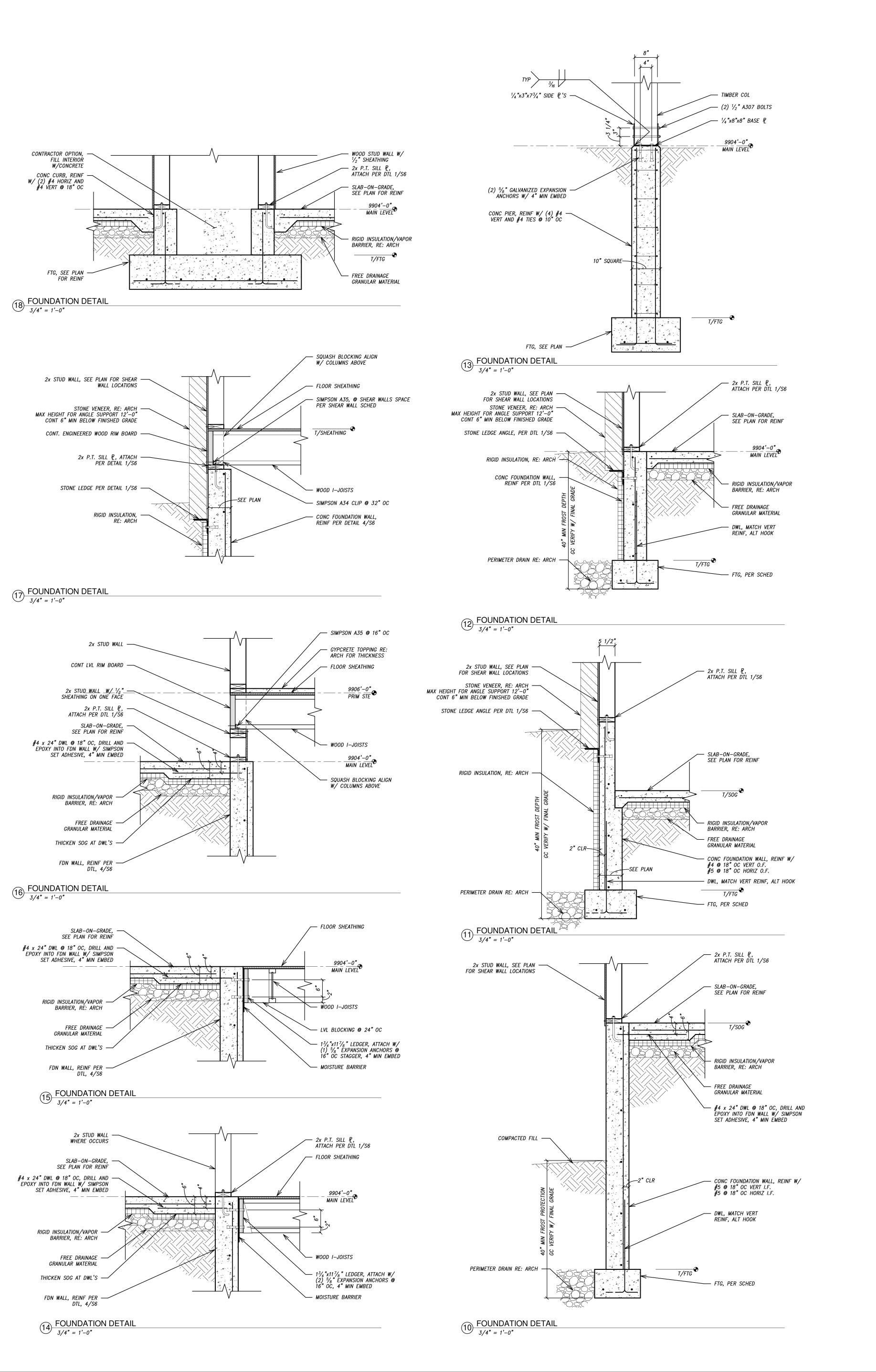
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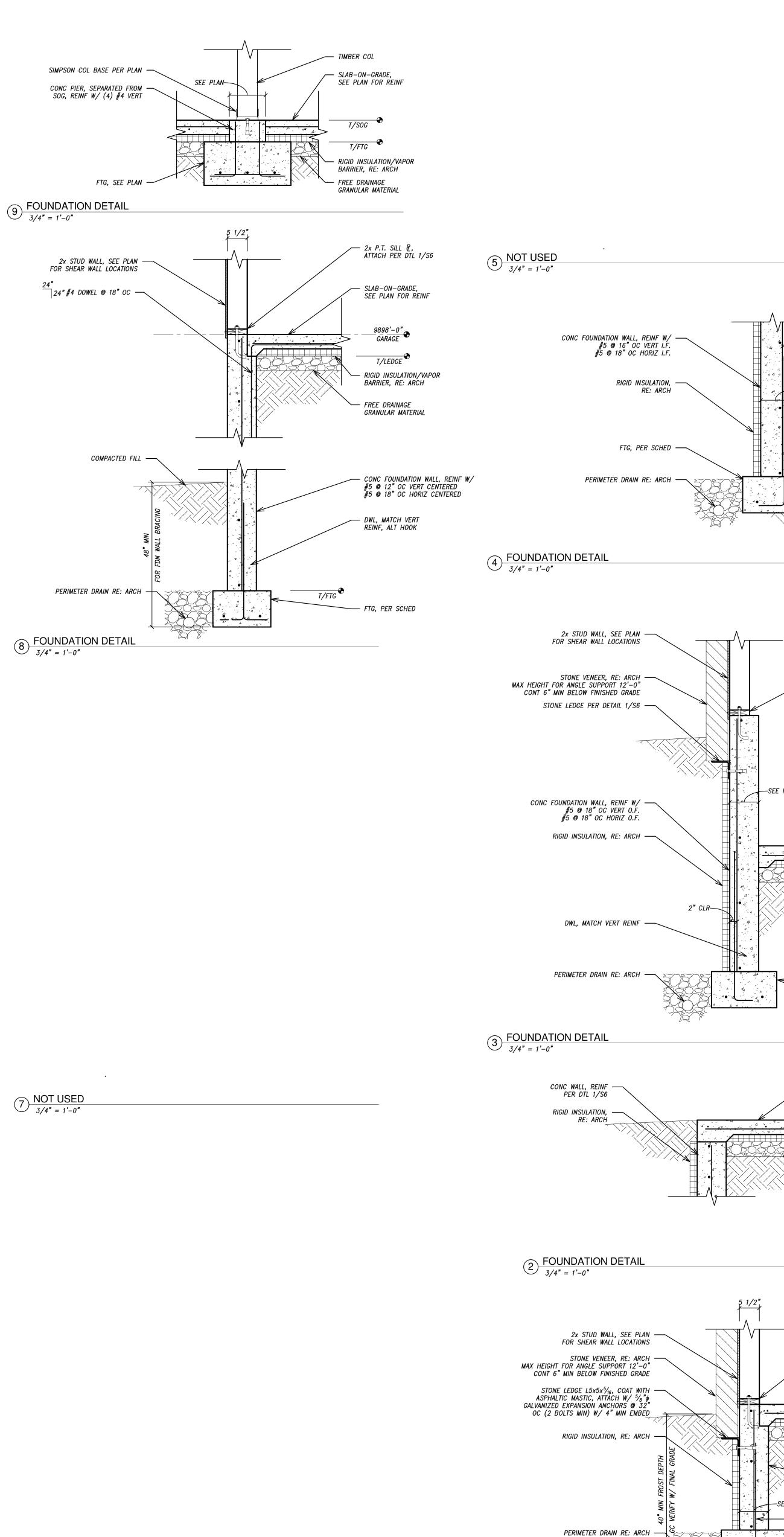
 $1 \frac{\text{ROOF FRAMING PLAN}}{1/4" = 1'-0"}$ 

ROOF FRAMING PLAN NOTES: 1. TYPICAL ROOF FRAMING IS 5/8" SHEATHING SUPPORTED BY WOOD I—JOISTS, AND TIMBER BEAMS, UNO
2. SEE ARCHITECTURAL DRAWINGS FOR SLOPES, TOP OF PLATE ELEVATIONS, AND RIDGE ELEVATIONS.
3. SEE THE BCI BUILDERS GUIDE FOR ALLOWABLE JOIST AND LVL BEAM PENETRATIONS. FOR 3. SEE THE BCT BUILDERS GUIDE FOR ALLOWABLE JOIST AND LVL BEAM PENETRATIONS. FOR ALL OTHER BEAM PENETRATIONS NOT SHOWN ON PLAN CONTACT THE STRUCTURAL ENGINEER FOR GUIDANCE.
4. SEE S1 FOR GENERAL NOTES AND LEGEND
5. SEE S2 FOR TYPICAL DETAILS AND HEADER SCHEDULE
6. SEE PLAN FOR HANGER SCHEDULE
7. SEE PLAN FOR OVER FRAMING SCHEDULE. OVER FRAMING DENOTED BY HATCHED REGIONS.

**DATE** STRUC REVIEW 6 JUN 2023 11 JUL 2023

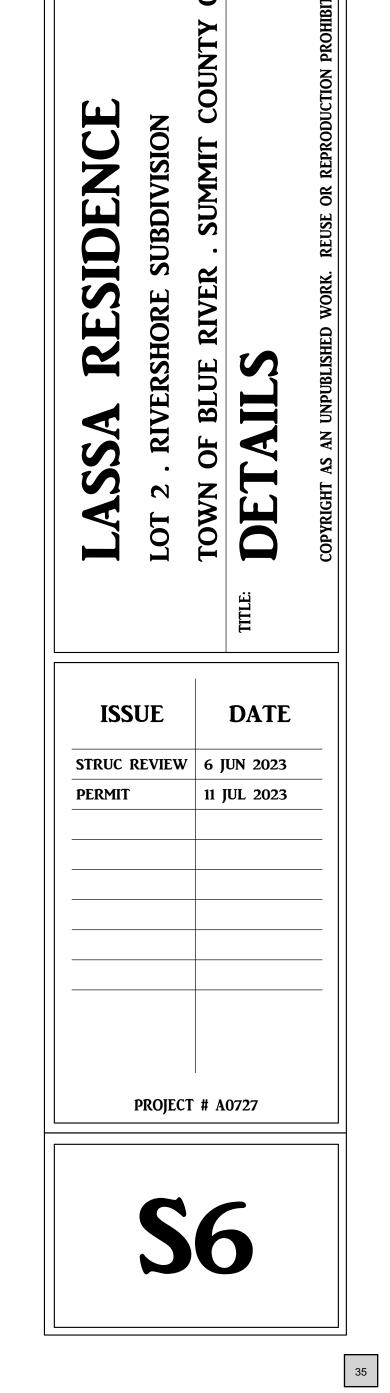
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1) FOUNDATION DETAIL 3/4" = 1'-0"

 $6 \frac{\text{NOT USED}}{3/4" = 1'-0"}$ 



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ALL OF THE REQUIREMENTS FOR THE CONSTRUCTION. WHERE CONFLICTS OCCUR CONTACT ARCHITECT FOR CLARIFICATION.

- DWL, MATCH VERT

REINF, ALT HOOK

· SLAB-ON-GRADE,

SEE PLAN FOR REINF

RIGID INSULATION/VAPOR

BARRIER, RE: ARCH

GRANULAR MATERIAL

FREE DRAINAGE

2x P.T. SILL P., ATTACH PER DTL 1/S6

SLAB-ON-GRADE,

SEE PLAN FOR REINF

RIGID INSULATION/VAPOR

BARRIER, RE: ARCH

GRANULAR MATERIAL

- FREE DRAINAGE

T/FTG ◆

— SLAB-ON-GRADE, SEE PLAN FOR REINF

RIGID INSULATION/VAPOR BARRIER, RE: ARCH

— 2x P.T. SILL P, ATTACH W/ 1/2" ANCHOR BOLTS @ 16" OC AT SHEAR WALLS AND 32" OC

ELSEWHERE, 7" MIN EMBED

SLAB-ON-GRADE,

SEE PLAN FOR REINF

- RIGID INSULATION/VAPOR BARRIER, RE: ARCH

— CONC FOUNDATION WALL, REINF W/ #4 @ 18" OC VERT CENTERED #5 @ 18" OC HORIZ CENTERED

- DWL, MATCH VERT REINF, ALT HOOK

- FREE DRAINAGE GRANULAR MATERIAL

T/FTG ◆

— FTG, PER SCHED

AT THRESHOLD

FREE DRAINAGE

GRANULAR MATERIAL

--- FTG, PER SCHED

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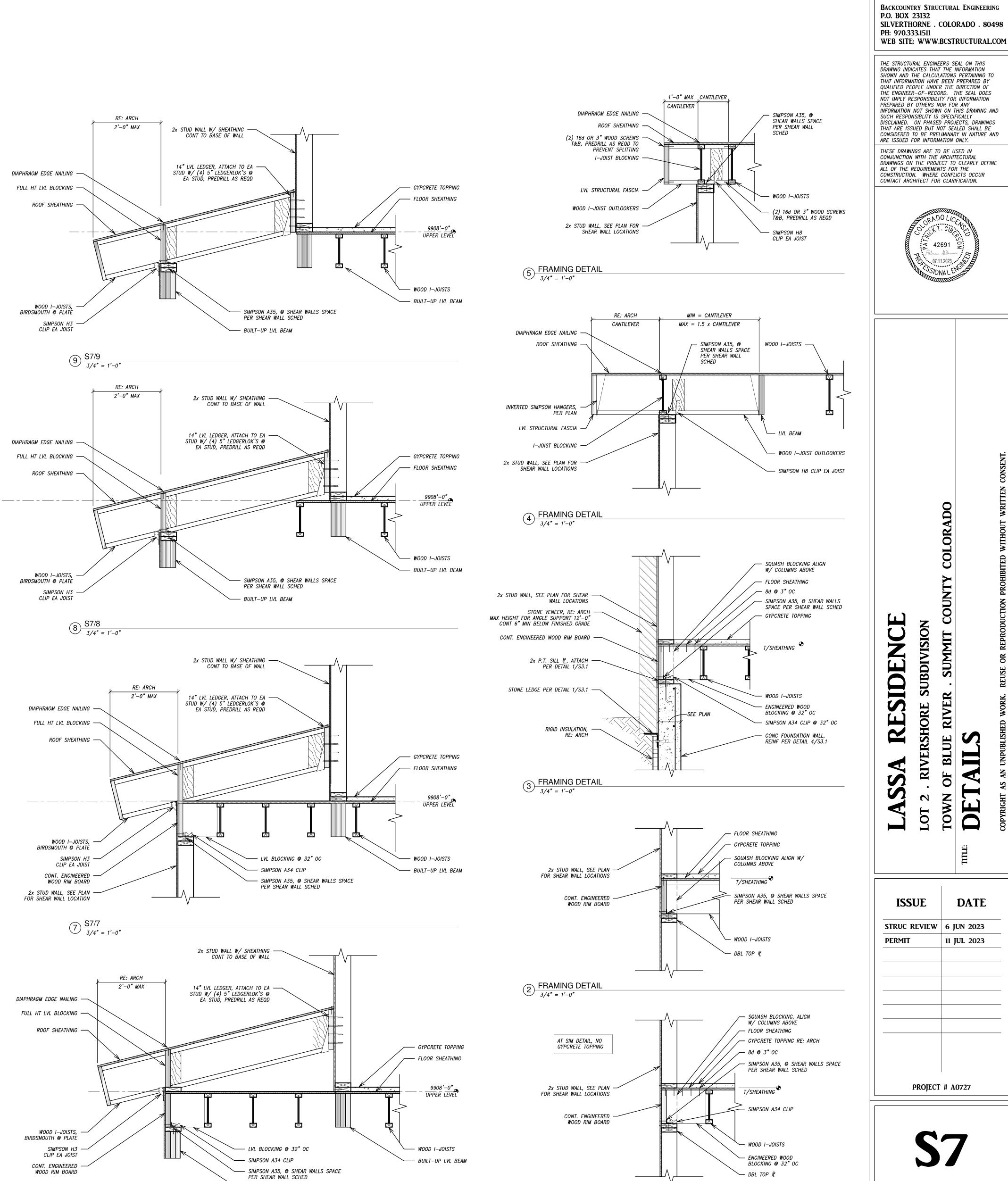
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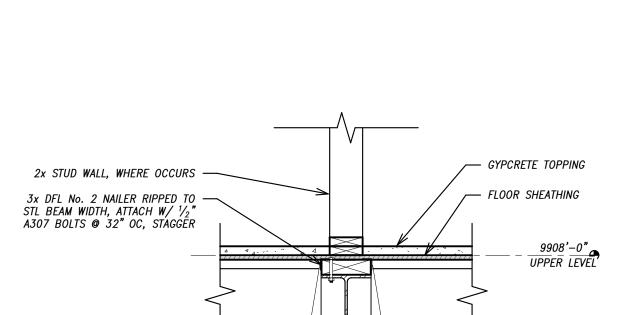
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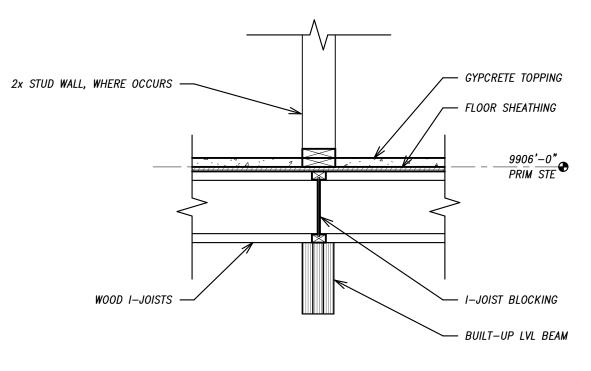
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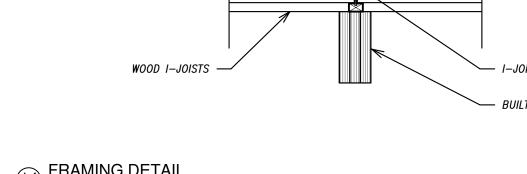
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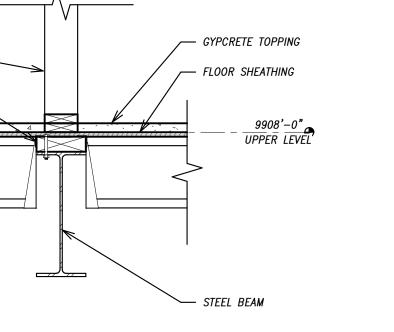


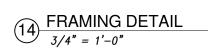


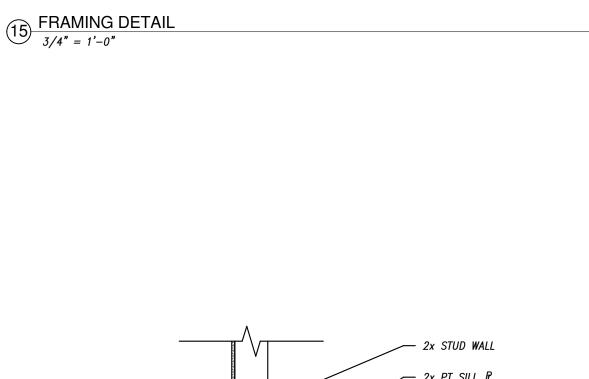
WOOD I-JOISTS -

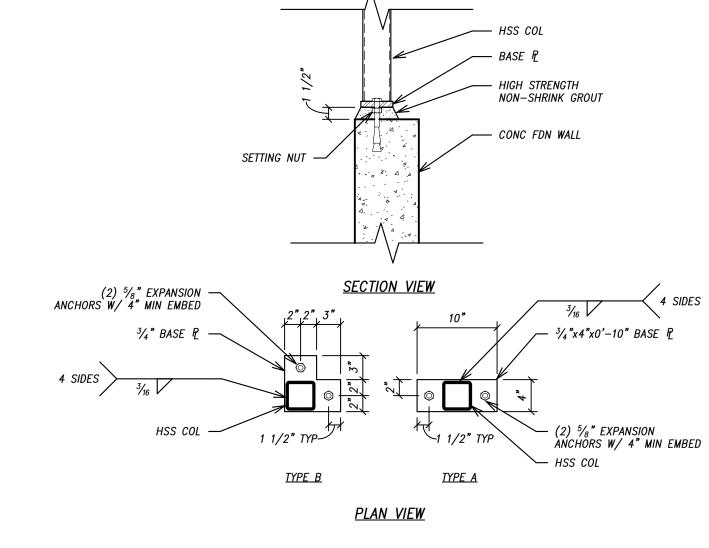


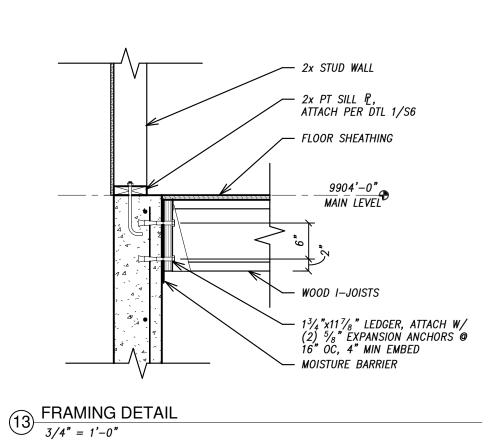


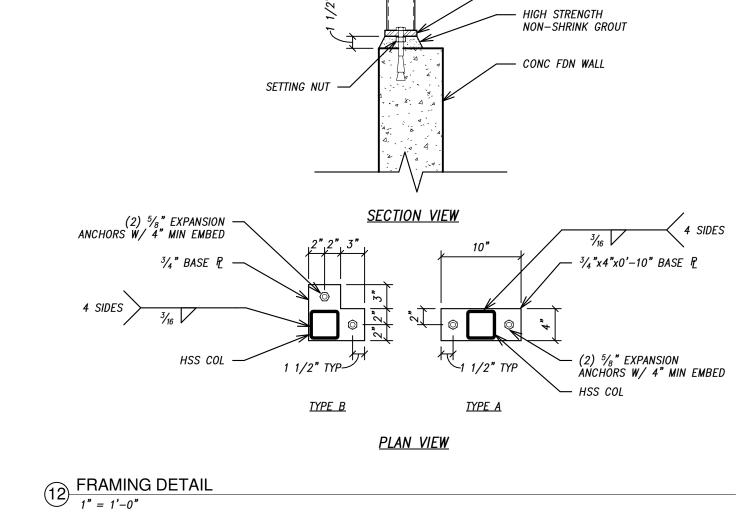








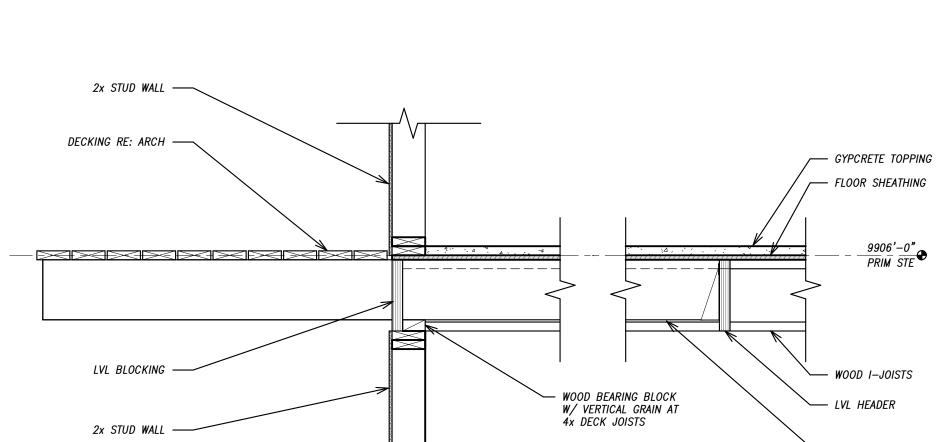


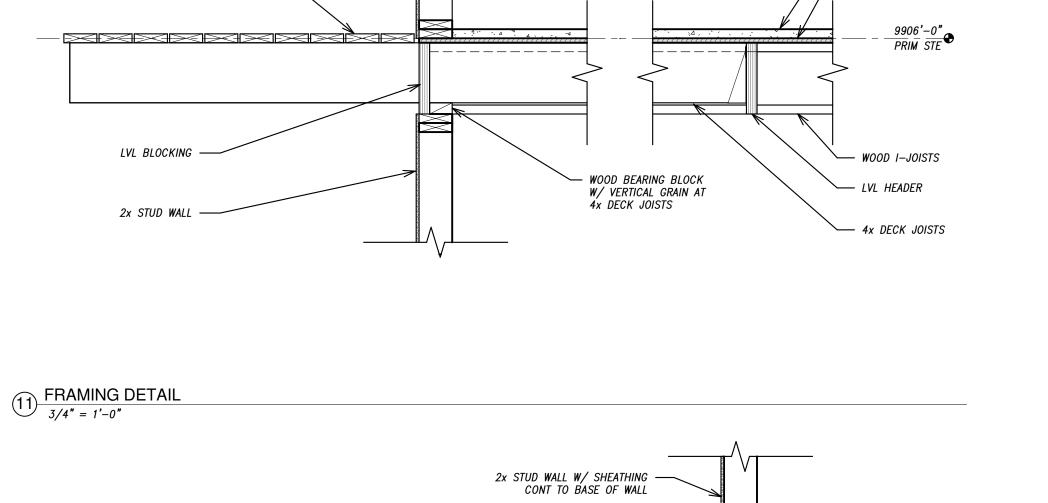


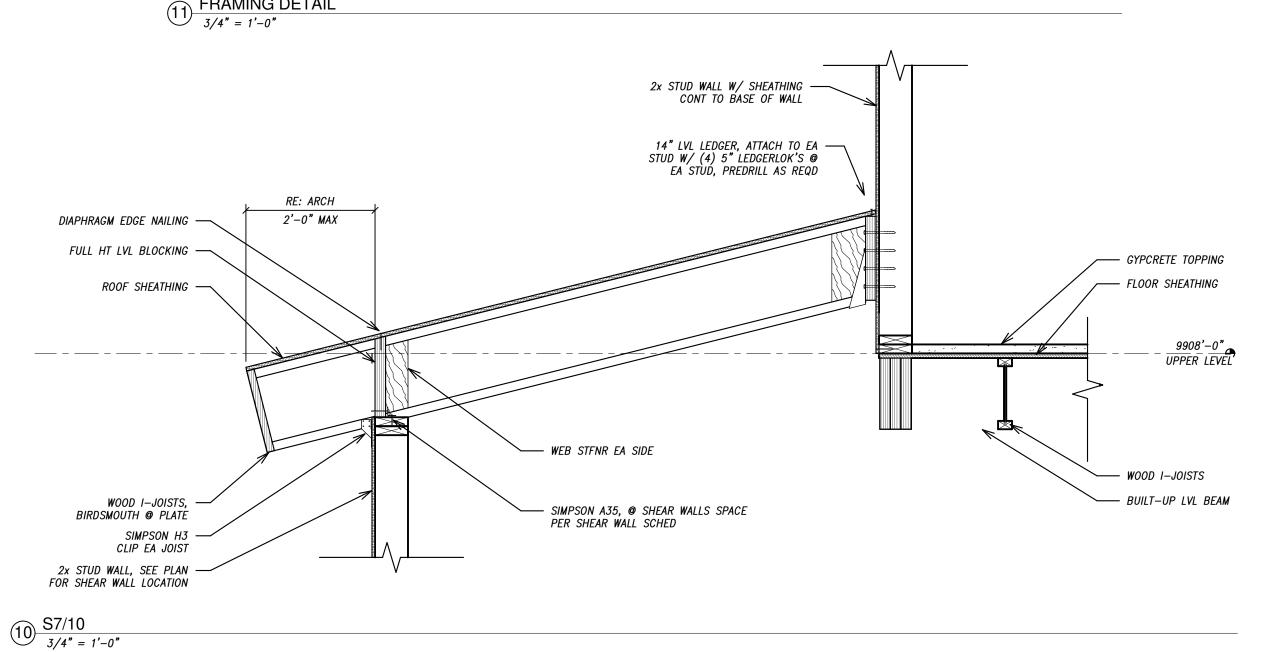
**WOOD RIM BOARD** 

 $6 \frac{57/6}{3/4" = 1'-0"}$ 

── BUILT-UP LVL BEAM







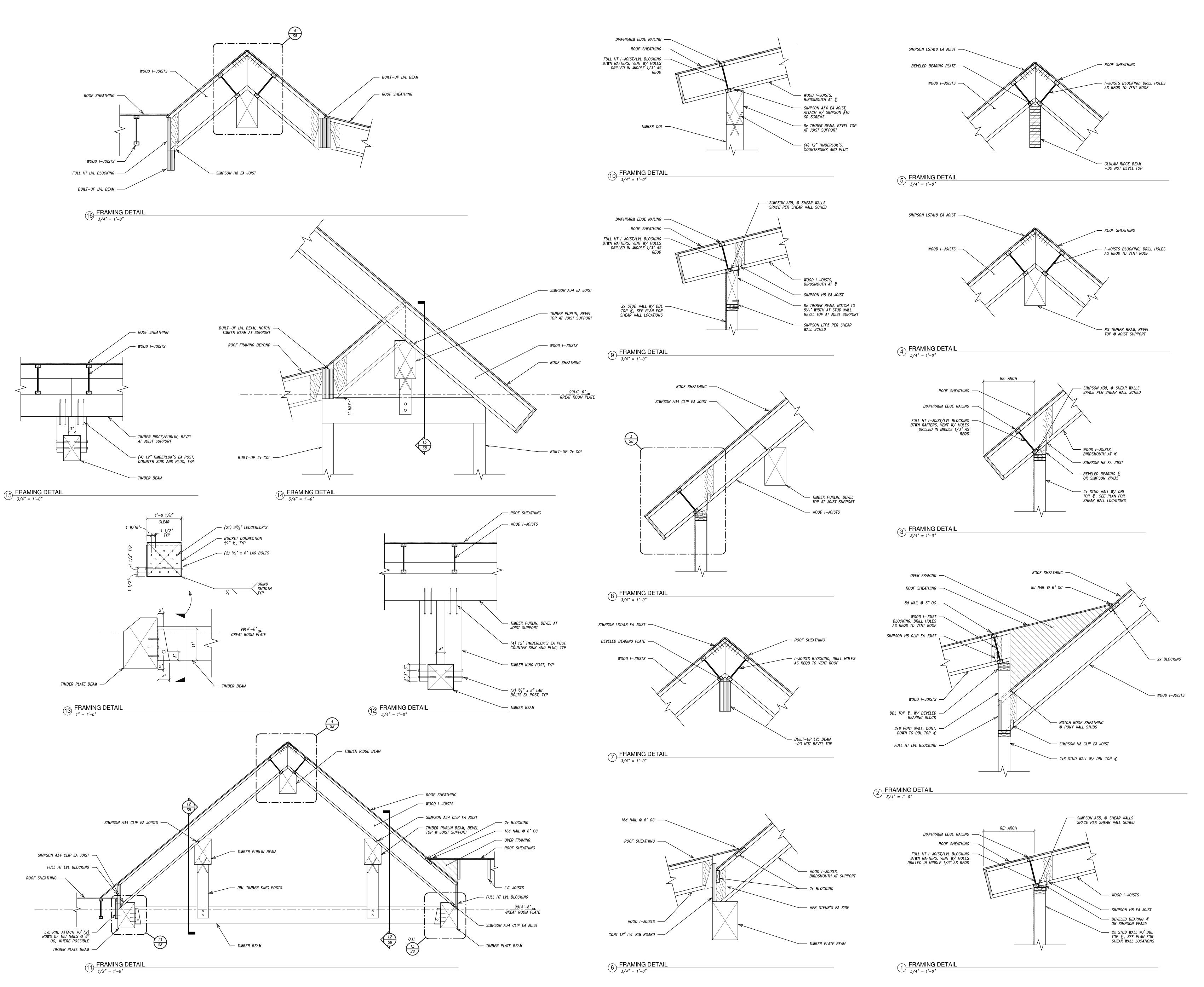
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**ISSUE** STRUC REVIEW 6 JUN 2023 **PERMIT** 11 JUL 2023 PROJECT # A0727

─ DBL TOP P

FRAMING DETAIL

3/4" = 1'-0"



Section III, ItemC.

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**DATE** 

11 JUL 2023

37



- 9904'-0" MAIN LEVEL

(3) 1<sup>3</sup>/<sub>4</sub>"x9<sup>1</sup>/<sub>2</sub>" LVL

(2) 2x6 HF No. 2 FLAT

- SIMPSON A34 CLIP TYP WHERE SHOWN

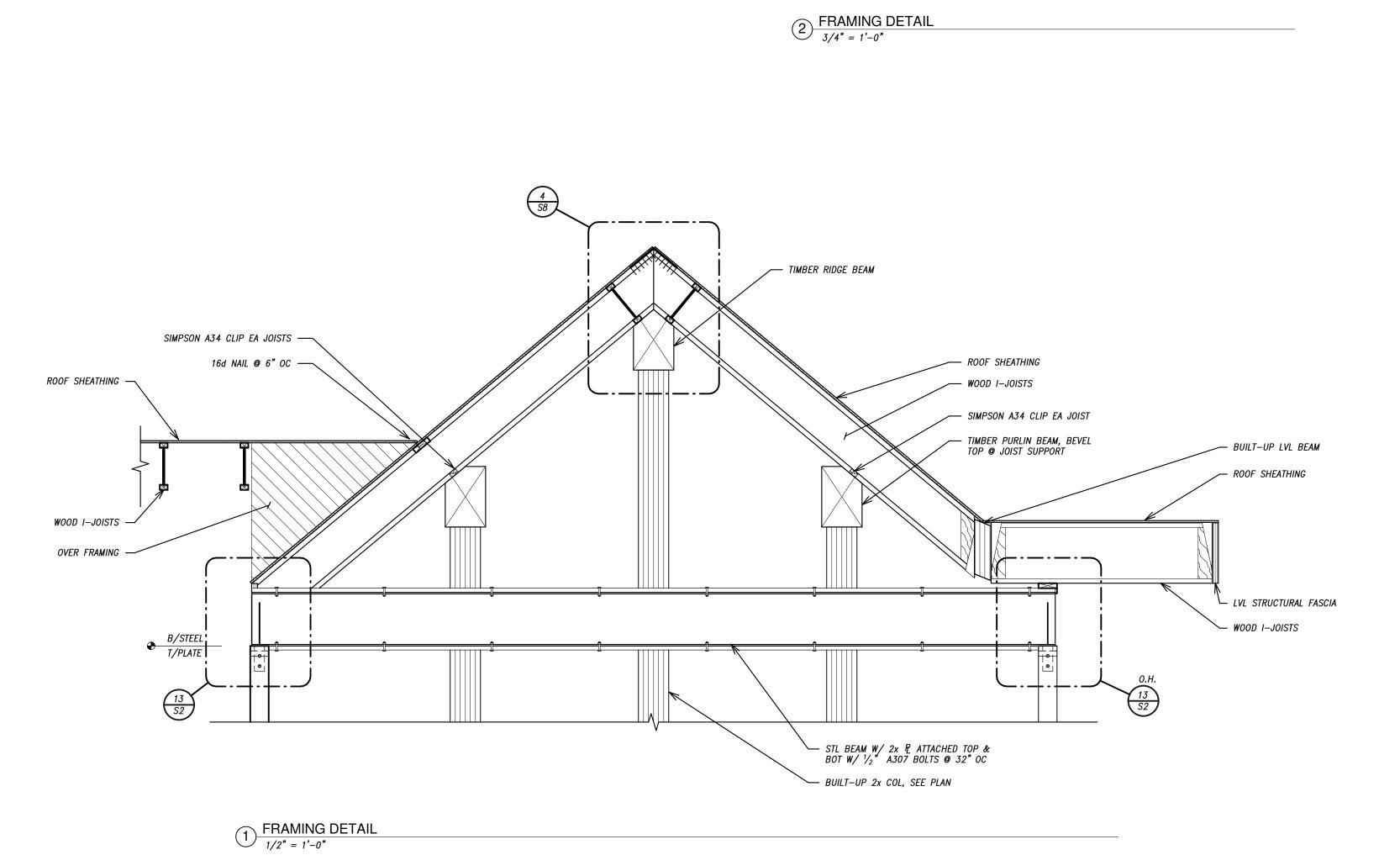
(3) 1<sup>3</sup>/<sub>4</sub>"x18" LVL

(2) 2x6 HF No. 2 FLAT

- SIMPSON HUCQ612-SDS FACE: 14-SDS 1/4×21/2 BEAM: 6-SDS 1/4×21/2 TYP EA END

(2) 2x6 HF No. 2 FLAT

3 ROOF FRAMING PLAN
1/2" = 1'-0"



2x BLOCKING —

SIMPSON H8 EA JOIST

DEVELED BEARING PC OR SIMPSON VPA35

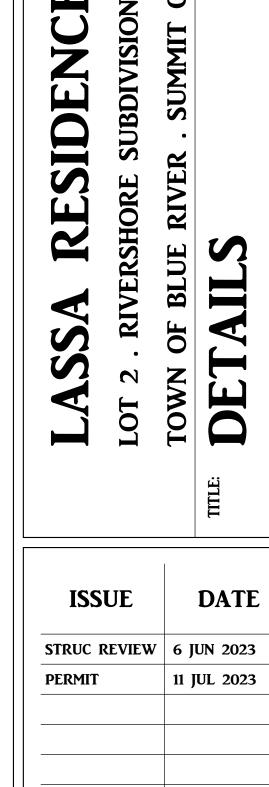
2x STUD WALL W/ DBL TOP P2

ROOF SHEATHING —

ROOF SHEATHING —

DIAPHRAGM EDGE NAILING —

FULL HT LVL RIM BOARD ——



**DATE** 

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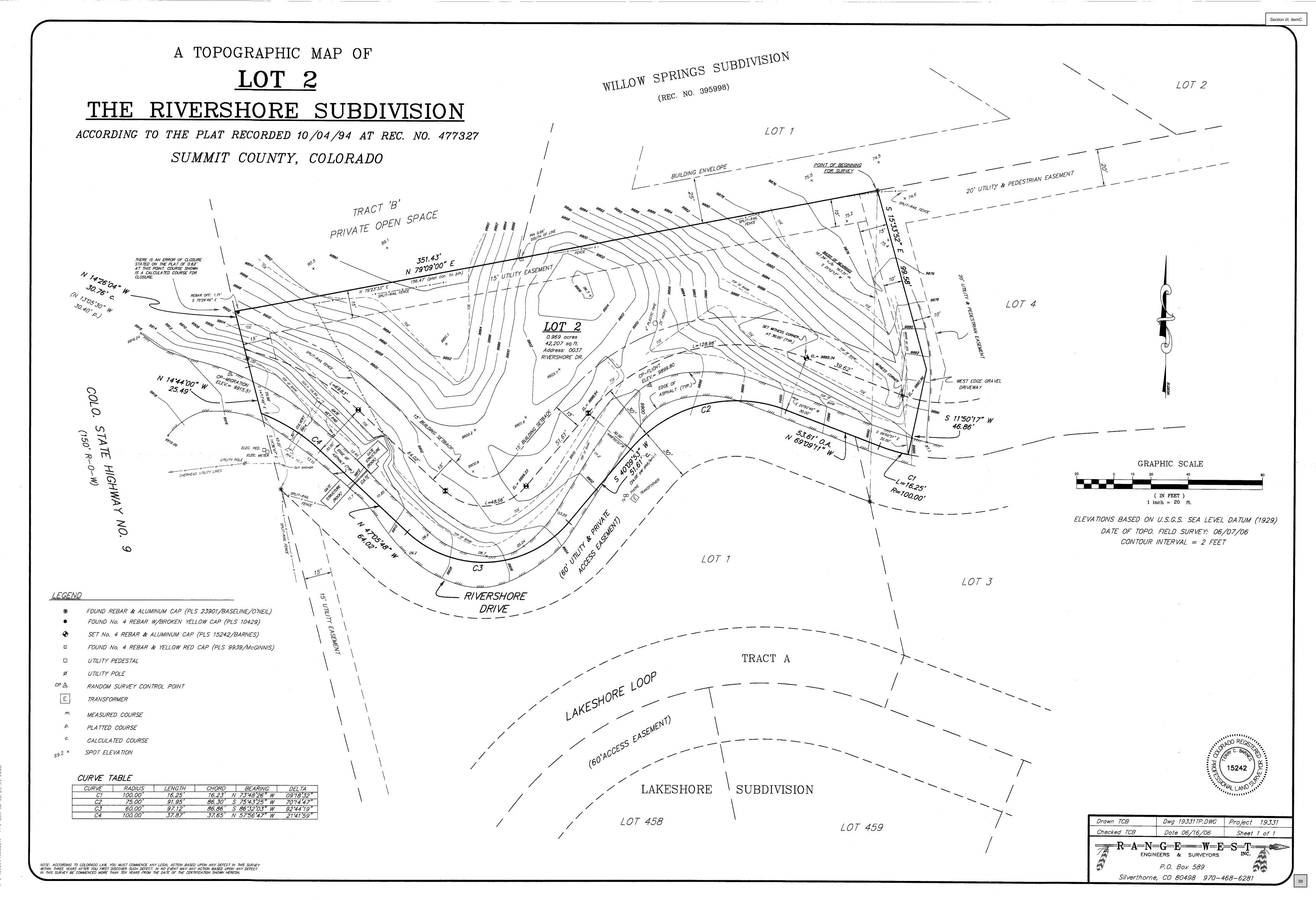
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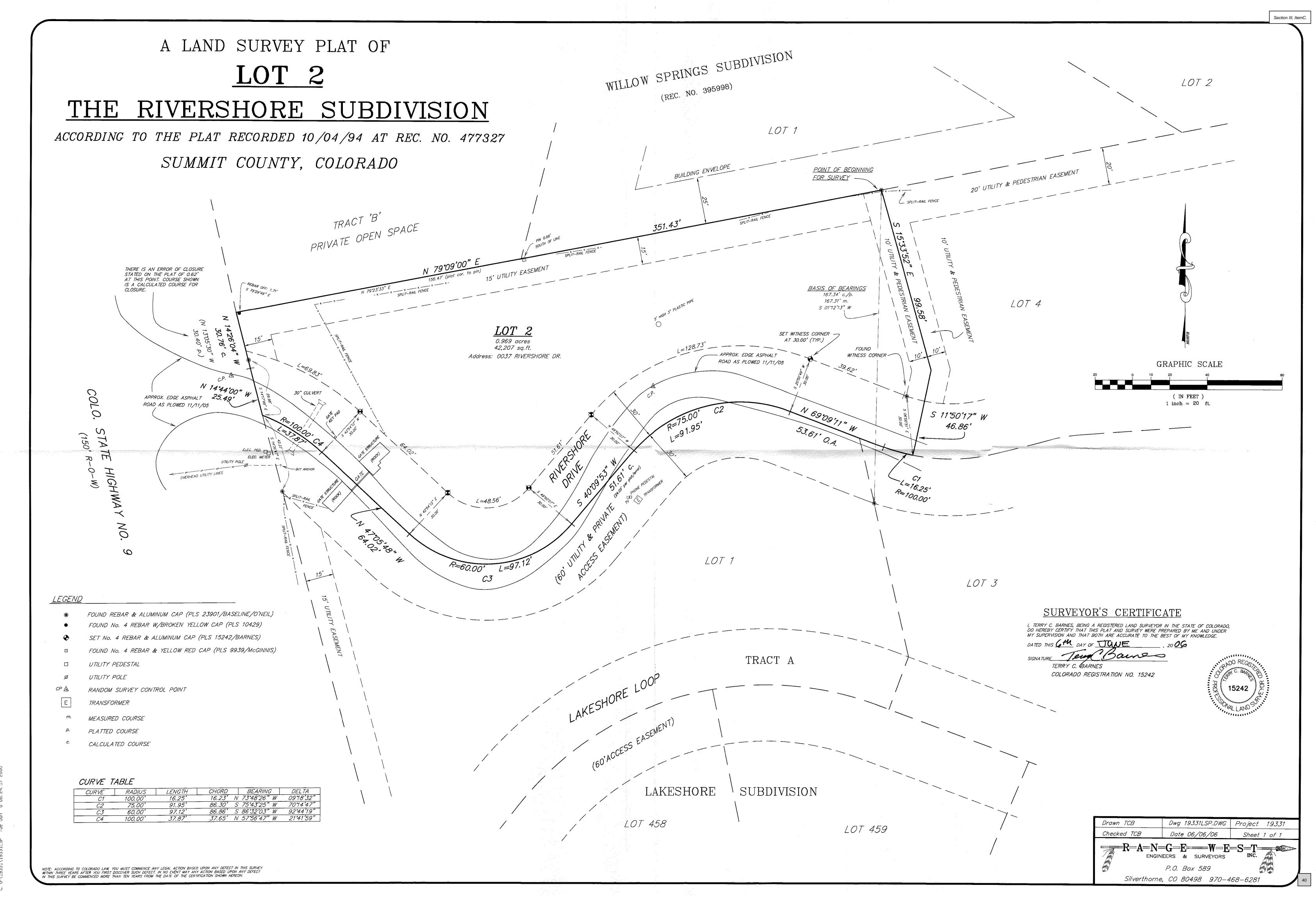
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38



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SUBSOIL STUDY
FOR FOUNDATION DESIGN
PROPOSED RESIDENCE
LOT 2, RIVERSHORE SUB
TOWN OF BLUE RIVER
SUMMIT COUNTY, COLORADO

JOB NO. 09-72

DATE: OCTOBER 21, 2009

#### PREPARED FOR:

ALLEN-GUERRA DESIGN BUILD P.O. BOX 7404 BRECKENRIDGE, COLORADO 80424

PREPARED BY:

WALTER O. SCHULTZ P.E. P.O. BOX 1957 DILLON, CO 80435

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PROCTOR TEST RESULTS	
TYPICAL WALL DRAIN DETAIL	

### PURPOSE AND SCOPE OF STUDY

This report presents the results of a subsoil study for a proposed residence to be located on Lot 2, Rivershore Sub, Summit County, Colorado. The project site is shown on Fig. 1. The purpose of the study was to develop recommendations for the foundation design. The study was conducted in accordance with the verbal agreement for geotechnical engineering services to Allen-Guerra Design Build dated October 19, 2009.

A field exploration program consisting of an exploratory pit was conducted to obtain information on subsurface conditions. Samples of the subsoils obtained during the field exploration were tested in the laboratory to determine their classification, compressibility or swell and other engineering characteristics. The results of the field exploration and laboratory testing were analyzed to develop recommendations for foundation types, depths and allowable pressures for the proposed building foundation. This report summarizes the data obtained during this study and presents my conclusions, design recommendations and other geotechnical engineering considerations based on the proposed construction and the subsoil conditions encountered.

### PROPOSED CONSTRUCTION

The proposed residence will be a two to three story structure over a walkout basement. Grading for the structure is to be moderate with cut depths between about four to ten feet. I assume moderately heavy foundation loadings, typical of the proposed type of construction.

If buildings loadings, location or grading plans change significantly from those described above, I should be notified to reevaluate the recommendations contained in this report.

#### SITE CONDITIONS

The lot was vacant at the time of the study. The lot is irregular in shape with the southern boundary serpentine following Rivershore Drive and the northern boundary along Tract B private open space and Lot 1 Willow Springs Sub. The western boundary of the lot is Colorado Highway 9. Occupied Lot 4 borders on the east. The building site is located in the central area of the lot and is a medium sized knoll with sides sloping moderately to steeply downward from the building site. Vegetation consists of scattered pines. Several boulders were noted at the surface.

## FIELD EXPLORATION

The field exploration for the project was conducted on October 20, 2009.

One exploratory pit was excavated at the location shown on Fig. 1 to evaluate the subsurface conditions. The pit was dug with a rubber-tracked mini-backhoe. The pit was logged.

Samples of the subsoils were taken with disturbed sampling methods. Depths at which the samples were taken are shown on the Test Pit Log, Fig. 2. The samples were returned to the laboratory for review by the project engineer and testing of the natural coarse granular soils is shown.

#### SUBSURFACE CONDITIONS

Logs of the subsurface conditions encountered at the site are shown on Fig. 2. The subsoils consist of about four inches of topsoil overlying relatively dense sandy gravel containing cobbles and boulders.

Laboratory testing performed on samples obtained from the pit included gradation analysis, Proctor value and Atterberg limits. Results of the laboratory preformed on the samples of the natural coarse granular soils are shown.

No free water was encountered in the pit at the time of excavation and the subsoils were slightly moist to moist.

# **DESIGN RECOMMENDATIONS**

#### **FOUNDATIONS**

Considering the subsoil conditions encountered in the exploratory pit and the nature of the proposed construction, I recommend the building be founded with spread footings bearing on the natural granular soils. The design and construction criteria presented below should be observed for a spread footing foundation system.

- 1) Footings placed on the undisturbed granular soils should be designed for an allowable soil bearing pressure of 4000 psf. Based on experience, I expect settlement of footings designed and constructed as discussed in this section will be about 1 inch or less.
- 2) The footings should have a minimum width of 16 inches for continuous walls and 2 feet for isolated pads.
- 3) Exterior footings and footings beneath unheated areas should be provided with adequate soil cover above their bearing elevation for frost protection. Placement of foundations at least 40 inches below exterior grade is typically used in this area.
- 4) Continuous foundation walls should be reinforced top and bottom to span local anomalies (by assuming an unsupported length of at least 5 feet.) Foundation walls acting as retaining structures should also be designed to resist lateral earth pressures as discussed in the "Foundation and Retaining Walls" section of this report.
- All existing fill, topsoil and any loose or disturbed soils should be removed and the footing bearing level extended down to relatively dense natural granular soils or on properly compacted fill. If water seepage is encountered, the footing areas should be dewatered before concrete placement.

- 6) Any fill placed for foundation support should be granular and compacted to at least 100% ASTM D-698 Proctor density. Fill should be placed in maximum loose lifts of 8 inches at a moisture content of 2 % to + 1% of optimum moisture content with a maximum depth of two feet.
- 7) A representative of the geotechnical engineer should observe all footing excavations prior to concrete placement to evaluate bearing conditions.

#### FOUNDATION AND RETAINING WALLS

Foundation walls and retaining structures which are laterally supported and can be expected to undergo only a slight amount of deflection should be designed for lateral earth pressure computed on the basis of an equivalent fluid unit weight of 55 pcf for backfill consisting of the on-site soils. Cantilevered retaining structures which are separated from the building and can be expected to deflect sufficiently to mobilize the full active earth pressure condition should be designed for a lateral earth pressure computed on the basis of an equivalent fluid unit weight of 35 pcf for backfill consisting of the on-site soils.

All foundation and retaining structures should be designed for appropriate hydrostatic and surcharge pressures such as adjacent footings, traffic, construction materials and equipment. The pressures recommended above assume drained conditions behind the walls and a horizontal backfill surface. The buildup of water behind a wall or an upward sloping backfill surface will increase the lateral pressure imposed on a foundation wall or retaining structure. An underdrain should be provided to prevent hydrostatic pressure buildup behind walls.

Backfill should be placed in uniform lifts and compacted to at least 90% of ASTM D-698 Proctor density (at a moisture content near optimum.) Backfill in pavement and walkway areas should be compacted to at least 95% of ASTM D-698 Proctor density. Care should be taken not to over compact the backfill or use large equipment near the wall, since this could cause excessive lateral pressure on the wall. Some settlement of deep foundation wall backfill should be expected, even if the material is placed correctly, and could result in distress to facilities constructed on the backfill.

I recommend on-site granular soils for backfilling foundation walls and retaining structures because their use results in lower lateral earth pressures. Subsurface drainage recommendations are discussed in more detail in the "Underdrain System" section of this report. Imported granular wall backfill should contain less than 10% passing the No. 200 sieve, be similar to the onsite granular soil and have a maximum size of 6 inches.

The lateral resistance of foundation or retaining wall footings will be a combination of the sliding resistance of the footing on the foundation materials and passive earth pressure against the side of the footing. Resistance to sliding at the bottoms of the footings can be calculated based on a coefficient of friction of 0.40. Passive pressure of compacted backfill against the sides of the footings can be calculated using an equivalent fluid unit weight of 400 pcf. The coefficient of friction and passive pressure values recommended above assume ultimate soil strength. Suitable factors of safety should be included in the design to limit the strain, which will occur at the ultimate strength, particularly in the case of passive resistance. Fill placed against the sides of the footings to resist lateral loads should be a nonexpansive granular material compacted to at least 95% of ASTM D-698 Proctor density at a moisture content near optimum.

#### FLOOR SLABS

The natural on-site soils, exclusive of topsoil, are suitable to support lightly to moderately loaded slab-on-grade construction. To reduce the effects of some differential movement, floor slabs should be separated from all bearing walls and columns with expansion joints which allow unrestrained vertical movement. Floor slab control joints should be used to reduce damage due to shrinkage cracking. The requirements for joint spacing and slab reinforcement should be established by the designer based on experience and the intended slab use. A (minimum) 4-inch layer of free-draining gravel may be placed beneath basement level slabs to facilitate drainage. This material should consist of minus 2-inch aggregate with at least 50% retained on the No. 4 sieve and less than 2% passing the No. 200 sieve.

All fill materials for support of floor slabs should be compacted to at least 95% of ASTM D-698 Proctor density at a moisture content near optimum. Required fill can consist of the on-site soils devoid of vegetation, topsoil and oversized rock.

#### UNDERDRAIN SYSTEM

Although free water was not encountered during the exploration, it has been my experience in mountainous areas that local perched groundwater may develop during times of heavy precipitation or seasonal runoff. Frozen ground during spring runoff can create a perched condition. I recommend below-grade construction, such as retaining walls, crawlspace and basement areas, be protected from wetting and hydrostatic pressure buildup by an underdrain system. Refer to attached detail.

The drains should consist of drainpipe placed in the bottom of the wall backfill surrounded above the invert level with free-draining granular material. The drain should be placed at each level of excavation and at or below lowest adjacent footing grade and sloped at a minimum 1% to a suitable gravity outlet. Free-draining granular material used in the underdrain system should contain less than 2% passing the No. 200 sieve, less than 50% passing the No. 4 sieve and have a maximum size of 2 inches. The drain gravel backfill should be at least 1 ½ feet deep. Geocomposite wall drain should extend to within 1 to 2 feet of finish grade and connect to the drain gravel or pipe.

# SITE GRADING

The risk of construction-induced slope instability at the site appears low. I assume the cut depths for the basement level will not exceed, one level, about 8 to 10 feet. Fills should be limited to about 8 to 10 feet deep, especially at the downhill side of the residence where the slope steepens. Embankment fills should be compacted to at least 95% of ASTM D-698 Proctor density near optimum moisture content. Prior to fill placement, the subgrade should be carefully prepared by removing all vegetation and topsoil and compacting to 95% ASTM D-698 Proctor density. The fill should be benched into the portions of the hillside exceeding 20% grade.

Permanent unretained cut and fill slopes should be graded at 2 horizontal to 1 vertical or flatter and protected against erosion by revegetation or other means. The risk of slope instability will be increased if seepage is encountered in cuts and flatter slopes may be necessary. If seepage is encountered in permanent cuts, an investigation should be conducted to determine if the seepage will adversely affect the cut stability. This office should review site-grading plans for the project prior to construction.

### SURFACE DRAINAGE

The following drainage precautions should be observed during construction and maintained at all times after the building has been completed:

- Inundation of the foundation excavations and underslab areas should be avoided during construction.
- Exterior backfill should be adjusted to near optimum moisture and compacted to at least 95% of ASTM D-698 Proctor density in pavement and slab areas and to at least 90% of ASTM D-698 Proctor density in landscape areas.
- 2) The ground surface surrounding the exterior of the building should be sloped to drain away from the foundation in all directions. I recommend a minimum slope of 12 inches in the first 10 feet in unpaved areas and a minimum slope of 3 inches in the first 10 feet in paved areas.

  Free- draining wall backfill should be capped with about 2 feet of the on-site soils to reduce surface water infiltration.
- 4) Roof downspouts and drains should discharge will beyond the limits of all backfill.
- 5) Landscaping which requires regular heavy irrigation should be located at least 10 feet from foundation walls.
- 6) Consideration should be given to use of xeriscape to reduce the potential for wetting of soils below the foundation caused by irrigation.

#### **LIMITATIONS**

This study has been conducted in accordance with generally accepted geotechnical engineering principles and practices in this area at this time. I make no warranty either expressed of implied. The conclusions and recommendations submitted in this report are based upon the data obtained from the exploratory pit excavated at the location indicated on Fig. 1, the proposed type of construction and my experience in the area. My findings include interpolation and extrapolation of the subsurface conditions identified at the exploratory pit and variations in the subsurface conditions may not become evident until excavation is performed. If conditions encountered during construction appear different from those described in this report, I should be notified so that re-evaluation of the recommendations may be made.

This report has been prepared for the exclusive use by the client for design purposes. I am not responsible for technical interpretations by others of the information. As the project evolves, I should provide continued consultation and field services during construction to review and monitor the implementation of my recommendations, and to verify that the recommendations have been appropriately interpreted. Significant design changes may require additional analysis or modifications to the recommendations presented herein. I recommend on-site observation of excavation and foundation bearing strata and testing of structural fill by a representative of the geotechnical engineer.

Sincerely,

WALTER O. SCHULTZ I

# **TEST PIT LOG**

# LOT 2, RIVERSHORE SUB, TOWN OF BLUE RIVER SUMMIT COUNTY, COLORADO

# PIT NO.1

0-4" TOPSOIL AND VEGETATION

4"-8' SANDY GRAVEL W/ COBBLE & BOULDERS, DENSE, MOIST BROWN

NO FREE WATER ENCOUNTERED DURING EXCAVATION

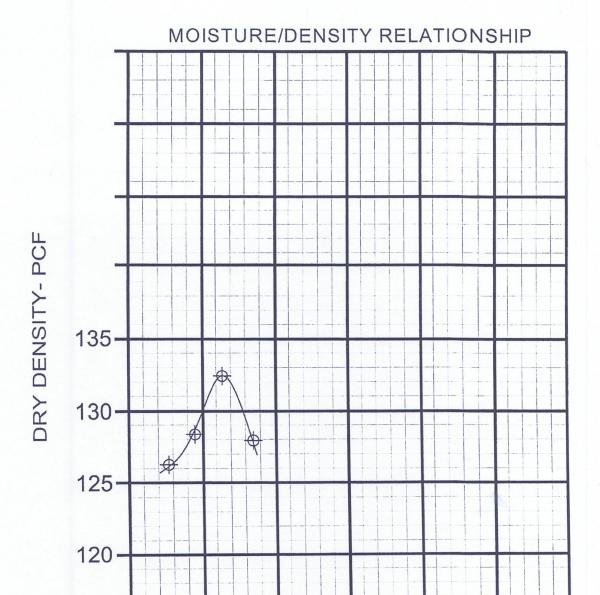
COMPOSITE SAMPLES TAKEN

JOB NO: 09-72 FIGURE 2 COLO. STATE HWY 9

TEST PIT LOCATION LOT 2, RIVERSHORE SUB TOWN OF BLUE RIVER SUMMIT COUNTY, CO



# WALTER O. SCHULTZ P.E. P.O. BOX 1957 DILLON, CO 80435



MOISTURE CONTENT- PER CENT DRAWING NO. 09-72P

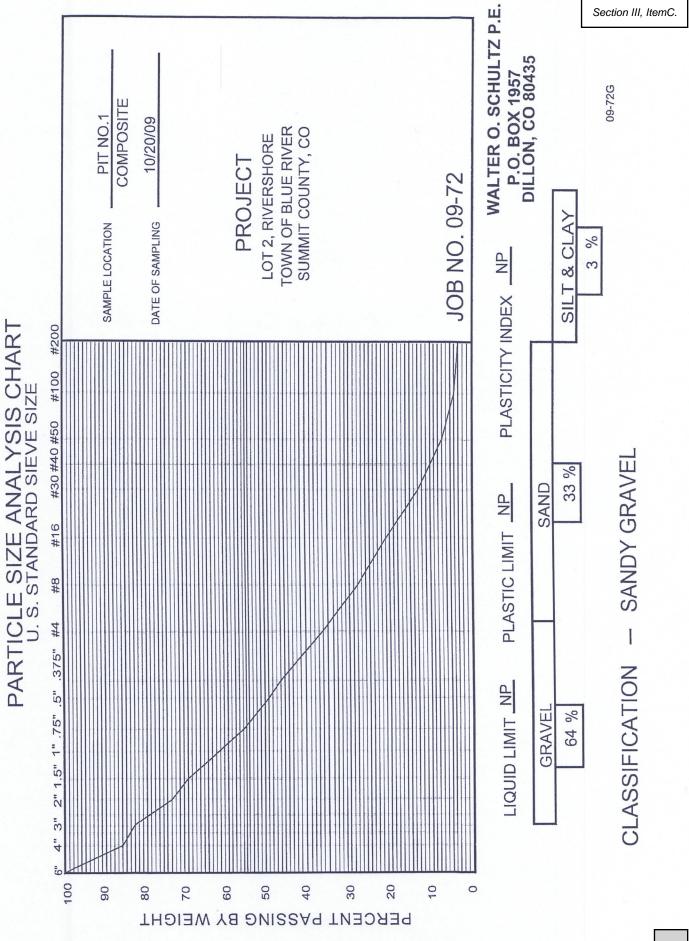
15

PIT NO.	SAMPLE NO.	DATE	DEPTH FEET	MAXIMUM DRY DENSITY	OPTIMUM MOISTURE	PROCTOR METHOD
1	1	10/20/09	COMPOSITE	132.4 PCF	6.4 %	ASTM D- 698D
SOIL TYPE SANDY GRAVEL			PROJECT NO. 09-72 LOT 2, RIVERSHORE TOWN OF BLUE RIVER SUMMIT COUNTY, CO			

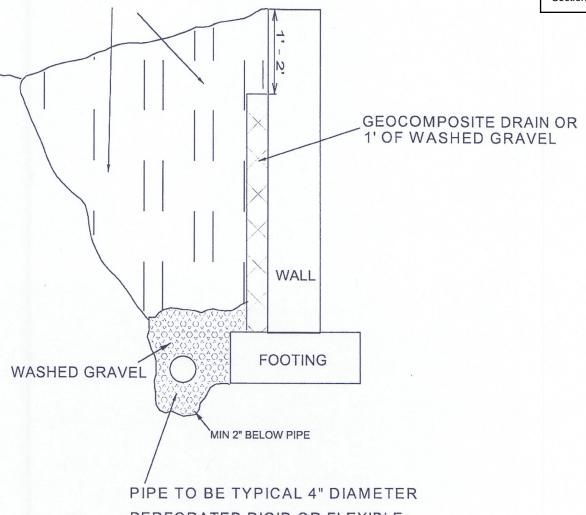
10

115

5



Section III, ItemC.



PERFORATED RIGID OR FLEXIBLE WRAPPED IN FILTER FABRIC

PIPE MAY BE AT OR BELOW BOTTOM OF FOOTING

WALLDRAIN

# TYPICAL DRAIN SYSTEM CROSS SECTION

WALTER O. SCHULTZ P.E. P.O. BOX 1957 **DILLON, CO 80435** 

#### STAFF REPORT

**TO:** Planning Commission

FROM: Kyle Parag, Building Official

**RE:** Modification to Town adopted Ground Snow Load

**DATE:** January 14, 2024

#### **BACKGROUND/ANALYSIS:**

Every jurisdiction is required to determine a snow load as part of the local climatic conditions specific to the location of the jurisdiction. This snow load is used in calculations by engineers for structural loads imposed on all portions of a structure and used by inspectors to determine structural stability of structures within the Town Limits of Blue River. Historically, Blue River's snow load has been determined to be 100 lbs/sqft, roof snow load. As a roof snow load, the IRC does not permit any reduction for the loads actually imposed on the structure other than pitch reductions.

Heavy snow fall weighs about 1.5 lbs/sqft\*inch, which means the current 100 lbs/sqft design criteria equates to a snow accumulation of about 66". In addition to the weight of the compounding snow accumulation, freeze-thaw cycles can create ice, exponentially increasing the total weight. Ice/snow mixtures weigh about 5lbs/sqft\*inch. With the analysis of the ice and snow combination, the 100 lb/sqft can be exceeded with only about 20" of late season snow and ice combination accumulation.

Most of the building safety industry uses ground load rather than roof load, which is typically converted by reducing the ground load by 30%. With that conversion, and for comparisons in this document, Blue River would have a current design ground snow load of 142 lb/sqft.

Newer recent data that uses this ideology has indicated the 2% snow load with the addition of the loading for ice for some of the Blue River properties goes up to 227lbs/sqft. 2% snow load is derived from similar methods of the rainfall statistics, such as 100-year rain. A 2% snow load would mean that the load is expected to be obtained with a chance of 2% in any given year, and/or expected every 50 years.

When using the tool below, residential structures are Risk Category II.

https://asce7hazardtool.online

For additional information:

 $\underline{https://assets.ccaps.umn.edu/documents/CPE-Conferences/structural/2022Structural722ASCE.pdf}$ 

The current snow load design criteria the Town uses (142 lb/sqft) is currently on par with the highest snow loads required by jurisdictions throughout Colorado. This snow load poses challenges for the design of structures and promotes steeper roof designs. A significant increase in snow load will create additional costs for the construction of new homes. However, with recent data indicating the relative probability of significantly exceeding the current design loads is likely, I recommend an increase in design snow loads.

As the building official, I am recommending increasing the roof snow load to 140 lbs/sqft (200 lbs/sqft ground snow load).

The above snow load has been determined based on the latest data available and to the best of the staff's knowledge, provides the safest and most reasonable design conditions for the Town of Blue River, without creating undue costs and burdens on the community.

This change will be written in the Town code as part of the climatic conditions table of the IRC, and the value will be used for the local determination in accordance with 1608.2 of the IBC.

#### STAFF RECOMMENDATION

Staff recommends the commission provide a recommendation to the Board of Trustees to approve an ordinance to increase the design snow load for the Town of Blue River.