

CITY OF BELLE ISLE, FL CITY COUNCIL MEETING - UPDATED

Held in City Hall Chambers 1600 Nela Avenue Belle Isle FL Held the 1st and 3rd Tuesday of Every Month Tuesday, April 18, 2023 * 6:30 PM **AGENDA**

City Council Commissioners

Nicholas Fouraker, Mayor

Vice-Mayor – Jim Partin, District 7

District 1 Commissioner – Ed Gold | District 2 Commissioner – Anthony Carugno | District 3 Commissioner – Karl Shuck | District 4 Commissioner – Randy Holihan | District 5 Commissioner – Beth Lowell | District 6 Commissioner – Stan Smith

Welcome

Welcome to the City of Belle Isle City Council meeting. Agendas and all backup material supporting each agenda item are available in the City Clerk's office or website at www.belleislefl.gov. If you are not on the agenda, please complete the yellow "Request to Speak" form to be handed to the City Clerk. The Council is pleased to hear relevant comments and has set a three-minute limit. Rosenberg's Rules of Order guide the conduct of the meeting. Order and decorum will be preserved at all meetings. Personal, impertinent, or slanderous remarks are not permitted. Please silence all technology during the session. Thank you for participating in your City Government.

- 1. Call to Order and Confirmation of Quorum
- 2. Invocation and Pledge to Flag Comm Gold, District 1
- 3. Consent Items These items are considered routine and previously discussed by the Council. One motion will adopt them unless a Council member requests before the vote on the motion to have an item removed from the consent agenda and considered separately.
 - a. Approval of City Council Meeting Minutes April 4, 2023
- 4. Citizen's Comments Persons desiring to address the Council must complete and provide the City Clerk a yellow "Request to Speak" form. When the Mayor recognizes you, state your name and address and direct all remarks to the Council as a body, not individual council members, staff, or audience. Citizen comments and each section of the agenda where public comment is allowed are limited to three (3) minutes. Questions will be referred to staff and answered within a reasonable period following the meeting date.
- 5. Unfinished Business
- a. Discuss the Wallace Park Use Agreement (Comm Carugno)

6. New Business

- a. Ordinance 23-02 First Reading and Consideration AN ORDINANCE OF THE CITY OF BELLE ISLE, FLORIDA REZONING THOSE CERTAIN PROPERTIES OWNED BY THE CITY OF BELLE ISLE LOCATED AT 5903 RANDOLPH AVENUE, 906 WALTHAM AVENUE, 6300 HANSEL AVENUE, "SUB OF HARNEY HOMESTEAD C/53 LOT 9 (LESS W 224.28 FT OF LOT 9 & LESS E 228.47 FT OF SAID LOT 9 & LESS R/W ON N & S)", AND 6049 RANDOLPH AVENUE, ALSO KNOWN AS ORANGE COUNTY TAX PARCELS # 24-23-29-3400-00-073, 24-23-29-3400-00-092, 24-23-29-3400-00-094, 24-23-29-3400-00-095, AND 24-23-29-3400-00-114, FROM C-1, C-2, R-1A, AND R-2 ZONING DISTRICTS TO PLANNED DEVELOPMENT DISTRICT (PD); REZONING THAT CERTAIN PROPERTY OWNED BY CORNERSTONE CHARTER ACADEMY, INC. LOCATED AT 5929 HANSEL AVENUE, ALSO KNOWN AS ORANGE COUNTY TAX PARCEL #24-23-29-8820-00-050, FROM C-1 ZONING DISTRICT TO PLANNED DEVELOPMENT DISTRICT (PD); REZONING THAT CERTAIN PROPERTY OWNED BY PINE CASTLE METHODIST CHURCH, INC. LOCATED AT 942 FAIRLANE AVENUE, ALSO KNOWN AS ORANGE COUNTY TAX PARCEL #24-23-29-3400-00-093, FROM C-1 ZONING DISTRICT TO PLANNED DEVELOPMENT DISTRICT (PD); PROVIDING FOR CERTAIN PERMITTED, PROHIBITED AND SPECIAL EXCEPTION USES AND DEVELOPMENT STANDARDS AND REQUIREMENTS WITHIN THE PLANNED DEVELOPMENT; DESCRIBING THE PLANNED DEVELOPMENT INCLUDING ALL THE AFORESAID PROPERTIES AS THE CORNERSTONE CHARTER ACADEMY PD; PROVIDING FOR SEVERABILITY AND AN EFFECTIVE DATE.
- b. Travel Reimbursement for Howard Brown
- c. Approval of Bing Grant for Winward on the Lake
- d. Adopt BIPD Pay Plan Amendment and Approve Interim City Manager, Acting Chief, and Acting Deputy Chief Stipend
- e. Discussion on Reinstating Council Work Sessions
- 7. Attorney's Report

[&]quot;If a person decides to appeal any decision made by the Council with respect to any matter considered at such meeting or hearing, he/she will need a record of the proceedings, and that, for such purpose, he/she may need to ensure that a verbatim record of the proceedings is made, which record includes the testimony and evidence upon which the appeal is to be based." (F. S. 286.0105). "Persons with disabilities needing assistance to participate in any of these proceedings should contact the City Clerk's Office (407-851-7730) at least 48 hours in advance of the meeting." –Page 1 of 2

8. City Manager's Report

- a. Acting Chief Report
- b. Public Works Report
- 9. Mayor's Report
- 10. Items from Council
- 11. Adjournment
 - a. Cornerstone Charter Academy Expansion Documents

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CITY OF BELLE ISLE, FL CITY COUNCIL MEETING

Tuesday, April 04, 2023, * 6:30 pm **MINUTES**

<u>Present was:</u> District 1 Commissioner – Ed Gold District 2 Commissioner – Anthony Carugno District 3 Commissioner – Karl Shuck District 4 Commissioner – Randy Holihan District 5 Commissioner – Beth Lowell District 7 Commissioner – Jim Partin <u>Absent was:</u> Nicholas Fouraker, Mayor District 6 Commissioner – Stan Smith

1. City Council Swear-In District 1 and District 7

Interim City Manager Grimm administered Oath of Office for Comm Gold, District 1, and Comm Partin, District 7.

2. Call to Order and Confirmation of Quorum

Vice Mayor Partin called the meeting to order at 6:30 pm, and the City Clerk confirmed the quorum. Also present were Interim City Manager Grimm, Attorney Chumley, Acting Chief Millis, Public Works Director Phil Price, and City Clerk Yolanda Quiceno.

3. Invocation and Pledge to Flag

Vice Mayor Partin gave the invocation and led the pledge to the flag.

4. Presentations

- a. Introduction of Code Enforcement and Community Service Officer Julie Wilk
- b. Swear-In of Officer Britzzy Diaz
- c. Promotion of Corporal Lugo to Sergeant and Officer Gargano to Corporal

Acting Chief Millis swore in Officer Diaz and presented Sgt Lug and Corporal Gargano with their promotions. Acting Chief Millis welcomed and introduced Service Officer Wilk.

Vice Mayor Partin called for a motion to excuse Mayor Fouraker and Comm Smith from the meeting. Comm Holihan moved to excuse Mayor Fouraker and Comm Smith. Comm Gold seconded the motion, which passed unanimously 6:0.

5. Consent Items

- a. Approval of the City Council Meeting Minutes March 20, 2023
- b. Approval of the City Council Meeting Minutes March 21, 2023
- c. Approval of the City Council Special Called Session March 27, 2023
- d. Proclamation Declaring May Neurofibromatosis Awareness Month

Comm Holihan moved to approve the consent items as presented. Comm Lowell seconded the motion, which passed unanimously 6:0. Vice Mayor Partin read the Proclamation for the record.

6. Citizen's Comments

Vice Mayor Partin called for citizen comments.

Mark Cronkite residing at 1925 Hoffner Avenue, opposed allowing RV parking on residential properties.

Jeanine Hollingsworth asked why a Code Enforcement Officer can't serve citations without having a resident report the citation and not suffer the consequences. In addition, why are we discussing RVs in residential districts? She said it had been discussed in detail at a previous meeting. She shared her concern and asked for clarification on placement, use, and front yard size.

7. Unfinished Business

a. Discuss Criteria for RV Parking and Survey Monkey Circulation

Interim City Manager Grimm said the City Council directed staff to look at the language that would allow for a waiver or special exception to allow recreational equipment to park in the front yard. Comm Carugno said he addressed the issue because some constituents requested clarification under Section 30-133(4). He opposes RVs in the front yard. However, several properties are 300-500 deep and may accommodate the use. He requested approval to add language to allow exceptions for RVs if screened from the street on a residential property.

Comm Carugno moved to direct the staff to draft an ordinance to change the RV parking code that would allow exceptions to RV parking on residential property to include "if lots that can store an RV in the back or front of the residents only if there is a home directly in front that would screen the RV from the street."

Comm Shuck seconded the motion.

Comm Lowell asked if the change to the Code would be easy for the Code Enforcement Officer to determine and if the homeowner is aware since this is a rental.

Interim City Manager Grimm said approval should be done in an open forum and asked if the special exception could come before the Council for approval.

Mr. Cronkite said he called the City on an RV parked on his neighbor's rental property for two years. The RV is obstructing his view and is allowed by the City. A few days later, Mr. Francis gave the resident a permit to allow temporary parking with the intention that the owner registers the RV and moves it to the south of the property; neither happened. This is his home, not a rental property, and he would like the City's help and follow the Code.

Comm Gold said this could cause an unsightly view and create a disservice to another resident.

The motion failed 3:3.

b. Discuss Change to the Municipal Code for Fences and Survey Monkey Circulation

Interim City Manager Grimm said the City Council requested discussion on possible changes to the Code on fencing to provide an overlay on Hoffner Avenue. Attorney Geller said general applications such as this do not provide a pecuniary interest, and a commissioner is allowed to vote. Comm Carugno said he would like to recommend a slight change in the code to allow for the repair and replacement of an existing front yard fence which will mirror the existing configuration without applying for a variance. Any other deviation will require variance approval.

Comm Lowell said she has a constituent with a similar situation and would like to include existing front yard fences that do not meet the current standards.

Comm Gold said a repair might not need a permit if repaired in stages. Attorney Chumley said he would have to review the code; it becomes a de facto matter at a certain point.

Vice Mayor Partin opened for public comment.

Holly Bobrowski residing at 2400 Hoffner Avenue, said she has a similar situation; however, to replace the dilapidated fence, she must undergo a variance process. She recommends Council provide specific information on allowable material.

There being no further comment, Vice Mayor Partin closed public comment.

The Council consensus was to move this to the Planning & Zoning Commission for further review and recommendation.

Comm Carugno asked for a 5-minute recess. Vice Mayor Partin called for a five-minute recess at 7:30 pm. The meeting resumed at 7:40 pm.

Vice Mayor Partin asked for Council approval to reopen public comment to allow for two comments. The Council agreed.

Susan Collins, District 3-OC Soil and Water Conservation (OCSWC) District said that the City was recognized for its affiliation as a Tree City and for being environmentally conscience. She also said the OCSWC would sponsor the poster contest at the Arbor Day event.

Betty Butler residing at 5231 Hawford Circle, spoke on removing a non-permitted large Oak Tree in her neighborhood. She said her husband researched a process that would save the tree while working around repairing the sidewalk. She urged Council for a moratorium on removing trees to allow for staff to review the information her husband provided.

8. New Business

a. <u>At-Large Appointment of Planning & Zoning Member District 3 (Per Section 42-32)B)(3)</u> Interim City Manager Grimm said the P&Z seat for District 3 has been vacant for more than six months, and Comm Shuck, with every effort, has been unable to find anyone in his district interested in filling the vacancy per Section 42-32(b)(3). Staff is recommending posting an At-Large appointment for the vacancy for the term May 2023-2026.

Comm Lowell moved to direct the staff to post the vacancy At-Large for Council consideration at the April 18 Council meeting.

Comm Gold seconded the motion, which passed unanimously 6:0.

b. Approval of BING Grant for Conway Lakes HOA, Inc.

Interim City Manager Grimm presented a BING Grant for Conway Lakes HOA for landscaping and irrigation repairs on Judge Road and Conway Lake Drive totaling \$12,824.67. Comm Smith has approved 50% of the total cost and would like to grant the request.

Comm Gold moved to approve the BING Grant for Conway Lakes HOA for \$6,412.33, District 6. Comm Lowell seconded the motion, which passed unanimously 6:0.

c. Approve Extension of Albert Moore's Contract Agreement

Interim City Manager Grimm presented a two-year extension agreement of Albert Moore's Tree Service Contract to provide tree removal services to the City. Staff recommends approval for the two-year extension with the same terms to allow services to continue.

Comm Shuck moved to approve Albert Moore's tree Service contract with the same services for two years.

Comm Carugno seconded the motion.

Comm Carugno asked who is responsible for maintaining the Tree City guidelines of removing and replacing trees on a City right-of-way if the City is the applicant. Council discussion was that it might fall under the Public Works Department to follow up. He would like the staff to report that there is a process for tracking, replacing, and removing trees.

Comm Gold said he wants to ensure the City gets the best value. Interim City Manager Grimm noted that with the past hurricanes, we were the first clean City in Orange County because of their service. The trees that have been removed are to prevent dangers during storms.

Comm Lowell asked if there is a formal policy for tree removals. Mr. Grimm said the staff could inform Council about emergency removals. Attorney Chumley said the State had curtailed the City regulating tree removals about two years ago.

After discussion, the motion passed unanimously at 6:0.

d. <u>Approval of Surplus Miscellaneous Office Equipment</u> Interim City Manager Grimm requested approval for a surplus of a LaserJet Pro and picture frames to donate to the Russell Home.

Comm Gold moved to approve the surplus items as presented. Comm Lowell seconded the motion, which passed unanimously 6:0.

9. Attorney's Report - na

10. City Manager's Report

Interim City Manager Grimm said the temporary transition has been moving in the right direction. Colin Baenziger has reported that they are finalizing the salary assessment and have met with all Council members. They said they would have a final brochure for approval shortly.

a. Chief's Report

Acting Chief Millis reported the following,

- Lake Take Over went well 37 citations were issued, and a few calls for noise were received. Council briefly discussed some concerns and partnering continued partnerships with Orange County.
- A 6-foot alligator was found on someone's doorstep FWC was notified.
- 1615 Idaho caught fire over the weekend and is officially condemned. Attorney Chumley said this home has a pending lien for the sewer replacement. The City has filed a motion of default.
- Acting Chief Millis reported 27 reports, 761 Red Light Camera violations, and YTD 633 citations on Hoffner.
- b. Public Works Report

Mr. Price said the City had received the Dump Trailer. He provides a sample of the PW Project update that will go out to the residents to notify them of upcoming projects.

11. Mayor's Report - na

12. Items from Council

- Council thanked the Police Officers and their families.
- Comm Carugno spoke briefly on finding new facilities for the Belle Isle Police Department. Comm Carugno motioned to cancel the Wallace Field Use Agreement with CCA to build a Police Department. Comm Carugno restated his motion to add the discussion to the next Council agenda.
- Comm Carugno summarized the last meeting. He added that the next ANAC Meeting would be held on April 21, 2023.
- Comm Gold said he would like to add, to the next agenda, a discussion on compensation for the Interim City Manager.

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- Holly Bobrowski invited everyone to the Easter Egg Hunt/Arbor Day Event at 10 am on Saturday, April 8, 2023.

13. Adjournment

There being no further business, Vice Mayor Partin called for a motion to adjourn. The motion passed unanimously at 8:35 pm.

CITY OF BELLE ISLE, FLORIDA CITY COUNCIL AGENDA ITEM COVER SHEET

Meeting Date:	April 18, 2023
То:	Honorable Mayor and City Council Members
From:	T. Grimm, Interim City Manager
Subject:	Ordinance 23-02 CCA Zoning Change from C1 to PD

Background: The City Council read Ordinance 21-12 for the first time at the October 5 Council meeting and reviewed the CCA Lease Agreement. The agreement, since then, had a few small changes for grammar and language necessary to proceed with an FDFC bond but nothing that would significantly change the agreement. City and CCA Attorneys reviewed the request and amendment, and changes were made to the original amendment.

On April 11, 2023, the CCA Board presented Ordinance 23-1 to the Planning & Zoning Board (P&Z) to change FROM C-1 ZONING DISTRICT TO PLANNED DEVELOPMENT DISTRICT (PD); PROVIDING FOR CERTAIN PERMITTED, PROHIBITED AND SPECIAL EXCEPTION USES AND DEVELOPMENT STANDARDS AND REQUIREMENTS WITHIN THE PLANNED DEVELOPMENT; DESCRIBING THE PLANNED DEVELOPMENT INCLUDING ALL THE AFORESAID PROPERTIES AS THE CORNERSTONE CHARTER ACADEMY PD.

The P&Z Board recommended approval 4:1 with the following conditions,

- 1. Randolph Avenue be closed only during the pick-up and drop-off hours of the school and shall include appropriate pedestrian safety measures.
- 2. Eliminate the "Required and "Allowable" columns on the site plan and reflect the floor area ratio, impervious surface ratio, and building setbacks standard.
- 3. The "Site Notes" on page 9 of the concept plans reference the final plans shall be at Final Construction Plan. Under sec. 54-77 (e) (5), the code requires the next phase of the PD process to govern "the use of land and the construction, modifications, or alterations of any buildings, structures or other improvements" on the property. The Site Note must be modified to reference the Development Plan, not the Final Construction Plans.
- The Title of the Plan Set needs to be Cornerstone Charter School CONCEPT Plan, not Development Plan. Consistency with Code verbiage is important to eliminate any confusion in the future.

Please note that the correct Ordinance number should be read as 23-02 and the following corrections have been agreed upon and made to the ordinance to reflect the following,

G. Intensity. There shall be a maximum of floor area ratio of 0.45; provided however, the building square footage developed on the Property shall be consistent with the Concept Plan.

H. Maximum Impervious Surface. The maximum impervious surface area shall be 0.7.

Staff Recommendation: Approve the amendment as recommended by the P&Z Board.

Suggested Motion: <u>I move that we read Ordinance 23-02 for the first time and post for Second</u> <u>Reading and Adoption on May 2, 2023.</u>

Alternatives:	Do not approve.
Fiscal Impact:	None
Attachments:	Ordinance

a.

ORDINANCE 23-01-02

AN ORDINANCE OF THE CITY OF BELLE ISLE, FLORIDA REZONING THOSE CERTAIN PROPERTIES OWNED BY THE CITY OF BELLE ISLE LOCATED AT 5903 RANDOLPH AVENUE. 906 WALTHAM AVENUE. 6300 HANSEL AVENUE, "SUB OF HARNEY HOMESTEAD C/53 LOT 9 (LESS W 224.28 FT OF LOT 9 & LESS E 228.47 FT OF SAID LOT 9 & LESS R/W ON N & S)", AND 6049 RANDOLPH AVENUE, ALSO KNOWN AS ORANGE COUNTY TAX PARCELS # 24-23-29-3400-00-073, 24-23-29-3400-00-092, 24-23-29-3400-00-094, 24-23-29-3400-00-095, AND 24-23-29-3400-00-114, FROM C-1, C-2, R-1A, AND R-2 ZONING DISTRICTS TO PLANNED DEVELOPMENT DISTRICT (PD); REZONING THAT CERTAIN PROPERTY OWNED BY CORNERSTONE CHARTER ACADEMY, INC. LOCATED AT 5929 HANSEL AVENUE, ALSO KNOWN AS ORANGE COUNTY TAX PARCEL #24-23-29-8820-00-050, FROM C-1 ZONING DISTRICT TO PLANNED DEVELOPMENT DISTRICT (PD); REZONING THAT CERTAIN PROPERTY OWNED BY PINE CASTLE METHODIST CHURCH, INC. LOCATED AT 942 FAIRLANE AVENUE, ALSO KNOWN AS ORANGE COUNTY TAX PARCEL #24-23-29-3400-00-093, FROM C-1 ZONING DISTRICT TO PLANNED DEVELOPMENT DISTRICT (PD): PROVIDING FOR CERTAIN PERMITTED, PROHIBITED AND SPECIAL EXCEPTION USES AND DEVELOPMENT STANDARDS REQUIREMENTS WITHIN THE PLANNED DEVELOPMENT: AND DESCRIBING THE PLANNED DEVELOPMENT INCLUDING ALL THE AFORESAID PROPERTIES AS THE CORNERSTONE CHARTER ACADEMY PD; PROVIDING FOR SEVERABILITY AND AN EFFECTIVE DATE.

WHEREAS, Cornerstone Charter Academy, Inc. (hereinafter "Developer") with the consent of the City of Belle Isle and Pine Castle Methodist Church, Inc. has made application for the rezoning of (i) those certain properties owned by the City of Belle Isle located at 5903 Randolph Avenue, 906 Waltham Avenue, 6300 Hansel Avenue, and 6049 Randolph Avenue, also known as Orange County Tax Parcels *#* 24-23-29-3400-00-073, 24-23-29-3400-00-092, 24-23-29-3400-00-094, 24-23-29-3400-00-095, and 24-23-29-3400-00-114, from C-1, C-2, R-1A, and R-2 zoning districts to Planned Development District (PD); (ii) that certain property

²⁵ owned by Cornerstone Charter Academy, Inc. located at 5929 Hansel Avenue, also known as

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Orange County Tax Parcel #24-23-29-8820-00-050, from C-1 zoning district to Planned Development District (PD); and (iii) that certain property owned by Pine Castle Methodist Church, Inc. located at 942 Fairlane Avenue, also known as Orange County Tax Parcel #24-23-29-3400-00-093, from C-1 zoning district to Planned Development District (PD) and legally described in the boundary survey attached hereto as **Exhibit "A"** (hereinafter "the Property"); and

WHEREAS, the Property has Low-Density Residential and Commercial Future Land Use Map designations; and

WHEREAS, the Planning and Zoning Board of the City of Belle Isle has reviewed the Developer's request at a public hearing and has made a recommendation to the City Council; and

WHEREAS, after public notice and due consideration of public comment, the City Council of the City of Belle hereby finds and declares the adoption of this Ordinance and the proposed development of the Property is consistent with the City of Belle Isle Comprehensive Plan and the land development regulations set forth in the City of Belle Isle Code of Ordinances; and

WHEREAS, based on competent substantial evidence in the record, the requested rezoning and preliminary concept plan set forth in this Ordinance meets all applicable criteria specified in the City of Belle Isle Comprehensive Plan and the land development regulations set forth in the Code of Ordinances; and

WHEREAS, this Ordinance and the Whereas clauses herein shall constitute the written findings of fact in support of issuing this rezoning development order pursuant to general law.

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NOW, THEREFORE, BE IT ORDAINED by the City Council of the City of Belle Isle, Florida as follows:

SECTION 1. The Property described above and in **Exhibit** "**A**" attached hereto is hereby rezoned from Retail Commercial District (C-1), General Commercial District (C-2), Single-Family Dwelling District (R-1A), and Multiple-Family Dwelling (R-2) zoning districts to Planned Development (PD) subject to the following conditions and restrictions:

A. *Concept Plan.* The Cornerstone Charter Academy Preliminary Concept Plan attached hereto as **Exhibit "B"** ("Concept Plan") is hereby approved. All development of the Property must conform to Concept Plan, including the development standards and requirements identified on the Concept Plan. Should any conflict be found between this Ordinance and the Concept Plan then the standards and conditions established by this Ordinance shall control.

B. *Code Provisions*. Unless specifically noted elsewhere in the Concept Plan attached hereto, or expressly provided for herein, all development on the Property must comply with the general zoning requirements of the Planned Development zoning district. Such requirements include any approval or amendment procedures pertaining to the Planned Development zoning district. The Land Development Code of the Belle Isle Code of Ordinances, as amended, shall govern the development of the Property with respect to any matter not addressed by this Ordinance or its attached exhibits.

- C. Permitted Uses. The permitted uses for the Property are as follows: (i) Elementary School,
 (ii) Middle School, (iii) High School, (iv) Other Community or Educational Meeting Space and Facilities, (v) Active and Passive Recreational and Sports Fields, Gymnasiums and Other Facilities, (vi) Open Space and Parks, (vii) Stormwater Ponds and Facilities, (viii)

Preservation of Historic Home (not for residential use) shown on the Concept Plan and use thereof for meeting or educational purposes; and (viii) the customary accessory uses and structures associated with the foregoing permitted uses. All uses that are not permitted uses or special exception uses as set forth in this Ordinance are prohibited.

- D. Special Exception Uses. The special exception uses for the Property are as follows: (i) Municipal and Governmental Facilities; (ii) uses that are substantially similar and compatible with the permitted uses of this Ordinance and that are not incompatible with surrounding area as determined by the City Council; and (iii) the customary accessory uses and structures associated with the foregoing special exception uses. If a special exception use is approved based on a finding by the City Council that such meets the applicable criteria set forth in the City Code, the approval may be conditioned and restricted.
- E. *Maximum Building Height*. The maximum height of structures shall be four stories and no more than 57 feet; provided however, each building constructed shall be consistent with the heights shown on the Concept Plan.
- F. Setbacks. The setbacks shall be consistent with those depicted on the Concept Plan.
- G. Intensity. There shall be a maximum of floor area ratio of 0.45; provided however, the building square footage developed on the Property shall be consistent with the Concept Plan.
- H. Maximum Impervious Surface. The maximum impervious surface area shall be 0.7.
- I. Architectural Features. The Property shall be developed consistent with the architectural elevations and concepts set forth in the Concept Plans.

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J. Vehicular Parking/Ingress/Egress. The vehicular parking and ingress and egress to and from the Property shall be developed and maintained consistent with the Concept Plans.

- K. *Lighting*. Lighting on the Property shall be development consistent with the lighting plan set forth in the Concept Plans.
- L. *Landscaping, Landscaping Buffers, Recreation and Open Space*. The Property shall be developed and maintained to preserve recreational area, open space, landscaping and landscaping buffers as depicted on the Concept Plan.
- M. *Planned Development Name*. The Planned Development adopted by this Ordinance governing the development of the Property will be known as the "Cornerstone Charter Academy PD."
- N. Development Plan/PD Expiration. Developer shall submit a development plan for review and approval pursuant to the provisions of Chapter 54, Section 54-77 (e) (4) of the Land Development Code of the Belle Isle Code of Ordinances. If Developer fails to obtain final approval of said development plan within one year of the Effective Date of this Ordinance, the entitlements under this Ordinance shall become null and void and the zoning classification of the Property shall revert to its previous zoning designations or other appropriate zoning designations as determined by the City Council in accordance with the provisions of Section 54-77 (e) (4), Land Development Code of the Belle Isle Code of Ordinances.
- O. Violation. A violation of this Ordinance or any of its Exhibits is considered a violation of the Land Development Code of the Belle Isle Code of Ordinances and zoning of the Property.

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ORD 23-0102 - 5 OF 7

a.

SECTION 2. Zoning Map. The City Manager is hereby authorized and directed to amend the Official City of Belle Isle Zoning Map consistent with the provisions of this Ordinance.

SECTION 3. Severability. If any word, phrase, sentence, clause or other portion of this Ordinance is determined to be invalid, void or unconstitutional, the remainder of this Ordinance shall remain in effect.

SECTION 4. Effective date. This Ordinance shall take effect immediately upon adoption.

First Reading was held this 18th day of April 2023

Second Reading held this 2nd day of May 2023

5			YES	N	0	4	ABSENT
:	Ed Gold						
,	Anthony Carug	jno					
5	Karl Shuck			_		-	
,	Randy Holihan	۱.		_		-	
:	Beth Lowell			_		-	
)	Stanley Smith			_		-	
)	Jim Partin					-	
	CITY OF BELL	LE ISLE					
	NICHOLAS FO	DURAKER, MA	YOR				
5	ATTEST:			_			
:		Yolanda Quicer	no, CMC-City Clerk				
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1	STATE OF FLORIDA
2	COUNTY OF ORANGE
3	I, Yolanda Quiceno, CITY CLERK of the City of Belle Isle, do hereby certify that the above and
4	foregoing document ORDINANCE 23-01 02 was duly and legally passed by the Belle Isle City Council,
5	in session assembled on the day of2023. At this session, a quorum of its members
6	was present.
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8	Yolanda Quiceno, CMC-City Clerk
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	ORD 23- 01 02 - 7 OF 7 0927036\169750\5545890v3

a.

CITY OF BELLE ISLE, FLORIDA CITY COUNCIL AGENDA ITEM COVER SHEET

Meeting Date:	April 18, 2023
То:	Honorable Mayor and City Council Members
From:	T. Grimm, Interim City Manager
Subject:	Reimbursement of Travel Fees for Candidate Howard Brown

Background: On March 20, 2023, the City Council scheduled a Special Called meeting for candidates who applied for the Interim City Manager position. The top 4 selected candidates were called at this meeting and confirmed their availability. At the March 7, 2023, City Council meeting, the Council motioned that Reasonable expenses may be allowed.

On April 11, 2023, the staff received a request from Howard Brown requesting reimbursement of \$232.93 (per 2023 IRS-millage and tolls) as follows,

- Palm Beach Garden to City of Belle Isle 320 miles (209.60)
- Sun Pass Tolls (23.23)

Staff Recommendation: The staff recommendation is to discuss the request and decide if the requests of Mr. Brown are reasonable. The City Attorney will also give an opinion on this matter.

Suggested Motion: <u>I move that we reimburse Howard Brown for his mileage and tolls of</u> <u>\$232.93.</u>

Alternatives: Do not pay them or pay some of the request.

Fiscal Impact: \$232.93

Attachments: Request and receipts dated April 5, 2023

Howard W. Brown, Jr.

11875 Dunbar Court Palm Beach Gardens, FL 33412 (305) 788-9647 howardwbrownjr@gmail.com

April 5, 2023

City of Belle Isle Attn: Interim City Manager 1600 Nela Avenue Belle Isle, FL 32809

RE: Request for Reimbursement for Interim City Manager Interview on March 20, 2023 OF \$232.93

Dear Mr. Interim Manager:

It was a pleasure to meet the City Council at the interview. While I was not chosen, I am sure the City Council will be in excellent hands. I did enjoy seeing the beautiful City of Belle Isle.

I am respectfully requesting \$232.93 reimbursement for (2023 IRS) mileage and tons:

- Palm Beach Gardens to City of Belle Isle 320 miles (\$209.60)
- Sunpass Tolls (\$23.33)

Should you have any questions, please do not hesitate to contact me directly.

Respectfully submitted,

Howard W. Brown,

Applicant

C: City Clerk

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/Enclosure – sunpass print out and mapquest directions

11875 Dunbar Ct to Belle Isle City Hall (City of Belle Isle Florida)

2 hr 28 min

160.3 miles

IRS reimbursement:

\$93.75

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Head toward Marlamoor Ln on Dunbar Ct. Go for 509 ft.

Then 0.10 miles

Turn left onto Marlamoor Ln. Go for 0.3 mi.

Then 0.34 miles

Turn left onto Bay Hill Dr. Go for 0.2 mi.

Then 0.15 miles

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Turn right onto Northlake Blvd (CR-809A). Go for 4.2 mi.

Then 4.25 miles

Turn right onto Beeline Hwy (SR-710). Go for 1.2 mi.

Then 1.20 miles

Turn left onto N Jog Rd toward Florida's Turnpike-TOLL. Go for 0.2 mi.

Then 0.23 miles

Turn right toward Florida's Turnpike-TOLL. Go for 0.2 mi.

Keep left toward Florida's Turnpike North. Go for 0.1 mi

Then 0.14 miles

Turn left and take ramp onto Florida's Tpke. Go for 148 mi.

Then 148.18 miles

Take exit 254 onto US-17 N/US-92 E/US-441 N (S Orange Blossom Trl). Go for 1.0 mi.

Then 0.96 miles

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Turn right onto W Landstreet Rd (CR-527A) toward SR-528-TOLL E. Go for 2.3 mi.

Then 2.26 miles

Turn left onto S Orange Ave (CR-527). Go for 1.8 mi.

Then 1.83 miles

Turn right onto Nela Ave. Go for 0.3 mi.

Then 0.31 miles

Keep left onto Nela Ave. Go for 0.1 mi.

Then 0.10 miles

Turn right. Go for 56 ft.

Then 0.01 miles



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4/5/23,6	02 PM			SunPass	Activity		
	Posted Date	Transaction Date	Transaction Time	Transponder/ License Plate	Description	Debit Credit (-) (+)	Balanc ^{b.}
03/	20/2023	03/20/2023	11:55:36 PM		PAYMENT & ADJUSTMENTS	\$25.00	\$33.87 View Receipt
03/	20/2023	03/20/2023	09:25:48 PM	028837170110	SR91 BECKER RD MAIN SB MP138	\$0.40	\$8.87 View Receipt
03/	20/2023	03/20/2023	09:22:51 PM	028837170110	SR91 PT ST LUCIE MAIN SB MP141	\$0.32	\$9.27 View Receipt
03/	20/2023	03/20/2023	09:29:44 PM	028837170110	SR91 STUART MAIN SB MP133	\$1.36	\$9.59 View Receipt
03/	20/2023	03/20/2023	09:16:21 PM	028837170110	SR91 MIDWAY RD MAIN SB MP150	<mark>\$0.8</mark> 0	\$10.95 View Receipt
03/	20/2023	03/20/2023	09:12:56 PM	028837170110	SR91 FT PIERCE MAIN SB MP154	\$3.27	\$11.75 View Receipt
03/	20/2023	03/20/2023	08:04:47 PM	028837170110	SR91 THREE LAKES MAIN SB MP236	\$3.82	\$15.02 View Receipt
03/	20/2023	03/20/2023	07:43:30 PM	028837170110	SR91 KISSIMMEE/ST CLOUD MP244	\$1.34	\$18.84 View Receipt
03/	20/2023	03/20/2023	04:15:17 PM	028837170110	SR528 BCHLINE WEST MAIN EB MP6	\$ <mark>2.03</mark>	\$20.18 View Receipt
03/	20/2023	03/20/2023	03:37:24 PM	028837170110	SR91 THREE LAKES MAIN NB MP236	\$3.82	\$22.21 View Receipt
03/2	20/2023	03/20/2023	02:31:50 PM	0, ,	SR91 FT PIERCE MAIN NB MP154	\$3.27	\$26.03 View Receipt
03/:	20/2023	03/20/2023	02:28:03 PM	0, ,	SR91 MIDWAY RD MAIN NB MP150	\$0.80	\$29.30 View Receipt
03/:	20/2023	03/20/2023	02:17:54 PM	0, ,	SR91 BECKER RD MAIN NB MP138	<mark>\$0.40</mark>	\$30.10 View Receipt
03/:	20/2023	03/20/2023	02:21:04 PM	028837170110	SR91 PT ST LUCIE MAIN NB MP141	\$0.32	\$30.50 View Receipt
03/2	20/2023	03/20/2023	02:12:57 PM	028837170110	SR91 STUART MAIN NB MP133	\$1.36	\$30.82 View Receipt
03/2	20/2023	03/20/2023	01:56:47 PM	0, ,	SR91 JUPITER	\$0.56	\$32.18 View Rec 22
https://ww	w.sunpass.com	/vector/account/transact	ions/webtransactionSea				3/18

4	//5/23, 6:02 PM			SunPass	: Activity		
	Posted Date	Transaction Date	Transaction Time	Transponder/ License Plate	Description	Debit Credit (-) (+)	Balanc ^{b.}
					MP113		
	03/20/2023	03/20/2023	01:53:02 PM	028837170110	SR91 PGA BLVD MAIN NB MP108	\$ 0.2 5	\$32.74 View Receipt
	03/20/2023	03/20/2023	12:15:10 PM	028837170110	SR91 45TH STREET MAIN NB MP104	\$0.64	\$32.99 View Receipt
	03/20/2023	03/20/2023	12:07:46 PM	028837170110	SR91 FOREST H BLV MAIN NB MP96	\$0.32	\$33.63 View Receipt
	03/20/2023	03/20/2023	12:10:02 PM	028837170110	SR91 BELVEDERE RD MAIN NB MP98	\$0.25	\$33.95 View Receipt
	03/20/2023	03/20/2023	11:57:47 AM	028837170110	SR91 LANTANA MAIN NB MP88	\$0.56	\$34.20 View Receipt
	03/20/2023	03/20/2023	11:33:47 AM	028837170110	SR91 POMPANO BCH MAIN NB MP65	\$1.07	\$34.76 View Receipt
	03/20/2023	03/20/2023	11:17:13 AM	028837170110	SR91 DOLPHIN CENTER NB ON MP2	\$0.80	\$35.83 View Receipt
	03/20/2023	03/20/2023	06:40:54 AM	028837170110	SR91 DOLPHIN CENTER SB OFF MP2	\$0.80	\$36.63 View Receipt
	03/20/2023	03/20/2023	06:25:04 AM	028837170110	SR91 POMPANO BCH MAIN SB MP65	\$1.07	\$37.43 View Receipt
	×		<u>1</u>	<u>2 Next Page Last</u>	<u>Page</u>		



- Account Number: 8688142
- Transaction Date/Time: 03/20/2023 11:55:36 PM
- Transaction Amount: \$25.00
- Payment Reference Number : 2486544538
- Payment Type: VISA
- Account ending with : 0025
- Balance: \$33.87

Description Amount

VISA REPLENISHMENT \$25.00

Print Close



Receipt

- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 BECKER RD MAIN SB MP138
- Date: 03/20/2023
- Time: 09:25:48 PM
- Transaction Amount: (\$0.40)
- Transaction Reference Number : 37724251277
- Transaction Type : Transponder Toll
- Balance: \$8.87



Receipt

- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 PT ST LUCIE MAIN SB MP141
- Date: 03/20/2023
- Time: 09:22:51 PM
- Transaction Amount: (\$0.32)
- Transaction Reference Number : 37724243062
- Transaction Type : Transponder Toll
- Balance: \$9.27



- Account Number: 8688142
- **Posted Date:** 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 STUART MAIN SB MP133
- Date: 03/20/2023
- Time: 09:29:44 PM
- Transaction Amount: (\$1.36)
- Transaction Reference Number : 37724224122
- Transaction Type : Transponder Toll
- Balance: \$9.59



Receipt

- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 MIDWAY RD MAIN SB MP150
- Date: 03/20/2023
- Time: 09:16:21 PM
- Transaction Amount: (\$0.80)
- Transaction Reference Number: 37724202696
- Transaction Type : Transponder Toll
- Balance: \$10.95

Print Close



- Account Number: 8688142
- **Posted Date:** 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 FT PIERCE MAIN SB MP154
- Date: 03/20/2023
- Time: 09:12:56 PM
- Transaction Amount: (\$3.27)
- Transaction Reference Number : 37724179227
- Transaction Type : Transponder Toll
- Balance: \$11.75

Print Close
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SUN PASS

Receipt

- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 THREE LAKES MAIN SB MP236
- Date: 03/20/2023
- **Time:** 08:04:47 PM
- Transaction Amount: (\$3.82)
- Transaction Reference Number : 37724038727
- Transaction Type : Transponder Toll
- Balance: \$15.02

Print	Close
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- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 KISSIMMEE/ST CLOUD MP244
- Date: 03/20/2023
- Time: 07:43:30 PM
- Transaction Amount: (\$1.34)
- Transaction Reference Number : 37723964251
- Transaction Type : Transponder Toll
- Balance: \$18.84



Receipt

- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR528 BCHLINE WEST MAIN EB MP6
- Date: 03/20/2023
- Time: 04:15:17 PM
- Transaction Amount: (\$2.03)
- Transaction Reference Number : 37721509141
- Transaction Type : Transponder Toll
- Balance: \$20.18

Print	Close
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- Account Number: 8688142
- **Posted Date:** 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 THREE LAKES MAIN NB MP236
- Date: 03/20/2023
- Time: 03:37:24 PM
- Transaction Amount: (\$3.82)
- Transaction Reference Number: 37721280887
- Transaction Type : Transponder Toll
- Balance: \$22.21



Receipt

- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 FT PIERCE MAIN NB MP154
- Date: 03/20/2023
- Time: 02:31:50 PM
- Transaction Amount: (\$3.27)
- Transaction Reference Number: 37720780370
- Transaction Type : Transponder Toll
- Balance: \$26.03

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- Account Number: 8688142
- **Posted Date:** 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 MIDWAY RD MAIN NB MP150
- Date: 03/20/2023
- Time: 02:28:03 PM
- Transaction Amount: (\$0.80)
- Transaction Reference Number : 37720720781
- Transaction Type : Transponder Toll
- Balance: \$29.30



- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 BECKER RD MAIN NB MP138
- Date: 03/20/2023
- Time: 02:17:54 PM
- Transaction Amount: (\$0.40)
- Transaction Reference Number: 37720691145
- Transaction Type : Transponder Toll
- Balance: \$30.10

Print	Close
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- Account Number: 8688142
- **Posted Date:** 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 PT ST LUCIE MAIN NB MP141
- Date: 03/20/2023
- Time: 02:21:04 PM
- Transaction Amount: (\$0.32)
- Transaction Reference Number : 37720689033
- Transaction Type : Transponder Toll
- Balance: \$30.50



- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 STUART MAIN NB MP133
- Date: 03/20/2023
- Time: 02:12:57 PM
- Transaction Amount: (\$1.36)
- Transaction Reference Number : 37720678658
- Transaction Type : Transponder Toll
- Balance: \$30.82

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- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 JUPITER MAIN NB MP113
- Date: 03/20/2023
- Time: 01:56:47 PM
- Transaction Amount: (\$0.56)
- Transaction Reference Number: 37720545144
- Transaction Type : Transponder Toll
- Balance: \$32.18



- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 PGA BLVD MAIN NB MP108
- Date: 03/20/2023
- Time: 01:53:02 PM
- Transaction Amount: (\$0.25)
- Transaction Reference Number : 37720524797
- Transaction Type : Transponder Toll
- Balance: \$32.74

Print	Close
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- Account Number: 8688142
- **Posted Date:** 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 45TH STREET MAIN NB MP104
- Date: 03/20/2023
- Time: 12:15:10 PM
- Transaction Amount: (\$0.64)
- Transaction Reference Number : 37720036883
- Transaction Type : Transponder Toll
- Balance: \$32.99



- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 FOREST H BLV MAIN NB MP96
- Date: 03/20/2023
- Time: 12:07:46 PM
- Transaction Amount: (\$0.32)
- Transaction Reference Number : 37719927608
- Transaction Type : Transponder Toll
- Balance: \$33.63

Print	Close
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- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 BELVEDERE RD MAIN NB MP98
- Date: 03/20/2023
- Time: 12:10:02 PM
- Transaction Amount: (\$0.25)
- Transaction Reference Number: 37719882582
- Transaction Type : Transponder Toll
- Balance: \$33.95



- Account Number: 8688142
- Posted Date: 03/20/2023
- Transponder Number/License Plate: 028837170110
- Agency Name : Florida Turnpike Enterprise
- Axle: 2
- Location: SR91 LANTANA MAIN NB MP88
- Date: 03/20/2023
- Time: 11:57:47 AM
- Transaction Amount: (\$0.56)
- Transaction Reference Number : 37719861168
- Transaction Type : Transponder Toll
- Balance: \$34.20

Print	Close
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- Search the FAQs...
- EN
 - ENGLISH
 - <u>ESPAÑOL</u>
 - Hi,HOWARD
 - ACCOUNT OVERVIEW
 - <u>LOGOUT</u>
- <u>MY SUNPASS</u>
 - Overview
 - Account Profile
 - Add Funds
 - <u>Manage Payment Methods</u>
 - <u>Transponders and Vehicles</u>
 - SunPass Plus Parking
 - <u>Plans</u>
 - <u>Activity</u>
 - <u>Statements and</u> <u>Correspondence</u>
 - Pay Toll Enforcement Invoice
 - Linked Accounts
 - <u>Customer Support</u>
- SUNPASS
 - About SunPass
 - SunPass PRO
 - Where to purchase SunPass
 - SunPass Plus Parking
 - How to Add Funds to Your Account
 - User Manual
- TRAVELER INFORMATION
 - <u>Tolls</u>
 - <u>Rental Vehicles</u>
 - Florida Managed Lanes
 - Contact Us
- UNPAID TOLLS
 - <u>Toll Enforcement Invoice</u>
 - Pay Collection Notice
 - Vehicle Registration Stop
 - File A Dispute

Activity

- <u>Home</u>
- <u>My SunPass</u>
- Activity

4/5/23, 6:02 PM

ACCOUNT # 8688142

- <u>Overview</u>
- Account Profile
- <u>Add Funds</u>
- <u>Manage Payment Methods</u>
- <u>Transponders and Vehicles</u>
- SunPass Plus Parking
- <u>Plans</u>
- <u>Activity</u>
- <u>Statements and Correspondence</u>
- <u>Pay Toll Enforcement Invoice</u>
- Linked Accounts
- <u>Customer Support</u>

Activity

Account activity can be accessed for a period of up to 90 days, and may be viewed or downloaded for the past 36 months.

Note: When viewing or downloading large volumes of transactions, it is advised that the search parameters be limited to the shortest time period necessary.

Filter By			
All		\mathbf{v}	
Date Type	~		
Posted Date	×		
Start Date		-4) 	
03/20/2023			
End Date			
03/20/2023			
Transponder Nu	mber		
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Transponder Nu Plate Number Agency Name			
Plate Number Agency Name			
Plate Number			

Export: PDF | Spreadsheet

VIEW

Note: Transponder Number and License Plate Number are optional. If left blank all transactions on your account for the date range selected will be returned.

1 2 Next Page Last Page

h.

CITY OF BELLE SLE, FLORIDA CITY COUNCIL AGENDA ITEM COVER SHEET

Meeting Date:	April 18, 2023
То:	Honorable Mayor and City Council Members
From:	T. Grimm, Interim City Manager
Subject:	BING Grant

Background: The Winward on Lake Conway (District 5) is applying for a BING Grant for surveillance cameras. The proposal has been developed with the Belle Isle Police Department coordination. The proposed project will work similarly to the surveillance cameras installed at Belle Isles' own LPR network and will enhance the safety of all residents. The project is \$15,227, with a grant request of \$7,000. The grant meets the criteria, and District 5 has \$7,000 in grant funds. To fund the entire grant amount, other Commissioners may contribute funds to this project.

Staff Recommendation: Approve the grant and determine if it will be funded in full (\$15,2270) or partially (\$7,000) if other Commissioners do not provide funding.

Suggested Motion:

- a. I move to approve the BING Grant for The Windward on Lake Conway for the surveillance cameras project in the grant amount of \$7,000.
- Alternatives: Do not approve.
- Fiscal Impact: \$7,000
- Attachments: Grant application



April 13, 2023

с.

Belle Isle City Council 1600 Nela Avenue Belle Isle, FL 32809

Dear Council Members,

Windward on Lake Conway is a condominium development located on the southeast corner of the south lake of the Lake Conway chain. The community was established through its Declaration and construction in 2008, with occupancy starting in 2012.

Windward includes 29 three-bedroom individually owned units, spread across four resident buildings. Each of these four buildings has a set of garages, with additional resident or guest parking. There is a pool cabana with restrooms available to all residents.

One the lake front, Windward has its own boat launch, which has been used by Orange County Fire and Rescue to support an emergency unrelated to our community. The community has two docks with 9 boat slips.

Attached find a package seeking council approval for a BING grant to support purchase of surveillance cameras at Windward. This proposal has been developed with Belle Isle Police Department coordination. As always, content generated from Windward's own surveillance cameras will be available to BIPD upon request.

The Windward surveillance camera BING request is presented as a complement to Belle Isle's own LPR camera network. Windward would be pleased to be a test case for the use of LPR surveillance at private community entrances in Belle Isle.

Together – Windward's own surveillance camera installation and Belle Isle's own LPR network – we will work for enhanced safety of all residents in Belle Isle.

Thank you for this opportunity to work with the City of Belle Isle.

With Regards,

Dennis Tierney Project Manager – Sarveillance Cameras Windward on Lake Conway Condominiums

Cc: Windward Board



CITY OF BELLE ISLE NEIGHBORHOOD PRIDE GRANTS

Grant Application

Submit the original application along with any attachments to The City of Belle Isle, 1600 Nela Avenue, Belle Isle FL 32809. Grants will be awarded on a first come, first served basis by district. (PLEASE PRINT)

	Ap	oplicant Contact Info	ormation
Applicant Organization Name:	WINDWARD O	N LAKE CONWAY	
Project Contact Name:	DENNIS TIERNEY / WINDWARD BOARD		
Mailing Address:	7836 HOLIDAY	ISLE CIRCLE, UNIT 301	
· · ·	BELLE ISLE, FL_		32812
	City, State		Zip
Daytime Phone:	917 288 7032_		Evening Phone: 917 288 7032
Email:	TIERNEDA@miamioh.edu		
	ALTER	NATIVE CONTACT IN	FORMATION
Alternate Contact Name: SHER	RY KENNEDY	STAN KACZMAREK	RALPH ROCHEFORD
Daytime Phone: 407 8	08 2877	407 694 5824	941 232 8309
Email: <u>SHERRYAKENNE</u>	DY@YAHOO.COM	STANKZ@AOL.COM	r99venice@verizon.net
		GRANT INFORMAT	
Type of Project — please select a			
o Landscaping o Reade	Board	o	Fountains
Sign o Ground Lighting	I	Х	Other SURVEILLANCE CAMERA INSTALLATION

Wall/Fence pressure washing and or

painting o Irrigation "Repairs"

C Other SURVEILLANCE CAMERA INSTALLATION Project Street Address or Nearest Intersection: 7818 HOLIDAY ISLE CIRCLE, BELLE (NEAR DAETWYLER/TRENTWOOD)____

- o Total amount of project: \$15,227*_____
- o Grant amount requested: \$7,000____
- Neighborhood participation amount (remainder of invoice) \$8,227_____

* The Windward community has a quote from SecurityPro for 10 cameras at \$15,227. We are in late discussion with one additional vendor and would accept a bid which is equal (or better) in coverage and lower (or equal) in cost. In any case, the community seeks BING grants of 50% funding for this initiative – capped as necessary at the \$7,000 shown above.

PROJECT INFORMATION

Please provide the answers to the following questions.

 <u>Description of the Project.</u> - This summary should provide an overview of the entire project; include what improvements will be constructed, installed, or applied. Remember to demonstrate the need for the project. The Daetwyler Road area of Belle Isle has been subjected to burglary incidents. Cars parked at Windward Condominiums were broken into in early 2022. Current surveillance – using uncoordinated and unreliable DIY solutions – have been inadequate to either deter the crime or assist with investigating crimes. The association will purchase a professionally installed series of 10 standard cameras. These will augment Belle Isle's LPR (License Plate Reader) camera network (see WINDWARD BING 2). All of Windward's cameras will be focused on the parking/driveway area of the property or the fenced security perimeter to the north and south.

BIPD will be provided access to content from surveillance cameras upon request for their own investigations.

- 2. <u>State the location and land ownership of the proposed project</u> Is the project on public property? (Right-of-way use agreement/permit will be required.) Please state the exact location of the project, including an address or cross streets. The project is located on private property, though some camera views will capture activity outside of the gates of the condominium community. The community is located at 7818 Holiday Isle Drive, just north of the intersection of Daetwyler Drive and Trentwood Dr. There are 29 units with 29 separate owners at Windward Condominiums.
- 3. Attach 2-5 photos, and include a brief description of each photo. Please also provide the original color photos.
- 4. Project Maintenance: Describe how the property has been maintained in the past, and how the project will be maintained and by whom after it is completed.

The property has used home-wired Nest cameras for limited surveillance, which has been unreliable in service and unsatisfactory in quality.

5. <u>Describe why this project is important to the community</u>. Provide a brief summary of how the project will enhance the quality of life in the community. How will this project empower your organization to work together to accomplish common goals and objectives? (i.e., to improve neighborhood communication and participation).

The project will provide owners and residents with greater security, preventing criminal activity because of the surveillance presence. This BING request ("Windward BING 1") is part 1 of a two-part package of funding, where Windward will install its own surveillance camera network plus, (in separate request "Windward BING 2") support BIPD installation of LPR cameras at Windward Condominiums as part of its overall Belle Isle LPR network. Together, this enhanced surveillance will support the greater Belle Isle community with deterrence and support for BIPD investigations. Potential notification of bad actors entering the Windward Condominium compound will help law enforcement across our Belle Isle community.

BING TEAM ROSTER

Each organization is required to have at least a 3 to 5 member team who will help plan and implement your community project. Team members will be required to sign the team member roster as a part of the grant application. Each team member must indicate his or her role/responsibility on the team.

PRINT NAME & SIGNATURE	ADDRESS/PHONE/EMAIL	ROLE/RESPONSIBILITY
Print Dennis Tierney		Project Manager
Signature		Application Writer
Print Sherry Kennedy		Board Member (President)
Signature		14
Print Ralph Rocheford		Asst Project Manager
Signature		Board Member (Secretary/Treasurer)
Print Stan Kaczmarek		Board Member (Vice President)
Signature		
Print Trapper Martin		Board President Emeritus
Signature		

TEAM ROLES: <u>PROJECT MANAGER</u>, — Team Captain. Responsible for leading project, getting a group consensus on which project the group wants to pursue. <u>ASST PROJECT MANAGER</u> — Co-Captain. Will work in concert with the project manager and assist obtaining quote(s) once the project idea has been decided upon. This position can also serve as the "Fund Watcher monitoring project expenses. <u>APPLICATION WRITER</u> — will work with project manager in organizing and developing BING application and submitting final report and pictures upon completion of project.

BELLE ISLE NEIGHBORHOOD GRANTS (BING)

BUDGET AND GRANT REQUEST			
NAME OF BUSINESS	TOTAL COST	DESCRIPTION OF SERVICES	
SecurityPro Florida	\$15,227	Provide equipment: 1 network video recorder/controller;10 fixed lens turret camera;Installation: establish communication links betweencontroller and cameras, install electric access as needed,install controller and 10 cameras, cameras housed inNema enclosureLicense Plate Recognition (LPR) camera(s), by separateapplication, augment this request.	
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TOTAL AMOUNT OF PROJECT	\$15,227	c
GRANT AMOUNT REQUESTED	\$ 7,613	
NEIGHBORHOOD PARTICIPATION AMOUNT (REMAINDER OF INVOICE)	\$ 7,614	

Location of security cameras



Installation includes:

- Three cameras on a north tower (photo, left) pointing east and west along property perimeter, and south across community parking and drive

- Three cameras on a south tower (photo, right) pointing east and west along property perimeter including boat ramp, and north across community parking and drive; in the lower-cost option, this tower is replaced by a camera on the poolside building focused on the south perimeter

- Three cameras across from entrance gate (photo, top) pointing north and south across community/visitor parking and east at the exit gate

- One BIPD-controlled LPR camera is shown with green arrow installed on entrance security pillar, and one near the poolside building capturing the exit gate (separate BING request)

- Buildings A, B, C and D are resident buildings, with 9, 6, 9 and 5 units respectively. Each resident building has a corresponding adjacent garage structure

- The equipment controller will be housed in the pool equipment room, located in the southeast corner of the pool cabana (to the right in photo, or south, of the pool itself)



The majority of cameras planned for Windward on Lake Conway's surveillance needs are the LTS[®]CMHT1752WE-28F 5 Megapixel HD fixed lens, matrix IR camera.

CITY OF BELLE SLE, FLORIDA CITY COUNCIL AGENDA ITEM COVER SHEET

Meeting Date:	April 18, 2023
То:	Honorable Mayor and City Council Members
From:	T. Grimm, Interim City Manager
Subject:	Adopted BIPD Pay Plan Stipend for Interim City Manager, Acting Chief, and Deputy Chief

Background: At the April 04, 2023, City Council meeting, Commissioner Gold asked the Council to consider compensation for out-of-classification work for the Interim City Manager, Acting Chief, and Acting Deputy Chief.

Staff is asking the Council to adopt an updated Belle Isle Police Department (BIPD) Pay Plan and remove the Deputy Chief position from the BIPD Pay Plan based on the recent changes in the department—the staff request approval for the entry-level Deputy Chief of \$96,194.80. The DC position may receive those cost-of-living salary adjustments and other benefit increases which may be granted to other employees.

On April 04, 2023, the Interim City Manager assigned Deputy Chief Millis to Acting Chief of Police. During this time of the transaction, I am asking for a ten percent salary increase for this position until he is reassigned to Deputy Chief.

On April 05, 2023, Sgt. McCormick was appointed Acting Deputy Chief, per the BIPD Pay Plan. Sgt. McCormick will receive the entry level of Deputy Chief until he is reassigned to the position of Sgt.

Staff Recommendation: Adopt the updated PD Play Plan and approve a temporary stipend for Acting Chief and Acting Deputy Chief and Council to determine temporary compensation for the Interim City Manager.

Suggested Motion:

I move to adopt the Belle Isle Police Department Pay Plan effective April 4, 2023, as follows,

- Approve a 10% Salary Increase for the Acting Chief
- Approve the Interim City Manager Stipend as agreed upon by Council.

Alternatives: Disapprove.

Attachments: Updated BIPD Pay Plan

Belle Isle Police Department Salary Adjustment and Agreement

March 31, 2020 (Revised July April 1813, 20232)

Officers from the Belle Isle Police Department officers are committed to protecting the Belle Isle community. The Department has faced some challenges over the past several years in attracting and keeping police officers. The pool of qualified candidates who want to become police officers has dwindled, partly due to negativity towards police. Larger agencies attract quality candidates with higher pay, more opportunities for advancement, and a wide array of specialized units. The competition to fill vacancies often boils down to agency salaries. Most qualified police applicants are given offers by several agencies.

The Belle Isle City Council recognized the need to provide better benefits and salaries in order to keep the highly trained and professional officers here. In doing so, over the past four years, the Council provided an increase in salaries and benefits to the officers. However, even with these increases, the Belle Isle Police Department remains one of the lowest-paid departments in Central Florida. The Belle Isle Police Department lost six valued officers over the past three years to larger agencies. These agencies offer better pay and benefits. The cost to train a new police officer is approximately

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\$32,000.When an officer leaves the Belle Isle Police Department, other communities get the benefit of the cost of this training.

An extensive study of salary step plans of other law enforcement agencies revealed that retention is significantly increased when an agency has a guaranteed salary and benefits plan. This plan only applies to certified police officers, hereafter referred to as "employees". Employees will be placed in the grade they would have obtained based on their original date of hire or date of rank for Sergeant and above.

ARTICLE 1

PENSION PLAN

1. The City, as the annual budget allows, will contribute 17.5% of the employee's salary to a 401(a) held by the Florida League of Cities up to 19%.

 Employees may also opt to contribute to a personal 457(b) account managed by the Florida League of Cities but the City does not contribute to this plan.

ARTICLE 2

WORKING OUT OF CLASSIFICATION

An employee who is specifically assigned by the Chief or his/her designee to temporarily accept the responsibilities and duties incident to a position senior to that of his/her regular grade and who is expected to exercise the authority and responsibility of the position shall be paid at the rate of such higher position from that date onward as long as such duties and responsibilities are carried out.

ARTICLE 3

WORKWEEK & WORK SHIFT

- The payroll workweek shall begin at 0001 hours, Thursday, and end at 2400 hours, Wednesday. The work cycle shall be a twenty-eight (28) day work period, under the FLSA 7(K) exemption. Employees assigned to ten and a half (10.5) hour shifts shall be scheduled to work forty (42) hours per seven (7) day week. Officers assigned to work twelve (12) hour shifts shall be scheduled to work eighty-four (84) hours, per a fourteen (14) day pay period.
- 2. Employees are permitted a 15-minute paid rest break for each four-hour work period. Breaks are not permitted at either the beginning or end of the workday to offset arrival and departure times. Employees who voluntarily work through their rest breaks will not be paid additional compensation.
- 3. Employees who work eight or more hours in a day may take a paid meal break of 30 minutes. Meal breaks are counted toward hours worked. Employees are not completely relieved from duty during their meal break.

Article 4

OVERTIME

 Employees working in excess of 86 hours in a pay period shall be paid at the overtime rate of 1.5 times their regular hourly rate. Paid Time Off (PTO) will not be counted in the 86 hours worked.

Article 5

EXTRA TIME PROVISIONS

- 1. Employees will be paid a minimum of three (3) hours "Call Back" time when asked to return to work outside a regular shift. Time of work begins upon notice to report.
- 2. Employees who are required to attend department business outside of their normal shift hours will be compensated for their actual time but no less than two (2) hours of straight pay. Business immediately appended to the employee's normal shift shall not be subject to the two (2) hour minimum.
- An employee ordered to attend any meeting that occurs outside of regular working hours shall be compensated for actual hours worked.
- 4. If a paid holiday falls on an employee's regularly scheduled day off, the employee will be compensated for an additional shift at regular straight-time pay. If an employee must work on a holiday (other than Floating Holidays), the employee will be paid for hours worked plus an additional shift at regular straight time. The number of hours for the additional shift of pay will be determined by the employee's normally scheduled shift, i.e., 8-hour, 10-hour, 12-hour shift.

Article 6

COURT TIME

1. Officers required to appear in court during their off duty hours, on behalf of the City and as a result of exercising their lawful authority, will receive the equivalent of no less than three (3) hours of straight pay. The three (3) hour minimum shall not apply when the court appearance is scheduled to begin within one hour of the start or end of the employee's shift. In such circumstances, the employee's shift will be extended, and the employee will be paid for hours actually worked. If an employee is required to appear in court two or more times on the same date, an employee may only receive one "three-hour minimum" if the proceedings are conducted within the same three-hour period. If the court appearances begin within the same three-hour period, the employee will be granted pay for those hours, or portions thereof, that exceed the original three-hour allocation. Only when the court appearances are scheduled to begin outside the "three-hour minimum" time periods may employees receive an additional three-hour minimum pay.

2. Employees shall be permitted to keep any subpoena fees legally due to them.

ARTICLE 7

EDUCATIONAL REIMBURSEMENT

- Employees are encouraged to attend institutions of higher learning. Employees who are attending college may be allowed to attend classes while in a paid status by using PTO or adjust work hours, workload permitting, and with a supervisor's approval.
- Reimbursement for educational expenses will be in accordance with all provisions of the City's Policy and Procedures. The maximum reimbursement shall be one thousand eight hundred (\$1,800.00) dollars per person per year.

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- 3. Employees seeking educational reimbursement must be enrolled in courses related to law enforcement.
- 4. Reimbursement will be only be granted based on the following scale:
 - 80-100% 100%
 - 70-79% (Pass/Fail Grade is considered 70%)- 75%
 - Below 70% (or Incomplete) 0%

ARTICLE 8

PAY PLAN

The Belle Isle Police Department shall define "good standing" as,

- A. No evaluation grade of "below standards" on any dimension on the annual Performance Appraisal.
- B. No more than two (2) separate Internal Investigations leading to sustained disciplinary findings, or any demotion, within the past twelve months.

Officer

 From the effective date of this Agreement, Officers in good standing will advance within Grades 1-11 as set forth in the chart below on their date of hire. Officers hired with previous experience will start at the following grades: two (2) to five (5) years of experience - Grade 2; six (6) to ten plus (10) years of experience - Grade 3. Experienced Officers starting salary within Grades two (2) or three (3) will advance within Grades as set forth in the chart below on their date of hire.

Annual PayScale Effective 10/01/2020			
Grade	Salary		
1	\$ 44,124.29		
2	\$ 47,212.99		
3	\$ 50,423.47		
4	\$ 52,389.99		
5	\$ 54,485.59		
6	\$ 56,665.01		
7	\$ 58,931.61		
8	\$ 61,288.88		
9	\$ 63,740.43		
10	\$ 66,290.05		
11	\$ 68,941.65		

<u>Corporal</u>

2. From the effective date of this Agreement, Corporals in good standing will advance within Grades 1-11 as set forth in the chart below on their date of hire.

Annual PayScale Effective 10/01/2020			
Grade	Salary		
1	\$ 45,889.26		
2	\$ 49,101.51		
3	\$ 52,440.41		

4	\$ 54,485.59
5	\$ 56,665.01
6	\$ 58,931.61
7	\$ 61,288.88
8	\$ 63,740.43
9	\$ 66,290.05
10	\$ 68,941.65
11	\$ 71,699.32

Sergeant

3. From the effective date of this Agreement, Sergeants in good standing will advance

within Grades 12-16 as set forth in the chart below on their date of rank.

Annual PayScale Effective 10/01/2020			
Grade	Salary		
12	\$ 69,633.56		
13	\$ 72,418.90		
14	\$ 75,315.66		
15	\$ 78,328.28		
16	\$ 81,461.42		

Lieutenant

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4. From the effective date of this Agreement, Lieutenant in good standing will advance within Grades 17-21 as set forth in the chart below on their date of rank.

Annual PayScale Effective 01/04/2022			
Grade	Salary		
17	\$	86,443.98	
18	\$	89,901.74	
19	\$	93,497.81	
20	\$	97,237.72	
21	\$	101,127.23	

** If promoted to Deputy Chief the salary will hold until next step above current salary is

achieved**

Deputy Chief

5. From the effective date of this Agreement, Deputy Chief in good standing will advance

within Grades 22-26 as set forth in the chart below on their date of rank.

Annual Pays	cale Effective 10/01/2020	
Grade	Salary	Formatted: Strikethrough
22	\$ 87,910.13	 Formatted: Strikethrough
23	\$ 91,426.54	 Formatted: Strikethrough
24	\$ 95,083.60	 Formatted: Strikethrough
25	\$ 98,886.94	 Formatted: Strikethrough

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26	\$ 102,842.42	Formatted: Strikethrough

- 5. Field Training Officers (FTO) shall be compensated at the rate of thirty-six dollars (\$36.00) per day for each day they perform the duties of a Field Training Officer.
- It is agreed that the Criminal Justice Standards Training Center educational incentive monies shall be paid monthly in one (1) biweekly payment and shall be received on or before the first of each month.
- Officers and Sergeants permanently assigned to Special Operations (Criminal Investigations, Traffic/Marine, and SRO) will receive \$25.00 bi-weekly, and Sergeants shall receive \$30.00 bi-weekly.
- 8. Date of rank or date of hire adjustments that fall within the first seven (7) days of the pay period will be effective from the beginning of that pay period. If the date falls within the 8th to 14th day, the adjustment will become effective at the beginning of the next pay period.
- 9. Employees who meet approved Bi-lingual Certification standards will be compensated at the rate of \$25.00 bi-weekly.

ARTICLE 9

LONGEVITY

 Employees with five or more years of service will be paid longevity pay based on years of service. Longevity pay will be distributed annually on the first pay period in October of each year. Effective October 1, 2020, the following longevity schedule will be used.

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Years of Service	Amount Per Year
5 to less than 10 years	\$500.00
10 to less than 15 years	\$750.00
15 to less than 20 years	\$1,000.00
20 to less than 22 years	\$1,250.00
22 years or more	\$1,500.00

ARTICLE 10

DURATION

Upon approval by the Belle Isle City Council, this Agreement shall take effect on October 1, 2020, and shall continue in full force and effect until September 30, 2023. At that time, the City Manager and the Police Department will review the pay plan for the appropriate Cost of Living adjustments.

ARTICLE 11

RULES

All employees covered by this Agreement shall also be covered by the terms of the City of Belle Isle Personnel Policy, as amended from time to time. It is specifically agreed and understood that this Agreement shall supersede all inconsistent provisions of the City of Belle Isle Personnel Policy

CITY OF BELLE ISLE, FLORIDA CITY COUNCIL AGENDA ITEM COVER SHEET

Meeting Date:	April 18, 2023
То:	Honorable Mayor and City Council Members
From:	T. Grimm, Interim City Manager
Subject:	Reinstate Council Workshops

Background: During the Strategic Planning Session and at a few City Council meetings, Council suggested going back to holding Council Workshops. Workshops, or Work Sessions, are allowed under the Bl Municipal Code, Section 2-53. The purpose of a City Council workshop is to provide an open forum for City Council and City staff members to discuss and share ideas on various subjects related to City operations, projects, and planning. Citizens are welcome to attend; however, since the City Council does not take official action during workshop sessions, citizen comments on workshop items are made only at the request of the Mayor or Council Members.

Staff Recommendation: Workshops can provide productive dialogue and open discussions between Council members less structured than regular meetings, which can lead to a positive direction for staff. The main problem encountered in the past has been to hold the workshop, provide direction, and then when it comes to the regular meeting agenda, the same discussions occur at the regular meeting done in the workshops.

If the Council wishes to go back to workshops, then issues discussed at the workshop should have limited discussion when placed on the regular meeting agenda for formal action. We can hold the same schedule. However, the first Tuesday meeting will be held for Action Items, and the third Tuesday can be for a Work Session.

Suggested Motion: <u>I move that, starting in May 2023, the Council meeting in a workshop on the third Tuesday of the month starting at 6:30 PM.</u>

Alternatives: Do not hold workshops and continue with two regular meetings every month.

Fiscal Impact: N/A

Attachments: None

4/18/23 **CONCEPT PLAN Co-Applicants: Cornerstone Charter** Ordinance 23-02 Academy **Pine Castle United Methodist Church City of Belle Isle Project Team FEG Engineering CIVICA** Architecture **TPD Engineering**

CORNERSTONE CHARTER ACADEMY

City Belle Isle, Florida **City Council Meeting**









Cornerstone Charter School CONCEPT Plan Ordinance 23-02

Co-Applicants

Cornerstone Charter Academy City of Belle Isle, Florida Pine Castle United Methodist Church

> Project Team FEG Engineering CIVICA Architecture TPD Engineering

CCA Class of 2023 Accomplishments

- 8 students graduating with AA degrees in 2023
- 5 students graduating as certified Bio-Medical Technicians (25 over the past 5 years)
- National Hispanic recognition award from College Board
- 2022 school winner for the Heisman High School Scholarship
- One of the 100 chosen (out of thousands nationally) for Disney Dreamer and Doers with his name in Times Square and on Good Morning America
- Two of our student athletes have signed with colleges to play at the collegiate level
- Winner of the Mayors MLK Humanitarian Award for community service
- 5 out of the top 10 graduates this year are Legacy Students
- 60% of our graduates earned the Scholar Diploma Designation

CCA Past Accomplishments

- Last year's Valedictorian attended West Point
- 2019 CCA graduate is graduating from the **Naval Academy** this year with an appointment to become a **Marine pilot**
- 2 National Merit Scholarship winners and multiple finalists
- 25%-30% of our graduates qualify for **Bright Futures** every year with **over \$1 million in additional scholarships** each year
- 5 Eagle Scouts have completed their service projects in and around CCA and Belle Isle
- Football team won their first ever SSAC State Title this year
- Over 250 students have taken dual enrollment since 2020
- Thriving theatre department that has won multiple Dr. Phillips Applause Awards
- Service Learning Club has raised tens of thousands of dollars for Childrens Miracle Network each year

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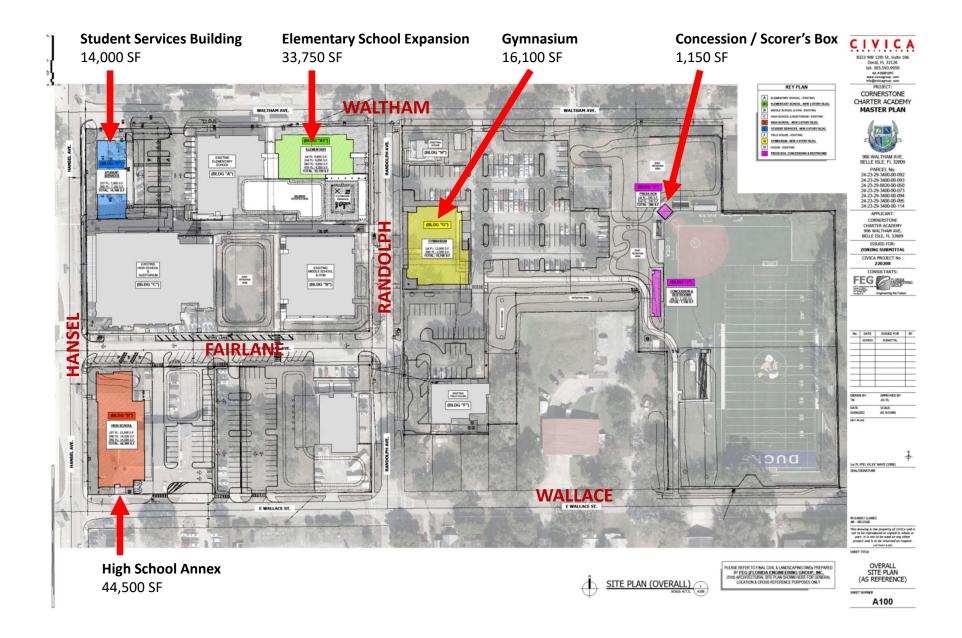




CORNERSTONE CHARTER ACADEMY PROPOSED MASTER PLAN

03.28.2023

CIVICA Architecture



Traffic Assessment



a.

Background

- Existing Shifts:
 - Grades K-5:
 - Drop-off 7:15 AM
 - Pick-up 2:35 PM
 - Grades 6-12:
 - Drop-off 7:30 AM
 - Pick-up 2:45 PM



Background

- Comprehensive Traffic Analysis
 - Traffic data collection
 - Traffic observation
 - Existing traffic analysis
 - Trip generation analysis
 - Buildout traffic analysis with suggested improvements
 - Queueing analysis
- Purpose of the analysis is to provide recommendations for traffic circulation



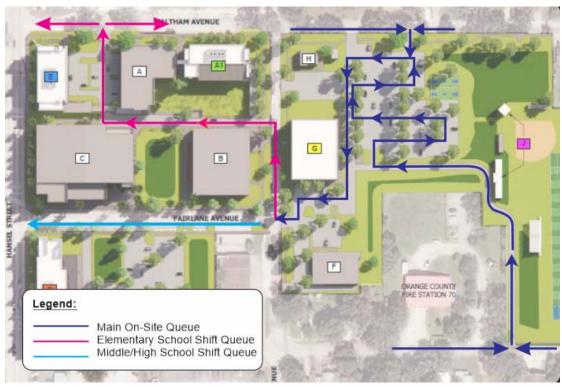
Proposed Improvements

- Drop-off / Dismissal will operate in three (3) shifts - minimum of ½-hour apart
- Site will have on-site queueing for all 3 shifts
- New entrance on Wallace Street to enhance on-site queueing.
- Site will have on-site queueing for all 3 shifts within the parking lot on the property east of Randolph Ave



Proposed On-Site Queueing

- Entry on Waltham Ave for traffic coming from the north
- Entry on Wallace Street for traffic coming from the south
- Queueing will form withing parking lot in <u>conga</u> <u>line configuration</u>
- Randolph Ave will be closed at Waltham Ave and on Wallace St during drop-off/pick-up
- Elementary School Shift:
 - Drop-off/Pick-up at Building A
 - Exit on Waltham Ave (can turn right and left)
- Middle/High School Shift:
 - Drop-off/Pick-up on Fairlane Ave
 - Exit right at the intersection of Fairlane Ave



Proposed Improvements

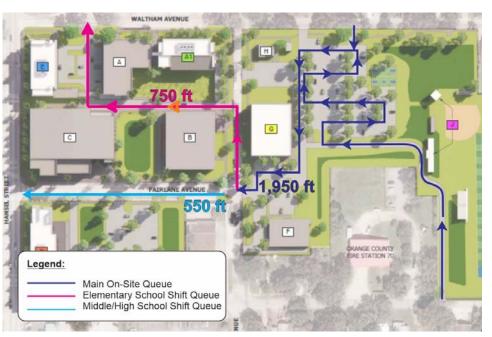
	Existing On-Site Queue Length Provided (Estimated from Traffic Observations)	Proposed On-Site Queue Length (Provided Within Site)***
Elementary School	750 ft *	2,700 ft
Middle/High School	770 ft **	2,500 ft

* 795 ft additional queue backs up on Waltham Ave, Randolph Ave and Wilks Ave

** 1,580 ft additional queue backs up on Wallace St and Randolph Ave south of Wallace St

*** Total stacking provided within the site





a.

Proposed Conditions Traffic Simulation – Elementary School Shift



Proposed Conditions Traffic Simulation – High School Shift



Recommendations

- Three (3) shifts for Elementary, Middle and High School, approximately 30-minutes apart.
- **On-site vehicle stacking** within the parking lot on the property east of Randolph Avenue.
- At the City's request, Randolph Avenue will be closed to traffic from the north on Waltham Avenue and the south on Wallace Street during Arrival and Dismissal.
- Fairlane Avenue to become 1-Way only westbound, and vehicles to exit right-only on Hansel Avenue.
- **Police/traffic attendant guidance** at the intersections of Hansel Avenue/Waltham Avenue and Hoffner Avenue/Randolph Avenue.
- Station **staff members** / **traffic attendants** within the site and at the entry driveways to **guide traffic** into the school site queue and ensure proper circulation.
- Traffic study considered 100% student vehicle drop-off/pick-up as a conservative analysis.
 Large portion of pedestrians and "golf-cart" pick-up locations,
 - The school expansion will also **expand the after-school programs** which will account for a percentage of students staying on campus after dismissal times and will **reduce the number of vehicles** arriving to the site at drop-off/dismissal.

Response to P&Z Meeting Comments

- 1. What policy does the Traffic Study comply with?
- City of Belle Isle MuniCode Sec 46-62
- Orange County "Traffic Operational Analysis Methodology for Schools"
- 2. Projected conditions of roadway segments within 1-mile operating at deficient Levels of Service
- Traffic report findings (modified):
 - 1 segment is operating at deficient Level of Service under existing conditions
 - 1 segment is projected to operate at deficient Levels of Service at buildout conditions
- Using SCHOOL PEAK HOUR (2:00-4:00 P.M. Peak Hour) for existing traffic volumes, there will be no capacity deficiency on the Hansel Avenue segment.

Hansel Avenue Segment 176 - Roadway Capacity Analysis

Roadway Capacity	Existing Volumes		Committed Trips	School Trips	Total	Capacity Exceeded?
2 4 4 0	4:00 - 6:00 P.M.	2,337	1	214	2,552	Yes
2,440	2:00 - 4:00 P.M.	1,730	1	214	1,945	No

Response to P&Z Meeting Comments

3. Project Impact on Hoffner Avenue is too significant

Trips Generated by SCHOOL EXPANSION

Trip Generation Comparison

	No. of Students	A.M. Peak Hour			P.M. Peak Hour		
		Total	Enter	Exit	Total	Enter	Exit
Existing Highest Shift Traffic	905	751	383	368	600	300	300
Proposed Highest Shift Traffic	1,100	913	466	447	779	390	389
	Net New Trips	162	83	79	179	90	89

Impact of EXPANSION TRIPS on Hoffner Avenue is only 4.00% of Capacity

Roadway Capacity	% of Trips	# of Trips Added due to Expansion	% of Capacity	
800	35%	32	4.00%	

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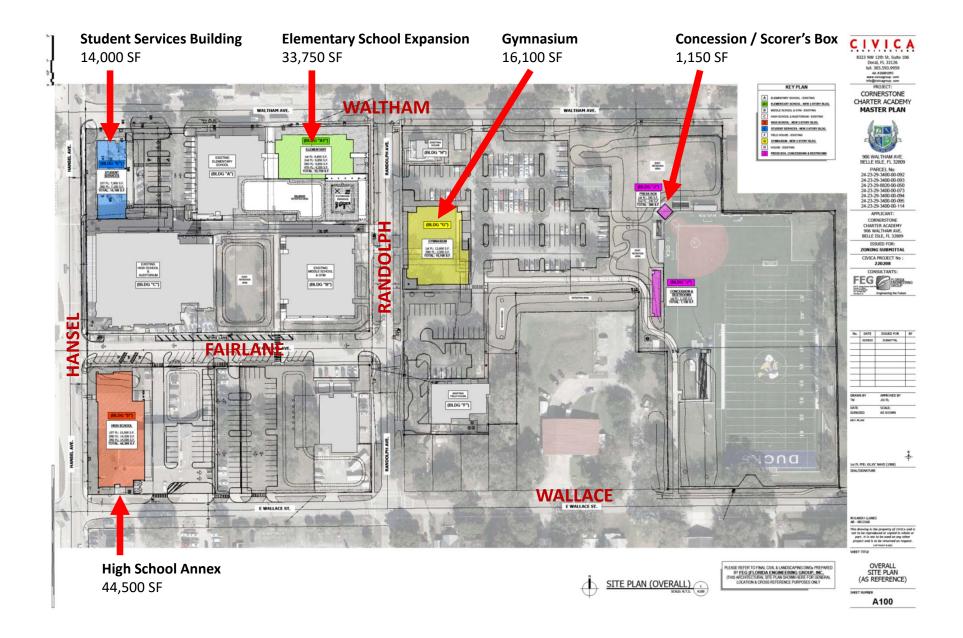
Response to P&Z Meeting Comments

- 4. Why were the intersections of Hansel Avenue/Oakridge Road and Orange Avenue/Oakridge Road excluded from analysis?
- Based upon our preliminary assessment of trip generation/ trip distribution, we determined that Oak Ridge Road would not be significantly impacted.
- Data was collected for further analysis and results show that these intersections will operate at acceptable Levels of Service at buildout.

	Orintral	Time	E	В	W	B	N	В	S	В	Ove	rall
Intersection	Control	Period	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Oak Ridge Rd & Hansel Ave Signa	Cinnal	A.M.	0.0	А			17.8	В			17.8	В
	Signal	P.M.	0.0	A		/	20.1	С		-	20.1	∕ C
Oak Bidge Bd & Orenge Ave	Signal	A.M.	19.7	В	18.3	В			8.4	А	11.9	В
Oak Ridge Rd & Orange Ave	Signal	P.M.	20.9	С	19.2	В		1	9.4	А	13.5	В
Hoffpor Ave & Marinell Dr	01	A.M.	0.0	А	3.7	В	29.6	D		/	4.0	Α
Hoffner Ave & Marinell Dr	Stop	P.M.	0.0	Α	2.3	Α	28.7	D		-	3.2	А

Response to P&Z Meeting Comments

- 5. <u>The roadway segments operating at deficient Levels of Service should follow Orange County's Concurrency requirements</u> for mitigating deficient Levels of Service.
- All public schools are exempt from Orange County's concurrency requirements.
- 6. <u>The stop-controlled intersection of Waltham Avenue/Hansel Avenue was not analyzed under police officer control during</u> <u>drop-off/pick-up</u>
- This intersection was analyzed under stop-control and under signal control in the traffic report. The signal control mimics the operation under police officer traffic guidance.
- The intersection will operate at satisfactory Levels of Service under signal (police officer) control.





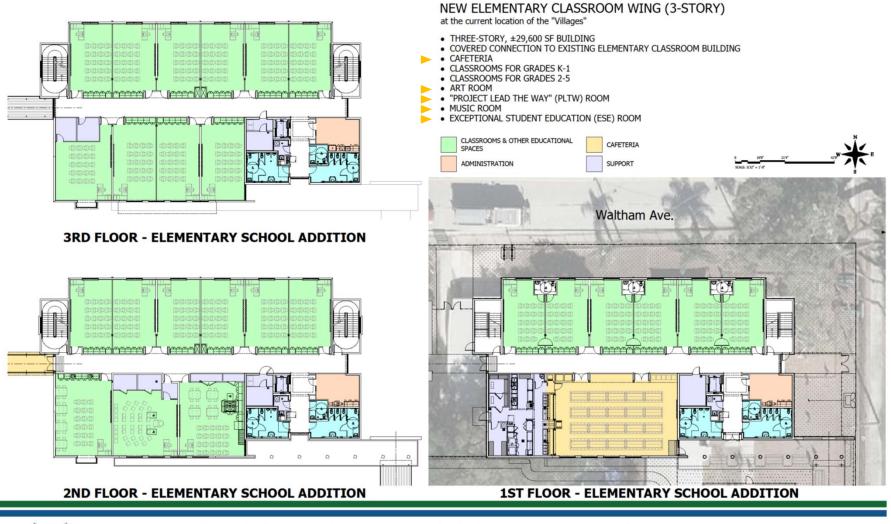
VIEW LOOKING NORTH EAST



CORNERSTONE CHARTER ACADEMY

03.28.2023

CIVICA Architecture

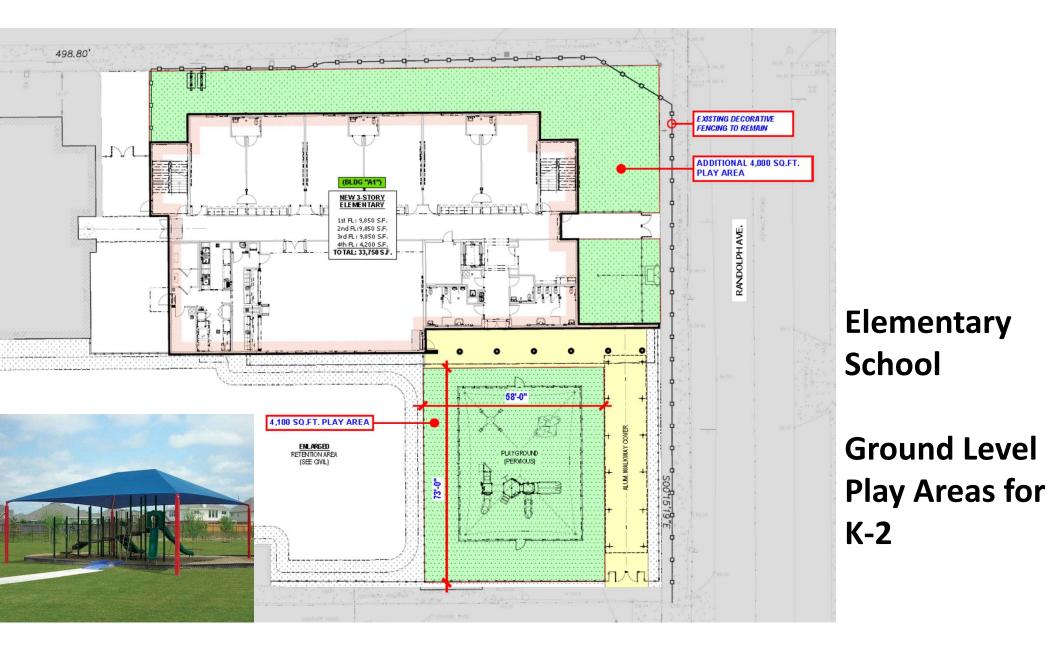




CORNERSTONE CHARTER ACADEMY NEW BUILDING A1 - ELEMENTARY SCHOOL ADDITION - FLOOR PLANS

03.28.2023

CIVICA Architecture





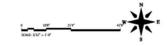


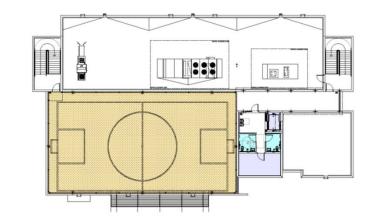
NEW ELEMENTARY CLASSROOM WING (3-STORY)

at the current location of the "Villages"

- THREE-STORY, ±29,600 SF BUILDING
- COVERED CONNECTION TO EXISTING ELEMENTARY CLASSROOM BUILDING
 CAESTERIA
- CAFETERIA
- CLASSROOMS FOR GRADES K-1
 CLASSROOMS FOR GRADES 2-5
- ART ROOM
- "PROJECT LEAD THE WAY" (PLTW) ROOM
- MUSIC ROOM
- EXCEPTIONAL STUDENT EDUCATION (ESE) ROOM







Elementary School Upper-Level Play Areas for 3-5

Fully Compliant with FBC, FFPC, FAC













NORTH ELEVATION



SOUTH ELEVATION

WEST ELEVATION

426.00

-854

-



EAST ELEVATION



willing.

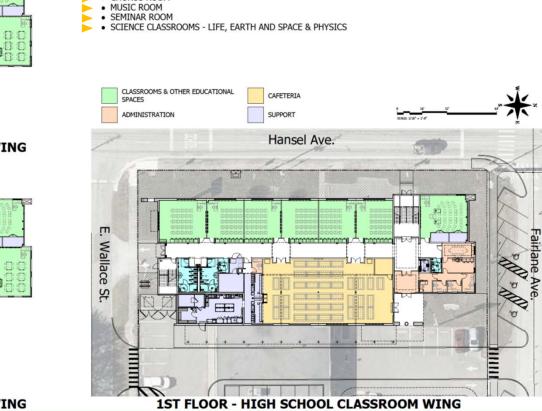
425-

\$Sim

Gillen

4 Miles

O TAN



NEW BUILDING "D" - HIGH SCHOOL CLASSROOM WING

at the old Bank of America parcel THREE-STORY, ±50,400 SF BUILDING HIGH SCHOOL ADMISTRATION OFFICE

GENERAL EDUCATION CLASSROOMS FOR GRADES 9-12

CAFETERIA

ART ROOMS

CHORUS ROOM



3RD FLOOR - HIGH SCHOOL CLASSROOM WING



2ND FLOOR - HIGH SCHOOL CLASSROOM WING

CORNERSTONE CHARTER ACADEMY

NEW BUILDING D - HIGH SCHOOL CLASSROOM WING - FLOOR PLANS

03.28.2023 CIVICA



WEST ELEVATION

NORTH ELEVATION

975

910m

4-07

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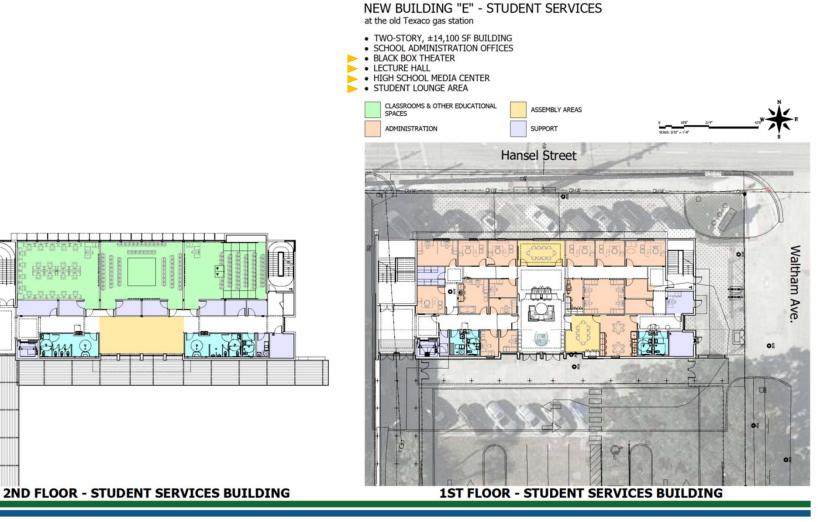
4000



EAST ELEVATION









1

CORNERSTONE CHARTER ACADEMY NEW BUILDING E - STUDENT SERVICES - FLOOR PLANS

03.28.2023

CIVICA Architecture



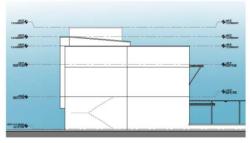
WEST ELEVATION



NORTH ELEVATION







SOUTH ELEVATION



а.

NEW BUILDING "J" - FIELD CONCESSIONS & RESTROOMS

- CONCESSIONS
- RESTROOMS
- COACH OFFICE

CONCESSIONS

OFFICE

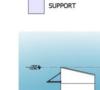
BASEBALL/SOFTBALL SCORER'S BOX

TWO-STORY, ±1,740 SF BUILDING





ELEVATION - SCORER'S BOX



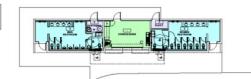
RESTROOM



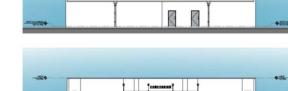
ELEVATIONS - CONCESSION + RESTROOM



2ND FLOOR - SCORER'S BOX



ROOF PLAN - CONCESSION + RESTROOM







CORNERSTONE CHARTER ACADEMY

NEW BUILDING J - FIELD CONCESSIONS+RESTROOM FLOOR PLANS AND ELEVATIONS

03.28.2023

CIVICA Architecture



 TWO-STORY, ±17,100 SF BUILDING GYMNASIUM / AUDITORIUM WITH BLEACHERS REGULATION BASKETBALL / VOLLEYBALL COURT A/V SYSTEM CONTROL ROOM CONCESSION STAND LOCKER ROOMS WEIGHT ROOM GYMNASIUM / AUDITORIUM LOCKERS & WEIGHT ROOMS OFFICES SUPPORT Randolph Ave. 0 44 E Я Lal LO 6 TE ToTe e 4004 10-1 TO 1 (CO) ------15-1 000 10-1 000 1 1 HC Ð . a THI DE D ₽⊙.

NEW BUILDING "G" - GYMNASIUM / AUDITORIUM

2ND FLOOR - GYMNASIUM / AUDITORIUM

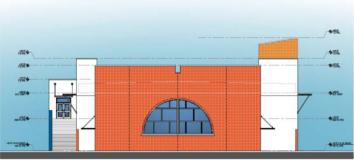
1ST FLOOR - GYMNASIUM / AUDITORIUM



CORNERSTONE CHARTER ACADEMY NEW BUILDING G - GYMNASIUM/AUDITORIUM FLOOR PLANS

03.28.2023

CIVICA Architecture а.



NORTH ELEVATION



SOUTH ELEVATION



-844

-114

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-554

CORNERSTONE CHARTER ACADEMY NEW BUILDING G - GYMNASIUM/AUDITORIUM ELEVATIONS

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41E

485m

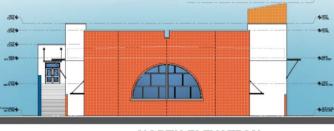
985-

4 Bicle

03.28.2023

SCALE 3/12" = 1'-4

CIVICA Architecture





EAST ELEVATION

WEST ELEVATION

CORNERSTONE CHARTER ACADEMY *City Belle Isle, Florida*

GYMNASIUM







STAFF REPORT

- Approval of the requested REZONING per Ordinance
 23-02 and per Belle Isle staff's recommendation of
 APPROVAL
 - "The proposed Planned Development rezoning is to create consistency in the zoning use of the entire campus where the school is and will be in operation."

STAFF RECOMMENDATION:

- "Staff has no objection to the proposed concept plan."
- "Staff recommends approval of the preliminary concept plan and PD district."

- Approval of the following per Ordinance 23-02
 - o Concept Plan
 - o Code Provisions
 - o Permitted Uses
 - Special Exception Uses
 - o Maximum Building Height
 - 4 stories / 57 feet
 - o Setbacks consistent with the Concept Plan
 - o Intensity
 - 0.45 Floor Area Ratio
 - Maximum Impervious Surface
 .70
 - o Architectural Features consistent with the Concept Plan
 - Vehicular Parking / Ingress / Egress
 - o Lighting
 - o Landscaping, Landscaping Buffers, Recreation, Open Space
 - o Planned Development Name
 - Cornerstone Charter Academy PD
 - Development Plan / PD Expiration
 - o Violation

Planning & Zoning recommended APPROVAL of Ordinance 23-02 w/ the following Conditions of Approval:

- 1. Randolph Ave to be closed ONLY during the Pick-up and Drop-off hours of the school and shall include appropriate safety measures.
- 2. Eliminate "Required" and "Allowable" columns on the site plan and reflect the floor area ratio, impervious surface ratio, and building setbacks standards.
- 3. Under sec.54-77(e) (5), the code requires the next phase of the PD process to govern "the use of land and the construction, modifications, or alterations of any buildings, structures or other improvements" on the property. The Site Note must be modified to reference the Development Plan, not the Final Construction Plans.
- 4. The Title of the Plan Set needs to be Cornerstone Charter School CONCEPT Plan, not Development Plan. Consistency with Code verbiage is important to eliminate any confusion in the future.

Staff Recommendation Approve the amendment recommended by the P&Z Board on 4/11/23

REQUEST

Read Ordinance 23-02 for the first time and post for Second Reading and Adoption on May 2, 2023 а.



5127 S. Orange Avenue, Suite 200 Orlando, FL 32809 Phone: 407-895-0324 Fax: 407-895-0325 2302 Parklake Drive, Suite 134 Atlanta, GA 30345 Phone: 1-877-857-1581 Fax: 1-877-857-1582



February 9, 2023

Mr. Bob Francis City Manager City of Belle Isle 1600 Nela Avenue Belle Isle, FL 32809

Subject: Cornerstone Charter Academy 906 Waltham Avenue Belle Isle, FL 32809 Project Description FEG Project No. 22-010

Dear Bob;

The purpose of this request is to rezone the parcels associated with the Cornerstone Charter Academy campus to Planned Development (PD) to have a consistent entitlements standards throughout the school site. The proposed project is located north of Wallace Street, East of Hansel Avenue and south of Waltham Avenue in the City of Belle Isle. The project consists of a combination of seven parcels with a total land area of approximately 14.9 acres as shown in the enclosed Parcel Map exhibit attached to this letter.

There are three separate entities that own these parcels where the City of Belle Isle being the largest landowner and the other two owners are Pine Castle Methodist Church and Cornerstone Charter Academy Inc. The project will consist of expanding the existing K-12 charter school to accommodate 2,420 Students from approximately 1,479 students under current conditions.

Zoning and Future Landuse:

The Cornerstone Charter Academy has been in operation for over a decade under the current different zoning districts and future land use designations. The existing zoning entails R-1A, R-2 and C-1 as detailed on the PD Master plan with Future Land use of Low Density Residential and Commercial Designations. As such, our request is to have a unified zoning designation along the entire school site with consistent entitlements standards.

Planned Development:

The enclosed PD Master Plan depicts the location of the new and existing buildings within the project site. It also provides the square footage per building. The new buildings will consist of three-story High School Annex Building located at the existing Bank of America site, a two-story Student Services Building at the old Texaco station, a four-story Elementary Classroom Building east of the existing elementary school building, a new Gymnasium on the east side of Randolph Avenue a Press Box Building and a Concession Building near the football field. The areas for each new building are shown on the PD master plan and on the accompanying architectural plans.

Moreover, setbacks, landscape buffers and open space are also defined as part of this PD request. A landscape plan also accompanies this submittal.

Stormwater and Utilities:

The proposed improvements will be served with additional stormwater ponds to be built as shown on the PD master plan to meet the water quality and flood control per the City of Belle Isle and the St. Johns River Water Management District (SJRWMD) requirements.

The proposed sewer system will connect to the exiting sewer system onsite. A lift station will be added near the new gymnasium to receive the sewer discharge from the Gymnasium building, the new Concession building and the existing Field House building. Orange County provides the sewer services in this area so the design will be in compliance with Orange County Utilities requirements. OUC will provide the water service to this project.

Traffic:

As requested by the City, a full Traffic Impact Assessment (TIA) Study was performed to assess the effect of the existing and proposed school expansion on the surrounding roadway network and on the local streets in the immediate vicinity of the school site. A copy of the full traffic impact assessment report is enclosed with this submittal.

As part of the traffic study, an analysis was conducted to ensure that the proposed site can accommodate parent vehicles during drop-off/pick up times. As a result of this analysis, it was recommended that three shifts be implemented 30 minutes apart to ensure queue clearance for the next shift and minimize queue spill on adjacent streets. New traffic patterns for queuing per shift was also recommended to ensure compliance with the traffic study recommendations.

Architectural Plans:

Colored building elevations and floor plans for each new building are also enclosed with this submittal as part of the PD package.

I trust this letter and the attached documents provide you with the necessary information to review and approve our PD zoning request. Should you have any questions or concerns, please do not hesitate to contact me at 407-895-0324 or, by email, at <u>JAbiaoun@feg-inc.us</u>.

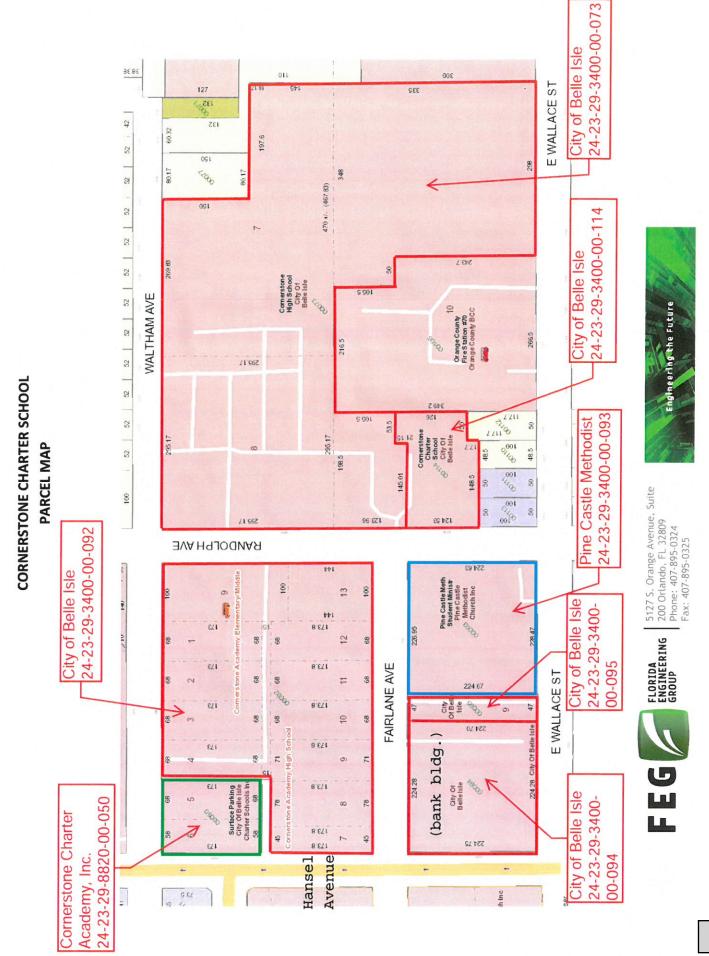
Sincerely, Florida Engineering Group, Inc.

Jean M. Abi-Aoun, P.E. Vice-President



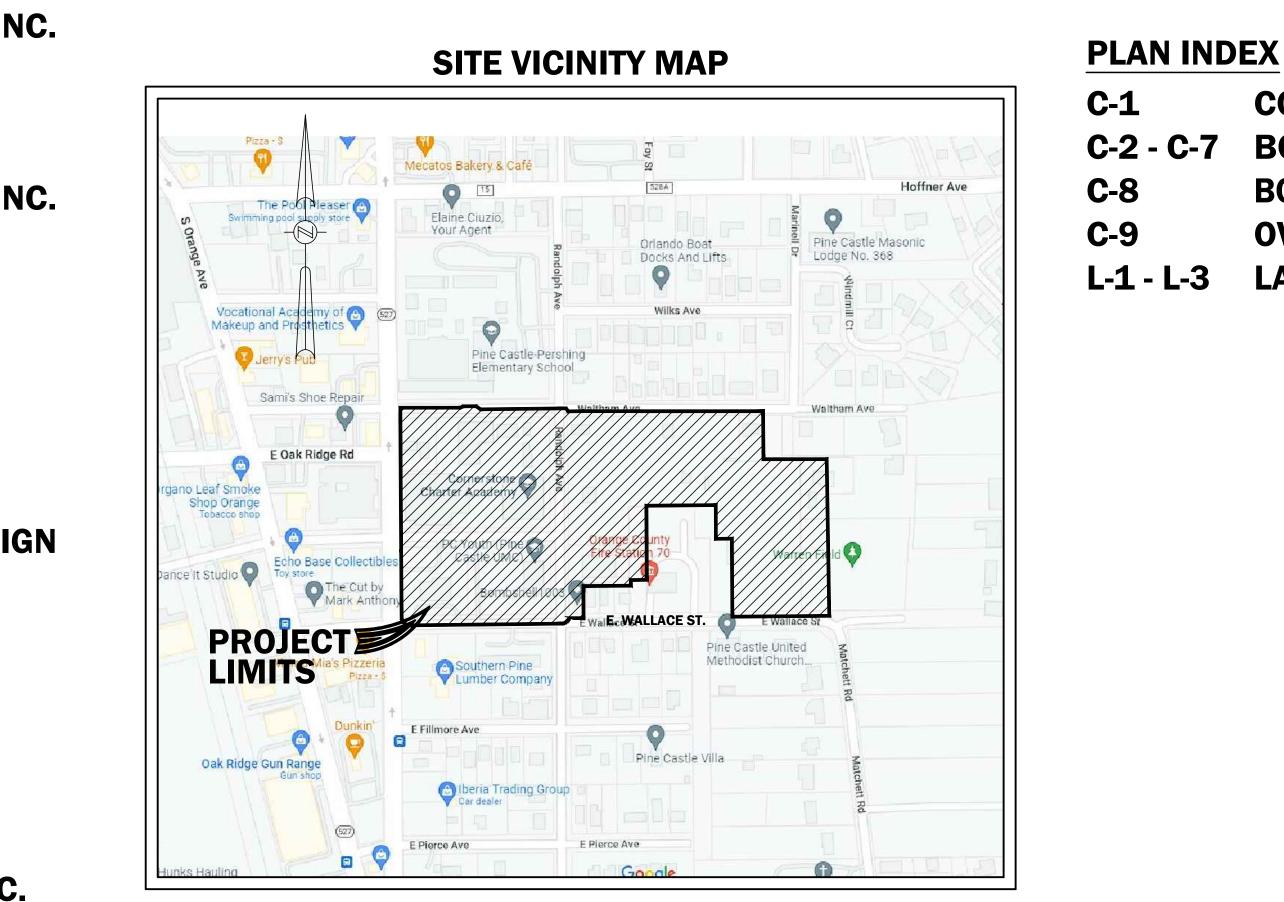
5127 S. Orange Avenue, Suite 200 Orlando, FL 32826 Phone: 407-895-0324 Fax: 407-895-0325 2302 Parklake Drive, Suite 134 Atlanta, GA 30345 Phone: 1-877-857-1581 Fax: 1-877-857-1582





CORNERSTONE CHARTER ACADEMY PLANNED DEVELOPMENT PLAN CITY OF BELLE ISLE, FLORIDA

APPLICANT:	CORNERSTONE CHART 906 WALTHAM AVE. CITY OF BELLE ISLE, F 407-608-7171	
OWNERS:	CORNERSTONE CHART 906 WALTHAM AVE. CITY OF BELLE ISLE, F 407-608-7171	
GEOTECHNICAL ENGINEER:	ECS FLORIDA. LLC 2815 DIRECTORS ROV ORLANDO, FL 32809 PHONE: (407) 859-837	, ,
ARCHITECT:	CIVICA ARCHITECTUR 8323 NW 12th St. SU DORAL, FL 33126 PHONE: (305) 593-995	TE 106
TRAFFIC:	TRAFFIC PLANNING & 535 VERSAILLES DR. S MAITLAND, FL 32751 PHONE: (305) 923-710	SUITE #200
SURVEYOR:	BISHMAN SURVEYING 32 W. PLANT STREET WINTER GARDEN, FL 3 PHONE: (407) 905 FAX: (407) 905	-8877
F.D.E.P.: WATER A CITY OF BELLE ISL ORANGE COUNTY	IRONMENTAL RESOURC AND WASTEWATER SYST E: SITE PLAN APPROVA	EM PERMITS
UTILITY COMPANI		
WATER: SEWER:	(407)-423-9018 (407)-254-9764	ORLANDO UTILITIES CO ORANGE COUNTY UTILI
ELECTRIC:	(877)-372-8477	DUKE ENERGY
TELEPHONE:	(800)-288-2020	AT&T
CABLE:	(855)-317-1263	CHARTER SPECTRUM



NOT TO SCALE





Engineering the Future

COVER SHEET

- **BOUNDARY AND TOPOGRAPHIC SURVEY (6) BOUNDARY AND TOPOGRAPHIC SURVEY - BANK OF AMERICA OVERALL MASTER SITE PLAN**
- LANDSCAPE PLAN (3)



THIS ITEM HAS BEEN DIGITALLY SIG SEALED BY JEAN M. ABI-AOUN

ON THE DATE ADJACENT TO THE SEAL PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES. LORIDA ENGINEERING GROUP. INC

5127 S. ORANGE AVE. SUITE 200 ORLANDO. FL 32809 CERTIFICATE OF AUTHORIZATION: EB-0006595 JEAN M. ABI-AOUN, P.E. 45128

Digitally signed by JEAN M ABI-AOUN DN: c=US, o=FLORIDA ENGINEERING GROUP INC., ou=A01410D00000172C 87078BF0000F7AE, cn=JEAN M ABI-AOUN Date: 2023.02.09 10:23:00 -05'00'

5127 S. Orange Avenue, Suite 200 Orlando, FL 32809 Phone: 407-895-0324 Fax: 407-895-0325 ORIDA ENGINEERING CERTIFICATE No. EB No 45128

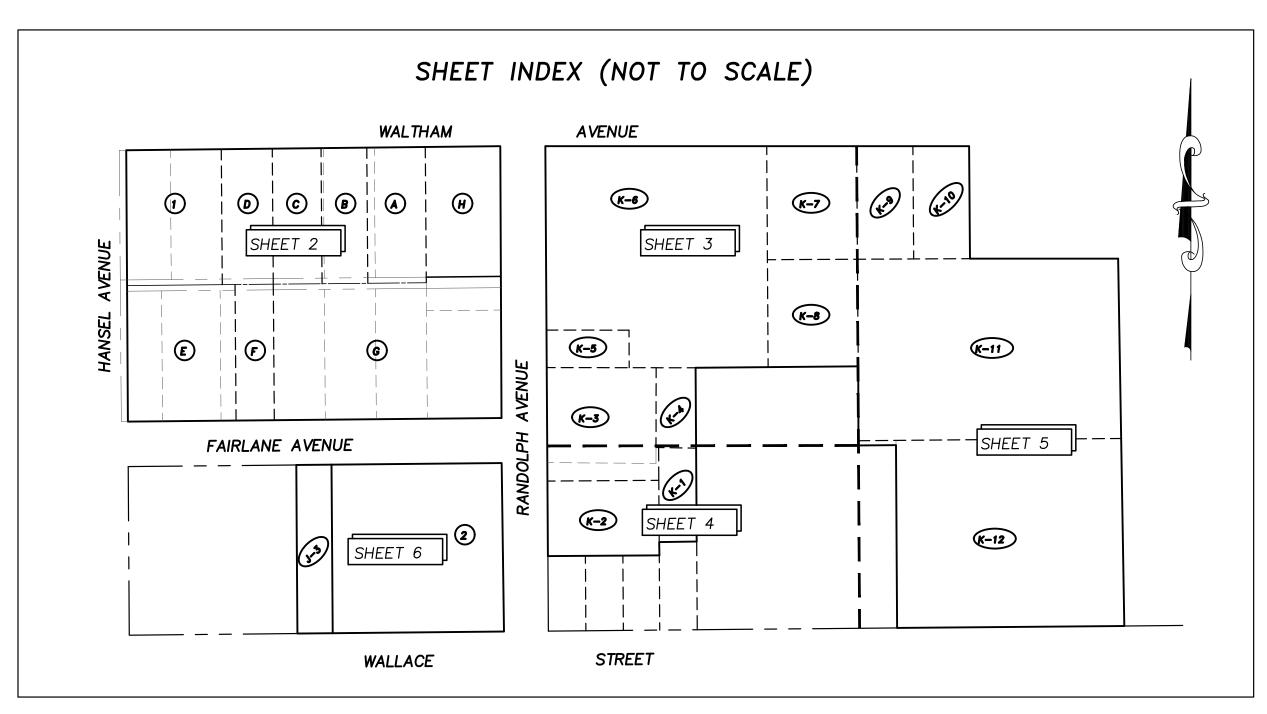
www.feg-inc.us

22-010_COVER.DWG

STATE O.

BOUNDARY AND TOPOGRAPHIC SURVEY

CORNERSTONE CHARTER ACADEMY



SURVEYORS NOTES;

1. NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL, OR DIGITAL SIGNATURE AND SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. PRINTED COPIES OF A DIGITAL SIGNED AND SEALED SURVEY ARE NOT VALID.

 LANDS SHOWN HEREON WERE NOT ABSTRACTED FOR RIGHTS OF WAY, EASEMENTS, OWNERSHIP, OR OTHER INSTRUMENTS OF RECORD, BY THIS FIRM.
 REVISIONS DO NOT CONSTITUTE A RE-CERTIFICATION OF THE EXISTING FIELD CONDITIONS OF THIS SURVEY.

4. BEARINGS SHOWN HEREON ARE BASED ON THE NNORTH RIGHT-OF-WAY LINE OF FAIRLANE AVENUE AS BEING S89°27'20"W (ASSUMED).

5. THE DESCRIPTION SHOWN HEREON WAS SUPPLIED BY THE CLIENT.

6. UNDERGROUND IMPROVEMENTS AND INSTALLATIONS HAVE NOT BEEN LOCATED.

7. THE LANDS SHOWN HEREON LIE ENTIRELY WITHIN ZONE X (AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOOD) ACCORDING TO "FIRM" MAP NO. 12095C0430 F, COMMUNITY NO. 120179, DATED SEPTEMBER 25, 2009.

8. THE ELEVATIONS SHOWN HEREON ARE BASED ON ORANGE COUNTY DATUM PER BENCH MARK NUMBER S1316–035 BEING A BOX CUT ON TOP OF CURB INLET, ELEVATION = 101.52 (NAVD 1988).

9. SITE BENCHMARKS ARE SHOWN HEREON

10. THE ELECTRONIC FILE FOR THIS PROJECT IS THE PROPERTY OF BISHMAN SURVEYING AND MAPPING, INC. AND IS NOT THE PROPERTY OF THE CLIENT.

		N	OT ALL SYMBOLS AND ABBREVIA	TIONS SHOWN HEREON MAY BE USED		
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\triangle	CENTRAL ANGLE		ELECTRICAL OUTLET	PVC POLYVINYL CHLORIDE PIPE	ЕНН	ELECTRIC HAND HOLE
L	LENGTH		TELEPHONE RISER	RCP REINFORCED CONCRETE PIPE		ELECTRIC HAND HOLE
СН	CHORD	HÇD	FIRE HYDRANT	CMP CORRUGATED METAL PIPE		CABLE TELEVISION RISER
CB	CHORD BEARING		FIRE HIDRANI	DIP DUCTILE IRON PIPE	TAC XX	TELEPHONE ACCESS CABINET
Т.В. (11)	TANGENT BEARING		MASTER WATER ASSEMBLY	VCP VITIOUS CLAY PIPE		
(M)	MEASURED	Ň	SEWER VALVE	CPP CORRUGATED PLASTIC PIPE		CATCH BASIN
(P)		X€	WATER VALVE		8	DRAIN
(C)			WATER VALVE		$\overline{\mathbf{Q}}$	CURB INLET
(D)	DESCRIPTION	<i>R₩V</i>	RECLAIMED WATER VALVE			CURB INLET WITHOUT MANHOLE
POB	POINT OF BEGINNING	IRV	IRRIGATION VALVE			
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P.C.	POINT OF CURVATURE	D	DRAINAGE MANHOLE	CONCRETE UTILITY POLE	PB #	POWER BOX
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۲	NAIL & DISC	-	GREASE TRAP MANHOLE	GUY ANCHOR	•	
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_		\bigcirc	POWER MANHOLE	T INVEST ELEVATION	¢	WALKWAY LIGHT
O	IRON PIPE	wso X	WATER SHUT-OFF VALVE	+ 68.51 EXISTING GROUND ELEVATION	tsb 	TRAFFIC SIGNAL BOX
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PSM	PROFESSIONAL SURVEYOR & MAPPER			SITE BENCH MARK (AS INDICATED)	SB	SOIL BORING
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DESCRIPTION: PARCEL A

Lot I and the East 10 feet of Lot 2 of J.G. TYNER'S SUBDIVISION, according to the Plat thereof, as recorded in Plat Book F, Page 44, of the Public Records of Orange County, Florida. TOGETHER WITH North Half of vacated alley way as described in Resolution recorded in Official Records Book 3723, Pages 2582 through 2584, Public Records of Orange County, Florida. AND

PARCEL B

The West 58 feet of Lot 2 and the East 3 feet of Lot 3, of J.G. TYNER'S SUBDIVISION, of a part of the North 391.8 feet of Lot 9, HARNEY'S HOMESTEAD, according to the Plat thereof, filed for record August 19, 1912, in Plat Book F, Page 44, Public Records of Orange County, Florida. TOGETHER WITH The North Half of vacated alleyway as described in Resolution recorded in Official Records Book 3723, Pages 2582 through 2584, Public Records of Orange County, Florida.

PARCEL C

AND

Lot 3, LESS the East 3 feet thereof, of J.G. TYNER'S SUBDIVISION of a part of the North 391.8 feet of Lot 9, HARNEY'S HOMESTEAD, according to the Plat there of, filed for record August 19, 1992, in Plat Book F, Page 44, Public Records of Orange County, Florida.

TOGETHER WITH The North Half of vacated alley way as described in Resolution recorded in Official Records Book 3723, Pages 2582 through 2584, Public Records of Orange County, Florida. AND

PARCEL D

Lot4 of J.G. TYNER'S SUBDIVISION of a part of North 391.8 feet of Lot 9,of HARNEY'S HOMESTEAD, according to the Plat thereof, as recorded in Plat Book F, Page 44, Public Records of Orange County, Florida.

TOGETHER WITH The North Half of vacated alley way as described in Resolution recorded in Official Records Book 3723, Pages 2582 through 2584, Public Records of Orange County, Florida. AND

PARCEL E

Lots Seven(7) and Eight(8) and the West Twenty Feet(20) of Lot Nine (9) of J.G.TYNER'S SUBDIVISION, according to the Plat there of, as recorded in Plat Book F, Page 44, Public Records of Orange County, Florida.

TOGETHER WITH South Half of vacated alley way lying North of said Lots 7 and 8 and the South Half of vacated alley way lying North of said West20 feet of said Lot 9 as described in Resolution recorded in Official Records Book 3723, Pages 2582 through 2584, Public Records of Orange County, Florida. AND

PARCEL F

Lot 9 (LESS the West 20 feet), J.G. TYNER'S SUBDIVISION, according to the Plat thereof, as recorded in Plat Book F, Page 44, Public Records of Orange County, Florida. TOGETHER WITH South Half of vacated alley way as described in Resolution recorded in Official Records Book 3723, Pages 2582 through 2584, Public Records of Orange County, Florida.

AND PARCEL G

Lots 10, 11, 12 and 13, of J.G. TYNER'S SUBDIVISION of a part of North 391.8 feet of Lot 9, HARNEY'S OMESTEAD, according to the Map or Plat of said HARNEY'S on record; the Plat of J.G. TYNER'S SUBDIVISION, being recorded in Plat Book F, Page 44, Public Records of Orange County, Florida.

ALSO, beginning at the Northwest corner of Lot 13, of J.G. TYNER'S SUBDIVISION, of a part of the North 391.8 feet of Lot 9, of HARNEY'S HOMESTEAD, according to the Map or Plat of said HARNEY'S HOMESTEAD on record, run North 29.8 feet; thence run East 100 feet; thence run South 29.8 feet; thence run West 100 feet to the POINT OF BEGINNING. Said land being located in Section 24, Township 23 South, Range 29 East, Orange County, Florida.

TOGETHER WITH The South Half of vacated alley way as described in Resolution recorded in Official Records Book 3723, Pages 2582 through 2584, Public Records of Orange County, Florida. AND

Extension of said 15 foot alley Easterly through a portion of Lot 9 of HARNEY'S HOMESTEAD, more particularly described as follows:

The North 15 feet of the South 44.8 feet of the North 217.8 feet of the East 100 feet of said Lot 9, together with any other interest of party of the first part in and to that part of said Lot 9, lying North of Lot 13, of J.G.TYNER'S SUBDIVISION, (Plat Book F, Page 44), recorded in Plat Book C, Page 53, Public Records of Orange County, Florida.

DESCRIPTION: (CONTINUED)

PARCEL H

The North 173 feet of the East 100 feet of Lot 9 of HARNEY HOMESTEAD, according to the Plat thereof, as recorded in Plat Book C, Page 53, Public Records of Orange County, Florida. ALSO DESCRIBED AS: Begin at a stone at the Northeast corner of land formerly belonging to C.J. SWEET AT PINE CASTLE, FLORIDA, situated in Section 24, Township 23 South, Range 29 East, run South 173 feet; thence West 100 feet; thence North 173 feet; thence East 100 feet to the POINT OF BEGINNING. AND

PARCEL J-3

Lot 9 of the HARNEY HOMESTEAD, as recorded in Plat "C", Page 53, of the Public Records of Orange County, Florida, LESS the Easterly 228.47 feet AND LESS the North 391.8 feet AND LESS the West 224.28 feet thereof;

AND LESS the road right-of-way on the South and being more particularly described as follows: Commence at the Southwest corner of Lot 9 of the HARNEY HOMESTEAD, as recorded in Plat Book "C", Page 53, of the Public Records of Orange County, Florida; thence run North 89 degrees 57minutes 29 seconds East along the North right-of-way line of Wallace Street as shown and depicted on the plat of KEEN-CASTLE, as recorded in Plat Book "P", Page 1, of said public records, a distance of 224.28 feet to the POINT OF BEGINNING; thence North 00 degrees 04 minutes 16 seconds East along the East line of the West 224.28 feet of said Lot 9, a distance of 224.70 feet to a point on the South right-of-way line of Fairlane Avenue; thence along said South line North 89 degrees 58 minutes 20 seconds East, a distance of 47.00 feet; thence leaving said South line South 00 degrees 18 minutes 56 seconds East, a distance of 224.67 feet to a point on the North right-of-way line of Wallace Street; thence along said North line South 89 degrees 57 minutes 29 seconds West, a distance of 47.00 feet to the POINT OF BEGINNING.

PARCEL K—I:

AND

North 126 feet of the South 243.7 feet of East 50 feet of West 198.5 feet of Lot 10, Subdivision of the HARNEY HOMESTEAD, according to the Plat there of, as recorded in Plat Book C, Page 53, of the Public Records of Orange County, Florida.

AND

PARCEL K-2:

The North 100 feet of the South 200 feet of the West 148.5 feet of Lot 10, SUBDIVISION OF THE HARNEY HOMESTEAD, according to the Plat there of, as recorded in Plat Book C, Page 53, Public Records of Orange County, Florida. AND

DESCRIPTION: (CONTINUED)

PARCEL K-3:

Begin at the Northwest corner of Lot 10, run East 145.0 feet along the North line of Lot 10, thence run South 00 degrees 07 minutes 04 seconds East 105.5 feet, thence run South 89 degrees 59 minutes 34 seconds East 3.5 feet more or less, to the Northwest corner of the above described Parcel K-1, thence South 00 degrees 07 minutes 04 seconds East 43.5 feet more or less, to the Northeast corner of the above described Parcel K-2, thence run North 89 degrees 59 minutes 34 seconds West along the North line of Parcel K-2, 148.5 feet more or less, to the Northwest corner of Parcel K-2, thence North 00 degrees 07 minutes 04 seconds West 149.0 feet more or less, to the POINT OF BEGINNING, all within the SUBDIVISION OF THE HARNEY HOMESTEAD, according to the Plat there of, as recorded in Plat Book C, Page 53, Public Records of Orange County, Florida.

From the Northwest corner of Lot 10, run East 145.0 feet along the North line of Lot 10; thence run South 00 degrees 02 minutes 36 seconds West 105.5 feet to the POINT OF BEGINNING; thence run East 3.5 feet to the Northwest corner of the above described Parcel K-1, thence South 00 degrees 02 minutes 36 seconds West 43.5 feet to the Northeast corner of the above described Parcel K-2, thence run West along the North line of Parcel K-2, 148.5 feet to the Northwest corner of Parcel K-2, thence North 00 degrees 02 minutes 36 seconds East 24.53 feet; thence South 89 degrees 13 minutes 04 seconds East 145.01 feet; thence North 00 degrees 02 minutes 36 seconds East 21.15 feet to the POINT OF BEGINNING, all within the SUBDIVISION OF HARNEY HOMESTEAD, according to the Plat there of, as recorded in Plat Book C, Page 53, Public Records of Orange County, Florida.

PARCEL K-4:

AND

AND

AND

AND

AND

AND

A portion of Lot 10, SUBDIVISION OF HARNEY HOMESTEAD, as recorded in Plat Book C, Page 53, of the Public Records of Orange County, Florida, being more particularly described as follows: Commence at the Northwest corner of said Lot 10; thence due East145.00 feet along the North line of said Lot 10 for a POINT OF BEGINNING; thence continue along said North line, due East 53.50 feet to the intersection of said North line and the Northerly prolongation of the East line of the North 126 feet of the South 243.7 feet of the East 50.00 feet of the West 198.50 feet of said Lot 10; thence along said east line, South 00 degrees 08 minutes 50 seconds West 105.50 feet of the West 198.50 feet of said Lot 10; thence from said point, due West 53.50 feet; thence North 00 degrees 08 minutes 50 seconds East 105.50 feet to the POINT OF BEGINNING.

PARCEL K-5

The West 110 feet of South 50 feet of Lot 8, SUBDIVISION OF THE HARNEY HOMESTEAD, according to the Plat thereof, as recorded in Plat Book C, Page 53, Public Records of Orange County, Florida.

PARCEL K-6:

Lot 8, LESS the West 110 feet of South 50 feet of Lot 8, SUBDIVISION OF THE HARNEY HOMESTEAD, according to the Plat thereof, as recorded in Plat Book C, Page53, Public Records of Orange County, Florida.

AND PARCEL K-7

The West 119.83 feet of the North 150 feet of Lot 7, SUBDIVISION OF THE HARNEY HOMESTEAD, according to the Plat thereof, as recorded in Plat Book C, Page 53, Public Records of Orange County, Florida.

PARCEL K-8:

The West 120 feet of the South 145 feet of Lot 7, SUBDIVISION OF THE HARNEY HOMESTEAD, according to the Plat thereof, as recorded in Plat Book C, Page 53, Public Records of Orange County, Florida.

PARCEL K-9:

The East 75 feet of the West 194.83 feet of the North 150 feet of Lot 7, HARNEY HOMESTEAD, according to the Map or Plat thereof, as recorded in Plat Book C, Page 53, Public Records of Orange County, Florida.

PARCEL K-10:

The East 75 feet of the West 269.83 feet of the North 150 feet of Lot 7, HARNEY HOMESTEAD, according to the Map or Plat thereof, as recorded in Plat Book C, Page 53, Public Records of Orange County, Florida

PARCEL K-11:

Begin 763 feet East and 250 feet North of the South west corner of Lot 10, HARNEY HOMESTEAD, as per Plat thereof, recorded in Plat Book C, Page 53, Public Records of Orange County, Florida, run North 251.51 feet, West 348 feet, South 251.5 feet, East 348 feet to POINT OF BEGINNING.

Less and except there from, that portion there of conveyed by Pine Castle Methodist Church, Inc., a Florida corporation, to Charles E. Maull, Jr. and June L. Maull, by Quit Claim Deed recorded August 21, 2003 in Official Records Book 7061, Page 4692, Public Records of Orange County, Florida, more particularly described as follows:

A portion of Lot 7, Subdivision of HARNEY HOMESTEAD, Plat Book "C", page 53, Public Records of Orange County, Florida, being more particularly described as follows:

Begin at the Southeast corner of the East 75 feet of the West 269.83 feet of the North 150 feet of said Lot 7; thence East 197.48 feet along the South line of the North 150 feet of said Lot 7 to a point on the East line of lands described in Official Records Book 6253, Page 6532, Public Records of Orange County, Florida; thence South 00 degrees 28 minutes 01 seconds East 11.10 feet along said East line; thence North 89 degrees 42 minutes 36 seconds West 197.60 feet to a point on a Southerly projection of the East line of the East 75 feet of the West 269.83 feet of the North 150 feet of said Lot 7; thence North 00 degrees 08 minutes 50 seconds East 10.10 feet along said southerly projection to the POINT OF BEGINNING.

PARCEL K-12:

AND

AND

AND

Beginning 465 feet East of the Southwest corner of Lot 10, HARNEY HOMESTEAD, in Section 24, Township 23 South, Range 29 East, as per Plat thereof, as recorded in Plat Book C, Page 53, Public Records of Orange County, Florida, run East 298 feet, North 250 feet, West 298 feet, and South 250 feet to the POINT OF BEGINNING.

PARCEL 1

Lots 5 and 6, less the West 10 feet of lot 6 for road right—of—way, J.G. TYNER SUBDIVISION, according to the map or lat thereof recorded among the Public Records of Orange County, Florida in Plat Book F, Page 44.

PARCEL 2:

The East 100 feet of Lot 9, (less the North 391.8 feet thereof), HARNEY HOMESTEAD, Plat Book C, Page 53, Public Records of Orange County, Florida.

F.E.G. PROJECT NO.

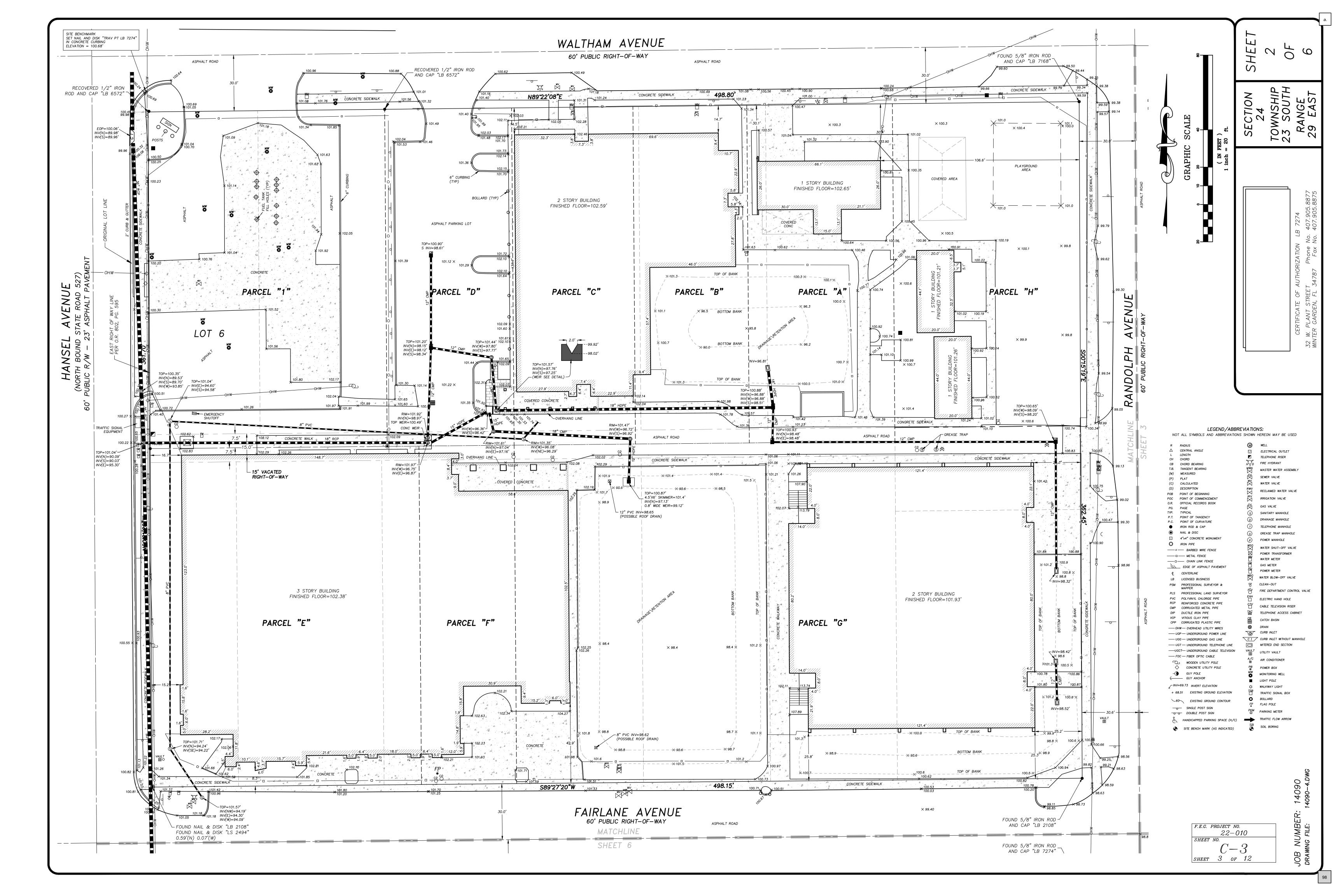
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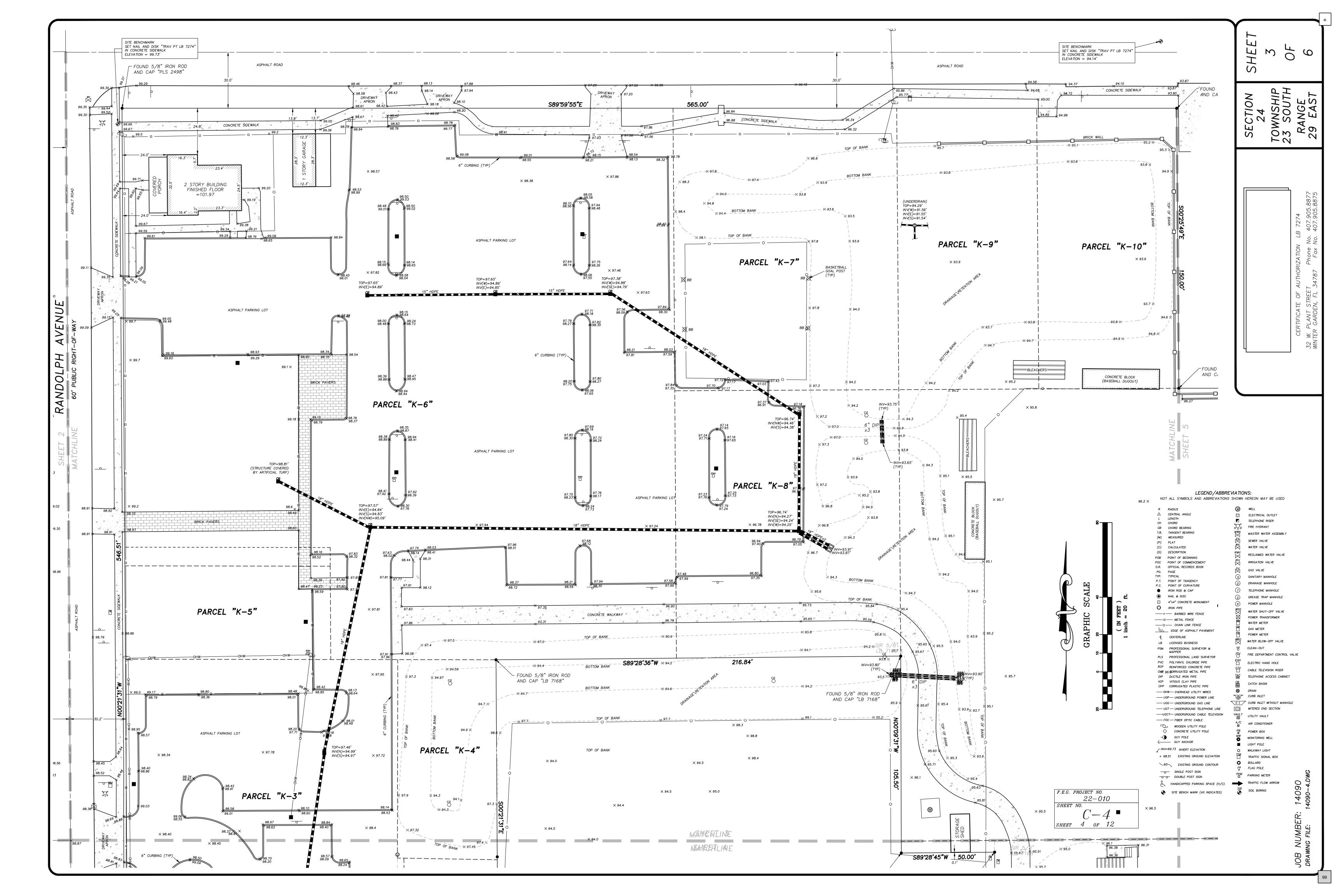
22-010

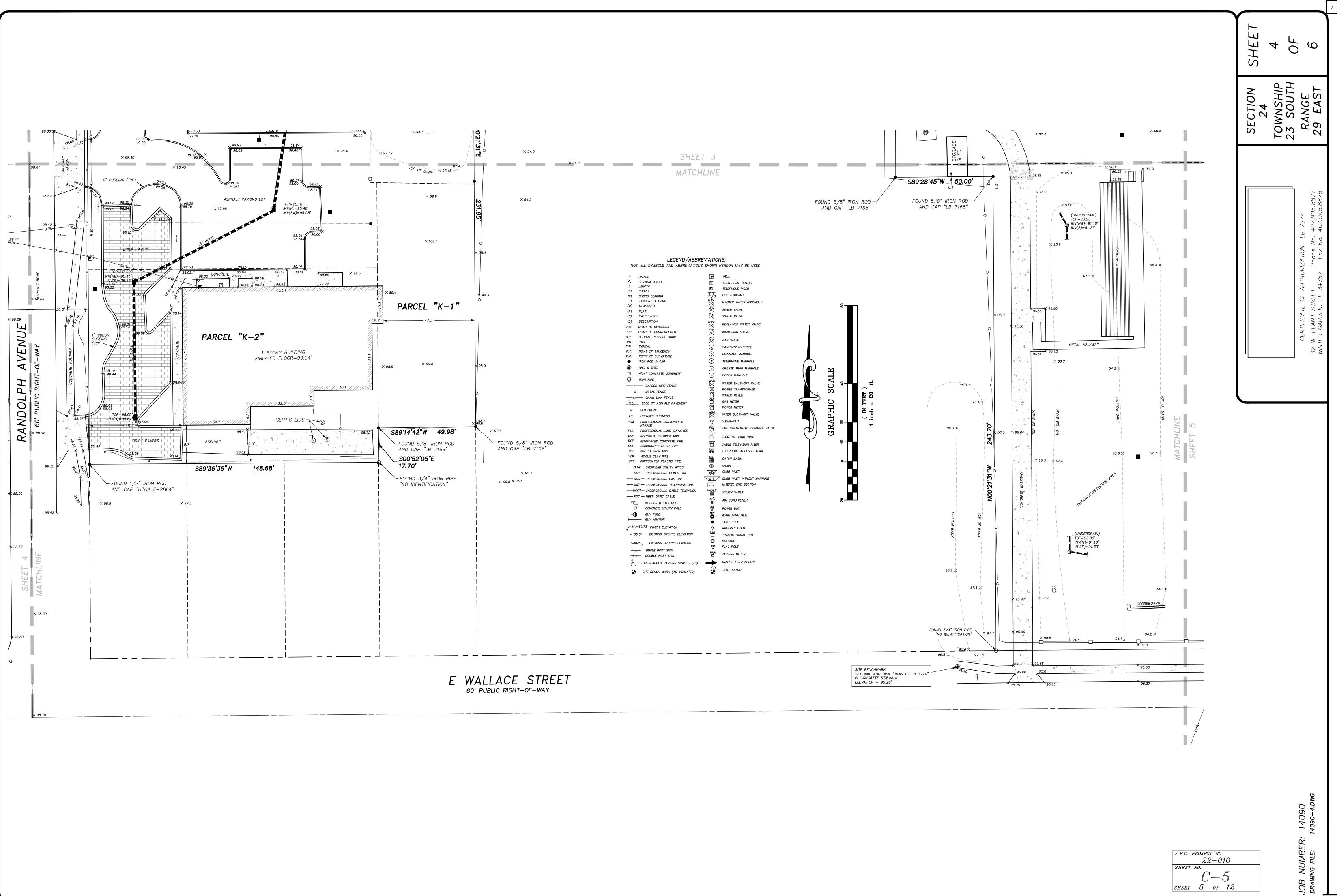
C-2

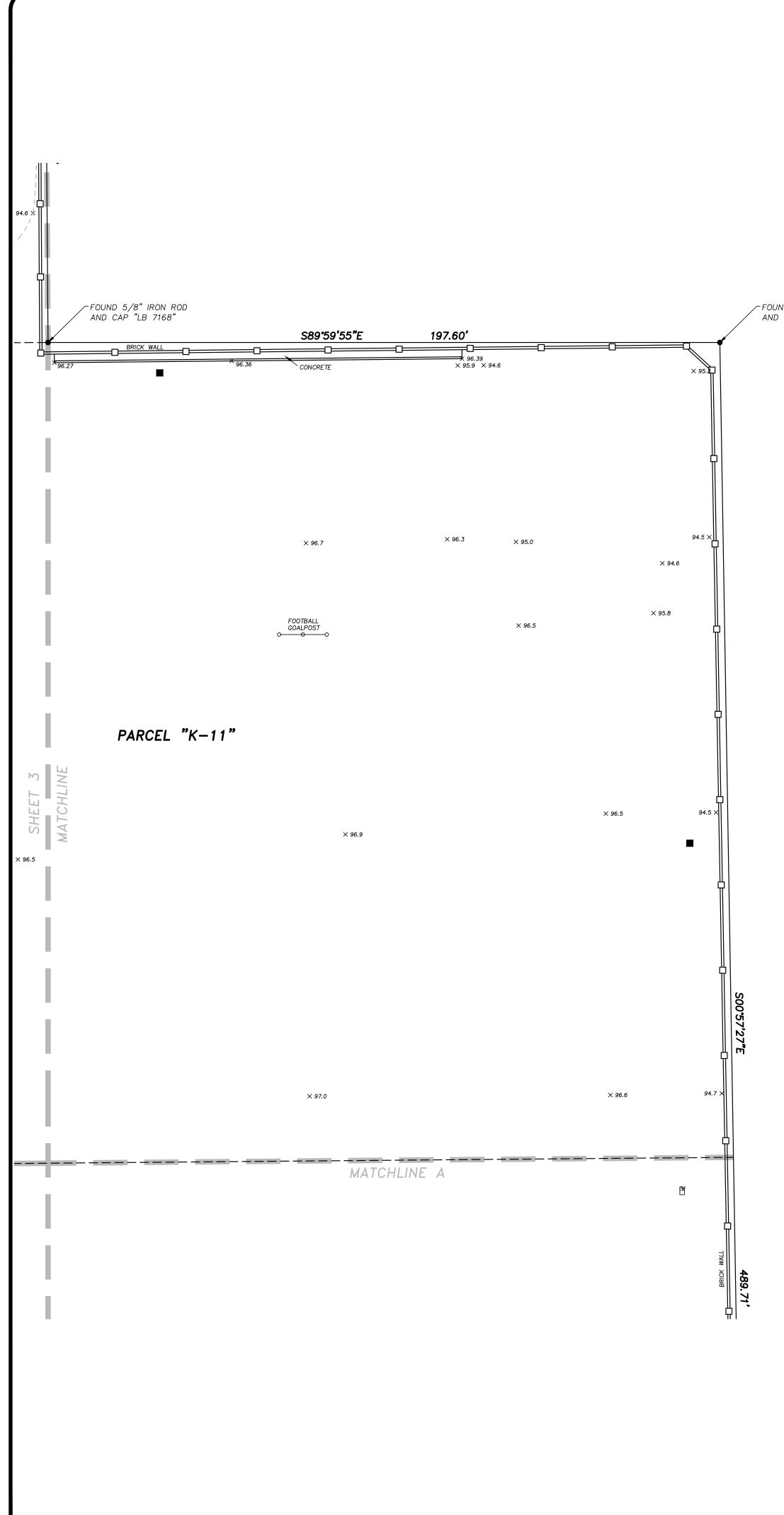
Sheet 2 of 12

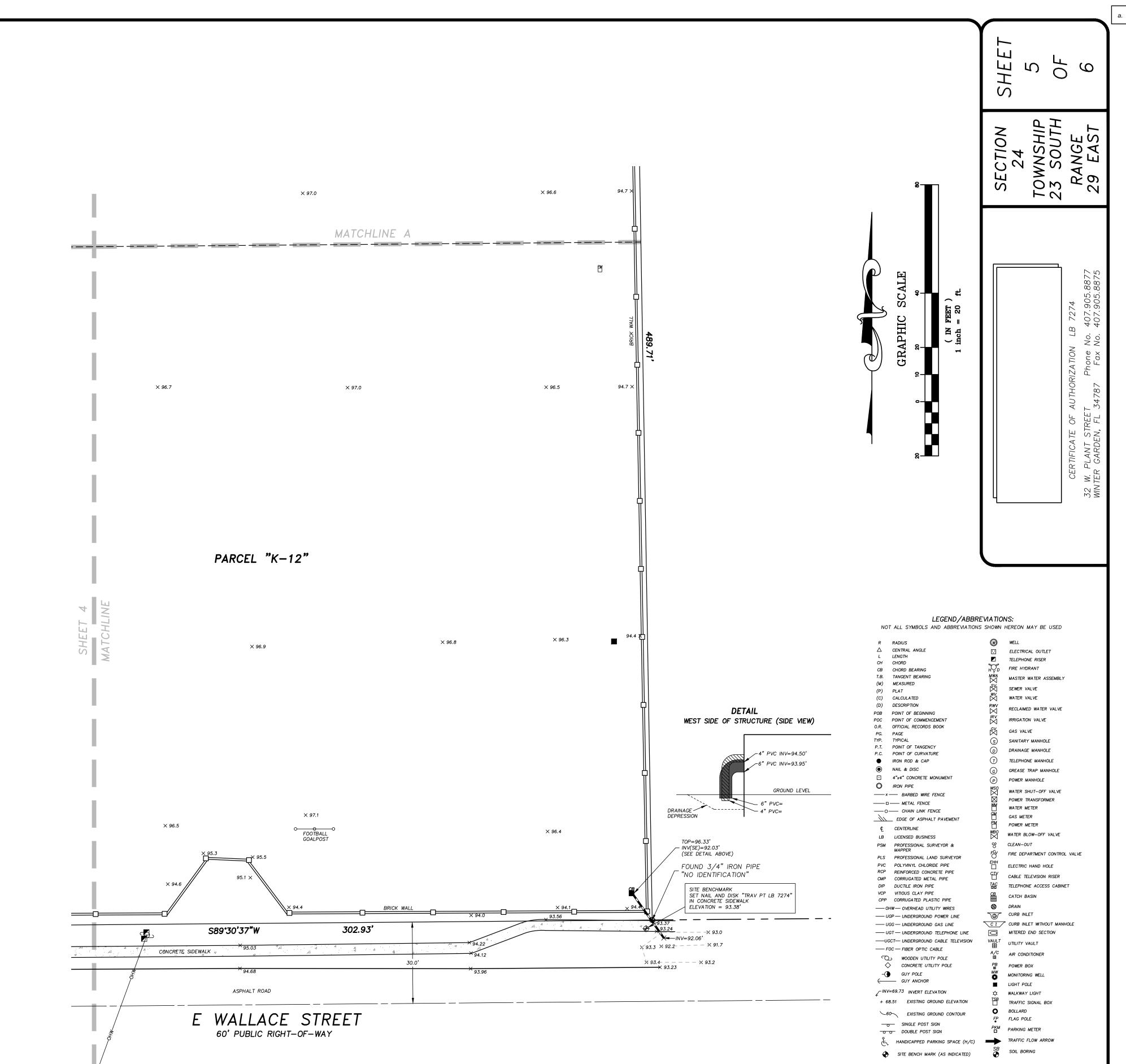
JOB NUMBER: 14090	REVISIONS:			SECTION	SHFFT
				ГС	
SURVEY DATE: 8–24–2021				7 4	7
FIELD BY: T. CONARD					
FIELD BOOK: 2002					
PAGES: 34–39, 73–76				23 SOUTH	
FIELD FILE: 14090–3.MJF				- - -)))	С С
			UERIIFICAIE UF AUIMURIZAIIUN EB 1214	RANGF	
DRAWING FILE: 14090–4.DWG			32 W. PLANT STREET Phone No. 407.905.8877		Ś
		ARON D. BISHMAN, P.S.M. FLURIDA REGISTRATION NO. 3000	WINTER GARDEN, FL 34787 Fax No. 407.905.8875	ZY EADI	





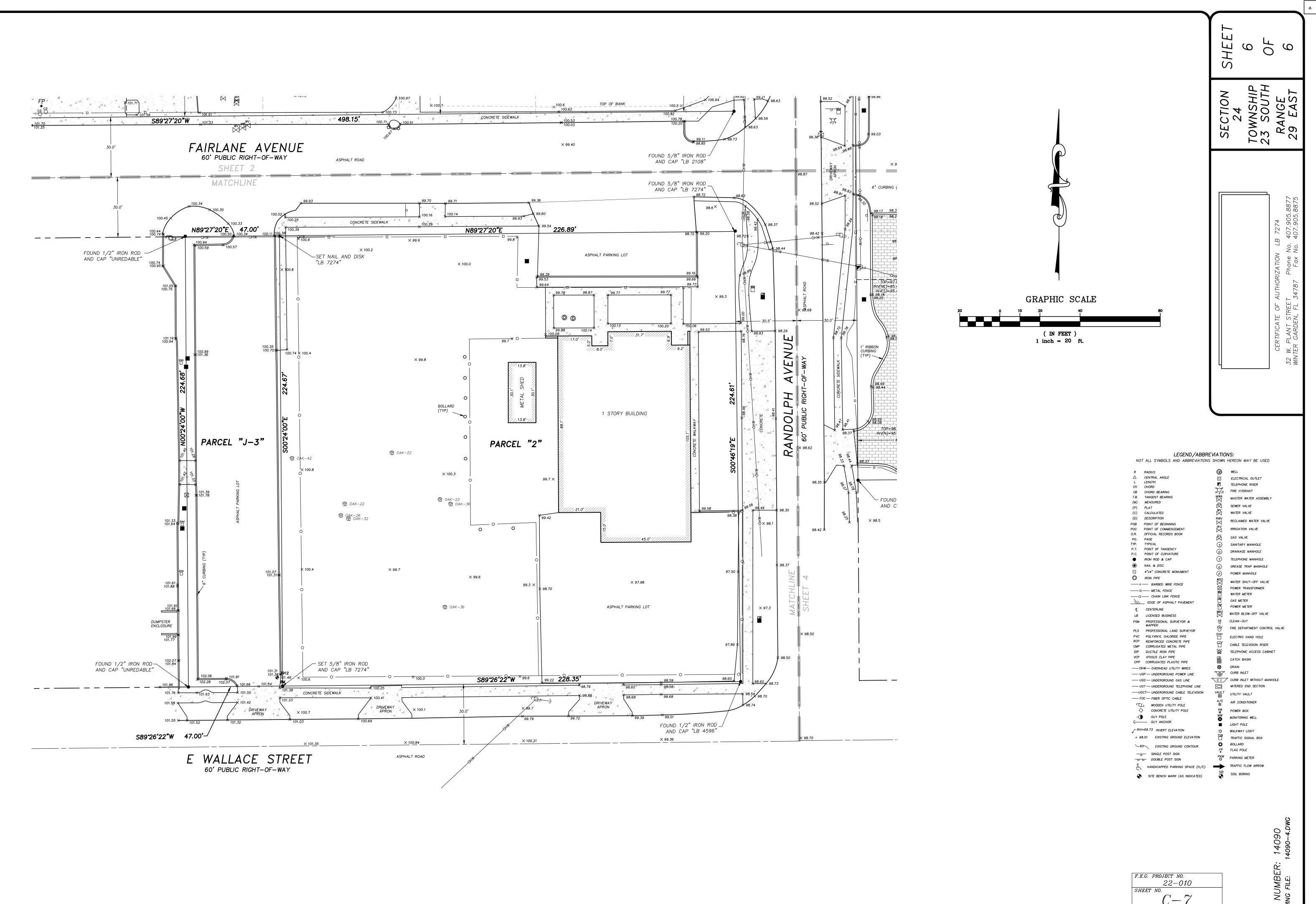






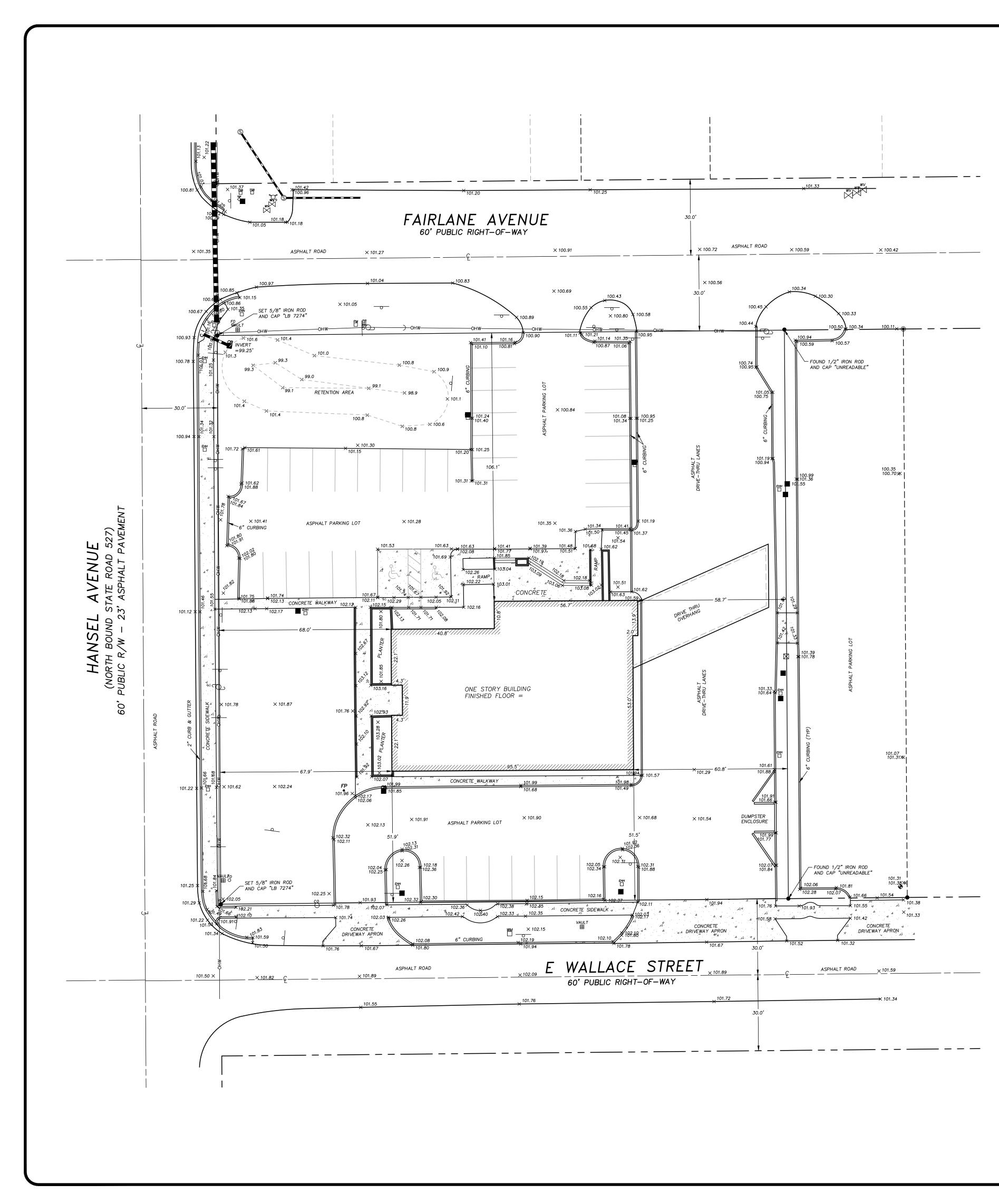
/ FOUND 5/8" IRON ROD AND CAP "LB 7168"

	14090 14090-4.DWG
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22-010	VG FILE:
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Sheet 6 of 12	drawing



.E.G. PRO.	JECT NO.
	22–010
HEET NO.	
1	C-7
heet 7	′оғ 12

JOB draw



BOUNDARY AND TOPOGRAPHIC SURVEY



LOT 9 OF HARNEY HOMESTEAD, AS RECORDED IN PLAT BOOK C, PAGE 53, OF THE PUBLIC RECORDS OF ORANGE COUNTY, FLORIDA, LESS THE EAST 100 FEET THEREOF; LESS THE NORTH 391.8 FEET THEREOF; LESS PORTIONS OF ROAD RIGHT-OF-WAY ON THE NORTH, BOUNDED BY FAIRLANE AVENUE, AND ON THE SOUTH, BOUNDED BY EAST WALLACE STREET, AS THE SAME MAY HAVE BEEN CONVEYED TO OR TAKEN BY THE CITY OF BELLE ISLE OF ORANGE COUNTY, FLORIDA FOR ROAD WIDENING PURPOSES.

LESS AND EXCEPT:

THAT PART OF LOT 9 CONVEYED TO THE STATE OF FLORIDA BY SPECIAL WARRANTY DEED RECORDED IN OFFICIAL RECORDS BOOK 779, PAGE 14, OF THE OFFICIAL RECORDS OF ORANGE COUNTY, FLORIDA, BEING DESCRIBED AS FOLLOWS:

THAT PART OF LOT 9, HARNEY HOMESTEAD SUBDIVISION, AS SHOWN IN PLAT BOOK "C", PAGE 53, SAID PUBLIC RECORDS, LESS THE NORTH 391.8 FEET OF SAID LOT 9; LYING WITHIN 30 FEET EASTERLY OF THE SURVEY LINE OF STATE ROAD 527, SECTION 75040, SAID SURVEY LINE BEING DESCRIBED AS FOLLOWS:

BEGIN AT THE EASTERLY EXTENSION OF THE NORTH LINE OF LOT 18, JOHN KEEN'S SUBDIVISION, PLAT BOOK "H", PAGE 11, PUBLIC RECORDS, ORANGE COUNTY, FLORIDA, AT A POINT 31.16 FEET EAST OF THE NORTHEAST CORNER OF SAID LOT 18, AND RUN THENCE NORTH 00"15'17" WEST, 579.36 FEET TO THE CENTER OF SECTION 24, TOWNSHIP 23 SOUTH, RANGE 29 EAST:

ALSO, THE EAST 30 FEET OF THE WEST 60 FEET OF THE SOUTH 30 FEET OF THE NORTH 421.8 FET OF SID LOT 9, HARNEY HOMESTEAD:

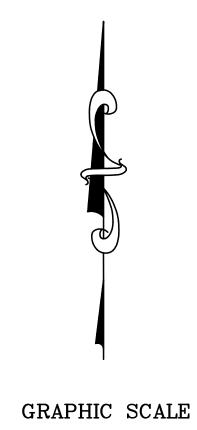
ALSO, THAT PART OF SAID LOT 9, HARNEY HOMESTEAD, LYING WITHIN 30 FEET NORTHERLY OF A LINE DESCRIBED AS FOLLOWS: COMMENCE ON THE EASTERLY EXTENSION OF THE NORTH LINE OF LOT 18, JOHN KEEN'S SUBDIVISION, PLAT BOOK "H", PAGE 11, PUBLIC RECORDS, ORANGE COUNTY, FLORIDA, AT A POINT 31.16 FEET EAST OF THE NORTHEAST CORNER OF SAID LOT 18, AND RUN THENCE NORTH 00"15'47" WEST, 33.70 FEET FOR A POINT OF BEGINNING; FROM SAID POINT OF BEGINNING RUN SOUTH 89°42'47" EAST, 60 FEET.

FURTHER LESS AND EXCEPT:

THAT PART CONVEYED TO PINE CASTLE METHODIST CHURCH, INC. BY SPECIAL WARRANTY DEED RECORDED IN OFFICIAL RECORDS BOOK 8382, PAGE 274, OF THE OFFICIAL RECORDS OF ORANGE COUNTY, FLORIDA, BEING DESCRIBED AS FOLLOWS:

COMMENCE AT THE SOUTHWEST CORNER OF LOT 9 OF THE HARNEY HOMESTEAD, AS RECORDED IN PLAT BOOK "C", PAGE 53, OF THE PUBLIC RECORDS OF ORANGE COUNTY, FLORIDA, THENCE RUN NORTH 89'57'29" EAST ALONG THE NORTH RIGHT-OF-WAY LINE OF WALLACE STREET AS SHOWN AND DEPICTED ON THE PLAT OF KEEN-CASTLE, AS RECORDED IN PLAT BOOK "P", PAGE "1", OF SAID PUBLIC RECORDS, A DISTANCE OF 224.28 FEET TO THE POINT OF BEGINNING; THENCE NORTH 00'04'16" EAST ALONG THE EAST LINE OF THE WEST 224.28 FEET OF SAID LOT 9, A DISTANCE OF 224.70 FEET TO A POINT ON THE SOUTH RIGHT-OF-WAY LINE OF FAIRLANE AVENUE; THENCE ALONG SAID SOUTH LINE SOUTH 0018'56" EAST ALONG THE WEST LINE OF THE EAST 100.00 FEET OF SAID LOT 9, A DISTANCE OF 224.65 FEET TO A POINT ON THE NORTH RIGHT-OF-WAY LINE OF WALLACE STREET; THENCE ALONG SAID NORTH LINE SOUTH 89°57'29" WEST, A DISTANCE OF 175.47 FEET TO THE POINT OF BEGINNING.

THE ABOVE DESCRIBED PARCEL CONTAINS A TOTAL OF 1.156 ACRES, MORE OR LESS.



(IN FEET) 1 inch = 20 ft.

SURVEYORS NOTES:

1. NOT VALID WITHOUT THE ORIGINAL SIGNATURE AND SEAL, OR DIGITAL SIGNATURE AND SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. PRINTED COPIES OF A DIGITAL SIGNED AND SEALED SURVEY ARE NOT VALID.

2. LANDS SHOWN HEREON WERE NOT ABSTRACTED FOR RIGHTS OF WAY, EASEMENTS, OWNERSHIP, OR OTHER INSTRUMENTS OF RECORD, BY THIS FIRM.

3. REVISIONS DO NOT CONSTITUTE A RE-CERTIFICATION OF THE EXISTING FIELD CONDITIONS OF THIS SURVEY. 4. BEARINGS SHOWN HEREON ARE BASED ON THE NORTH

RIGHT-OF-WAY LINE OF E WALLACE STREET (ASSUMED). 5. THE DESCRIPTION SHOWN HEREON WAS SUPPLIED BY THE CLIENT.

6. UNDERGROUND IMPROVEMENTS AND INSTALLATIONS HAVE NOT BEEN LOCATED.

7. THE LANDS SHOWN HEREON LIE ENTIRELY WITHIN ZONE X (AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOOD) ACCORDING TO "FIRM" MAP NO. 12095C0430F, DATED SEPTEMBER 25, 2009.

3. THE ELEVATIONS SHOWN HEREON ARE BASED ON ORANGE COUNTY DATUM PER BENCH MARK NUMBER S1316-035 BEING A BOX CUT ON TOP OF CURB INLET, ELEVATION = 101.52 (NAVD 1988). 9. SITE BENCHMARK IS SHOWN HEREON.

10. THIS SURVEY WAS PERFORMED IN ACCORDANCE WITH THE STANDARDS OF PRACTICE SET FORTH IN RULE 5J-17.052 ADOPTED BY THE FLORIDA BOARD OF SURVEYORS AND MAPPERS, PURSUANT TO FLORIDA STATUTES 472.027.

R	RADIUS		WELL
${\scriptstyle \bigtriangleup}$	CENTRAL ANGLE LENGTH		ELECTRICAL OUTLET
CH	CHORD		TELEPHONE RISER
СВ	CHORD BEARING	нÇD	FIRE HYDRANT
Т.В.	TANGENT BEARING	MŴA	MASTER WATER ASSEMBLY
(M)	MEASURED	ÍSV Í	SEWER VALVE
(P) (C)	PLAT CALCULATED	Ŵ	WATER VALVE
(0) (D)	DESCRIPTION	RWV	
POB	POINT OF BEGINNING	\bowtie	RECLAIMED WATER VALVE
POC	POINT OF COMMENCEMENT	$\overset{RV}{\bowtie}$	IRRIGATION VALVE
0.R.	OFFICIAL RECORDS BOOK	Ř	GAS VALVE
PG. TYP.	PAGE TYPICAL	S	SANITARY MANHOLE
P.T.	POINT OF TANGENCY	0 0	DRAINAGE MANHOLE
P.C.	POINT OF CURVATURE	-	
•	IRON ROD & CAP		TELEPHONE MANHOLE
\odot	NAIL & DISC	G	GREASE TRAP MANHOLE
	4"x4" CONCRETE MONUMENT	P	POWER MANHOLE
0	IRON PIPE	wso	WATER SHUT-OFF VALVE
x		\boxtimes	POWER TRANSFORMER
	WOOD FENCE	WW	WATER METER
<u> </u>	CHAIN LINK FENCE	Ē	GAS METER
	EDGE OF ASPHALT PAVEMENT	PM	POWER METER
£	CENTERLINE	₩ВО	WATER BLOW-OFF VALVE
LB	LICENSED BUSINESS		CLEAN-OUT
PSM	PROFESSIONAL SURVEYOR & MAPPER	°° ₽°∨	
PLS	PROFESSIONAL LAND SURVEYOR	'О' ЕНН	FIRE DEPARTMENT CONTROL VAL
PVC	POLYVINYL CHLORIDE PIPE		ELECTRIC HAND HOLE
RCP CMP	REINFORCED CONCRETE PIPE CORRUGATED METAL PIPE		CABLE TELEVISION RISER
DIP	DUCTILE IRON PIPE		TELEPHONE ACCESS CABINET
VCP	VITIOUS CLAY PIPE	CB	CATCH BASIN
CPP	CORRUGATED PLASTIC PIPE		
<u> </u>	HW— OVERHEAD UTILITY WIRES	<u></u>	
	JGP — UNDERGROUND POWER LINE		CURB INLET
	IGG — UNDERGROUND GAS LINE		CURB INLET WITHOUT MANHOLE
	JGT — UNDERGROUND TELEPHONE LINE	0	MITERED END SECTION
	GCT— UNDERGROUND CABLE TELEVISION	VAULT 目	UTILITY VAULT
	OC — FIBER OPTIC CABLE	A/C	AIR CONDITIONER
7	N	© PB	
	CONCRETE UTILITY POLE GUY POLE	⊞ MW	POWER BOX
	GUY FOLE	0	MONITORING WELL
			LIGHT POLE
+	/=69.73 INVERT ELEVATION	Ф TSB	WALKWAY LIGHT
+ 6	8.51 EXISTING GROUND ELEVATION		TRAFFIC SIGNAL BOX
$\searrow \epsilon$	EXISTING GROUND CONTOUR	0	BOLLARD
		FP	FLAG POLE
<u> </u>	- DOUBLE POST SIGN		PARKING METER
æ	HANDICAPPED PARKING SPACE (H/C)	\rightarrow	TRAFFIC FLOW ARROW
		SB	SOIL BORING
Ð	SITE BENCH MARK (AS INDICATED)	Ð	COL DOMINO

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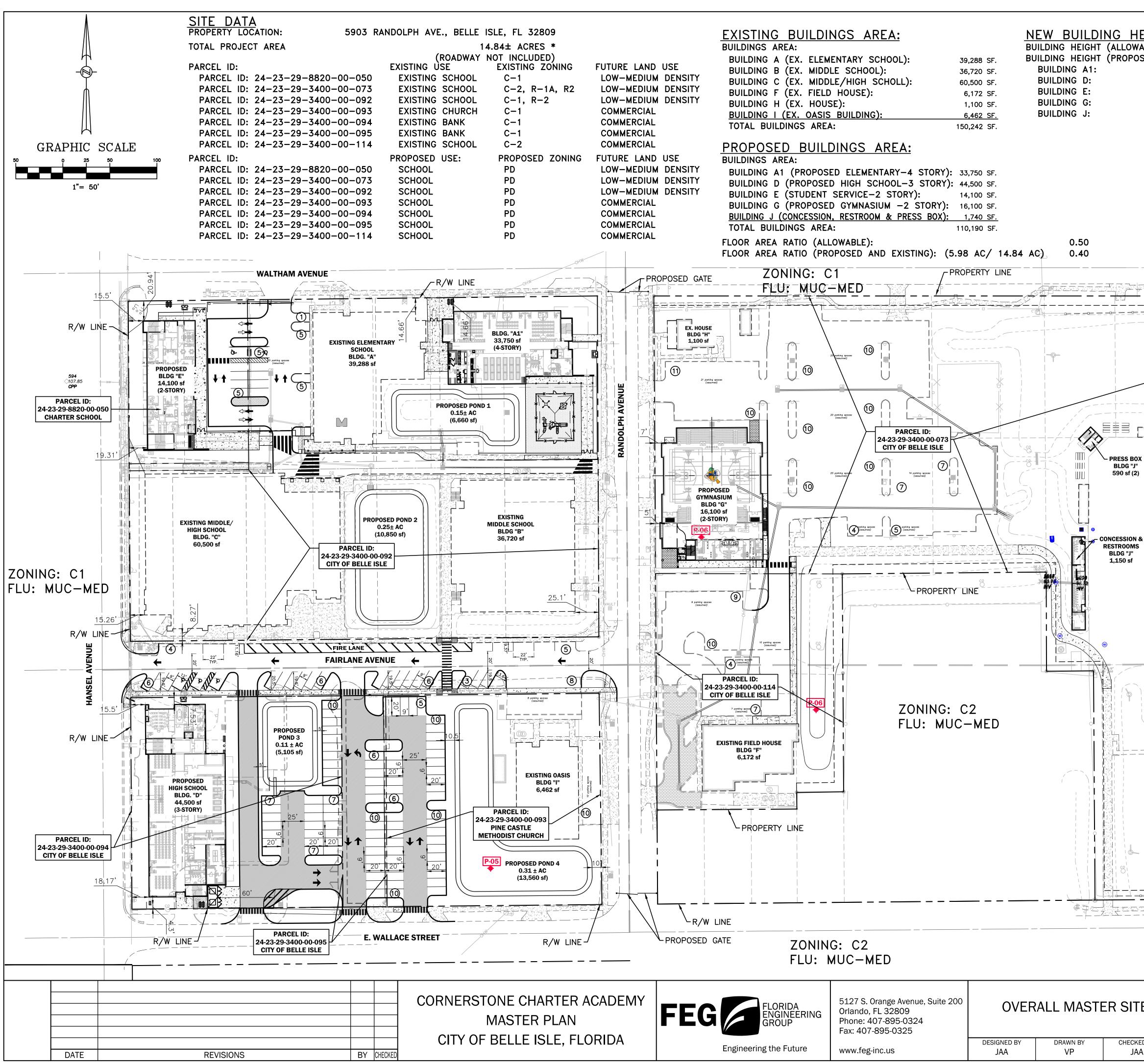
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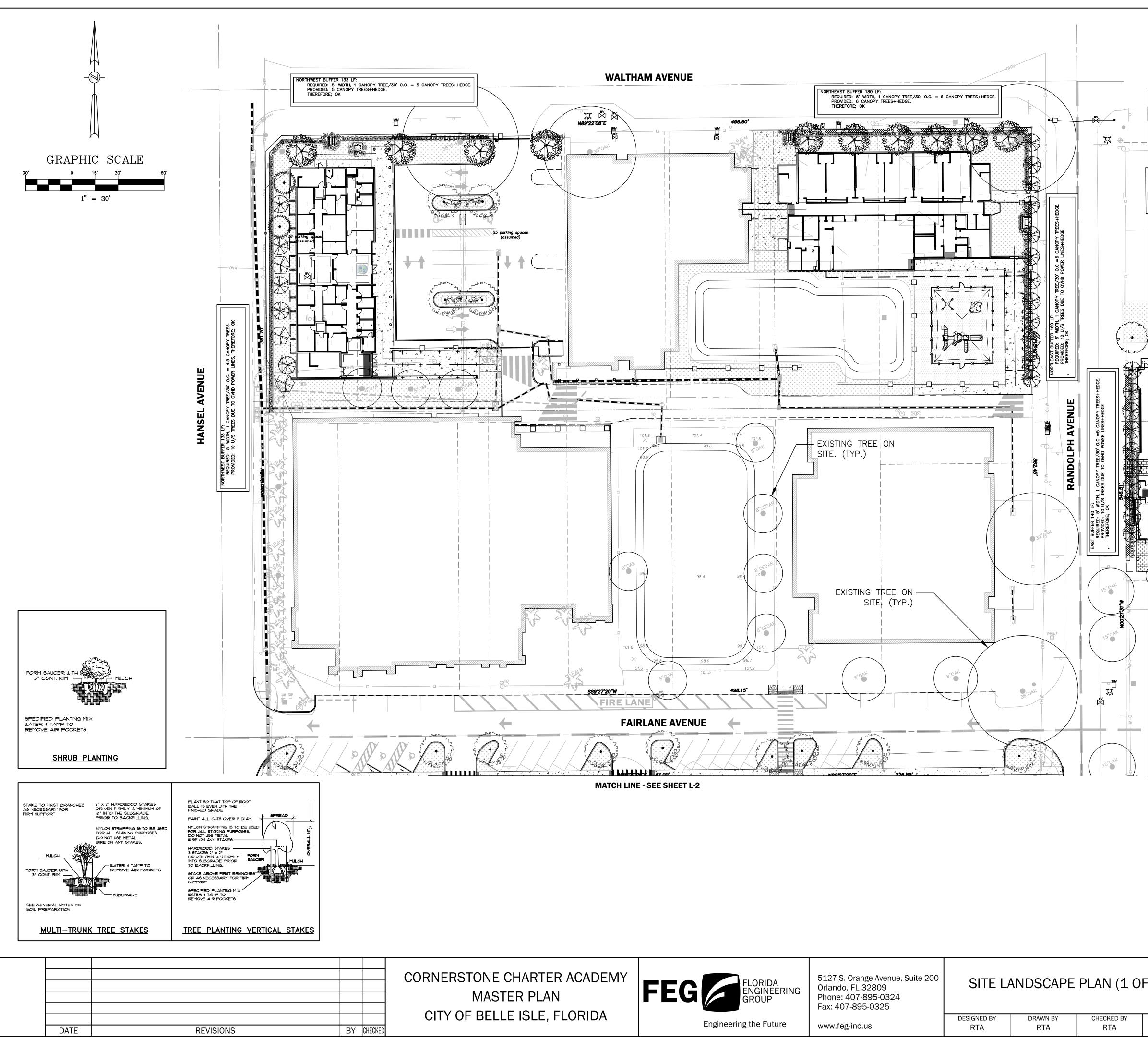
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SHEET 8 OF 12

SHFFT					1	
SECTION	24	TOWNSHIP	23 SOUTH	EANCE	29 EAST	
				CERTIFICATE OF AUTHORIZATION LB 7274	301 N. TUBB STREET, SUITE 106, OAKLAND, FLORIDA 34760 Phone No. 407.905.8877	
					ARON D. BISHMAN, P.S.M. FLORIDA REGISTRATION NO. 5668	
REVISIONS:						
JOB NUMBER: 22029.000	SURVEY DATE: 3–15–2022	FIELD BY: T. CONARD FIELD BOOK: 2201		FIELD FILE: 14090–3.MJF	DRAWING FILE: 22029.DWG	



<u>HT:</u>):	57'	PARKING PARKING REQUIRED STUDENTS 2 STAFF	2,420 STÚDENTS/13		186/ GRADE * 2) * ⁻ 1 SPACE PER STAFF)		7 SPACES
	56' 54'	VISITOR TOTAL PARKING REQUIRED SPACES REQUIRED TO BI (*) HIGH SCHOOL STUDE	2, D E RESERVED FOR H	,420/100 (*	1 SPACE PER 100 ST	UDENTS) 24 22	SPACES SPACES SPACES SPACES
	44' 50' 16'	PARKING PROVIDED EXISTING PARKING SPACE EXISTING PARKING SPACE NEW PARKING SPACES HANDICAP PARKING) ES)		-6 13	9 SPACES 7 SPACES 4 SPACES 5 SPACES
		TOTAL PARKING PROVIDED					1 SPACES
		BUILDINGS FOOTPRINT EXISTING BUILDINGS TOTAL EXISTING CONCRE TOTAL IMPERVIOUS AREA	TE, SIDEWALK & AS	•	75,729 ±S.F. 187,460 ±S.F. 263,189 ±S.F.		40.70 %
		EXISTING IMPERVIOUS AREA EXISTING IMPERVIOUS TO B EXISTING IMPERVIOUS TO B POND AREAS PERVIOUS AREA	BE REMOVED (OLD TEX	KACO SITE & OT NK OF AMER. S	[HERS)(-)41,222 ±S.F.	1.27 ±AC.	40.70 % 8.56 % 50.74 %
		TOTAL SITE AREA (*) ROADWAYS NOT INCL	.UDED IN THIS SITE	DATA	646,302 ±S.F.		100.00 %
		BUILDINGS FOOTPRINT EXISTING BUILDINGS		<u>ONS (P</u>	ROPOSED 8	<u>EXISTIN</u>	<u>G)</u>
	.	PROPOSED BUILDINGS <u>PROPOSED & EXISTING (</u> IMPERVIOUS AREA POND AREAS DEDVIOUS AREA		<u>_K & ASPHALT</u>	289,954 ±S.F. 71,185 ±S.F.	6.66 ±AC. 1.63 ±AC.	44.88 % 10.98 %
		<u>PERVIOUS AREA</u> TOTAL SITE AREA (*) ROADWAYS NOT INCL	UDED IN THIS SITE	DATA	<u>282,163 ±S.F.</u> 646,302 ±S.F.	<u>6.55 ±AC.</u> 14.84 ±AC.*	<u>44.14</u> % 100.00 %
		CITY OF BELLE ISLE ALLOWABLE IMPERVIOUS	SURFACE RATIO –	ISR	REMENTS		0.80
		PROPOSED IMPERVIOUS (SEE SITE AREA CALCUL)		ISR			0.45
		G: R1A ES-LOW					
		 = 					
		BUILDING SE	TBACKS		REQUIRED	PROPOSED	
		HANSEL AVENUE WALTHAM AVENUE FAIRLANE AVENUE E. WALLACE STREET RANDOLPH AVENUE (*) DUMPSTER WALL:	(BUILDINGS D & (BUILDINGS A1 (BUILDINGS B, (BUILDING D) (BUILDING G)	& É)	15' 14' 7' 4' 5'	PROPOSED 15.5' 14.6' 7.5' 4.3' (*) 5'	
		LANDSCAPE BUFFE HANSEL AVENUE	ERS (REQUIRED	· F	IDSCAPE BUFFER	15')
В	ASEBALL FIELD	WALTHAM AVENUE FAIRLANE AVENUE E. WALLACE STREET RANDOLPH AVENUE	14' 7' 4' 5'	F	VALTHAM AVENUE TAIRLANE AVENUE I. WALLACE STREET RANDOLPH AVENUE	14' 7' 4' 5'	
	FIELD			OPEN	SPACE CAL	CULATION	IS
				OPEN SPACE	PROVIDED:	44%	
<u>л</u>				SPORT FIELD	ATION AREA AREA:	2.66 AC	
				LIGHTING SHA	ALL MEET CITY OF BE	LLE ISLE	
				THE PROPOS	TER NOTE: SED STORMWATER SYS ^T D MEET THE CITY OF QUIREMENTS.		
		SPORT FIELD		DESIGNED TO	<u>NOTE:</u> SED WATER AND SEWE D MEET ORANGE COUN TLITIES COMMISSION R	ITY UTILITIES AND	
				SUBJECT TO	<u>E:</u> UCTION DETAILS ARE (REVIEW AND MODIFIC, F FINAL CONSTRUCTIO	ATION DURING THE	
			• Д				
		PROJECT NO.	FLORIDA ENGINEER		<u>igitally signed by JEA</u> BI-AOUN	THIS ITEM HAS BEE	
		PROJECT NO. 22-010 SCALE <u>1" = 50'</u> DATE	FLORIDA ENGINEER			THIS ITEM HAS BEE ELECTRONICALLY S SEALED BY (ENGINI NC., (DATE) USING A SHA AUTHENTICATION C	IGNED AND EER), P.E. ON A-1



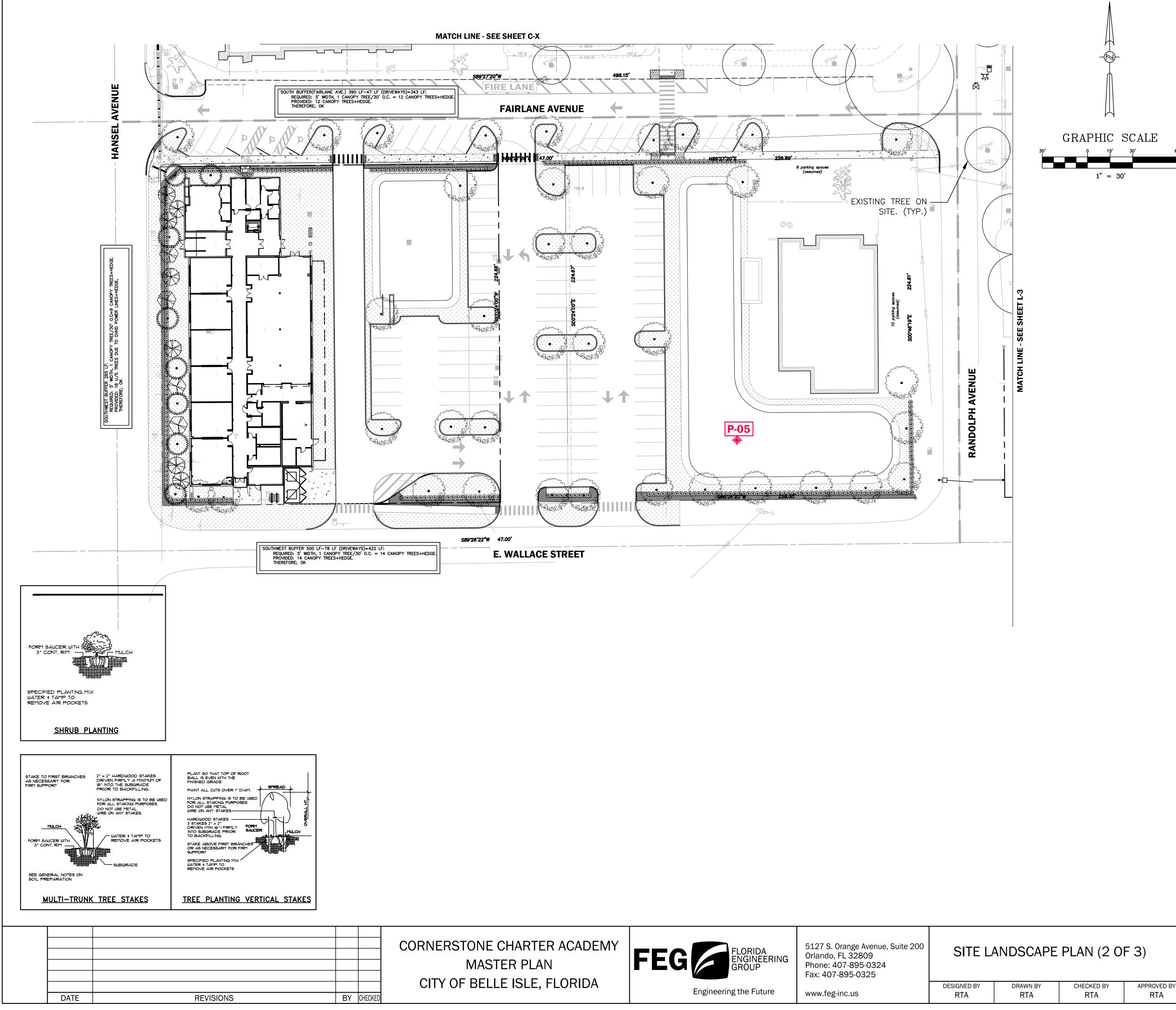
SITE L/	ANDSCAPE	PLAN (1 O	F 3)	PROJECT NO. 22-010 SCALE 1" = 30' DATE FEBRUARY 9, 2023 SHEET NO.	FLORIDA ENGINEERINGIGROUP, INC. CERTIFICATE No. EB 0006595	THIS ITEM HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY (ENGINEER), P.E. ON (DATE) USING A SHA-1 AUTHENTICATION CODE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SHA-1
DESIGNED BY RTA	DRAWN BY RTA	CHECKED BY RTA	APPROVED BY RTA	L-1 SHEET 10 OF 12	CF * FRICKABE A UCENSENO: 1321	AUTHENTICATION CODE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.
						22-010_Landscape Irrigation.dwg

		FLORIDA	NATIVE	LANDSCA	APE SO	CHEDULE	
		-	CAN	NOPY TREES		-	
SYMBOL	ABR.	BOTANICAL NAME	COMMON NAME	MINIMUM SIZE	QUANTITY	REMARKS	NATIV
	QV	QUERCUS VIRIGINIA 'HIGH RISE'	HIGH RISE LIVE OAK	12'HT. 4"CAL.	41	SPECIMEN	Y
•	AR	ACER RUBRUM	RED MAPLE	10'—12'HT. 3"CAL.	1	SPECIMEN	Y
	UA	ULMUS ALATA	WINGED ELM	10'-12' HT. 3" CAL.	10	SINGLE STRAIGHT TRUNK	Y
	LL	LIGUSTRUM LUCIDUM	TREE LIGUSTRUM	10'- HT.	10		Y
	IA	ILEX ATTENUATA 'EAGLESTON'	EAGLESTON HOLLY	6'—8''HT. 3"CAL.	35		Y
	LI	LAGERSTROEMIA INDICA 'NATCHEZ'	NATCHEZ CRAPE MYRTLE	10'HT	7		Y
				SHRUBS			-
*	VB	VIBURNUM ODORATISSIMUM	VIBURNUM	30"HT. 36"O.C.	Х		Y
۲	PA	PLUMBAGO AURICULATA	PLUMBAGO	3 GAL. 24" H 36" O.C.	іт. _Х		Y
	TA	DWARF ASIATIC JASMINE	TRACHELOSPERMUM ASIATICUM	1 GAL. 12" O.C.	Х		Y
			SC	DD/MISC.			
* * * * * * * * *	SOD	BAHIA HYBRID	(QTY. BY CONTRA	CTOR		

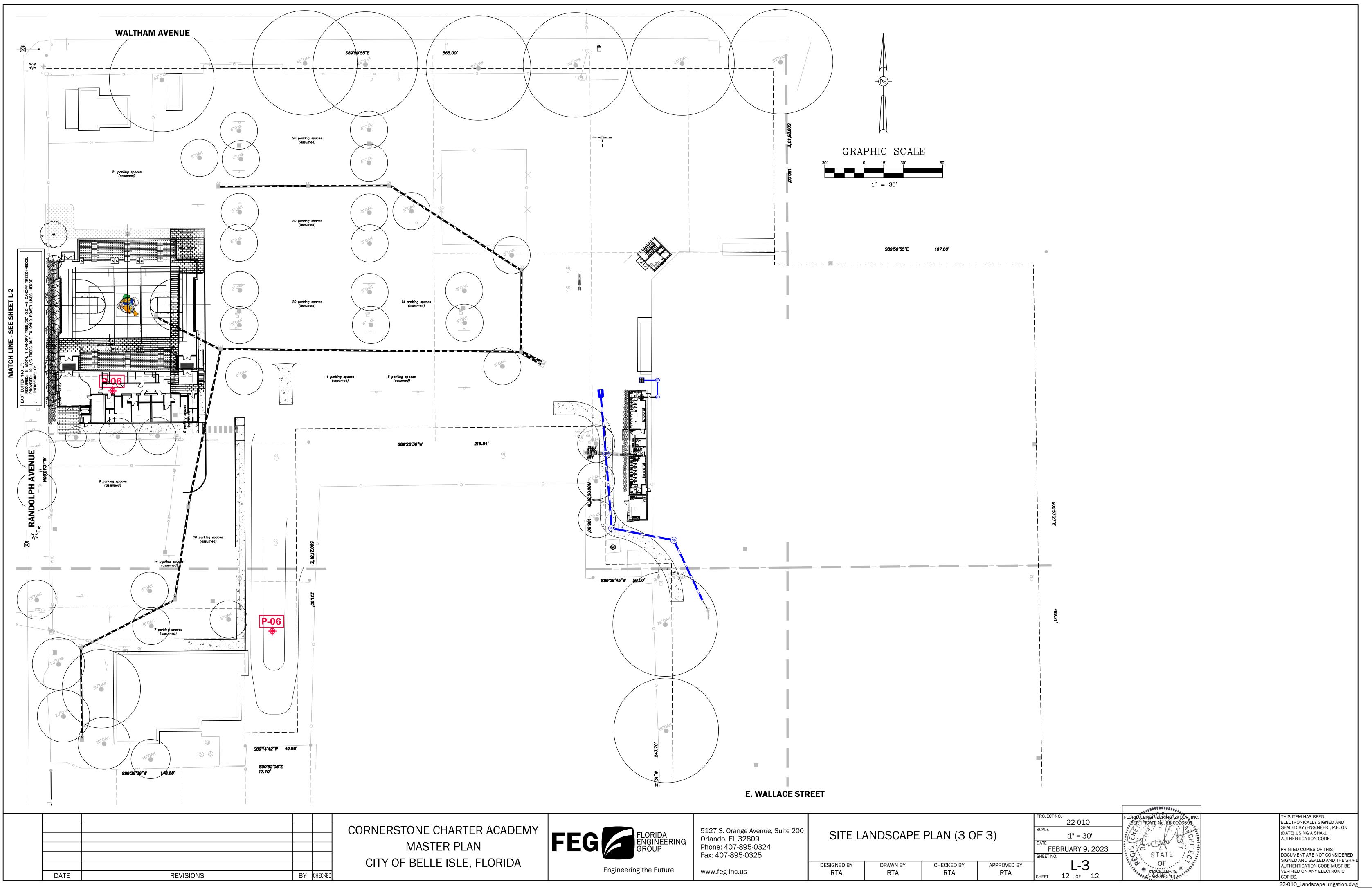
LANDSCAPE NOTES

- 1. IN THE EVENT OF A DISCREPANCY BETWEEN THE PLANT LIST AND THE ACTUAL NUMBER OF PLANTS SHOWN ON THE PLAN, THE PLAN SHALL CONTROL. THE LANDSCAPE CONTRACTOR SHALL NOTIFY FEG OF ANY DISCREPANCY PRIOR TO ISSUANCE OF BIDS.
- 2. ALL NEW PLANTINGS MUST COMPLY WITH THE SIZES AND QUANTITIES SHOWN IN THE PLANT LIST. ALL PLANTS MUST COMPLY WITH THE APPLICABLE JURISDICTIONAL CODE. PER THE FLORIDA GUIDES AND STANDARDS MANUAL ALL NEW TREES SHALL HAVE THEIR CALIPER MEASURED AT 6" ABOVE GRADE FOR ALL TREES UP TO AND INCLUDING 4" CALIPER TREES AND 12" ABOVE GRADE FOR ALL LARGER TREES.
- 3. ALL PLANTING BEDS AND TREE SAUCERS TO HAVE ORGANIC MULCH TO A DEPTH OF 2" MINIMUM. AREAS WITH SLOPES GREATER THAN 4:1 SHALL HAVE MULCH RINGS OR TERRACED IN LONGER BEDS DESIGNED TO CAPTURE WATER AND REDUCE EROSION. TOPS OF MULCH RINGS OR TERRACES ARE TO BE LEVEL WITH RESPECT TO GRADE AND MULCH ON THE HIGH SIDE OF THE PLANTS.
- 4. A TWELVE MONTH WARRANTY IS TO BE PROVIDED FOR ALL NEW PLANT MATERIALS STARTING FROM THE DATE OF FINAL CERTIFICATE OF OCCUPANCY OR ACCEPTANCE BY THE OWNER. WARRANTY CAN EXCLUDE PLANTS LOST DUE TO ACTS OF GOD OR VANDALISM. REPLACEMENT OF DEAD OR UNHEALTHY PLANT MATERIALS SHALL BE THE RESPONSIBILITY OF THE LANDSCAPE CONTRACTOR WITHIN 30 DAYS OF NOTIFICATION IN CONFORMANCE WITH THE APPROVED LANDSCAPE PLAN.
- 5. ALL DISTURBED AREAS WITHIN THE PROJECT LIMITS SHALL BE FULLY SODDED ASIDE FROM DEFINED PLANTING BEDS. SOD SHALL BE FREE OF WEEDS AND NOXIOUS PESTS OR DISEASES. DISTURBED AREAS OUTSIDE OF PROJECT LIMITS SHALL BE SEEDED AND MULCHED UNLESS STEEPER THAN 5:1. PROVIDE SOD ON SLOPES STEEPER THAN 5:1. ALL DRY PONDS CAN BE SEEDED AND MULCHED ON THE BOTTOMS AND SIDE SLOPES. WET PONDS ARE TO BE SODDED DOWN TO THE BOTTOM OF DESIGNED 5:1 OR 4:1 SIDE SLOPES.
- 6. LANDSCAPE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE OCCURRING ON SITE OR ADJACENT AREAS, INCLUDING, BUT NOT LIMITED TO BUILDINGS, PAVING, UTILITIES, ETC., WHICH IS CAUSED BY PREPARING OR INSTALLING ANY AND ALL PLANT MATERIAL.
- 7. GRADE, DRESS AND SOD OR SEED AREAS THAT HAVE BEEN DISTURBED AS SOON AS POSSIBLE TO PREVENT EROSION.
- 8. ALL PLANT MATERIAL SHALL BE SET BACK FROM THE BACK OF CURBS OR PAVEMENT. SET TREES 3 FEET BACK FROM CURBS. 9. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING IF THERE ARE ANY INVASIVE OR EXOTIC TREES LOCATED ON THE SITE AND IS REQUIRED TO REMOVE ALL INVASIVE SPECIES PER THE LOCAL JURISDICTION'S INVASIVE SPECIES LIST LOCATED IN THEIR CODE OF ORDINANCES. GENERAL CONTRACTOR TO CONFIRM WITH THE LANDSCAPE INSTALLER THAT NO INVASIVE OR EXOTIC PLANT MATERIALS EXIST ON SITE. INFORM LANDSCAPE ARCHITECT IF ANY OF THESE PLANTS EXIST.
- 10. ANY PRUNING OF EXISTING TREES OR VEGETATION TO REMAIN AS A LANDSCAPE ELEMENT MUST BE CONDUCTED BY A CERTIFIED ARBORIST HOLDING A CURRENT CREDENTIAL WITH THE INTERNATIONAL SOCIETY OF ARBORICULTURE (ISA). ALL PRUNING SHALL CONFORMING TO THE ANSI A300 STANDARDS, PART 1; PRUNING. THE SPECIFICATIONS FOR PRUNING SHALL BE DETERMINED BY THE CERTIFIED ARBORIST OR A PERSON QUALIFIED TO CREATE PRUNING SPECIFICATIONS ACCORDING TO THE ANSI STANDARD. THE GREATEST EMPHASIS IN PRUNING SHALL BE THE SAFETY, HEALTH AND WELFARE OF THE TRAVELING OR PEDESTRIAN TRAFFIC ESPECIALLY AS IT RELATES TO PEOPLE OR PROPERTY WITHIN THE FAILURE FOOTPRINT OF THE TREE OR VEGETATION TO BE PRUNED. WHEN THERE ARE EXISTING TREES OR VEGETATION TO REMAIN, CONTRACTOR SHALL EMPLOY A CERTIFIED TREE SURGEON TO SELECTIVELY PRUNE AND TRIM ALL EXISTING PLANT MATERIALS TO PROVIDE A BALANCED APPEARANCE WHILE ALSO ELIMINATING ANY INTERFERENCE WITH THE NEW CONSTRUCTION OR SAFETY, VISIBILITY CORRIDORS.
- 11. IRRIGATION (100% COVERAGE) SHALL BE PROVIDED IN ALL NEWLY PROPOSED PLANTING AREAS IN ACCORDANCE WITH THE JURISDICTIONAL LAND DEVELOPMENT CODE. SEE SHEET L-2, IRRIGATION PLAN FOR SPECIFICS.

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N (2 O	F 3)	PROJECT NO. 22-010 SCALE 1" = 30' DATE FEBRUARY 9, 2023 SHEET NO.	FLORIDA ENGINEERING (GROUP, INC. CERTIFICATE No. EB-0006595		THIS ITEM HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY (ENGINEER), P.E. ON (DATE) USING A SHA-1 AUTHENTICATION CODE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SHA-1
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Cornerstone Charter Academy Master Plan Community Meeting Summary

Meeting Date:January 12, 2023, 5:30 p.m. to 7 p.m.Location:Cornerstone Charter Academy AuditoriumAttendees:More than 100 community members, CCA families, neighboring residents

Cornerstone Charter Academy held a community meeting regarding its master plan on Thursday, January 12, 2023. The open house format gave school families and the surrounding community an opportunity to learn about the master plan, including building and traffic plans from school leaders and the school's planning and design consultants. More than 100 people attended the meeting.

Meeting Format: The meeting was an open house to which people were invited to attend



anytime between 5:30 p.m. and 7 p.m. to view project displays and speak with project team members.

Attendees were asked to sign in at a table set up outside the main auditorium. Each attendee was given a fact sheet and comment form and invited to view the displays set up around the auditorium. The displays included renderings of planned new buildings and the campus map, as well as the three proposed traffic plans for each of the three staggered lower, middle, and high school drop-off and dismissal times. Copies of the fact sheet, comment form and exhibits are attached.



Community Notification: The school prepared a Save the Date notification that was sent in advance of the meeting to the City of Belle Isle and to school families. A meeting invitation flyer was then developed with additional information and distributed electronically by the school via email newsletter, social media and was shared with the City of Belle Isle staff as well, who then also distributed it to their contacts. Copies of the flyer were also hand-delivered by project team members to homes in the surrounding neighborhoods.

During the distribution, the project team was able to speak with several of the homeowners to encourage their attendance at the meeting. Many

said they appreciated the opportunity. Copies of the "save the date" and invitation flyer are attached.

Feedback: A total of 21 comment forms were received. A few individuals also submitted comments by email. Of the comments received, 12 expressed support for the project and four were opposed. The remaining comments contained questions or suggestions without expressing an opinion on the plan directly. The CCA Master Plan areas mentioned in the comments included:

- Questions about how drop-off and dismissal will work when a parent has children in different grades.
- Golf cart accommodation
- How traffic increase will be handled
- Questions re: school bus transportation
- Project schedule
- Requests for specific facilities, such as elementary school recess areas/playground
- Safety & security during construction.











SAVE THE DATE!

Community Open House Cornerstone Charter Academy Master Plan January 12, 2023, 5:30 - 7 p.m. 925 Fairlane Ave., Belle Isle, FL 32809

Community Meeting January 12, 2023



Cornerstone Charter Academy invites you to a Community Open House Meeting on **Thursday, January 12**, any time between 5:30 p.m. and 7 p.m. to learn about the school's master plan. Cornerstone Charter Academy's success and status as a High Performing School has led to its continued growth in popularity, prompting the need to expand its facilities to achieve its vision and mission to serve Belle Isle, Edgewood, and nearby Orange County families.

The \$35 million investment being made by the school (with no local tax revenue) proposes construction of four new educational buildings. The school development plan includes steps to provide enhanced traffic flow and parking operations on campus and in surrounding neighborhoods.

The school welcomes community feedback as it finalizes the development plan. The plan will be submitted to the City of Belle Isle Planning and Zoning Board for review in early 2023.

What:	Community Open House Meeting
When:	January 12, 2023, 5:30 - 7 p.m.
Where:	Cornerstone Charter Academy 801 Fairlane Ave. Belle Isle, FL 32809

Public participation is solicited without regard to race, color, national origin, age, sex, religion, disability of family status. Persons who require language translation or interpretive services should contact Cassie Rylands, Administrative Assistant, 407-608-7171.

Cornerstone Charter Academy



Cornerstone Charter Academy opened in 2010 to serve students from kindergarten through 12th grade. The Academy's mission is to provide a college preparatory educational environment with a curricular emphasis on biotechnology and life sciences that furthers the philosophy of respect and high expectations for all, enabling students to become confident, self - directed, and responsible life-long learners.

By the second year of operation, the Academy achieved Orange County's only A+ rating as a K-12 charter school system with a 97% graduation rate.

Cornerstone Charter Academy's success and status as a High Performing School has led to its continued growth in popularity, with enrollment growing from 645 in its first year to nearly 1,500 students today. There is a waitlist of more than 1,700 prospective students.

To achieve its vision and mission to serve students and families in Belle Isle, Edgewood, and nearby Orange County communities, Cornerstone Charter Academy has developed the Master Plan Project. The \$35 million investment being made by the school (with no local tax revenue) proposes construction of four new educational buildings. The school development plan includes steps to provide enhanced traffic flow and parking operations on campus and in surrounding neighborhoods.

Cornerstone Charter Academy will seek approval of its master plan from the City of Belle isle with a goal of beginning construction in summer 2023. Construction is expected to take place over two years.

Master Plan Project FAQs

What is the Master Plan Project?

The Project involves the construction, renovation, expansion and equipping school facilities, including the construction of four new educational buildings. The Project also includes the renovation and expansion of the campus parking and stormwater retention system.

Who is paying for the Project?

The \$35 million Project will be funded through bonds and will not use any public taxpayer funds.

How many students will the improvements accommodate?

The new educational facilities will allow Cornerstone to serve up to 2,500 students in grades K-12.

How will Cornerstone manage traffic in the morning and afternoon?

Currently, Cornerstone Charter Academy has two different start times and dismissal times. The Project includes changes to the traffic patterns and higher queue lane capacity on property to better accommodate traffic flow.





Additionally, the school will have three dismissal times, 30 minutes apart.

When is construction starting and how long will it take?

The goal is to begin construction by summer 2023. The next step seeking approval of the building plan from the City of Belle Isle. Construction will be completed in two years.

Cornerstone Charter Academy Master Plan

Fulfilling a Mission & Vision

The Cornerstone Mission

To provide a college preparatory educational environment with a curricular emphasis on biotechnology and life sciences that furthers the philosophy of respect and high expectations for all, enabling students to become confident, selfdirected, and responsible life-long learners.







Master Plan Overview

- Four new educational buildings
- Improved traffic patterns and vehicle capacity on property to improve traffic flow
- Three dismissal times, 30 minutes apart
- Not funded by local taxpayer dollars
- The expansion is needed to serve the growing student demand





















The Cornerstone Vision

To provide a top shelf, innovative, public education opportunity of choice in a safe, sound environment which:

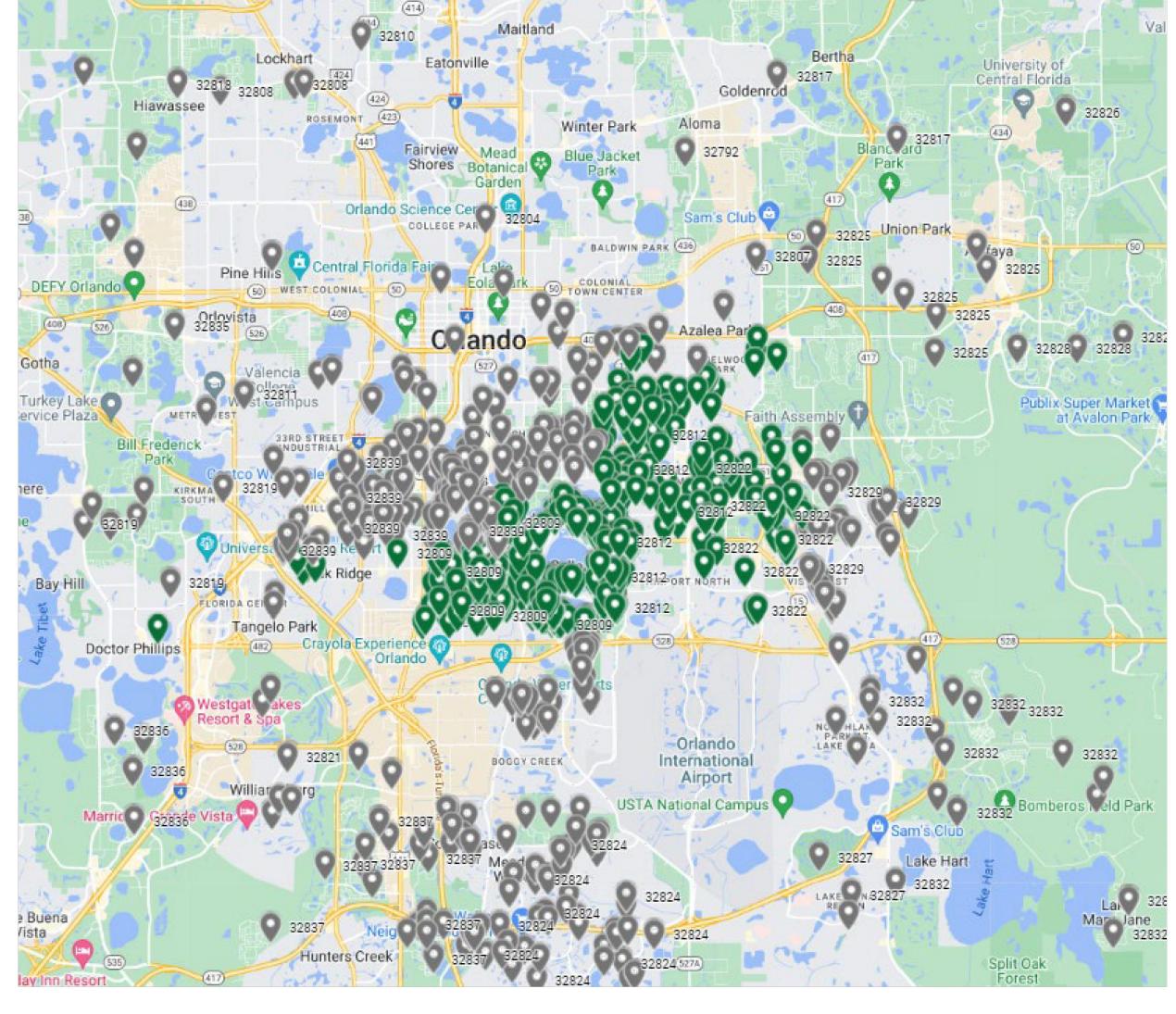
- Has as its foundation a solid, core academic curriculum;
- Offers curricular emphasis on life sciences and biotechnology;
- Attracts families to live in the community;
- Employs talented and passionate teachers and administrators.
- Utilizes technology tools conducive to advanced learning;
- Substantially involves parents, community and corporate partners;
- Provides a full spectrum of extracurricular activities;
- Instills a sense of community service and pride; and
- Prepares the students for college and a fulfilling, meaningful career.

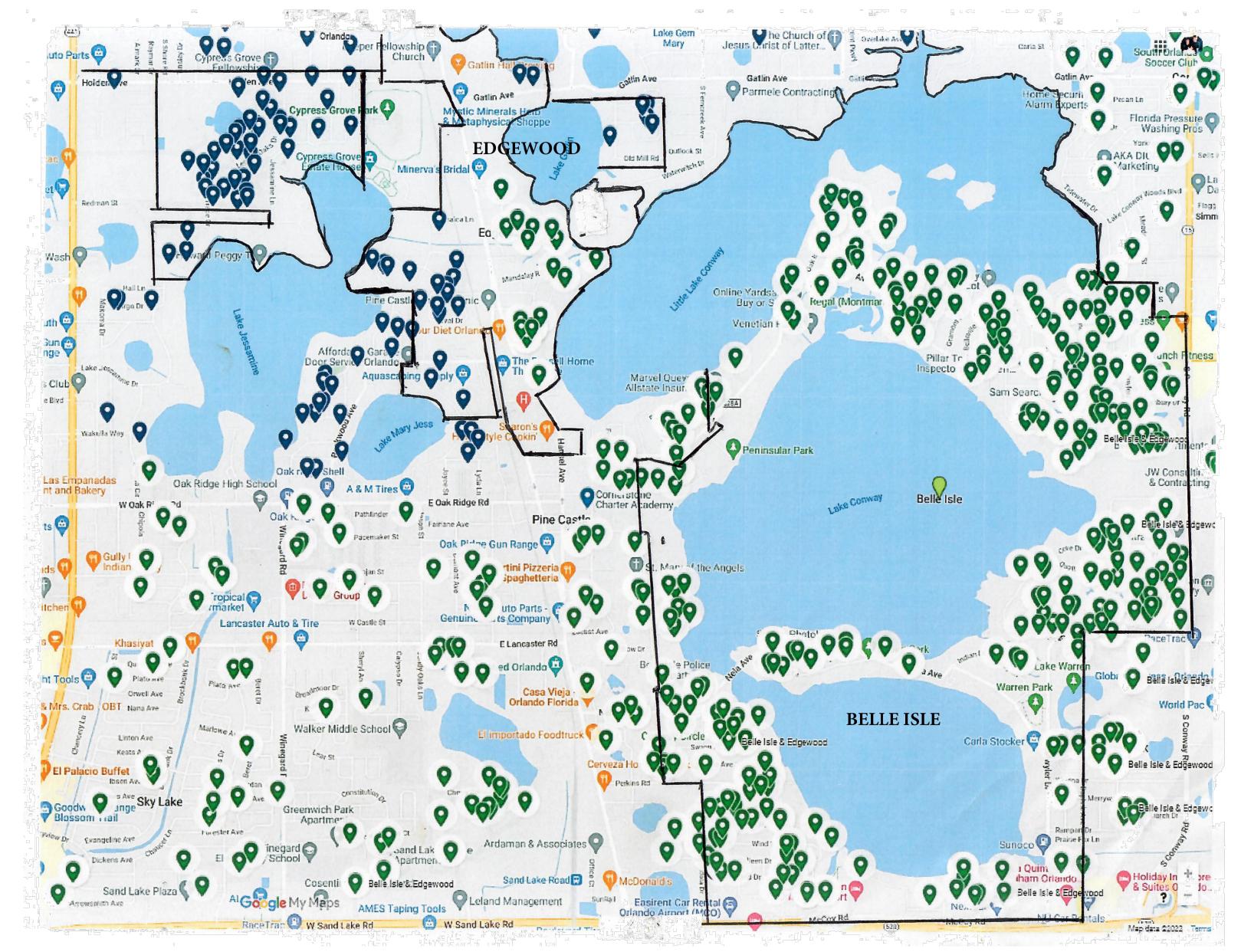




CORNERSTONE CHARTER ACADEMY STUDENT ENROLLMENT - HEAT MAPS

ALL STUDENTS





CITY LIMITS - BELLE ISLE & EDGEWATER

01.12.2023

CIVICA Architecture 129

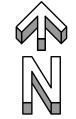




CORNERSTONE CHARTER ACADEMY PROPOSED MASTER PLAN

a.

ELEMENTARY SCHOOL - NEW 3-STORY BLDG. HIGH SCHOOL & AUDITORIUM - EXISTING STUDENT SERVICES - NEW 2-STORY BLDG. PRESS BOX, CONCESSIONS & RESTROOMS





01.12.2023

CIVICA Architecture 130





VIEW LOOKING NORTH EAST



01.12.2023

CIVICA Architecture 131







01.12.2023

VIEW LOOKING NORTH WEST





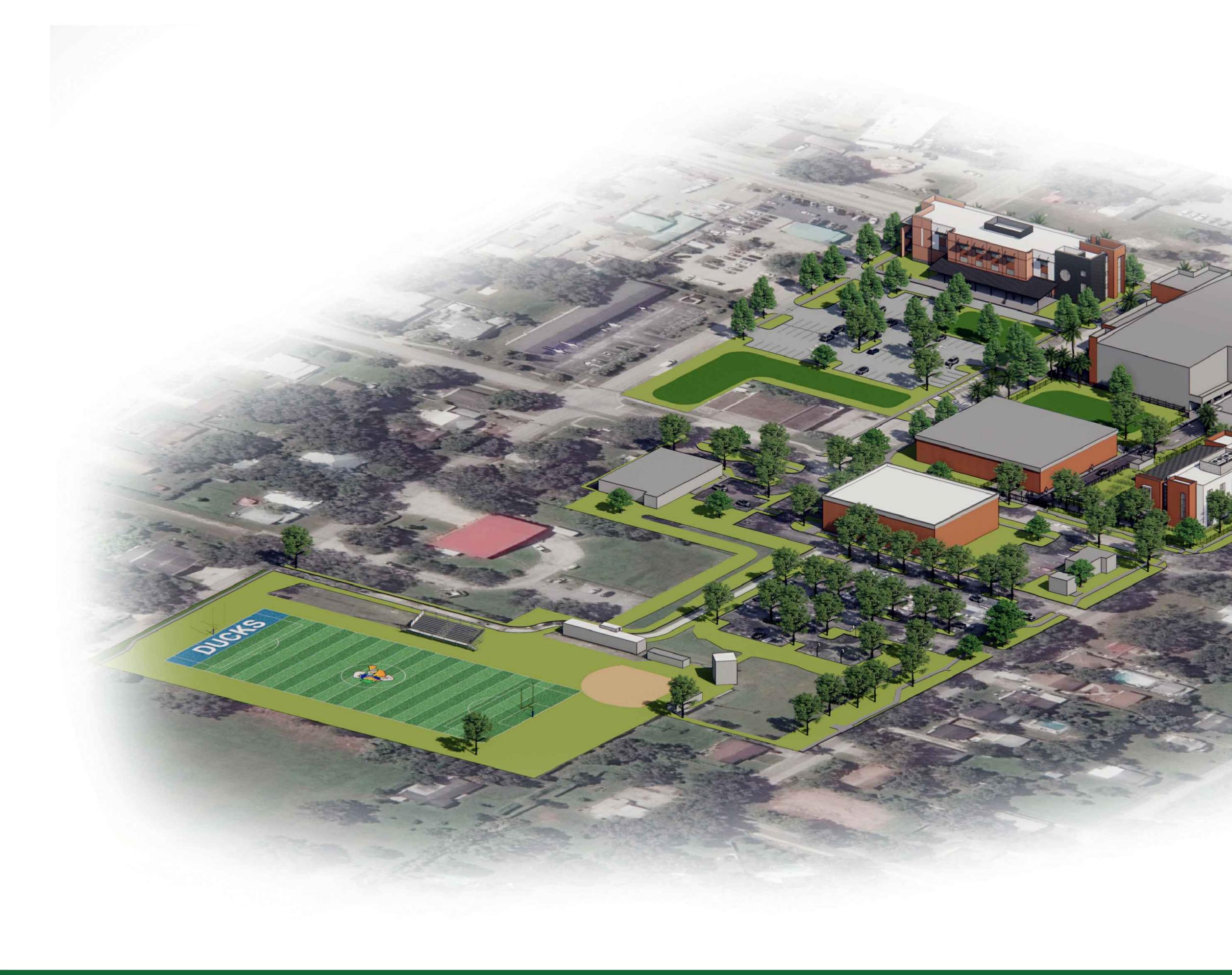


VIEW LOOKING SOUTH EAST



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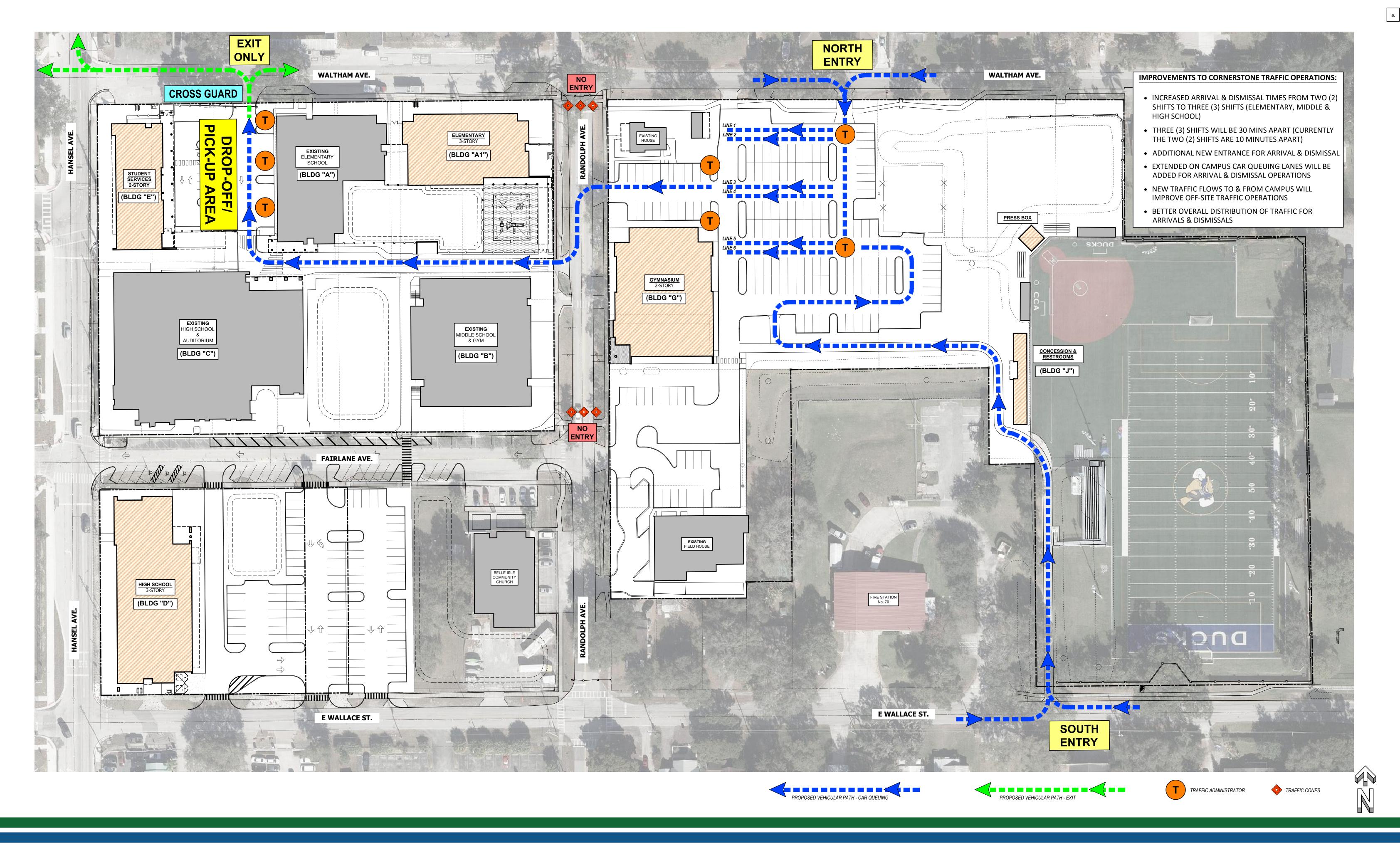




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VIEW LOOKING SOUTH WEST

CIVICA Architecture 134





CORNERSTONE CHARTER ACADEMY PROPOSED TRAFFIC MANAGEMENT PLAN - ELEMENTARY SCHOOL DROP-OFF & PICK-UP SHIFT

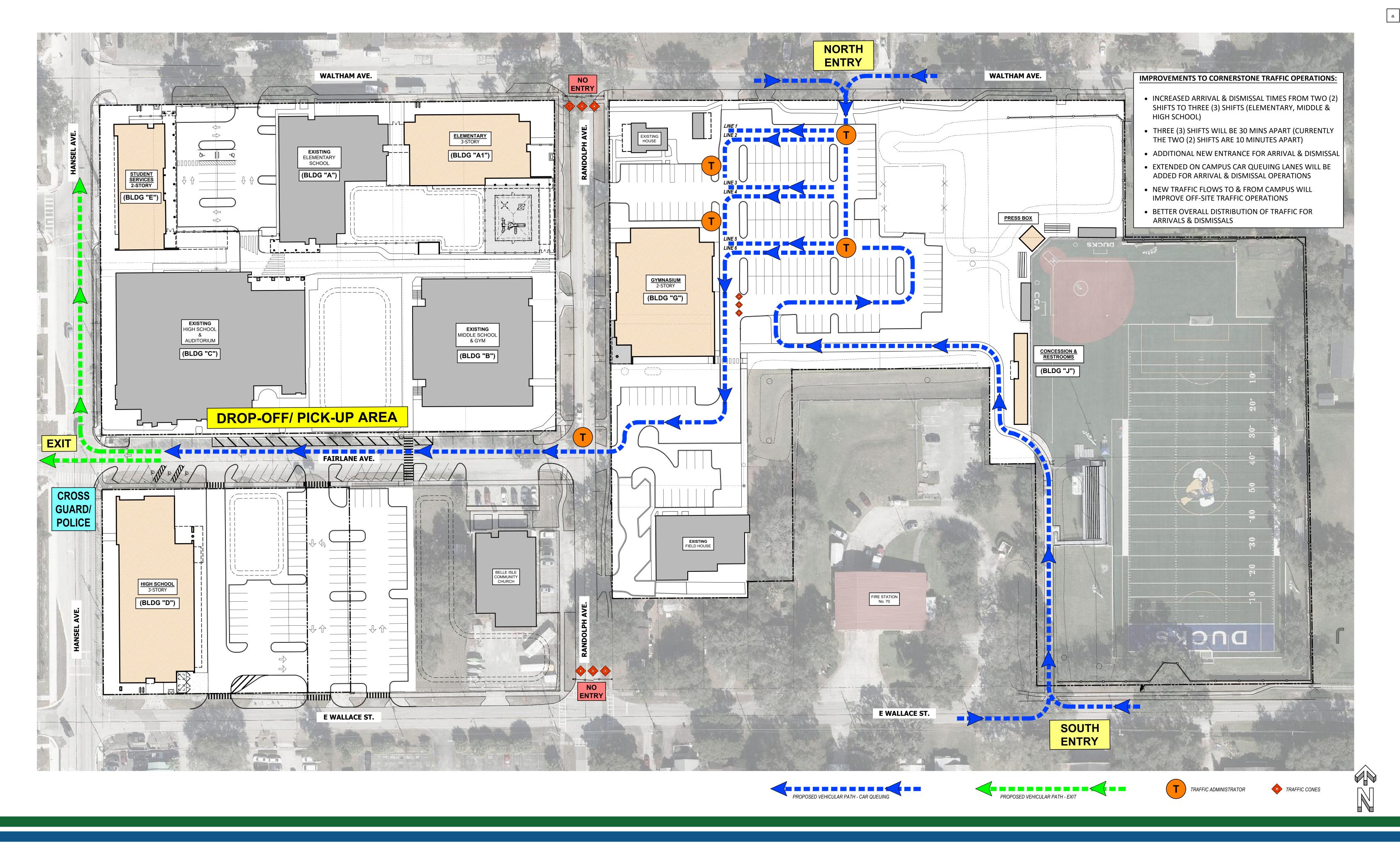


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Architecture

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CORNERSTONE CHARTER ACADEMY PROPOSED TRAFFIC MANAGEMENT PLAN - MIDDLE SCHOOL DROP-OFF & PICK-UP SHIFT

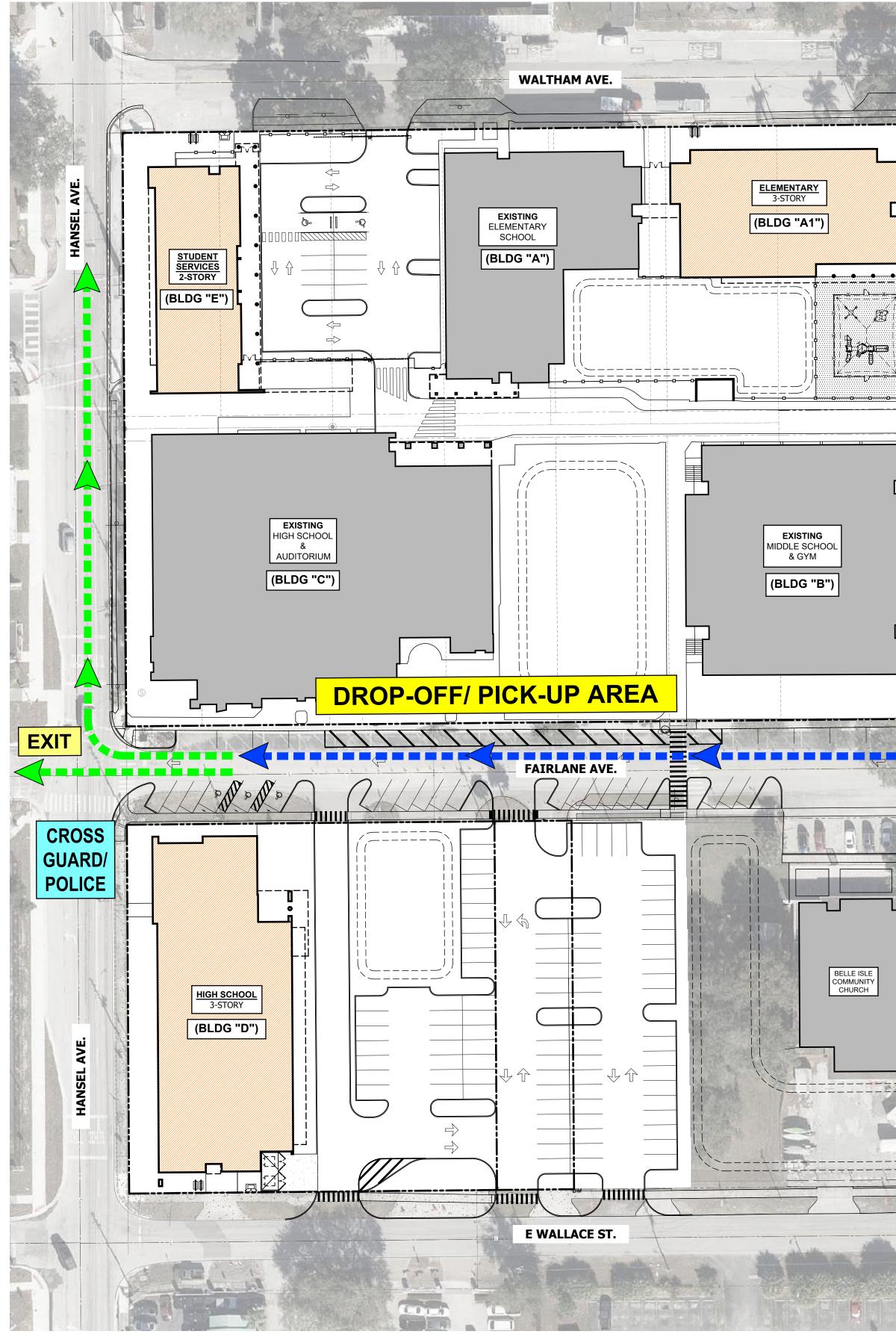


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CORNERSTONE CHARTER ACADEMY PROPOSED TRAFFIC MANAGEMENT PLAN - HIGH SCHOOL DROP-OFF & PICK-UP SHIFT

NORTH **ENTRY** NO ENTR R • • • • • • • • • LINE 4 <u>GYMNASIUM</u> 2-STORY (BLDG "G") \bigcirc AR ARAC EXISTING FIELD HOUSE BELLE ISLE COMMUNITY CHURCH FIRE STATION No. 70 E WALLACE ST.

PROPOSED VEHICULAR PATH - CAR QUEUING

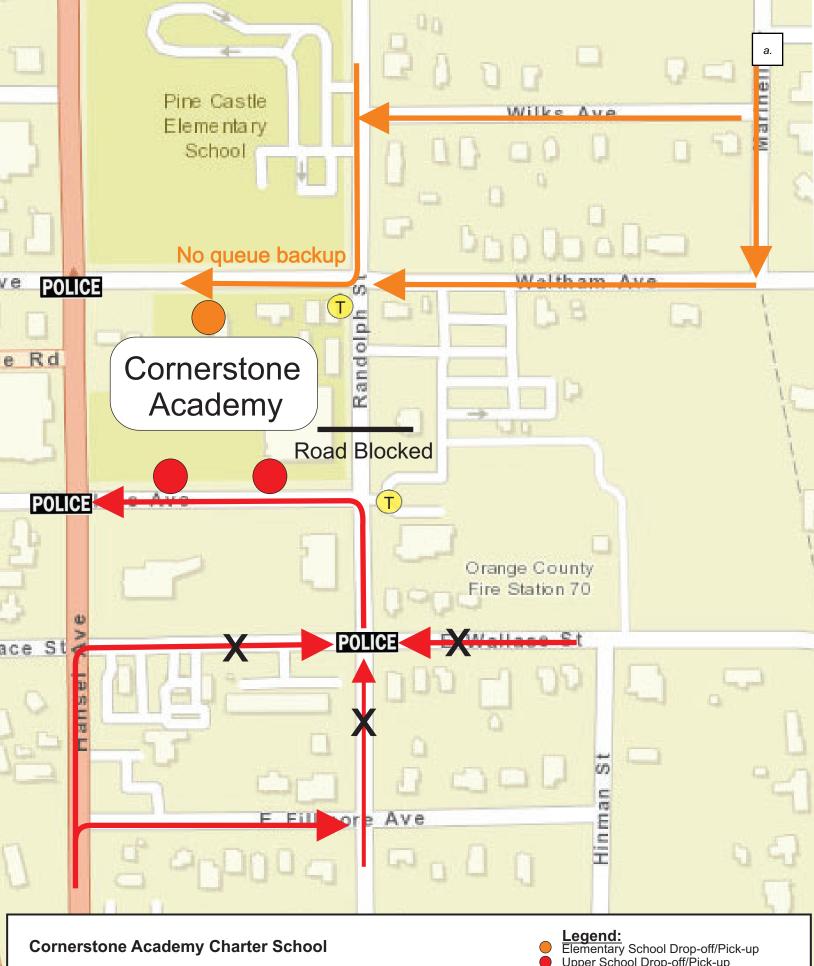


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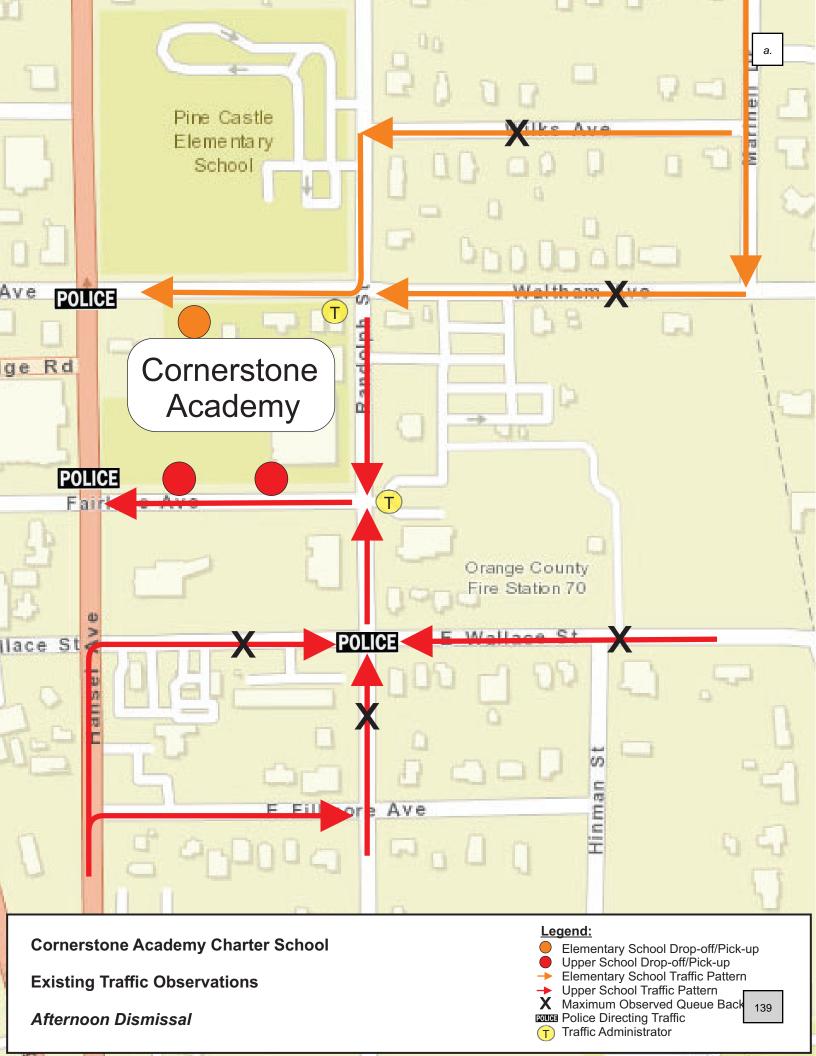
137



Existing Traffic Observations

Morning Drop-off

Elementary School Drop-off/Pick-up
 Upper School Drop-off/Pick-up
 Elementary School Traffic Pattern
 Upper School Traffic Pattern
 Maximum Observed Queue Back
 Police Directing Traffic
 Traffic Administrator



TRAFFIC OPERATIONS ANALYSIS STUDY

CORNERSTONE CHARTER ACADEMY CITY OF BELLE ISLE, FLORIDA



Prepared for:

Florida Engineering Group 5127 S. Orange Avenue, Suite 200 Orlando, Florida 32809

Prepared by:

Traffic Planning and Design, Inc. 535 Versailles Drive Maitland, Florida 32751 407-628-9955

February 2023

TPD № 5725

PROFESSIONAL ENGINEERING CERTIFICATION

I hereby certify that I am a Professional Engineer properly registered in the State of Florida practicing with Traffic Planning & Design, Inc., a corporation authorized to operate as an engineering business, EB-3702, by the State of Florida Department of Professional Regulation, Board of Professional Engineers, and that I have prepared or approved the evaluations, findings, opinions, conclusions, or technical advice attached hereto for:

- **PROJECT:** Cornerstone Charter Academy
- LOCATION: City of Belle Isle, Florida
- CLIENT: Florida Engineering Group

I hereby acknowledge that the procedures and references used to develop the results contained in these computations are standard to the professional practice of Transportation Engineering as applied through professional judgment and experience.

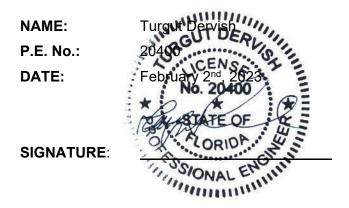


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INTRODUCTION

Cornerstone Charter Academy is a charter school located along the east side of Hansel Avenue, south of Waltham Avenue in the City of Belle Isle, Florida. **Figure 1** depicts the site location and surrounding roadways.

This traffic analysis study was performed to evaluate the proposed expansion in terms of traffic operations and related queueing and evaluate the ability of adjacent roadways to accommodate the additional traffic volumes, and to recommend transportation improvements, including traffic circulation and transportation improvements to mitigate congestion resulting from additional site traffic. The primary purpose of this traffic study is to ensure on-site vehicular and pedestrian facilities and circulation are adequately provided to protect public safety and maintain traffic flow efficiency for all users accessing the campus before, during and after school construction.

The existing school enrollment consists of 574 students (K-5) students, and 905 (grades 6-12) students with a total of 1,479 students operating in two drop-off/pick-up shifts. With the proposed expansion, the school will have a total enrollment of 2,420 students (750 k-5 students, 570 grades 5-8 students and 1,100 grades 9-12 students) operating in 3 shifts. The expansion will be on the existing school site in addition to the property on the south side of Fairlane Avenue that is currently occupied by a bank. **Figure 2** depicts the proposed site plan of the school expansion.







Cornerstone Charter Academy Project № 5725 Figure 1



Cornerstone Charter Academy Project № 5725 **Figure 2**



a.

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Proposed Site Plan

INVENTORY OF TRAFFIC CONDITIONS

The following section provides an overview of the data collected in support of the traffic analysis for the proposed school.

Study Area

The following roadways were identified from Orange County's CMS database to be included in the project's one-mile area: Hoffner Avenue, Orange Avenue to Oak Island Road Lancaster Road, Winegard Road to Orange Avenue Nela Avenue, Orange Avenue to Indian Drive Oak Ridge Road, Orange Blossom Trail to Orange Avenue Orange Avenue, Sand Lake Road to Hansel Avenue (S) Hansel Avenue (S) to Hansel Avenue (N) Hansel Avenue (N) to Holden Avenue

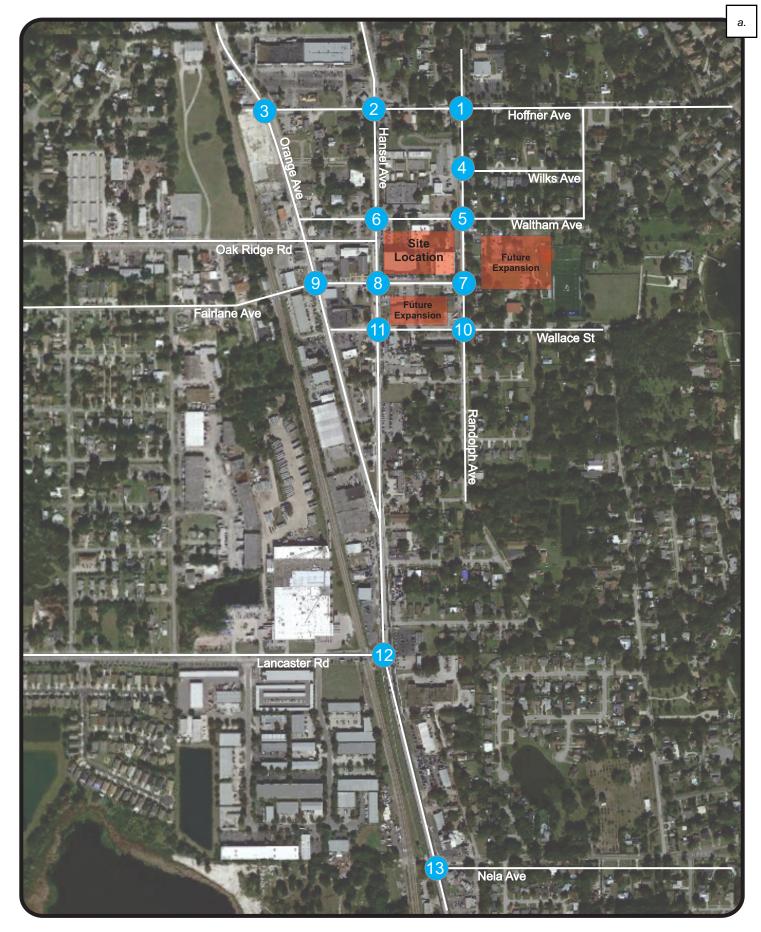
Orange Avenue/Hansel Avenue, Orange Avenue to Orange Avenue

Additionally, the following intersections were included in the study area:

- Hansel Avenue & Hoffner Avenue
- Hansel Avenue & Waltham Avenue
- Hansel Avenue & Fairlane Avenue
- Hansel Avenue & Wallace Street
- Randolph Avenue & Hoffner Avenue
- Randolph Avenue & Wilks Avenue
- Randolph Avenue & Waltham Avenue
- Randolph Avenue & Fairlane Avenue
- Randolph Avenue & Wallace Street
- Orange Avenue & Hoffner Avenue
- Orange Avenue & Fairlane Avenue
- Orange Avenue & Lancaster Road
- Orange Avenue & Nela Avenue

Figure 3 provides a diagram of the study intersection locations in the study area.







Cornerstone Charter Academy Project № 5725 Figure 3



Existing Conditions

Hansel Avenue is one-way state roadway running two-lanes northbound connecting to Orange Avenue from the south at Prince Street to the north at Mandalay Road. The posted speed limit along the school's site is 40 mile per hour (mph) and is provided with sidewalks on both sides in the vicinity of the school site.



Hansel Ave South of Fairlane Ave



Randolph Ave South of Waltham Ave

Randolph Avenue is a two-lane undivided local roadway running northsouth from Locust Avenue to Hoffner Avenue with a speed limit of 25 mph. The street has sidewalks on both sides along the vicinity of the school site.

Fairlane Avenue is a two-lane undivided local roadway running east-west from Randolph Avenue on the east and terminates at Jason Street to the west of Orange Avenue. The street gives direct access to Cornerstone Academy and is signalized at Hansel Avenue. The speed limit in the vicinity of the school is 25 mph. There is on-street parking on the north side of the roadway adjacent to the school site.



Fairlane Ave East of Hansel Ave





Waltham Ave East of Hansel Ave

Waltham Avenue is a two-lane undivided facility running east-west from Marinell Drive to the east and terminates at Orange Avenue to the west with a speed limit of 25 mph. The street is used for Elementary School drop-off/pick-up during school opening and dismissal times.

Wallace Street is a two-lane undivided facility running east-west from Matchett Road to the east and terminates at Orange Avenue to the west with a speed limit of 25 mph. The street provides access to vehicles arriving to the site from the south to Randolph Avenue and Fairlane Avenue.



Wallace St East of Hansel Ave



а.

Existing Traffic Patterns and Queueing

Traffic counts were collected at the study intersections and traffic patterns and queueing were observed.

The school currently operates in two shifts, one for lower school grades (Elementary K-5th) with drop-off at 7:15 AM and dismissal at 2:35 PM, and another for upper school grades (6th-12th) with drop-off at 7:30 AM and pick-up at 2:45 PM.

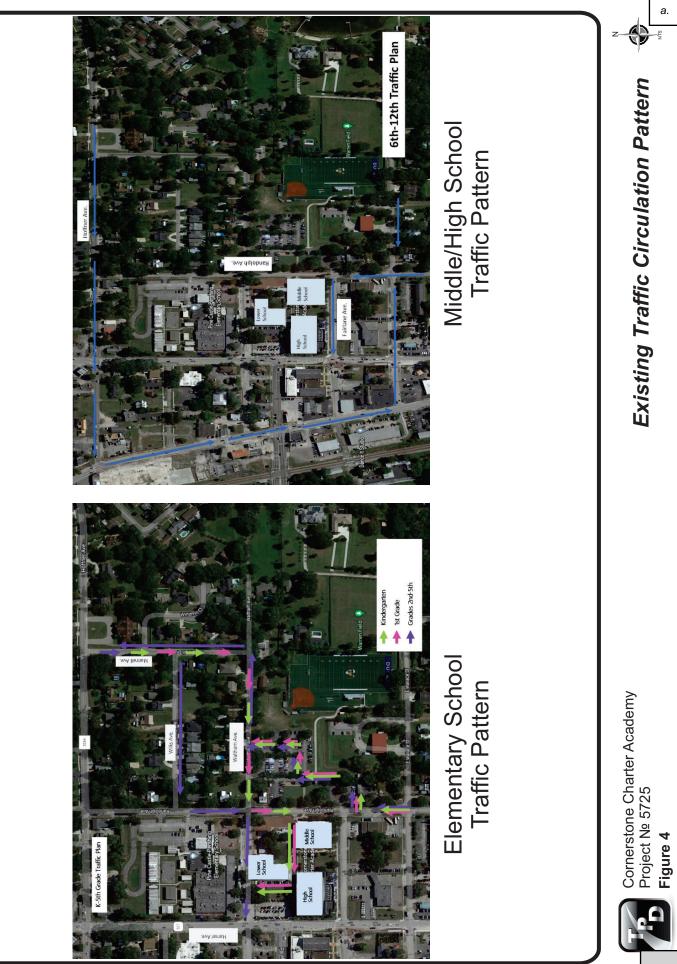
The school currently has two traffic circulation patterns. Elementary school students are picked up on the north side of the property. Kindergarten and 1st grade students are picked up inside the school campus with vehicles entering on Randolph Avenue and exiting on Waltham Avenue. The 2nd to 5th grade students drop-off/pick-up location is on Waltham Avenue. Middle and high school students drop-off and pick-up location is on Fairlane Avenue. Existing traffic circulation patterns are illustrated in **Figure 4**.

The observation during the morning and afternoon queuing showed significant queues forming on the adjacent roadways during drop-off/pickup.

Planned and Programmed Improvements

Based on Orange County's 2030 Long Range Transportation plan, there are no roadway improvements planned in the vicinity of the project area.





EXISTING OPERATIONAL ANALYSIS

Roadway Segment Analysis

The study roadway segments, obtained from Orange County's CMS database and supplemented with counts obtained by Traffic Planning and Design (TPD), were analyzed by comparing the existing P.M. peak hour directional volume for each roadway segment with the corresponding peak hour directional capacity at the adopted Level of Service (LOS) E standard. A summary of the existing roadway capacity analysis is presented in **Table 1**, which shows all the roadway segments are currently operating within their adopted LOS standard except for Hoffner Avenue from Orange Avenue to Oak Island Road, and Orange Avenue from Hansel Avenue (S) to Hansel Avenue (N), and from Hansel Avenue (N) to Holden Avenue. These segments are operating below their Level of Service standards in the existing conditions. The County's CMS Database is included in **Appendix A**.

Intersection Analysis

A capacity analysis was conducted for each intersection using the latest version of *Synchro Software* in accordance with the procedures of the latest *Highway Capacity Manual (HCM)*. The capacity analysis was performed using the existing intersection geometry and traffic volumes during the A.M. and P.M. peak hours. Turning movement counts were collected by TPD during the A.M. and P.M. peak hours to properly evaluate peak hour conditions. The counts were obtained on December 6, 2022 and on January 19, 2023 when the Florida Department of Transportation seasonal factor was 0.99 and 1.08, respectively. The counts were adjusted for the intersections with a seasonal factor higher than 1.00. The peak hour volumes at the study intersections are displayed in **Figures 5(a)** through **5(d)**, and the raw turning movement counts along with signal timings and SF reports are included in **Appendix B**.

The intersection capacity analysis as summarized in **Table 2** indicates that the study intersections currently operate at satisfactory Level is Service except for the following:

- Waltham Avenue & Hansel Avenue (EB and WB approaches) A.M. and P.M.
- Fairlane Avenue & Orange Avenue (WB approach) A.M. and P.M.
- Orange Avenue & Nela Avenue (WB Approach) A.M. only

Some of these deficient Levels of Service, as shown in Table 2, have V/C ratio less than 1.0, which indicates that the Level of Service is caused by delay, not a capacity deficiency. Detailed *Synchro* analysis worksheets are included in **Appendix C**.



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Roadway Segment	Mum	Seg Length	# of Lns	LOS Std	Total Cap	PM Pk	Peak Dir	Available Cap	LOS Met?
Hoffner Avenue									
Orange Ave to Oak Island Rd	185	1.33	2	ш	800	866	EB	0	z
Lancaster Road									
Winegard Rd to Orange Ave	248	1.01	4	ш	2,000	902	EB	1,294	٢
Nela Avenue									
Orange Ave to Indian Dr	293	1.46	2	Ш	800	151	EB	649	۲
Oak Ridge Road									
Orange Blossom Tr to Orange Ave	298	1.67	4	ш	1,700	1,518	EB	182	٢
Orange Avenue									
Sand Lake Rd to Hansel Ave (S)	330	1.14	5	ш	2,510	2,102	BN	408	۲
Hansel Ave (S) to Hansel Ave (N)	331	1.22	2 (One-Way)	Ш	2,040	2,324	BN	0	z
Hansel Ave (N) to Holden Ave	332	0.75	5	Ш	2,510	2,867	BN	0	z
Orange Avenue / Hansel Avenue									
Orange Ave to Orange Ave	176	1.23	2 (One-Way)	ш	2,400	2,337	SB	63	≻

a.

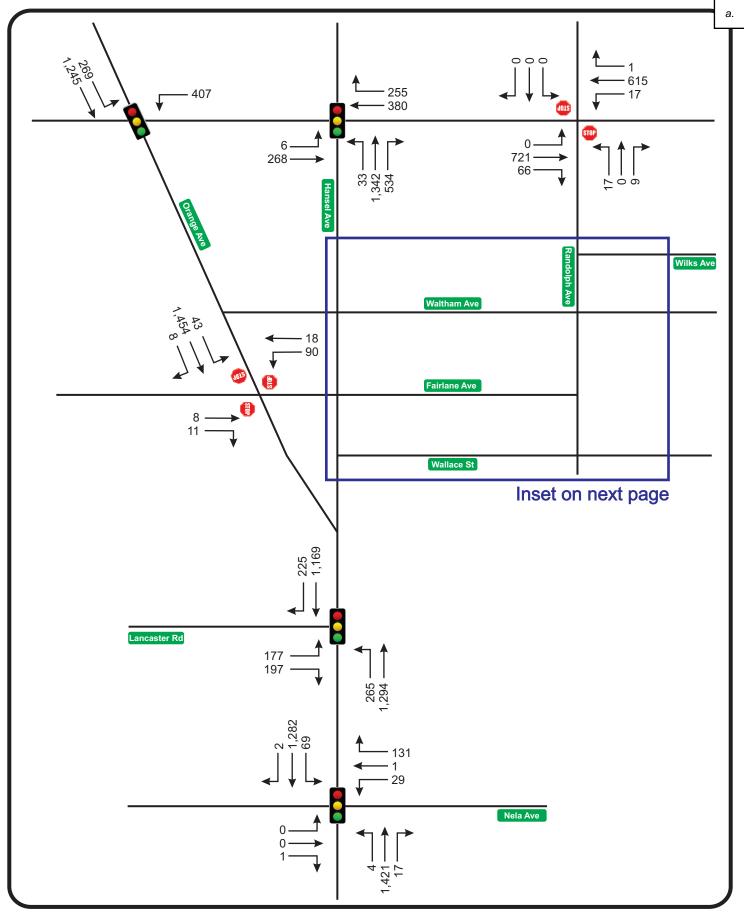
Analysis
Capacity
Intersection
Existing
Table 2:

1	Intersection	Control	Time	EB	8	WB	В	NB	3	SB	3	Overall	rall
			Period	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
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(

* V/C ratio is less than 1.0

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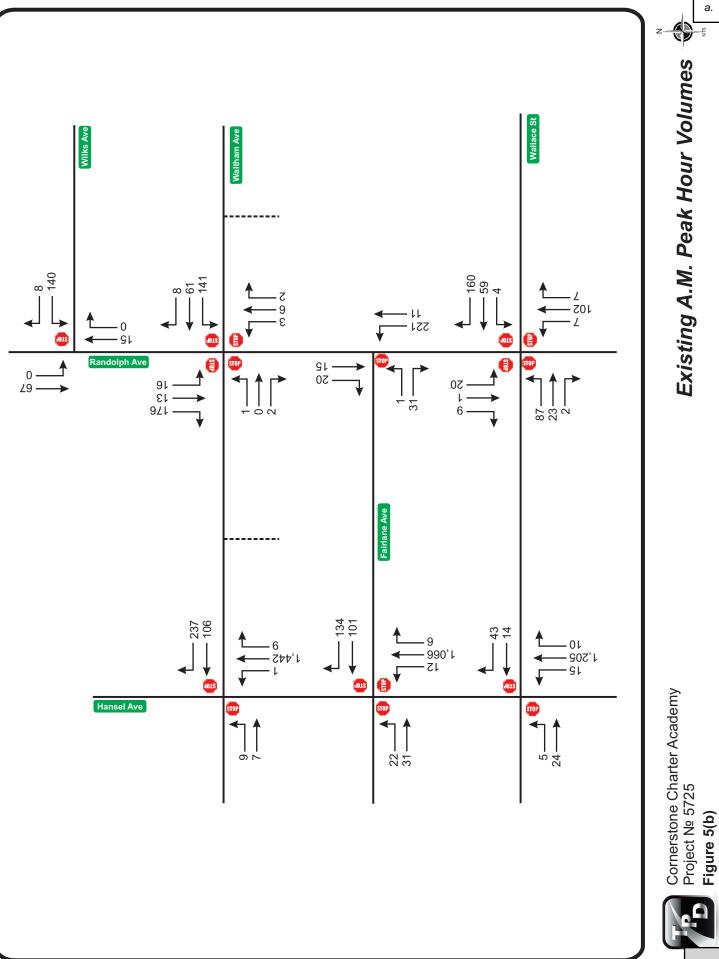
Cornerstone Charter Academy Project № 5725 Page 12

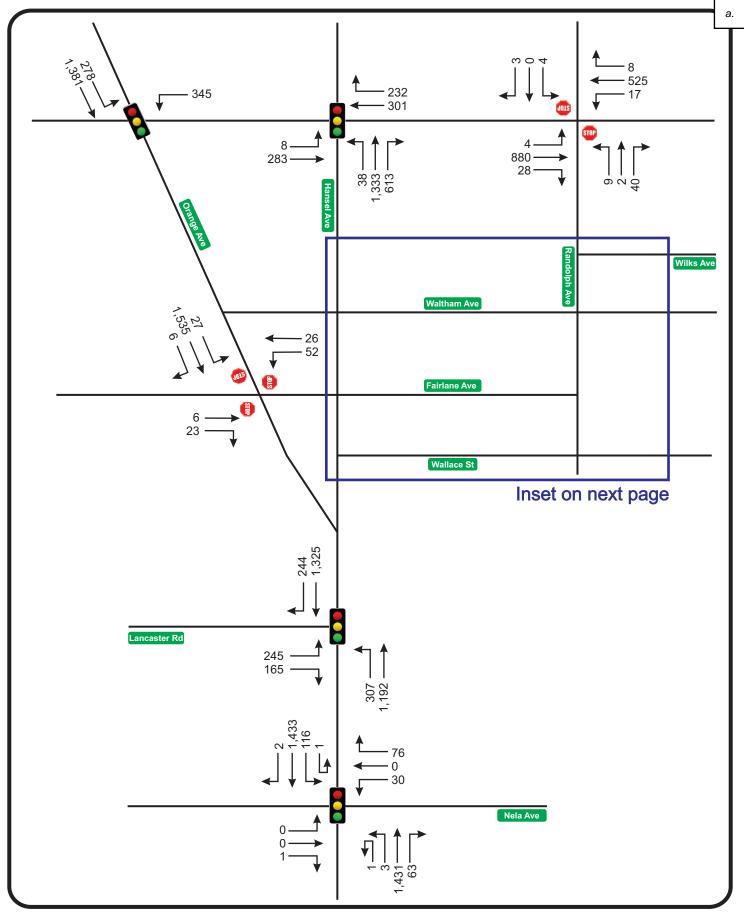




Cornerstone Charter Academy Project № 5725 **Figure 5(a)** Existing A.M. Peak Hour Volumes



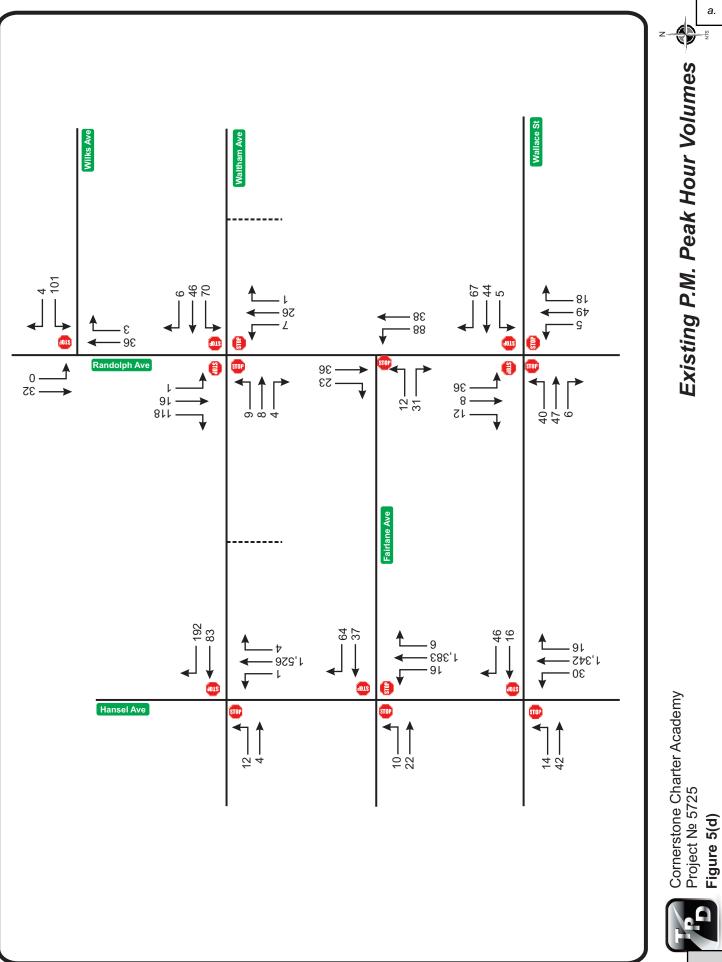






Cornerstone Charter Academy Project № 5725 **Figure 5(c)** Existing P.M. Peak Hour Volumes





TRIP GENERATION AND ASSIGNMENT

Trip Generation

The charter school currently has a total enrollment of 1,479 students (574 students grades K-5 students and 905 grades 6-12 students). The drop-off/pick up is currently operating in two shifts. The proposed expansion of the school will increase the total student enrollment to 2,420 (750 grades K-5 students, 570 grades 5-8 students and 1,100 grades 9-12 students) operating in 3 shifts. The shifts will occur in 30 minute increments.

Trip generation is performed utilizing data obtained from the Institute of Transportation Engineers (ITE) *Trip Generation Manual, 11th Edition*. A summary of the trip generation of the proposed school is shown in **Table 3**. The trip generation was calculated per shift for the existing conditions and proposed expansion as the resultant vehicular trip generation will not occur at the same time. Moreover, the drop-off/pick-up location and traffic pattern is different for lower school (grades K-5) than the upper school (grades 6-12). As shown in Table 3, the school expansion is expected to increase the trips by 782 A.M. peak hour trip and 633 P.M. peak hour trips. Relevant ITE trip generation sheets are included in **Appendix D**. It should be noted that the school currently has a large number of students that walk to school from neighboring residential areas, and that a portion of the high school students will drive and park at the designated parking area. Therefore, the vehicular traffic is expected to be less than the results given by ITE. Nonetheless, ITE rates were used in the traffic analysis to provide a conservative estimate.

Trip Distribution

The distribution of the project trips onto the study area roadways was determined using the Central Florida Regional Planning Model (CFRPM). The trip distribution was adjusted based on enrollment maps provided by the school and utilizing engineering judgement and knowledge of travel patterns in the area. This adjusted trip distribution pattern is illustrated in **Figure 6**. This distribution pattern was utilized to assign school trips to the area roadways. The model distribution plots and enrollment maps are included in **Appendix E**.



Table 3

Trip Generation Summary

				A.M. Peak Hour	Hour			P.M. Peak Hour	k Hour	
	Land Use	Size (Students)	Rate	Enter	Exit	Total	Rate	Enter	Exit	Total
			Existing	ting						
Shift 1										
538	Charter School (K-12) – Elementary School	574	0.83	243	233	476	0.51	148	147	295
Shift 2										
538	Charter School (K-12) – Middle & High School	905	0.83	383	368	751	0.66	300	300	600
		Total Exi	Total Existing Traffic	626	601	1,227		448	447	895
			Prop	Proposed						
Shift 1										
538	Charter School (K-12) – Elementary School	750	0.83	317	306	623	0.61	229	228	457
Shift 2										
538	Charter School (K-12) – Middle School	570	0.83	241	232	473	0.51	146	146	292
Shift 3										
538	Charter School (K-12) – High School	1,100	0.83	466	447	913	0.71	390	389	779
		Total Proje	Total Projected Traffic	1,024	985	2,009		765	763	1,528
		Trip Increase Due to Expansion	b Expansion	398	384	782		317	316	633

Notes:

1. Daily trip generation is not available for this ITE Land Use Code

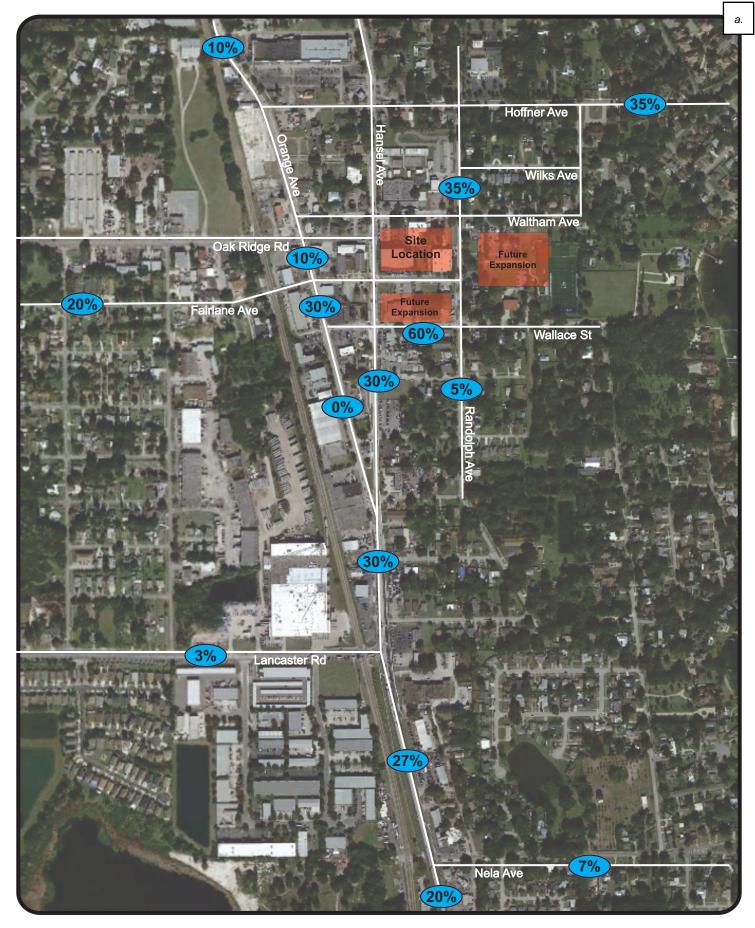
2. ITE's peak hour of adjacent street traffic was used for A.M. peak hour

3. ITE's peak hour of generator was used for P.M. peak hour

4. Equations were used where R^2 exceeded 0.75



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Cornerstone Charter Academy Project № 5725 **Figure 6**



PROJECTED OPERATIONAL ANALYSIS

Traffic conditions were analyzed for the study roadways/intersections for the school's anticipated expansion completion in 2024. Projected traffic volumes used in the roadway analysis consisted of existing traffic combined with committed trips and site generated traffic.

For the intersection analysis, background traffic was determined by expanding the traffic using an annual growth factor of 1% to existing traffic volumes. A trends analysis of the historical traffic volumes on the area roadways in the vicinity of the project revealed annual growth trends averaging 1.07%. Therefore, an average of 1.00% annual growth rate was used to develop background traffic. Trends analysis worksheets are included in **Appendix F**. Project trips were added to the background traffic estimation to obtain the total projected traffic volumes.

It should be noted that the school drop-off/dismissal is proposed to take place in three (3) shifts. Therefore, project trips from the most critical (highest trip generation) shift was used for the development of projected traffic volumes.

Roadway Segment Analysis

The projected roadway segment analysis was performed by comparing the projected traffic volume of each segment with the capacity at the adopted LOS E standard. The analysis as summarized in **Table 4** revealed that all the roadway segments are projected to operate satisfactorily within their adopted Levels of Service except for the following four segments;

- Hoffner Avenue from Orange Avenue to Oak Island Road
- Orange Avenue from Hansel Avenue (S) to Hansel Avenue (N)
- Orange Avenue from Hansel Avenue (N) to Holden Avenue
- Hansel Avenue from Orange Avenue (S) to Orange Avenue (N)

The first three segments operate below their Level of Service capacity in the existing conditions. The last segment fails as a result of adding committed and project trips. It should be noted that the school afternoon peak period (2-4 P.M.) is significantly earlier than the typical P.M. peak hour used by Orange County (4-6 P.M.). Nonetheless, the volumes were used to provide a conservative estimate.



Table 4: Projected Roadway Capacity Analysis

Roadway Segment	Num	Seg Length	# of Lns	LOS Std	Total Cap	PM Pk	Peak Dir	Com Trips	Trip Dist*	Project Trips	Total Trips	Available Cap	LOS Met?
Hoffner Avenue													
Orange Ave to Oak Island Rd	185	1.33	2	ш	800	866	EB	0	35%	136	1,134	0	z
Lancaster Road													
Winegard Rd to Orange Ave	248	1.01	4	ш	2,000	902	EB	6	5%	20	735	1,265	≻
Nela Avenue													
Orange Ave to Indian Dr	293	1.46	2	ш	800	151	EB	10	4%	16	177	623	≻
Oak Ridge Road													
Orange Blossom Tr to Orange Ave	298	1.67	4	Ш	1,700	1,518	EB	5	20%	78	1,601	66	Y
Orange Avenue													
Sand Lake Rd to Hansel Ave (S)	330	1.14	5	Э	2,510	2,102	NB	19	32%	125	2,246	264	Υ
Hansel Ave (S) to Hansel Ave (N)	331	1.22	2 (One- Way)	Ш	2,040	2,324	NB	-	45%	175	2,500	0	z
Hansel Ave (N) to Holden Ave	332	0.75	5	Э	2,510	2,867	NB	4	10%	39	2,910	0	Z
Orange Avenue / Hansel Avenue													
Orange Ave to Orange Ave	176	1.23	2 (One- Way)	ш	2,400	2,337	SB	-	55%	214	2,552	0	z

*Highest Percentage on Segment



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Intersection Analysis

A capacity analysis was conducted at the study intersections utilizing projected traffic volumes as shown in **Figures 7(a)**through **7(d)**. This was accomplished in accordance with the procedures of the latest version of the Highway Capacity Manual by utilizing *Synchro Software*. The results of this analysis are summarized in **Table 5** and indicate satisfactory traffic operating conditions at all intersections except for the following intersections that are projected to operate at deficient Levels of Service, similar to existing conditions:

- Waltham Avenue & Hansel Avenue EB and WB approaches (AM and PM)
- Fairlane Avenue & Orange Avenue WB approach (AM and PM), EB approach (PM only)
- Orange Avenue & Nela Avenue WB approach (AM)

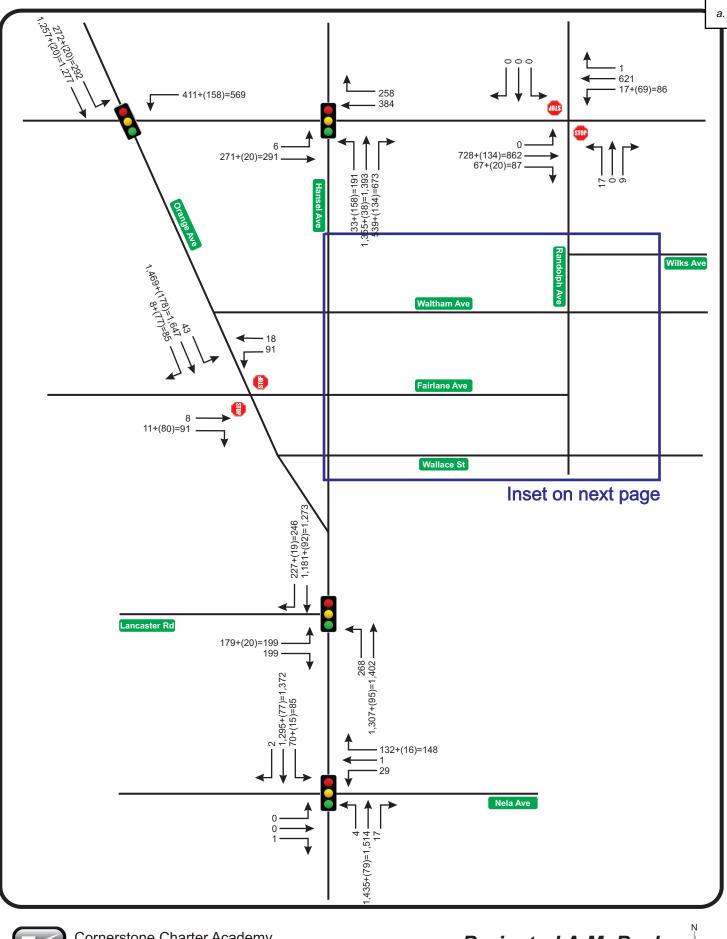
Additionally, the following intersections are projected to operate at deficient Levels of Service due to the addition of background traffic and/or project trips:

- Hoffner Avenue & Randolph Avenue- NB approach (AM and PM), SB approach (PM only)
- Wallace Street & Hansel Avenue EB approach (AM only), WB approach (PM only)
- Orange Avenue & Nela Avenue WB approach (PM)

Some of these deficient Levels of Service as indicated in Table 5 have V/C ratio less than 1.0, which indicates that the Level of Service is caused by delay, not a capacity deficiency. It should be noted that with the proposed traffic plan, the project is not adding any trips to the stop-controlled intersection of Orange Avenue and Fairlane Avenue failing EB/WB approaches or to the intersection of Wallace Street and Hansel Avenue WB approach.

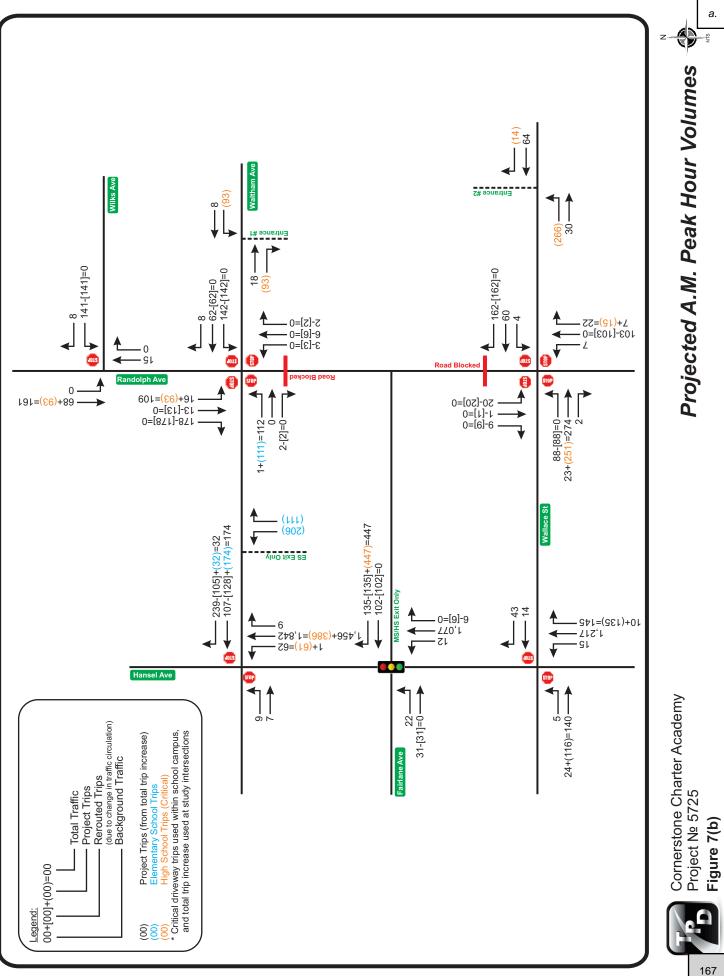
The intersection of Hansel Avenue and Waltham Avenue is a stop-controlled intersection. However, during pick-up and dismissal, this intersection is controlled by the City's police and/or traffic attendant that regulate the traffic in a pattern that resembles a signal. Reanalyzing this intersection as signalized for pick-up/drop-off times yields a satisfactory Level of Service, as shown in Table 5. Detailed *Synchro* intersection analysis worksheets are included in **Appendix G**.

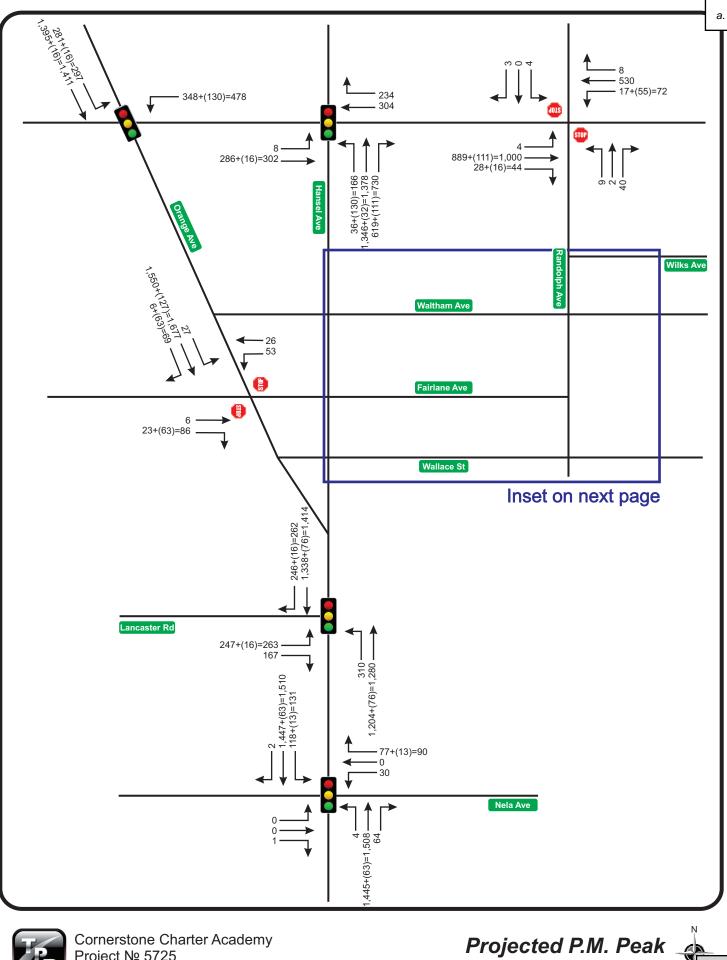




Cornerstone Charter Academy Project № 5725 **Figure 7(a)** Projected A.M. Peak Hour Volumes





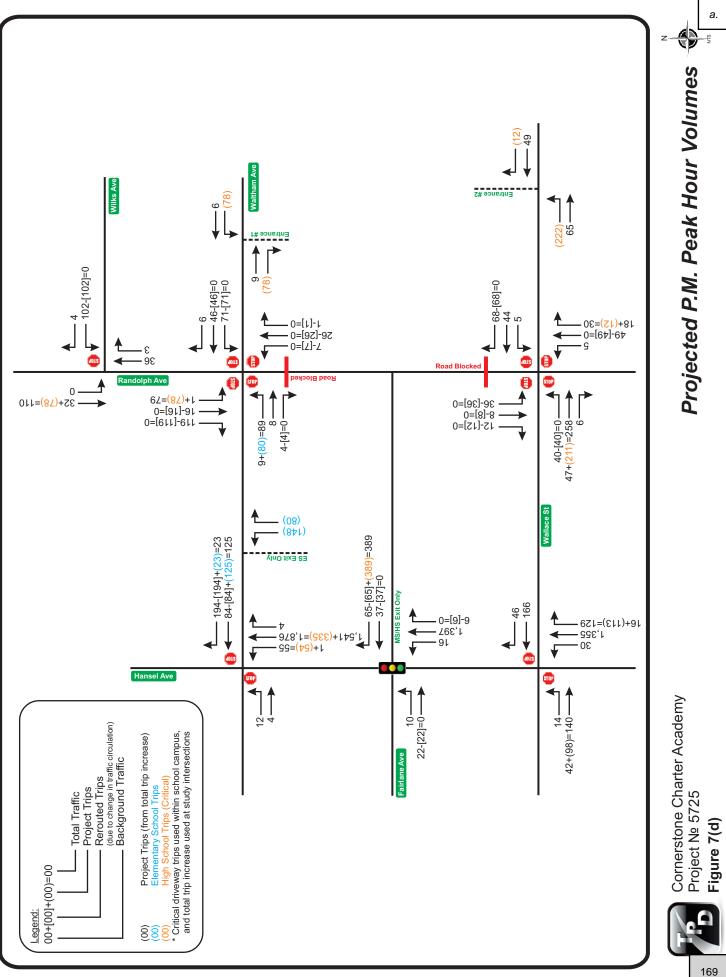


Project № 5725

Figure 7(c)

Projected P.M. Peak Hour Volumes





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Projected Traffic Intersection Capacity Analysis

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_		dois	P.M.	0.0	A	2.5	A	74.8	* 4	103.2	* 4	3.5	A
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N		olgilai	P.M.	64.9	Е	58.4	Е	15.4	В	ł	-	27.7	с
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o		olgilai	P.M.	ł	-	0.0	A	ł	-	0.6	A	0.6	A
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4		dois	P.M.	ł		8.5	A	0.0	A	0.0	A	0.1	A
ц	Mothom Avia & Dandalah Avia	U0/010	A.M.	9.3	A	7.3	A	0.0	A	10.4	В	9.9	А
r		2000	P.M.	8.4	А	7.0	А	0.0	А	8.8	А	8.6	А
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<u>u</u>		dois	P.M.	ł		769.3	F	0.0	A	ł	-	54.2	L
D		Ciccol	A.M.	67.0	Е	55.4	Е	55.5	ш	ł	-	55.6	ш
		วเยเาสเ	P.M.	76.8	Ш	49.5	D	26.9	С	1	-	28.9	С
7	Fairlane Ave & Randolph Ave	U2/010	A.M.	ł	ł	ł	ł	ł	ł	ł	ł	ł	ł
-	(Closed)		P.M.	ł	ł	ł	1	ł	ł	ł	1	ł	ł
0		Cianol	A.M.	71.5	ш	77.1	Е	16.9	В	ł	1	34.9	С
0		olgilai	P.M.	72.1	Е	72.3	Ш	18.5	В	ł	1	30.4	c
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* V/C ratio is less than 1.0



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2		DOWE	P.M.	9.1	A	7.6	A	7.4	A	0.0	A	8.7	A
7		0,00 0	A.M.	195.3	F	22.5	С	0.0	A	ł	ł	18.7	U
=		doie	P.M.	ł		437.0	F	0.0	A	ł	ł	49.3	ш
ç		Cicanol	A.M.	41.3	D	ł	-	11.9	В	18.5	В	18.0	В
N -	Olalige Ave & Lalicastel Ku	ыдпа	P.M.	54.6	D	1	-	17.4	В	39.9	D	31.9	C
C 7		Cicerol	A.M.	51.8	D	98.0	* ±	9.6	A	3.8	A	11.9	В
2		olylia	P.M.	64.8	Ш	84.9	* Ц	9.6	A	1.3	A	8.2	A
0:+													

* V/C ratio is less than 1.0

ON-SITE QUEUING ANALYSIS

Existing Queues

A preliminary analysis was conducted in order to ensure that the proposed site can accommodate parent vehicles during drop-off/pick-up times. This preliminary analysis was conducted using queuing data collecting in the field during drop-off and pick-up times. The longest queues is recorded in **Table 6**, and the queue data collection is included in **Appendix H**. The longest queue was observed for the High/Middle School afternoon pick-up, on the south side of Randolph Avenue extending east and west on Wallace Street, and on Randolph Avenue south of Wallace Street.

	Elementar	y School	Middle/Hig	gh School
	Queue (veh)	Queue (ft)*	Queue (veh)	Queue (ft)*
Drop-off	10	250	26	650
Pick-up	19	475	34	850

Table 6Observed Queue Length Summary

* veh X 25 ft/veh

The following is a summary of observations of queues and site circulation:

- The pick-up/drop-off for Elementary School and Middle/High School shifts happen ten (10) minutes apart.
- Elementary School pick-up and drop-off currently occurs on Waltham Avenue at designated stops. Kindergarten and 1st grade students are picked up inside the school campus with vehicles entering on Randolph Avenue and exiting on Waltham Avenue. The 2nd to 5th grade students drop-off/pick-up location is on Waltham Avenue. No significant queues were observed during the morning drop-off. However, vehicles start queueing and stop on the adjacent streets prior to dismissal, and queues are observed on Waltham Avenue. Wilks Avenue and Randolph Avenue.
- Traffic attendants and police are present at the intersection of Hansel Avenue and Waltham Avenue, and at the intersection of Waltham Avenue and Randolph Avenue, to direct exiting traffic onto Hansel Avenue.



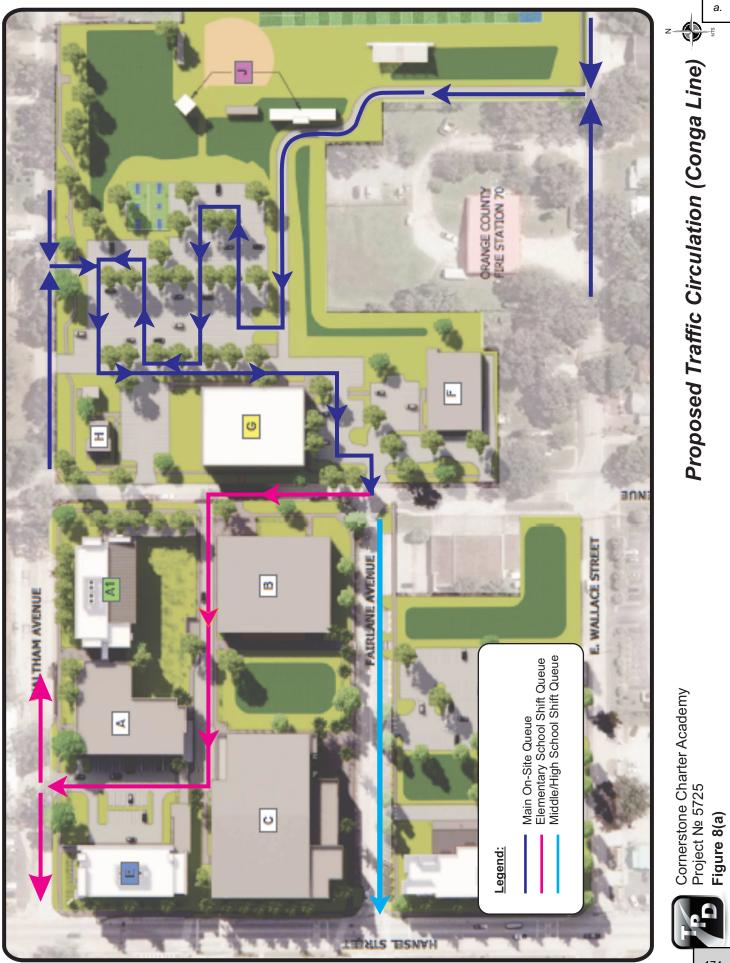
 Middle and high school students drop-off and pick-up location is on Fairlane Avenue. Queues are observed in the morning and afternoon with higher queues in the afternoon extending east and west of Randolph Avenue on Wallace Street, and on Randolph Avenue south of Wallace Street. Police directs the exiting traffic from Fairlane Avenue onto Hansel Avenue. Exiting traffic yields to large numbers of pedestrians crossing Hansel Avenue to and from the school.

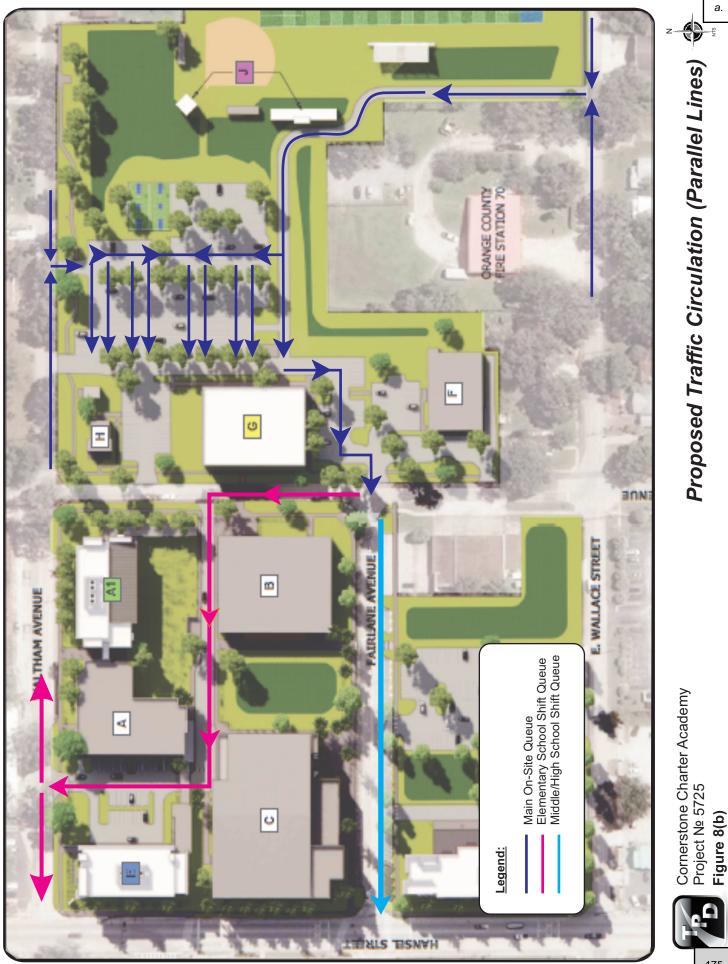
Proposed Site Circulation

The proposed school expansion will include changes to traffic pattern in efforts to accommodate all incoming morning and afternoon traffic within the school site. This can be achieved by creating on-site queueing on the property east of Randolph Avenue. It is proposed that traffic for all three shifts have two entry points, one on Wallace Street east of Randolph Avenue, and the other on Waltham Avenue east of Randolph Avenue. Traffic from the north and the south entry points will merge with the assistance of traffic attendants, and will be routed to the drop-off/pick up area through Randolph Avenue. Traffic will then be allowed to exit on Waltham Avenue for Elementary School shift, and on Fairlane Avenue for Middle/High School shifts. Randolph Avenue will be blocked during these times from both the north on Waltham Avenue and from the south on Wallace Street. Fairlane Avenue will become a one-way westbound street utilized for exiting traffic on Hansel Avenue.

On-site queueing is proposed via two methods, the first method is a conga line configuration with one-lane traffic circulating the existing parking lot, the second is a parallel line configuration that will utilize two parallel lanes in each parking lane. The two methods are displayed in **Figure 8(a)** and **Figure 8(b)**, respectively. The conga line method is estimated to provide 2,700 ft for Elementary School queueing, and 2,500 ft for Middle/High School queueing. The parallel line method is estimated to provide 2,200 ft for Middle/High School queueing.







Microsimulation Analysis

A microsimulation analysis was conducted for the A.M. and P.M. peak hours in order to determine if the proposed on-site queuing and traffic circulation will be sufficient to accommodate the increase in traffic during the drop-off/pick-up times and to determine the impact on the adjacent roadway segments. This was accomplished using the latest version of Synchro/SimTraffic software and utilizing a preliminary site plan and the proposed traffic circulation. Since the proposed site circulation with Conga Line configuration provides slightly more on-site queueing distance, this method was utilized in the analysis to more accurately depict traffic flow within the site. The queueing reports are included in **Appendix I** and are summarized in **Table 7**. As shown in the table, with implementing the proposed changes, the proposed on-site queueing distance is expected to accommodate the projected traffic during the highest shift without significantly impacting the adjacent roadways.

School Shift	Time Period	SimTraffic Queue (ft)	Available Length (ft)	Deficiency (ft)
Elementary	A.M.	3,115	2,700	Yes
	P.M.	2,857	2,700	Yes
Middle/High	A.M.	2,238	2,500	No
	P.M.	2,253	2,500	No

 Table 7

 Microsimulation On-Site Queueing Summary

Based on the SimTraffic microsimulation analysis, the highest queues occurred for Elementary School morning drop-off. Some back-up is expected on adjacent roadways, however, most of the queue will be contained within the site and spill-out occurred for a short period of time and dissipated quickly. Elementary school queue backup on adjacent roadways also occurred at afternoon pick-up, but was less significant than the morning queue. No queue spill-out occurred for Middle/High School morning drop-off or afternoon pick-up with the proposed changes.



FINDINGS AND RECOMMENDATIONS

This traffic analysis study was performed per the request of the City of Belle Isle to (a) evaluate the proposed expansion in terms of traffic operations and related queueing, (b) to evaluate the ability of adjacent roadways to accommodate the additional traffic volumes, and (c) to recommend transportation improvements, including traffic circulation and transportation improvements to mitigate congestion resulting from additional site traffic. The primary purpose of this traffic study is to ensure on-site vehicular and pedestrian facilities and circulation are adequately provided to protect public safety and maintain traffic flow efficiency for all users accessing the campus before, during and after school construction.

Cornerstone Charter Academy is a charter school located along the east side of Hansel Avenue, south of Waltham Avenue in the City of Belle Isle, Florida. The existing school enrollment consists of 574 students (K-5) students, and 905 (grades 6-12) students with a total of 1,479 students operating in two drop-off/pick-up shifts. With the proposed expansion, the school will have a total enrollment of 2,420 students (750 k-5 students, 570 grades 5-8 students and 1,100 grades 9-12 students) operating in 3 shifts. The expansion will be on the existing school site in addition to the property on the south side of Fairlane Avenue that is currently occupied by a bank

The following recommendation were developed as part of the Study:

Roadway and Intersection Findings and Recommendations

 The roadway segments are currently operating within their adopted LOS standard except for Hoffner Avenue from Orange Avenue to Oak Island Road, and Orange Avenue from Hansel Avenue (S) to Hansel Avenue (N), and from Hansel Avenue (N) to Holden Avenue. These segments are operating below their Level of Service standards in the existing conditions and will continue to do so with the addition of school-generated trips. Additionally, the segment of Hansel Avenue from Orange Avenue (S) to Orange Avenue (N) is expected to fail in the projected conditions as a result of adding background growth/committed trips and project trips.



- The study intersections included in this comprehensive traffic analysis currently operate at satisfactory Levels of Service under existing conditions except for the following intersections/approaches:
 - Waltham Avenue & Hansel Avenue EB and WB approaches (AM and PM)
 - Fairlane Avenue & Orange Avenue WB approach (AM and PM), v/c < 1.0
 - Orange Avenue & Nela Avenue WB Approach (A.M. only), v/c < 1.0

As indicated, some of these deficient Levels of Service have V/C ratio less than 1.0, which indicates that the Level of Service is caused by delay, not a capacity deficiency. These intersections will continue to operate at deficient Levels of Service upon the addition of school generated trips.

- In addition to intersections operating at LOS "F" under existing conditions, the following intersections are projected to operate at deficient Levels of Service due to the addition of background traffic growth and/or project trips:
 - Hoffner Avenue & Randolph Avenue NB approach (AM and PM), SB approach (PM only), v/c < 1.0
 - Fairlane Avenue & Orange Avenue EB approach ((PM Only), v/c < 1.0
 - Wallace Street & Hansel Avenue EB approach (AM only), WB approach (PM only)
 - Orange Avenue & Nela Avenue WB approach (PM), v/c < 1.0

As indicated, some of these deficient Levels of Service have V/C ratio less than 1.0, which indicates that the Level of Service is caused by delay, not a capacity deficiency. It should be noted that with the proposed traffic plan, the project is not adding any trips to the stop-controlled intersection of Orange Avenue and Fairlane Avenue failing EB/WB approaches or to the intersection of Wallace Street and Hansel Avenue WB approach. The intersection of Hansel Avenue and Waltham Avenue is a stop-controlled intersection. However, during pick-up and dismissal, this intersection is controlled by the City's police and/or traffic attendant that regulate the traffic in a pattern that resembles a signal. Reanalyzing this intersection as signalized for pick-up/drop-off times yields a satisfactory Level of Service



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Proposed School Site Findings and Recommendations

The following is a summary of observations of queues and site circulation under existing conditions:

- The pick-up/drop-off for Elementary School and Middle/High School shifts happen ten (10) minutes apart.
- Elementary School pick-up and drop-off currently occurs on Waltham Avenue at designated stops. Kindergarten and 1st grade students are picked up inside the school campus with vehicles entering on Randolph Avenue and exiting on Waltham Avenue. The 2nd to 5th grade students drop-off/pick-up location is on Waltham Avenue. No significant queues were observed during the morning drop-off. However, vehicles start queueing and stop on the adjacent streets prior to dismissal, and queues are observed on Waltham Avenue. Wilks Avenue and Randolph Avenue.
- Traffic attendants and police are present at the intersection of Hansel Avenue and Waltham Avenue, and at the intersection of Waltham Avenue and Randolph Avenue. To direct exiting traffic onto Hansel Avenue.
- Middle and high school students drop-off and pick-up location is on Fairlane Avenue. Queues are observed in the morning and afternoon with higher queues in the afternoon extending east and west of Randolph Avenue on Wallace Street, and on Randolph Avenue south of Wallace Street. Police directs the exiting traffic from Fairlane Avenue onto Hansel Avenue. Exiting traffic yields to large numbers of pedestrians crossing Hansel Avenue to and from the school.

The following improvements should be considered based on review of site plan and analysis of internal circulation:

 Under buildout conditions, the school is proposed to have three shifts for Elementary, Middle and High School. These shifts are recommended to be 30-45 minutes apart in order to ensure queue clearance for the next shift and prevent queue spill back on adjacent streets.



- Upon review of the proposed site plan, the parking lot on the property east of Randolph Avenue is recommended to be utilized to provide on-site queueing for incoming traffic. Providing on site-queueing within the school property will encourage drivers arriving early to wait within the school campus and will provide stacking for incoming vehicles. The proposed site circulation plan considers two entry points, one on Waltham Avenue for vehicles coming from the north via Randolph Avenue and Marinell Drive, and one from Wallace Street for vehicles coming from the south. The south entrance will utilize the existing pedestrian pathway which is recommended to be paved for vehicle use and stacking.
- On-site traffic circulation is suggested via two methods illustrated within the study where vehicles stack via a conga line or parallel lines within the parking lot east of Randolph Avenue and exit on Waltham Avenue for Elementary School shift, and on Fairlane Avenue for Middle School and High School shifts. This traffic circulation provides maximum utilization of the school property for on-site stacking. However, the efficient operation during pick-up and dismissal, with the help of traffic attendants and police similar to existing conditions, is a crucial part in managing the queues efficiently and preventing queue spill out on the adjacent streets. This includes the following:
 - Randolph Avenue road should be closed to traffic from both the north on Waltham Avenue and from the south on Wallace Street. This street can be used for overflow vehicles in two lanes if necessary.
 - Vehicles on Fairlane Avenue will be directed to *exit right only* on Hansel Avenue.
 Vehicles travelling to the south will utilize upstream intersections to turn left and travel south on Orange Avenue. This will prevent westbound queueing at the stopcontrolled intersection of Orange Avenue and Fairlane Avenue. This operation during drop-off/dismissal can be achieved with the help of proper signage and police/traffic attendant guidance.
 - Signal changes to the intersection of Hansel Avenue and Fairlane Avenue with split side streets and retiming of the sufficient green time for exiting vehicles on Fairlane Avenue while maintaining coordination timing on Hansel Avenue corridor.



- Change in pedestrian crossing location on Hansel Avenue to prevent vehicles turning right from Fairlane Avenue onto Hansel Avenue to yield to large numbers of pedestrian and create queueing on both the major and minor streets.
- Similar to existing conditions, police/traffic attendant guidance is needed at the intersection of Hansel Avenue and Waltham Avenue during Elementary School shift to create a signal-like operation at the stoop-controlled intersection and provide sufficient time for exiting vehicles from Waltham Avenue to Hansel Avenue.
- Station staff members / traffic attendants within the site and at the entry driveways to guide traffic into the school site queue and ensure proper circulation.

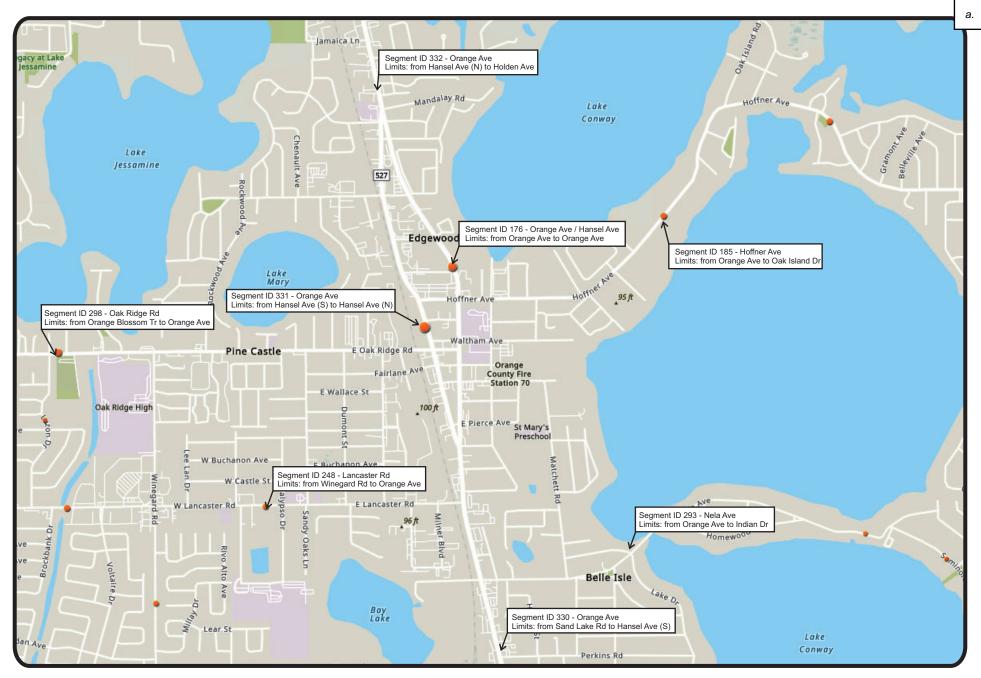
It should be noted that this study was analyzed for 100% student drop-off/pick-up as a conservative analysis. The school currently has a large portion of pedestrians and "golf-cart" pick-up locations, and will continue to facilitate and encourage other modes of transportation to mitigate additional traffic generated by the school expansion. Additionally, the school expansion will also expand the after-school programs which will account for a percentage of students staying on campus after dismissal times, and will reduce the number of vehicles arriving to the site at drop-off/dismissal.



APPENDICES

APPENDIX A

Orange County's CMS Database





Cornerstone Charter Academy Project № 5725 **Appendix A**

Orange County Segment Stations



184

Traffic Concurrency Management Program Concurrency Link Information Orange County, Florida

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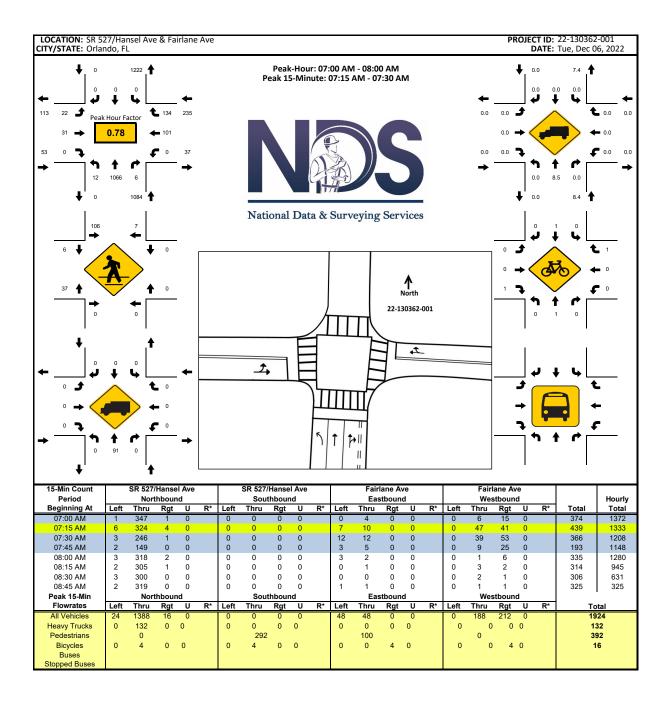
P. L. O. R. T. D. A.	Application Number:	inN uo	nber:								
ID From	To	Lgth	Maint Agency	Capacity Group	Min Total Ln LOS Cap	Min Total LOS Cap	AADT H	Comm AADT PmPk PkDir Trips	Comm ir Trips	Avail Cap* LOS	
Hoffner Ave		0	0	4		7			7	4	
185 Orange Ave	Oak Island Rd	1.33	Cnty	Urban - Class II 2	I 2 E	800	19,626	998 EB	3	0 F	
Lancaster Rd											
248 Winegard Rd	Orange Ave	1.01	Cnty	Urban - Class I	I 4 E	2000	14,009	706 EB	9	1,285 C	
Nela Ave											
293 Orange Ave	Indian Dr	1.46	Belle Isle	Urban - Class II	I 2 E	800	2,963	151 EB	3 10	639 C	
Oak Ridge Rd											
298 Orange Blossom Tr	Orange Ave	1.67	Cnty	Urban - Class II	I 4 E	1700	31,833	1,518 EB	3	177 D	
Orange Ave											
330 Sand Lake Rd	Hansel Ave (S)	1.14	ST	Urban - Class I	— 2 Е	2510	45,787	2,102 NB	3 19	389 C	
331 Hansel Ave (S)	Hansel Ave (N)	1.22	ST	Urban - Class II	I 2 E	2040	25,821	2,324 NB	1	0	
				(T-Way)							
332 Hansel Ave (N)	Holden Ave	0.75	ST	Urban - Class l	- 2 Е	2510	56,887	2,867 NB	3 4	0 F	
Orange Ave / Hansel Ave											
176 Orange Ave	Orange Ave	1.23	ST	Urban - Class I	2 E	2400	48,989	2,337 SB	3 1	62 D	
				(1-way)							

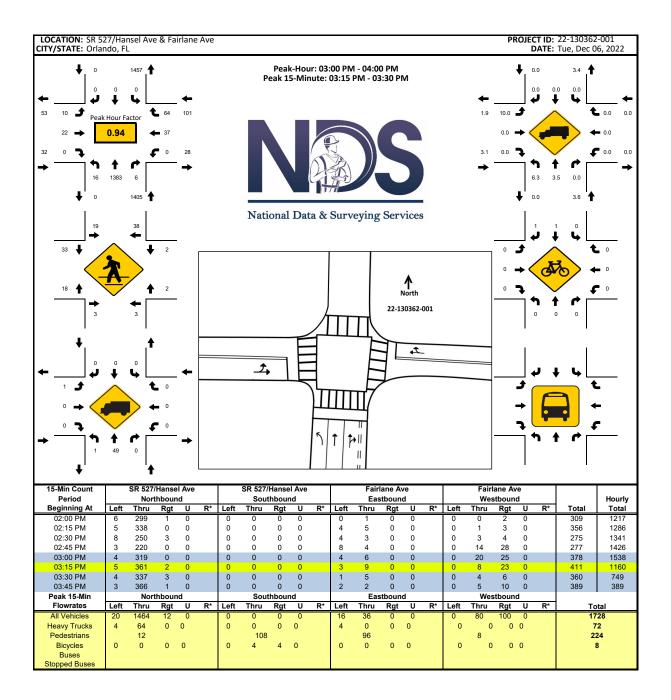
* It about the moted that the capacities indicated on this information sheet are a snapshot at this specific date and time. Available capacities are subject to at any time. 185

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APPENDIX B

Intersection Turning Movement Counts, Signal Timings and FDOT's Seasonal Reports









22-130362-001	12/06/2022	Sunny	Orlando	Orange	00:00 - 00:00	14:00 - 16:00
Site Code:	Date:	Weather:	City:	County:	Count Times:	

Control: Signalized

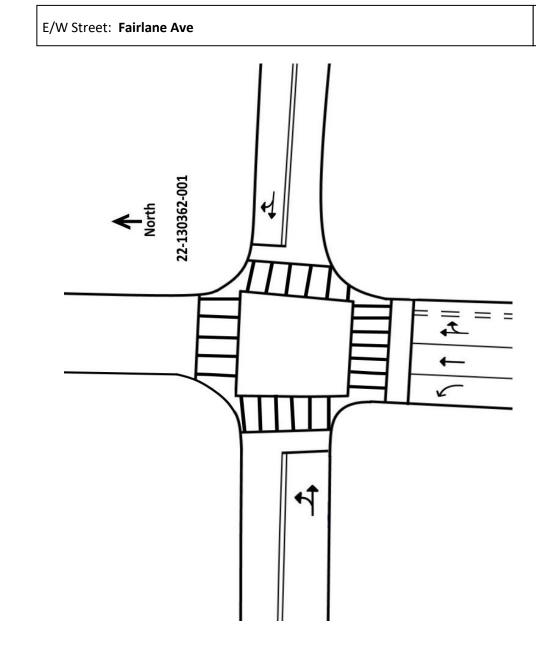
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PHASES	1	7	m
NL/NT	01:18	01:21	01:30
ET/WT	01:23	00:58	00:59

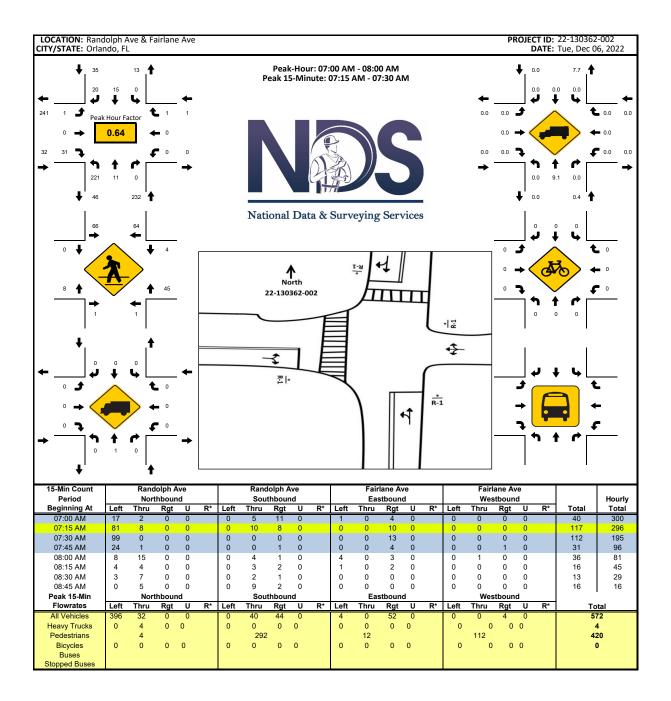


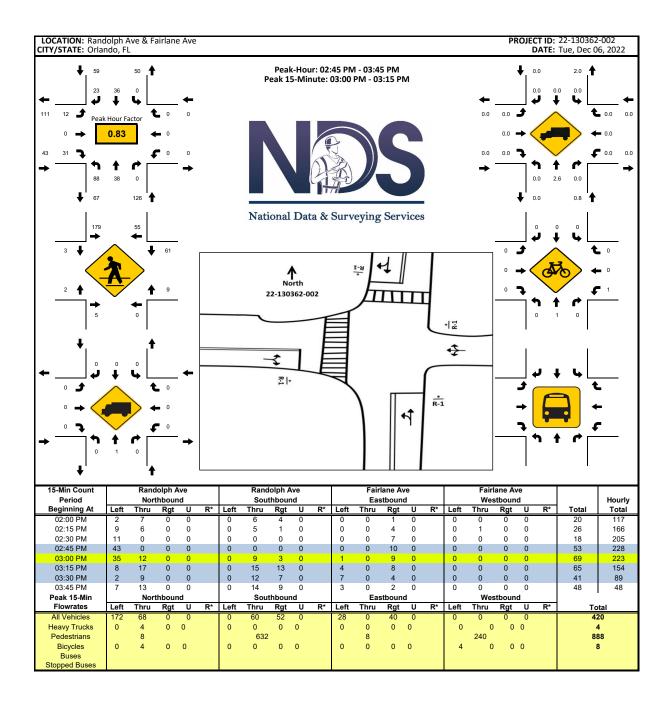
N/S Street: SR 527/Hansel Ave

Speed: 40 MPH



Speed: 25 MPH





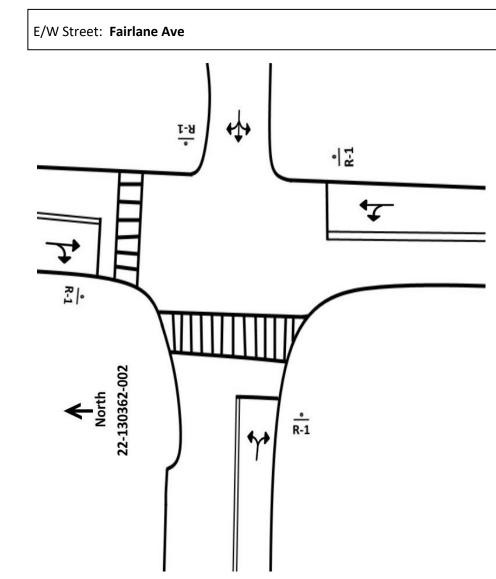


National Data & Surveying Services	ode: 22-130362-002	12/06/2022	ier: Sunny	Orlando	y: Orange	Times: 07:00 - 09:00	14:00 - 16:00	ol: 4-Way Stop
National Da	Site Code:	Date:	Weather:	City:	County:	Count Times:		Control:

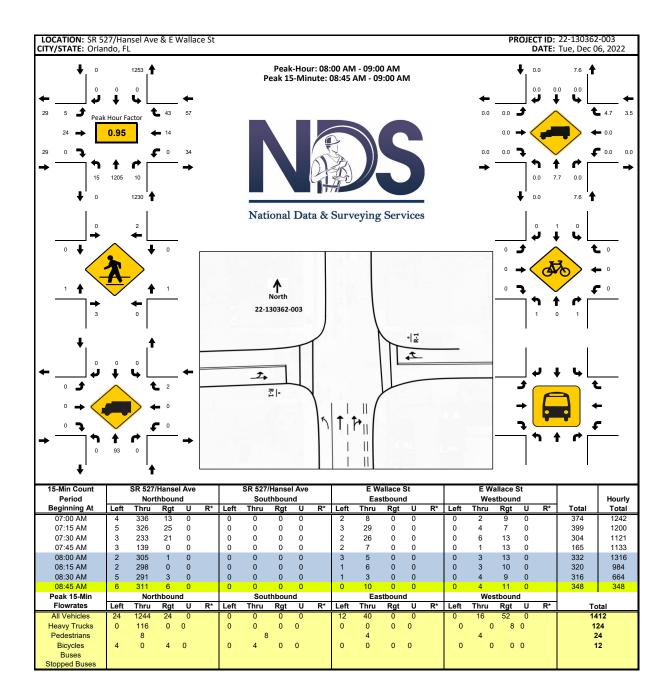


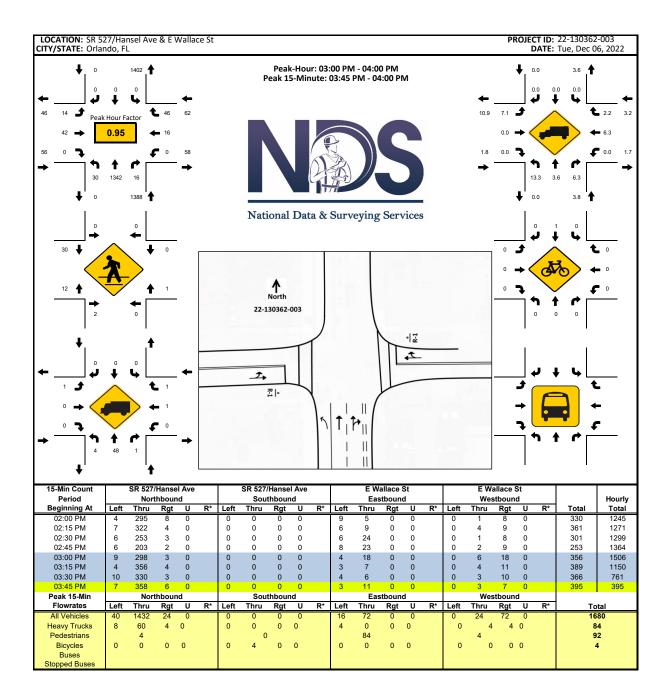
N/S Street: Randolph Ave

Speed: 25 MPH



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22-130362-003	12/06/2022	Sunny	Orlando	Orange	00:00 - 00:00	14:00 - 16:00
Site Code:	Date:	Weather:	City:	County:	Count Times:	

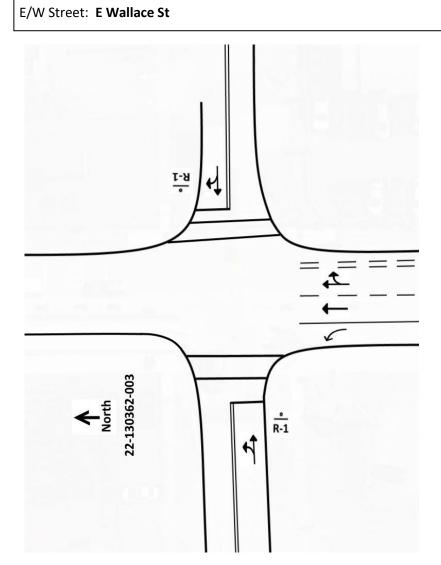
2-Way Stop(EB/WB)

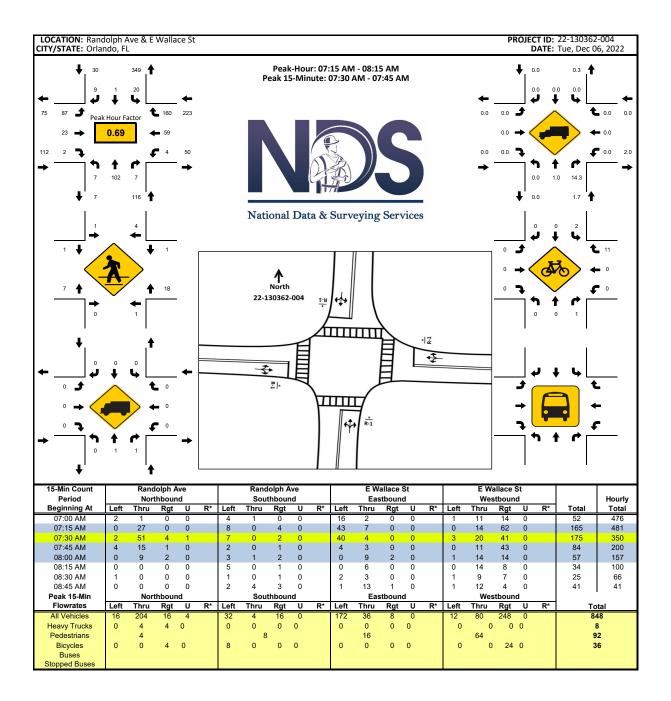
Control:

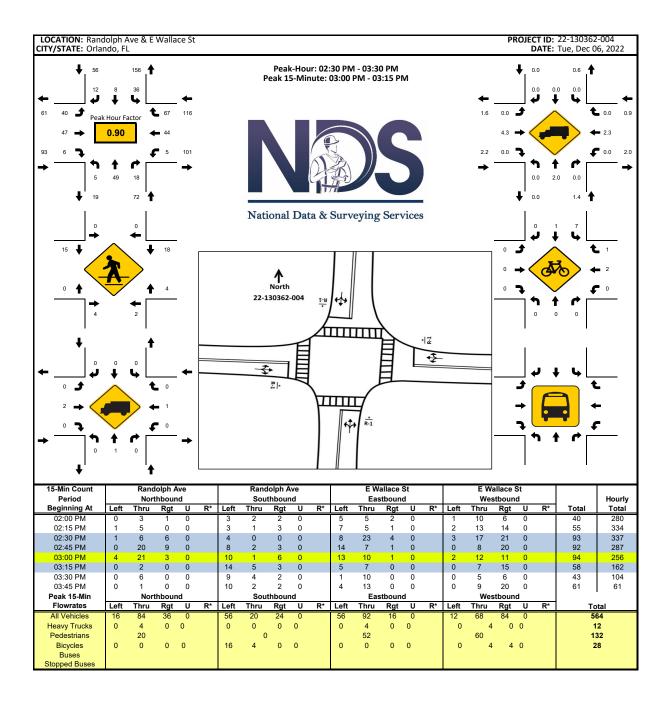


N/S Street: SR 527/Hansel Ave

Speed: 40 MPH

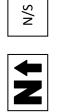






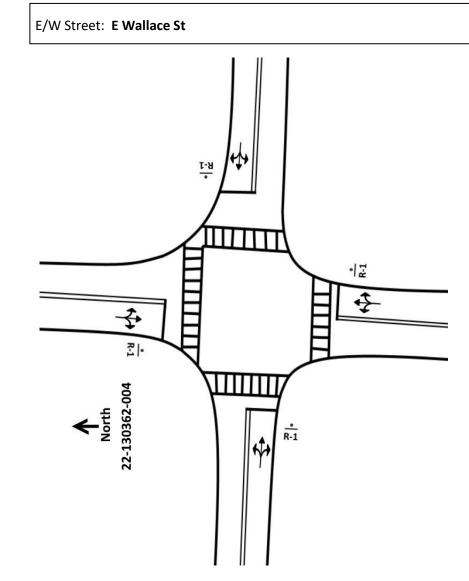


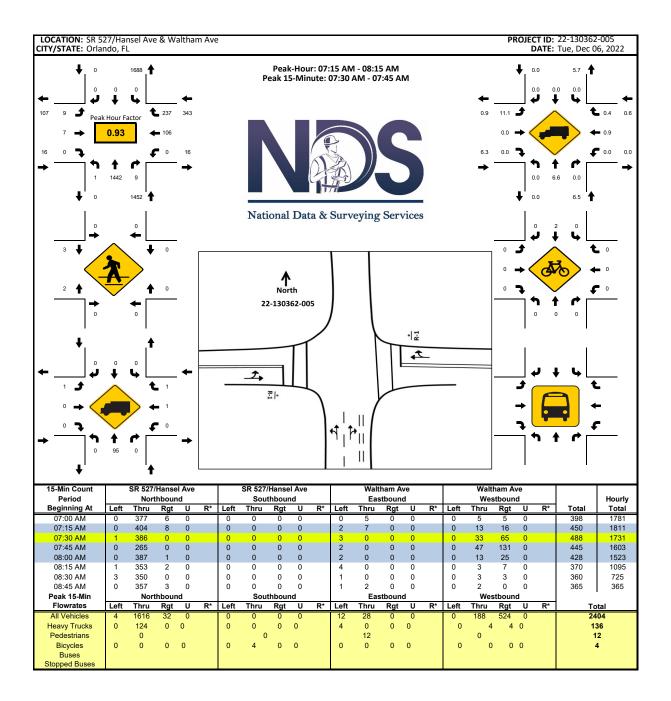
nalional Data & Surveying Servic	22-130362-004	12/06/2022	Sunny	Orlando	Orange	00:00 - 00:00	14:00 - 16:00	4-Way Stop
Nauonal Data	Site Code:	Date:	Weather:	City:	County:	Count Times:		Control:

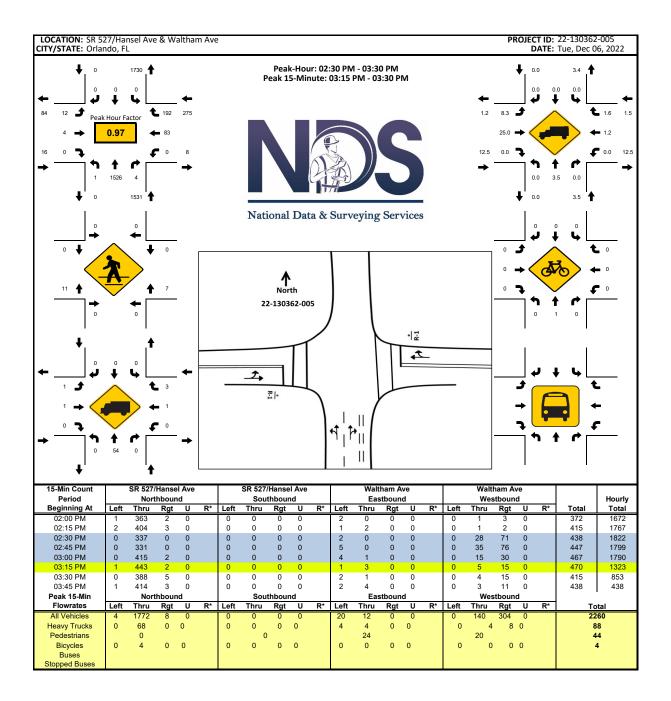


N/S Street: Randolph Ave

Speed: 25 MPH









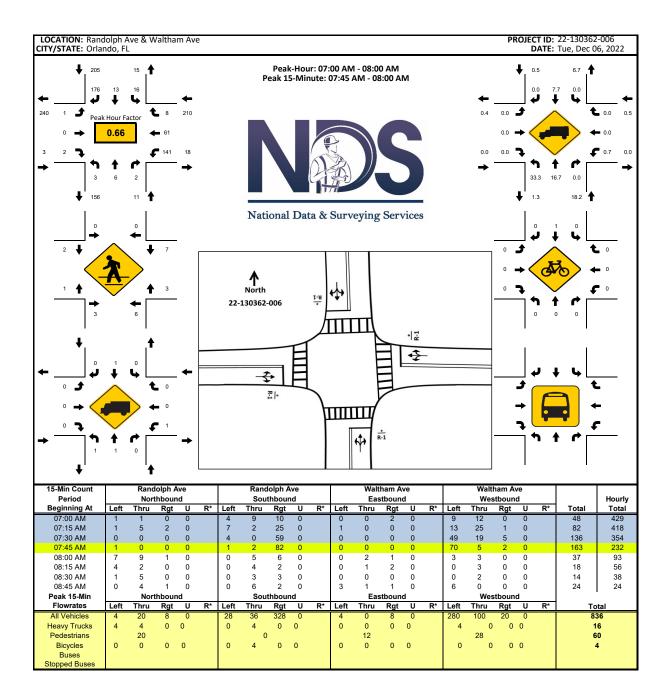
Speed: 40 MPH

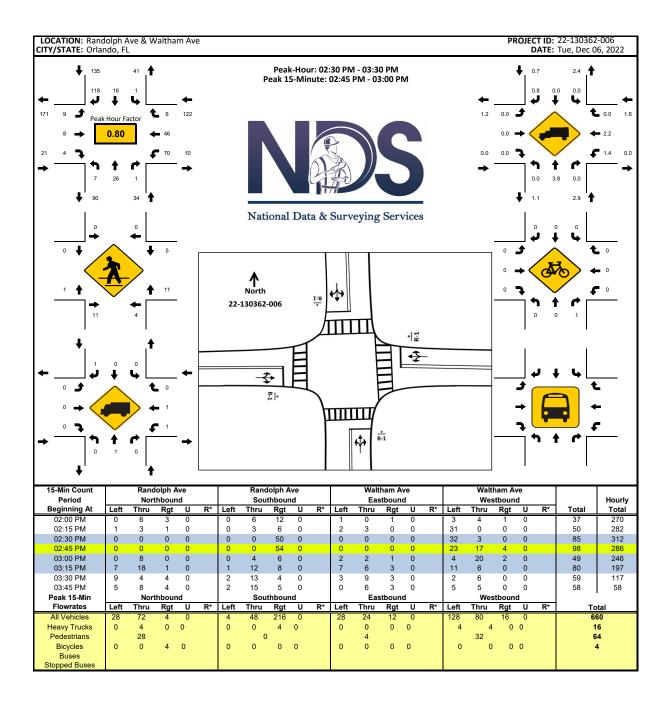
N/S Street: SR 527/Hansel Ave

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E	E/W S	Street	: Wa	ltham	Ave							
					Ţ	ا م ا	rt					
		<	North	22-130362-005				4	* R-1		-	
	National Data & Surveying Services		22-130362-005	12/06/2022	Sunny	Orlando	Orange	00:60 - 00:20	14:00 - 16:00	2-Way Stop(EB/WB)		
	National Data		Site Code:	Date:	Weather:	City:	County:	Count Times:		Control:		

Speed: 25 MPH







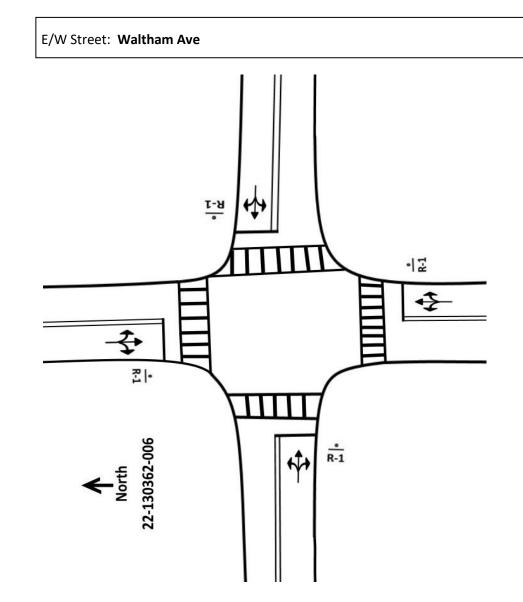


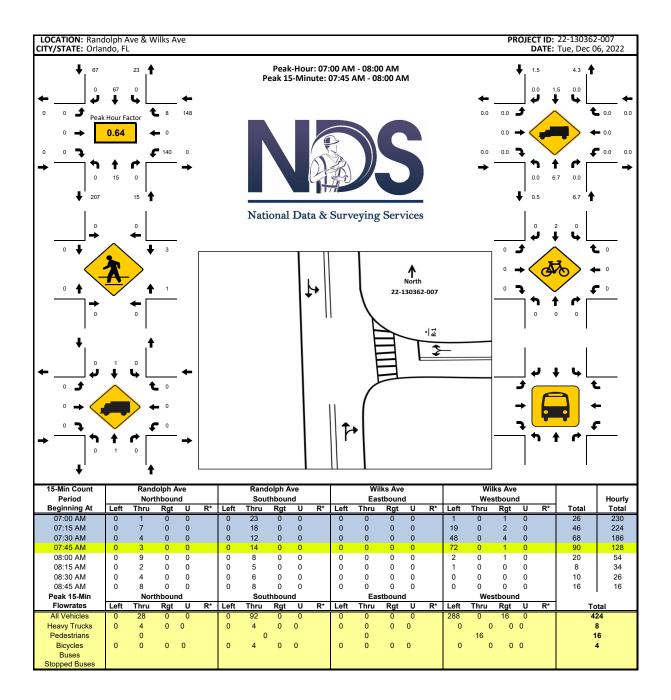
22-130362-006	12/06/2022	Sunny	Orlando	Orange	00:00 - 00:00	14:00 - 16:00	4-Way Stop
Site Code:	Date:	Weather:	City:	County:	Count Times:		Control:

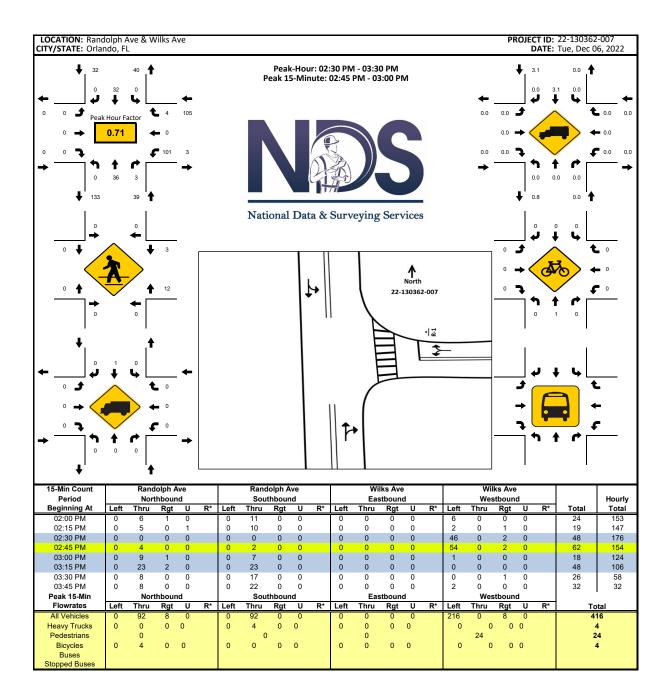


N/S Street: Randolph Ave

Speed: 25 MPH









22-130362-007	12/06/2022	Sunny	Orlando	Orange	07:00 - 09:00	14:00 - 16:00
Site Code:	Date:	Weather:	City:	County:	Count Times:	

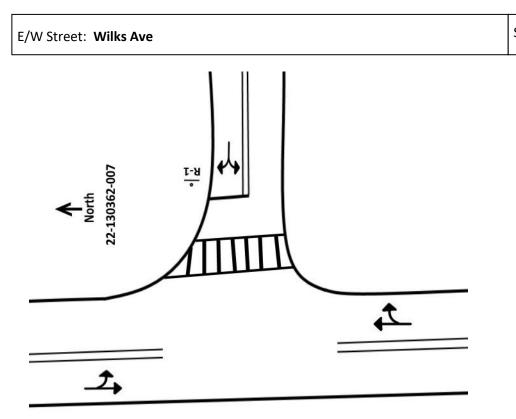
1-Way Stop(WB)

Control:



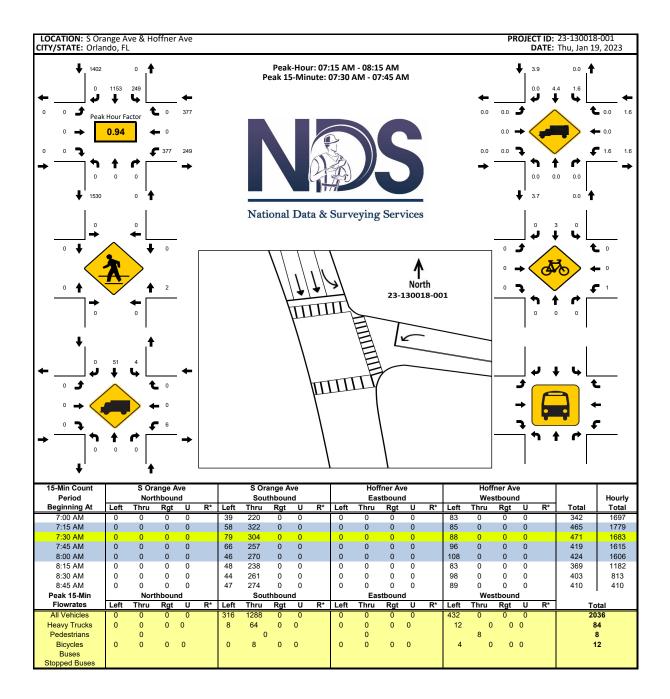
N/S Street: Randolph Ave

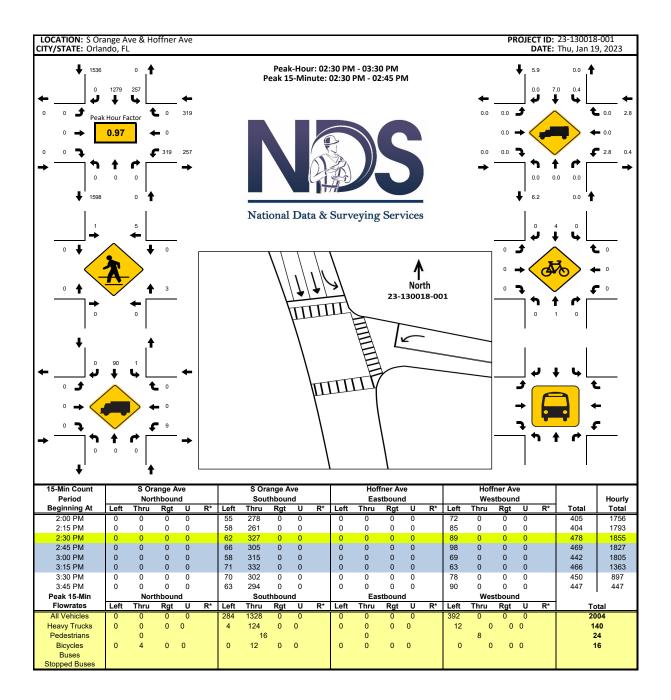
Speed: 25 MPH



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23-130018-001	01/19/2023	Sunny	Orlando	Orange	02:00 - 09:00
Site Code:	Date:	Weather:	City:	County:	Count Times:

14:00 - 16:00

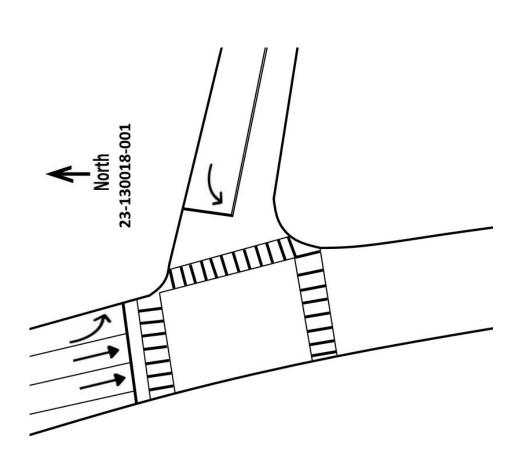
Control: Signalized

SIGNAL TIMING

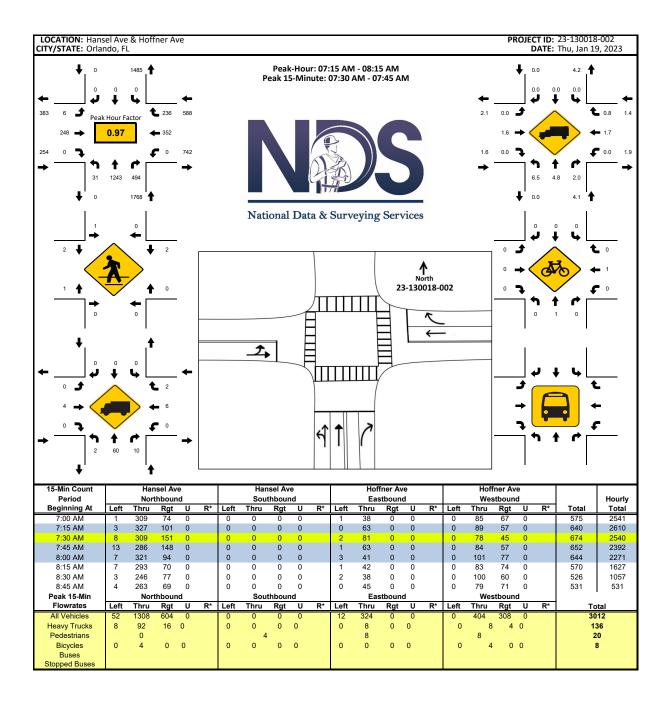
PHASES	7	7	ŝ
SL/ST	00:55	00:55	00:59
٨٢	00:35	00:20	00:32

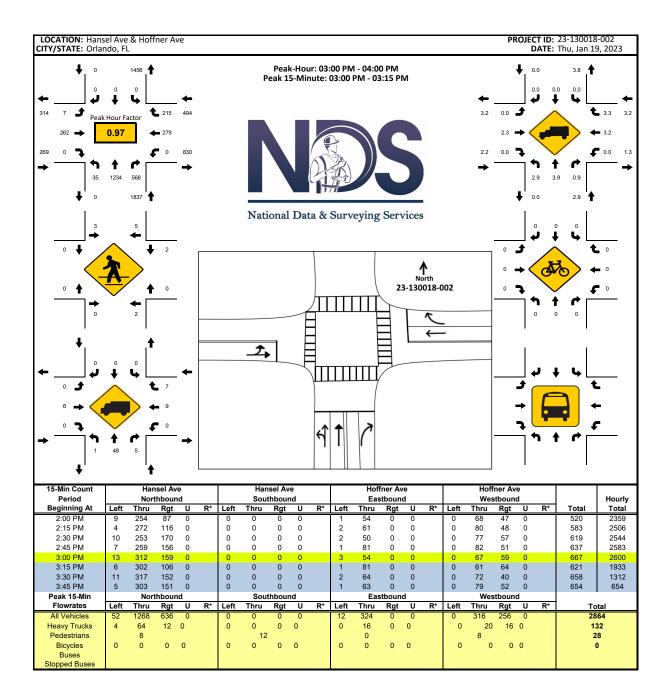


N/S Street: S Orange Ave



E/W Street: Hoffner Ave

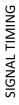




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Site Code:	23-130018-002
Date:	01/19/2023
Weather:	Sunny
City:	Orlando
County:	Orange
Count Times:	02:00 - 09:00
	14:00 - 16:00
Control:	Signalized



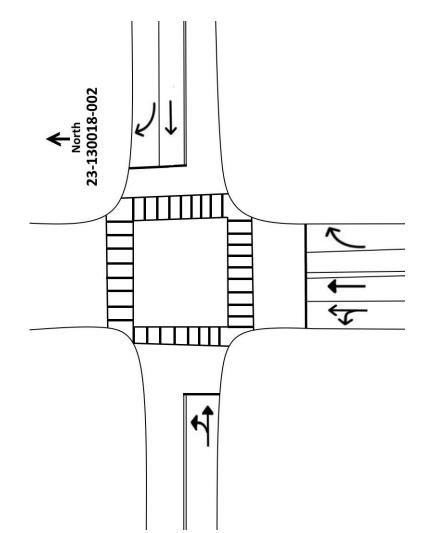
PHASES	7	7	m
NL/NT	01:32	01:17	01:21
ET/WT	00:53	00:52	00:52

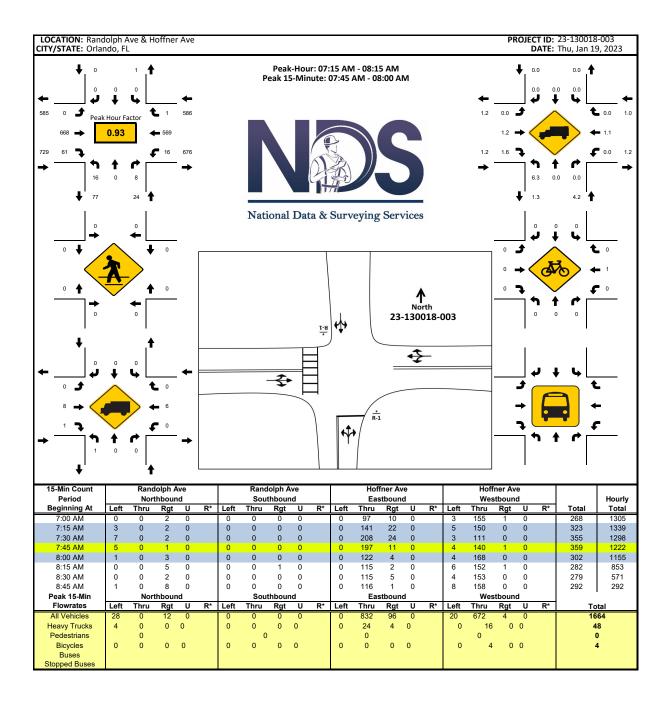


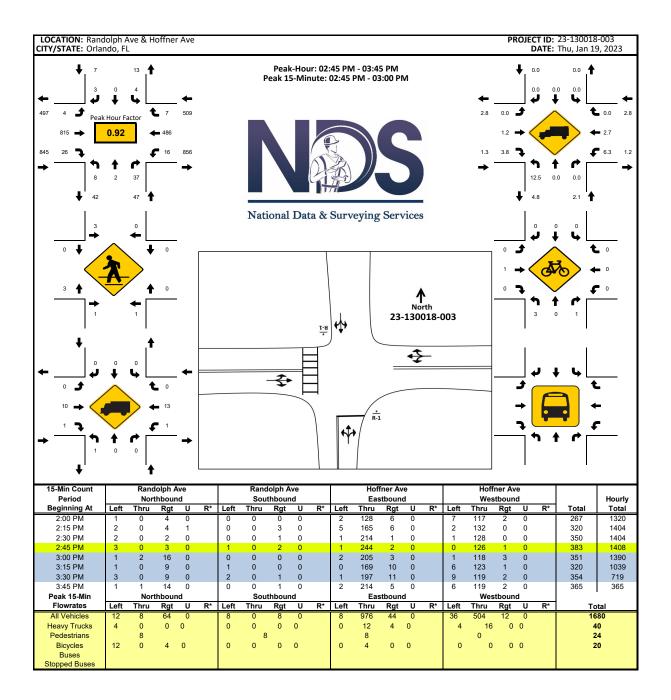
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N/S Street: Hansel Ave

Speed: 40 MPH









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National Data & Surveying Services	23-130018-003	01/19/2023	Sunny	Orlando	Orange	00:00 - 00:00	14:00 - 16:00	2-Way Stop(NB/SB)	
National Data	Site Code:	Date:	Weather:	City:	County:	Count Times: 07:00 - 09:00		Control:	

A North 23-130018-003

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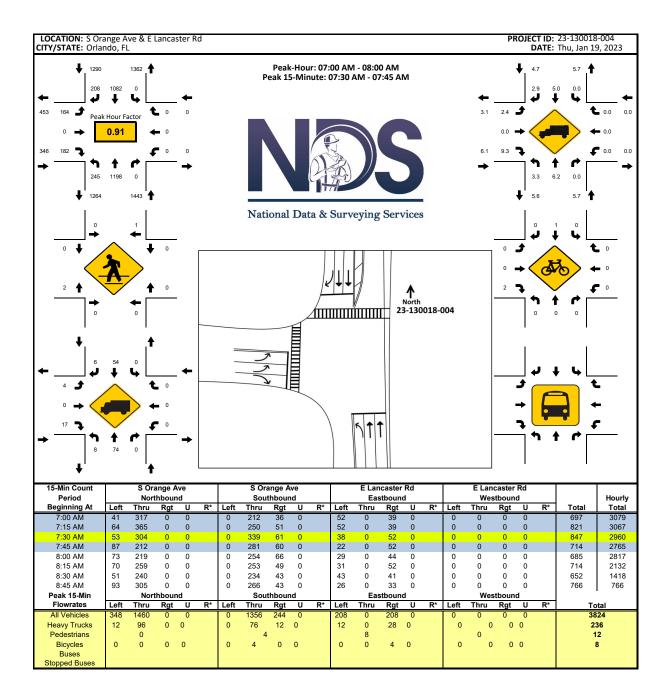
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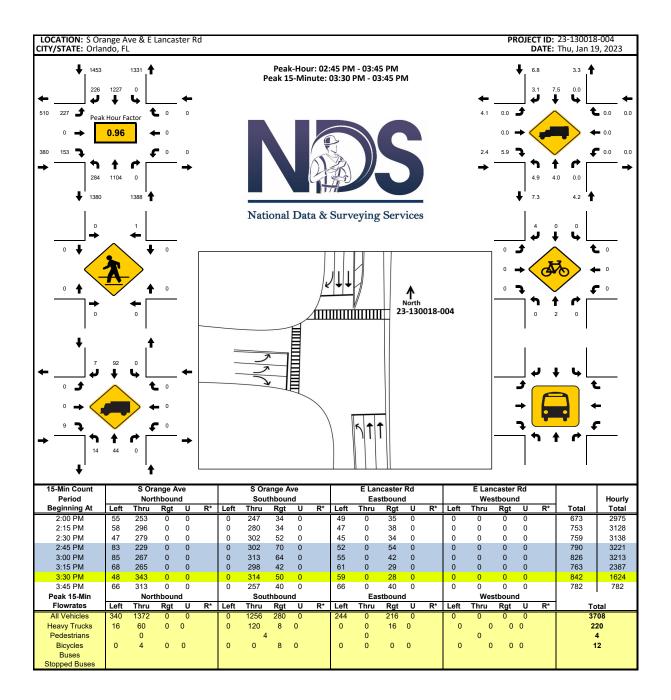
Speed: 25 MPH

N/S Street: Randolph Ave

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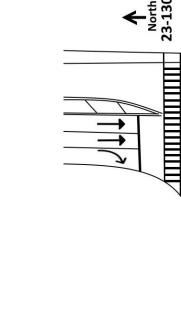


23-130018-004	01/19/2023	Sunny	Orlando	Orange	: 07:00 - 09:00	14:00 - 16:00	Signalized
Site Code:	Date:	Weather:	City:	County:	Count Times:		Control:

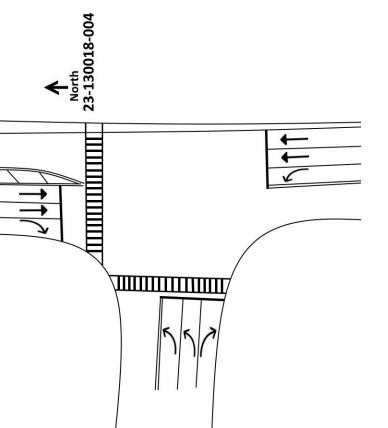
SIGNAL TIMING

PHASES	1	2	3
NL/NT	1	00:21	-
NT/ST	01:48	01:25	01:48
EL	00:24	00:22	00:24

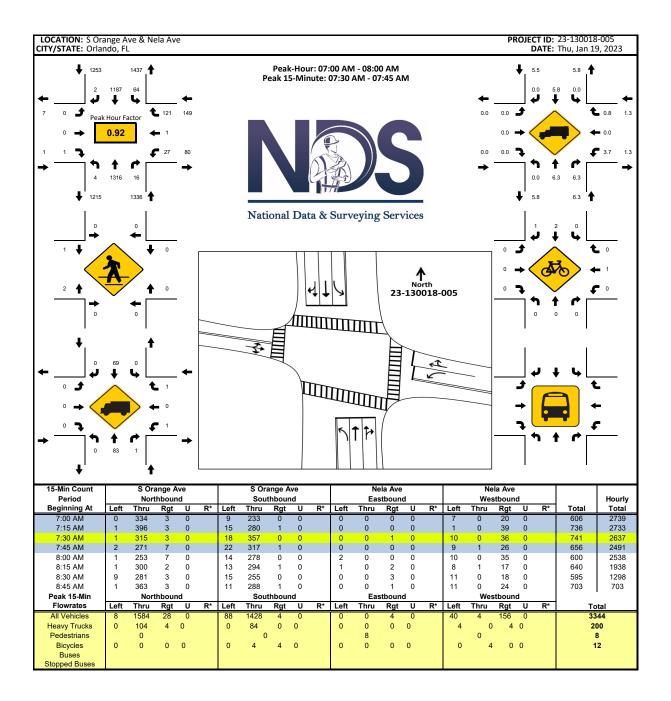
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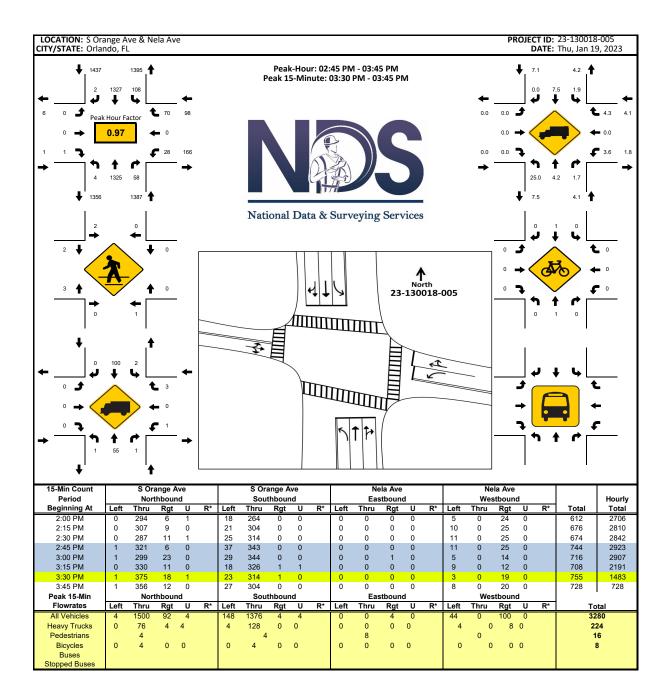


E/W Street: E Lancaster Rd



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National Data & Surveying Services

23-130018-005	01/19/2023	Sunny	Orlando	Orange	07:00 - 09:00	14:00 - 16:00
Site Code:	Date:	Weather:	City:	County:	Count Times:	

SIGNAL TIMING

Signalized

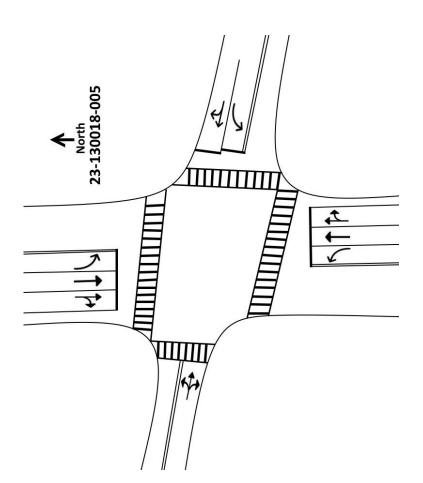
Control:

PHASES	1	2	3
SL/ST	I	00:16	00:15
NT/ST	01:38	01:21	01:37
ET/WT	00:32	00:19	00:16



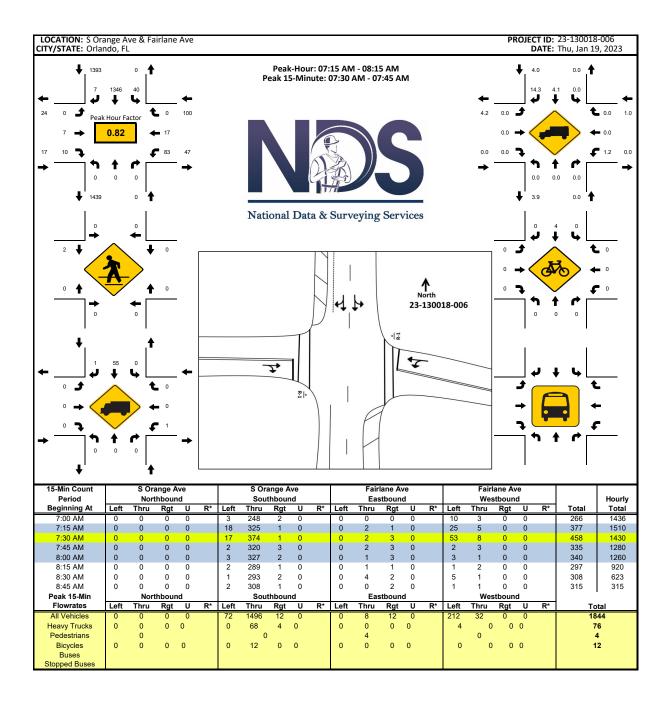
N/S Street: S Orange Ave

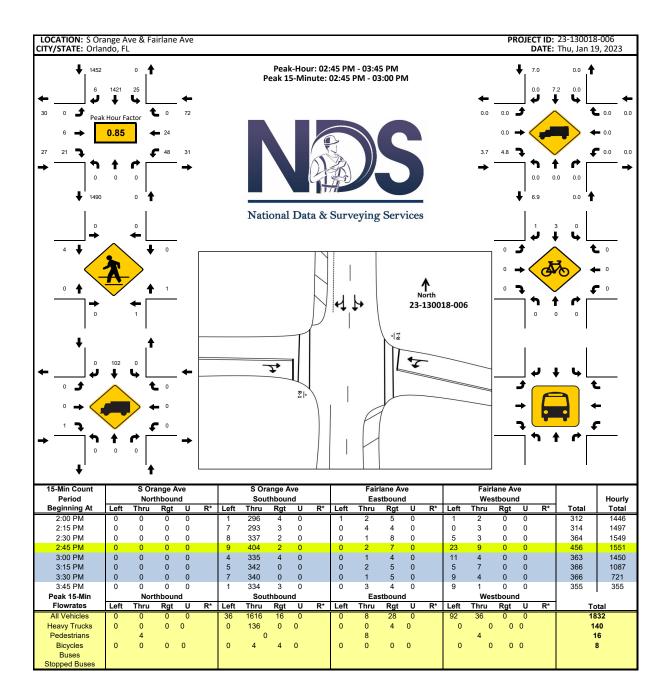
Speed: 45 MPH



E/W Street: Nela Ave

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		÷	North 23-130018-006		<u>1-8</u>	4				
	_		4 F			F	•	•. R-1		
National Data & Surveying Services		23-130018-006	01/19/2023	Sunny	Orlando	Orange	02:00 - 09:00	14:00 - 16:00	2-Way Stop(EB/WB)	
National Data		Site Code:	Date:	Weather:	City:	County:	Count Times:		Control:	

N/S Street: S Orange Ave

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E/W Street: Fairlane Ave

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CATEGO	DRI: 7500 ORANGE COUNTIWIDE	<u>.</u>	MOCF: 0.99
WEEK	DATES	SF	PSCF
53	12/26/2021 - 12/31/2021	1.09	1.10

* PEAK SEASON

08-MAR-2022 12:36:27

830UPD 5_7500_PKSEASON.TXT

Traffic

	Р	rograi	m		Patte	rn							ļ	Ph F	unc	;						
				Dial	Split	Offset	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
1	1	0	1	0	0	4																
2	1	10	0	2	1	1																
3	1	19	30	0	0	4																
4	2	0	1	0	0	4																
5	2	6	0	1	1	1																
6	2	9	0	2	1	1																
7	2	14	0	3	1	1																
8	2	18	30	2	1	1																
9	2	20	30	0	0	4																
10	7	0	1	0	0	4																
11	7	9	0	2	1	1																
12	7	19	45	0	0	4																

Vehicle Basic Timing

Phase Bank 1

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Min. Green	0	15	0	5	0	15	0	5	0	0	0	0	0	0	0	0
Passage	0	3	0	3.3	0	3	0	4	0	0	0	0	0	0	0	0
Maximum 1	0	45	0	20	0	45	0	20	0	0	0	0	0	0	0	0
Maximum 2	0	45	0	20	0	45	0	20	0	0	0	0	0	0	0	0
Yellow Change	4	4.4	4	3.4	4	4.5	4	3.7	4	4	4	4	4	4	4	4
Red Clearance	0	2	0	2	0	2	0	2	0	0	0	0	0	0	0	0
Green Delay	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Yellow Delay	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bike Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bike Passage	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Phase Bank 2

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Min. Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Passage	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Maximum 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Maximum 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Yellow Change	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Red Clearance	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Green Delay	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Yellow Delay	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bike Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bike Passage	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Phase Bank 3

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Min. Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Passage	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Maximum 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Maximum 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Yellow Change	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Red Clearance	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Green Delay	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Yellow Delay	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bike Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bike Passage	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Phase Bank 4																
Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Min. Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Passage	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Maximum 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Dial 1/Split 1

Cycle Length 130

Phase	1	2	3	4	5	6	7	8
Time	0	78	0	52	0	78	0	52
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time		Bnk1=23 Bnk2=5 Bnk3=5 Bnk4=5		Bnk1=12 Bnk2=5 Bnk3=5 Bnk4=5		Bnk1=23 Bnk2=5 Bnk3=5 Bnk4=5		Bnk1=12 Bnk2=5 Bnk3=5 Bnk4=5
Min Ped Time		Bnk1=19 Bnk2=0 Bnk3=0 Bnk4=0		Bnk1=30 Bnk2=0 Bnk3=0 Bnk4=0		Bnk1=25 Bnk2=0 Bnk3=0 Bnk4=0		Bnk1=27 Bnk2=0 Bnk3=0 Bnk4=0
Phase Reduction	0	0	0	0	0	0	0	0
Phase Extension	0	0	0	0	0	0	0	0

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time								
Phase Reduction	0	0	0	0	0	0	0	0
Phase Extension	0	0	0	0	0	0	0	0

Offset	1	2	3
Time	101	0	0
Mode	0 - Normal	0 - Normal	0 - Normal
Alt Sequence	0	0	0
Correction	0 - Normal	0 - Normal	0 - Normal
Special Function	0	0	0
Maximum Mode	0 - None	0 - None	0 - None
Ring 2 Lag Time	6	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

Dial 3/Split 1

Cycle Length 150

Phase	1	2	3	4	5	6	7	8
Time	0	95	0	55	0	95	0	55
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time		Bnk1=23 Bnk2=5 Bnk3=5 Bnk4=5		Bnk1=12 Bnk2=5 Bnk3=5 Bnk4=5		Bnk1=23 Bnk2=5 Bnk3=5 Bnk4=5		Bnk1=12 Bnk2=5 Bnk3=5 Bnk4=5
Min Ped Time		Bnk1=19 Bnk2=0 Bnk3=0 Bnk4=0		Bnk1=30 Bnk2=0 Bnk3=0 Bnk4=0		Bnk1=25 Bnk2=0 Bnk3=0 Bnk4=0		Bnk1=27 Bnk2=0 Bnk3=0 Bnk4=0
Phase Reduction	0	0	0	0	0	0	0	0
Phase Extension	0	0	0	0	0	0	0	0

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time								
Phase Reduction	0	0	0	0	0	0	0	0
Phase Extension	0	0	0	0	0	0	0	0

Offset	1	2	3
Time	11	0	0
Mode	0 - Normal	0 - Normal	0 - Normal
Alt Sequence	0	0	0
Correction	0 - Normal	0 - Normal	0 - Normal
Special Function	0	0	0
Maximum Mode	0 - None	0 - None	0 - None
Ring 2 Lag Time	6	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

Location: Hansel Ave (SR					AL TIMIN	G Hazem El-Ass	ar, P.E	
Equipment: Eagle			CDI: 2/	10/2021	CDO:		Date: 2/1	1/21
		В	ASIC TIM	IING				
Phase	1	2	3	4	5	6	7	8
Direction		NB		EB/WB				
Min Green (sec)		15		5				
Vehicle Gap (sec)		3.0		3.0				
Max Green 1 (sec)		40		30				
Max Green 2 (sec)		40		30				
Yellow (sec)		4.5		3.9				
All-Red (sec)	-	2.0		2.0				
Walk (sec)		7		7				
Flash Don't Walk (sec)		15		, 16				
	_	SF/LK		NL				
Recall/Memory		JF/LK					┨────	<u> </u>
Delay (sec)								
Detector Switching								
Dual Entry								
Overlap	_							
lash		Y		R				
Speed (mph)		40		30				
/ehicle Distance (ft)		69		81				
Crossing Distance (ft)		46.0		52.0				
Ped Clearence (sec)		13		15				
		COOR	DINATIO	N PLANS	i			
Coordination Pattern	1/1/1	2/1/1	3/1/1	•		Day	Time	Pattern
Cycle	130	110	150			1	0:01	FREE
Split 1	0	0	0			1	10:00	2/1/1
Split 2	75	78	100			1	19:30	FREE
Split 3	0	0	0			2	0:01	FREE
Split 4	55	1 22						1/1/1
		32	50			2	6:00	
	0	0	0			2	9:00	2/1/1
Split 6	0	0	0 0			2 2	9:00 14:00	2/1/1 3/1/1
Split 6 Split 7	0 0 0	0 0 0	0 0 0			2 2 2	9:00 14:00 18:30	2/1/1 3/1/1 2/1/1
Split 6 Split 7 Split 8	0 0 0 0	0 0 0 0	0 0 0 0			2 2	9:00 14:00 18:30 20:30	2/1/1 3/1/1 2/1/1 FREE
Split 6 Split 7 Split 8 Offset	0 0 0	0 0 0	0 0 0			2 2 2 2 2	9:00 14:00 18:30	2/1/1 3/1/1 2/1/1
Split 6 Split 7 Split 8 Offset Lagging Phases	0 0 0 0 111	0 0 0 0 76	0 0 0 72 0/0/0/0	Equate 4	Equate 5	2 2 2 2 7	9:00 14:00 18:30 20:30 0:01	2/1/1 3/1/1 2/1/1 FREE FREE
Split 6 Split 7 Split 8 Offset Lagging Phases Source Day	0 0 0 111 0/0/0/0 Equate 1	0 0 0 76 0/0/0/0 Equate 2	0 0 0 72 0/0/0/0 Equate 3		Equate 5	2 2 2 2 7 7 7	9:00 14:00 18:30 20:30 0:01 9:00	2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1
Split 6 Split 7 Split 8 Offset Lagging Phases	0 0 0 1111 0/0/0/0	0 0 0 76 0/0/0/0	0 0 0 72 0/0/0/0	Equate 4	Equate 5	2 2 2 2 7 7 7	9:00 14:00 18:30 20:30 0:01 9:00	2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1
Split 6 Split 7 Split 8 Offset Lagging Phases Source Day	0 0 0 111 0/0/0/0 Equate 1	0 0 0 76 0/0/0/0 Equate 2	0 0 0 72 0/0/0/0 Equate 3		Equate 5	2 2 2 2 7 7 7	9:00 14:00 18:30 20:30 0:01 9:00	2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1
Split 6 Split 7 Split 8 Offset Lagging Phases Source Day 2	0 0 0 111 0/0/0/0 Equate 1	0 0 0 76 0/0/0/0 Equate 2	0 0 0 72 0/0/0/0 Equate 3		Equate 5	2 2 2 2 7 7 7	9:00 14:00 18:30 20:30 0:01 9:00	2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1
Split 6 Split 7 Split 8 Offset Lagging Phases Source Day 2	0 0 0 111 0/0/0/0 Equate 1	0 0 0 76 0/0/0/0 Equate 2	0 0 0 72 0/0/0/0 Equate 3		Equate 5	2 2 2 2 7 7 7	9:00 14:00 18:30 20:30 0:01 9:00	2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1
Split 6 Split 7 Split 8 Offset Lagging Phases Source Day 2	0 0 0 111 0/0/0/0 Equate 1 3	0 0 76 0/0/0/0 Equate 2 4	0 0 0 72 0/0/0/0 Equate 3		Equate 5	2 2 2 2 7 7 7	9:00 14:00 18:30 20:30 0:01 9:00	2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1
Notes: 1. Offset referenced to start of t	0 0 0 111 0/0/0/0 Equate 1 3	0 0 76 0/0/0/0 Equate 2 4	0 0 0 72 0/0/0/0 Equate 3		Equate 5	2 2 2 2 7 7 7	9:00 14:00 18:30 20:30 0:01 9:00	2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1
Split 6 Split 7 Split 8 Offset Lagging Phases Source Day 2 Notes: 1. Offset referenced to start of the start	0 0 0 1111 0/0/0/0 Equate 1 3 mainstreet gree	0 0 76 0/0/0/0 Equate 2 4	0 0 0 72 0/0/0/0 Equate 3		Equate 5	2 2 2 2 7 7 7	9:00 14:00 18:30 20:30 0:01 9:00	2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1
Split 6 Split 7 Split 8 Offset Lagging Phases Source Day 2 Notes:	0 0 0 1111 0/0/0/0 Equate 1 3 mainstreet gree	0 0 76 0/0/0/0 Equate 2 4	0 0 0 72 0/0/0/0 Equate 3		Equate 5	2 2 2 2 7 7 7	9:00 14:00 18:30 20:30 0:01 9:00	2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1
Split 6 Split 7 Split 8 Offset Lagging Phases Source Day 2 Notes: 1. Offset referenced to start of the start	0 0 0 1111 0/0/0/0 Equate 1 3 mainstreet gree	0 0 76 0/0/0/0 Equate 2 4	0 0 0 72 0/0/0/0 Equate 3		Equate 5	2 2 2 2 7 7 7	9:00 14:00 18:30 20:30 0:01 9:00	2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1
Split 6 Split 7 Split 8 Dffset agging Phases Source Day 2 Notes: 2 . Offset referenced to start of the start	0 0 0 1111 0/0/0/0 Equate 1 3 mainstreet gree	0 0 76 0/0/0/0 Equate 2 4	0 0 0 72 0/0/0/0 Equate 3		Equate 5	2 2 2 2 7 7 7	9:00 14:00 18:30 20:30 0:01 9:00	2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1
Split 6 Split 7 Split 8 Offset Lagging Phases Source Day 2 Notes: 1. Offset referenced to start of the start	0 0 0 1111 0/0/0/0 Equate 1 3 mainstreet gree	0 0 76 0/0/0/0 Equate 2 4	0 0 0 72 0/0/0/0 Equate 3		Equate 5	2 2 2 2 7 7 7	9:00 14:00 18:30 20:30 0:01 9:00	2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1

Consultant Timing

APR 0 4 2018

Equipment: Siemens Eagle	R 527 SB) at	Hoffner Ave			Int. # Date:	11 7/13/2018	Node Address:	350
			BASIC T	MING				
Phase	1	2	3	4	5	6	7	8
Direction		SB		WB	T			0
/lin Green (sec)		15		5				
/ehicle Gap (sec)		3.0		5.0				
lax Green 1 (sec)		50		25				
lax Green 2 (sec)		50		25				
ellow Change Interval (sec)		4.1		3.8	-			
Red Clearance Interval (sec)		2.0		2.0				
Valk (sec)		7		7				
lash Don't Walk (sec)		15		14				
lin Split (sec)		29		27				
ecall/Memory		SF/LK		NL				
etector Delay (sec)		UT/LIX		CD 10				
etector Switching				CD 10				
ual Entry								
verlap								
lash		Y		R				
peed (mph)	L	35		30				
pproach Grades (%)		-0.5%		-1.9%				
eh Traversed Distance (ft)		70		-1.9%				
ed Crossing Distance (ft)		52		46				
ed Clearance (sec)		15		40				
ed-button to curb (ft)		12		14				
ed-button to far curb (ft)		64		58				
ed Clearance to far curb (sec	•	22		20				
		Contraction of the Contraction o		Statement of the local division of the local				
		COU	ORDINATIO	DN PLANS				
	1/1/1	2/1/1	3/1/1			- Day	Time	Pattern
oordination Pattern +	130	2/1/1 110	3/1/1 150			- Day 1	Time 0:01	Pattern FREE
ycle plit 1	130 0							
ycle plit 1 plit 2	130 0 77	110 0 78	150			1	0:01	FREE
ycle plit 1 plit 2 plit 3	130 0 77 0	110 0 78 0	150 0			1	0:01 10:00	FREE 2/1/1 FREE
ycle plit 1 plit 2 plit 3 plit 4	130 0 77 0 53	110 0 78	150 0 104			1 1 1	0:01 10:00 19:30	FREE 2/1/1 FREE
ycle plit 1 plit 2 plit 3 plit 4 plit 5	130 0 77 0 53 0	110 0 78 0 32 0	150 0 104 0			1 1 1 2	0:01 10:00 19:30 0:01	FREE 2/1/1 FREE FREE
ycle olit 1 olit 2 olit 3 olit 4 olit 5 olit 6	130 0 77 0 53 0 0	110 0 78 0 32 0 0	150 0 104 0 46			1 1 1 2 2	0:01 10:00 19:30 0:01 6:00	FREE 2/1/1 FREE FREE 1/1/1
ycle plit 1 plit 2 plit 3 plit 4 plit 5 plit 6 plit 7	130 0 77 0 53 0 0 0 0	110 0 78 0 32 0	150 0 104 0 46 0			1 1 2 2 2 2	0:01 10:00 19:30 0:01 6:00 9:00	FREE 2/1/1 FREE FREE 1/1/1 2/1/1
ycle plit 1 plit 2 plit 3 plit 4 plit 5 plit 6 plit 7 plit 8	130 0 77 0 53 0 0 0 0 0	110 0 78 0 32 0 0	150 0 104 0 46 0 0			1 1 2 2 2 2 2 2	0:01 10:00 19:30 0:01 6:00 9:00 14:00	FREE 2/1/1 FREE FREE 1/1/1 2/1/1 3/1/1
ycle blit 1 blit 2 blit 3 blit 4 blit 5 blit 6 blit 7 blit 8 ffset	130 0 77 0 53 0 0 0 0 0 0 109	110 0 78 0 32 0 0 0 0 0 0 90	150 0 104 0 46 0 0 0 0			1 1 2 2 2 2 2 2 2 2 2	0:01 10:00 19:30 0:01 6:00 9:00 14:00 18:30	FREE 2/1/1 FREE FREE 1/1/1 2/1/1 3/1/1 2/1/1
ycle blit 1 blit 2 blit 3 blit 4 blit 5 blit 6 blit 7 blit 8 ifset ugging Phases	130 0 77 0 53 0 0 0 0 0	110 0 78 0 32 0 0 0 0 0	150 0 104 0 46 0 0 0 0 0			1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2	0:01 10:00 19:30 0:01 6:00 9:00 14:00 18:30 20:30 0:01	FREE 2/1/1 FREE FREE 1/1/1 2/1/1 3/1/1 7REE FREE
ycle blit 1 blit 2 blit 3 blit 4 blit 5 blit 6 blit 7 blit 8 ifset Igging Phases burce Day	130 0 77 0 53 0 0 0 0 0 0 109	110 0 78 0 32 0 0 0 0 0 0 90	150 0 104 0 46 0 0 0 0 0 131	Equate 4	Equate 5	1 1 2 2 2 2 2 2 2 2 2 2 2 2 7	0:01 10:00 19:30 0:01 6:00 9:00 14:00 18:30 20:30	FREE 2/1/1 FREE FREE 1/1/1 2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1
ycle blit 1 blit 2 blit 3 blit 4 blit 5 blit 6 blit 7 blit 8 ffset gging Phases burce Day (Sunday) 1	130 0 77 0 53 0 0 0 0 109 0/0/0/0 Equate 1	110 0 78 0 32 0 0 0 0 0 90 0/0/0/0	150 0 104 0 46 0 0 0 0 131 0/0/0/0	Equate 4	Equate 5	1 1 2 2 2 2 2 2 2 2 2 2 7 7 7	0:01 10:00 19:30 0:01 6:00 9:00 14:00 18:30 20:30 0:01 9:00	FREE 2/1/1 FREE FREE 1/1/1 2/1/1 3/1/1 7REE FREE
ycle blit 1 blit 2 blit 3 blit 4 blit 5 blit 6 blit 7 blit 8 fset gging Phases burce Day (Sunday) 1 (Monday) 2	130 0 77 0 53 0 0 0 0 0 109 0/0/0/0	110 0 78 0 32 0 0 0 0 0 90 0/0/0/0	150 0 104 0 46 0 0 0 0 131 0/0/0/0	Equate 4	Equate 5	1 1 2 2 2 2 2 2 2 2 2 2 7 7 7	0:01 10:00 19:30 0:01 6:00 9:00 14:00 18:30 20:30 0:01 9:00	FREE 2/1/1 FREE FREE 1/1/1 2/1/1 3/1/1 2/1/1 FREE FREE 2/1/1
vcle blit 1 blit 2 blit 3 blit 4 blit 5 blit 6 blit 7 blit 8 fset gging Phases burce Day (Sunday) 1	130 0 77 0 53 0 0 0 0 109 0/0/0/0 Equate 1	110 0 78 0 32 0 0 0 0 90 0/0/0/0 Equate 2	150 0 104 0 46 0 0 0 0 131 0/0/0/0 Equate 3		Equate 5	1 1 2 2 2 2 2 2 2 2 2 2 7 7 7	0:01 10:00 19:30 0:01 6:00 9:00 14:00 18:30 20:30 0:01 9:00	FREE 2/1/1 FREE 1/1/1 2/1/1 2/1/1 7REE FREE 2/1/1

Consultant Timing

APR 0 4 2018

Intersection: Orange Ave (S Equipment: Eagle ATC	R 527) at Lai	icaster Rd			Int. # Date:	3 7/13/2018	Node Address:	168
			BASIC T	IMING				
Phase *	1	2	3	4	5	6	7	8
Direction	NBL	SB	EBL		T	NB	,	WB Peo
Min Green (sec)	5	15	5			15		0
Vehicle Gap (sec)	3.0	3.0	3.0			3.0		
Max Green 1 (sec)	25	60	30			60		1.0
Max Green 2 (sec)	25	60	30			60		10
Yellow Change Interval (sec)	4.8	4.8	4.5			4.8		4.5
Red Clearance Interval (sec)	2.3	2.0	2.3			2.0		2.3
Walk (sec)		7				2.0		2.3
Flash Don't Walk (sec)		21						25
Min Split (sec)	13	35	12			22		
Recall/Memory	NL	MIN/LK	NL			MIN/LK		39
Detector Delay (sec)	CD 10					WIIN/LK		NL
Detector Switching								
Dual Entry								
Overlap	CD		D					0.5
Flash		Y	R			Y		CD
Speed (mph)	45	45	40			45		
Approach Grades (%)	-0.3%	0.5%	-0.7%			-0.3%		
/eh Traversed Distance (ft)	99	119	98			101		
Ped Crossing Distance (ft)		72				101		00
Ped Clearance (sec)		21						86
Ped-button to curb (ft)		11						25
Ped-button to far curb (ft)		83						12
Ped Clearance to far curb (sec		28						98 33
		and the second se	ORDINATIO	ON PLANS	-			33
Coordination Pattern	1/1/1	2/1/1						
Cycle	130	110	3/1/1	1		Day	Time	Pattern
Split 1	24	18	150			1	0:01	FREE
Split 2	66	60	25 75			1	10:00	2/1/1
Split 3	20	20	26			1	19:30	FREE
Split 4	0					2	0:01	FREE
plit 5	0	0	0			2	6:00	1/1/1
plit 6	90	78				2	9:00	2/1/1
plit 7	0		100			2	14:00	3/1/1
plit 8	20	0	0			2	18:30	2/1/1
Offset	35	12	24			2	20:30	FREE
agging Phases	0/0/0/0	29	142			7	0:01	FREE
ource Day	Equate 1	0/0/0/0	0/0/0/0	.	-	7	9:00	2/1/1
(Sunday) 1	Equate	Equate 2	Equate 3	Equate 4	Equate 5	7	19:45	FREE
(Monday) 2	3	4	E	0				
(Saturday) 7	5	4	5	6				
otes:								
Offset referenced to start of ma	ainstreet area	n						
Use Plan Force-offs	union del giet	20				All Patte		
Use Max Inhibit during coordin					1	2	3	8

	ORANG		TY TRAF	IC SIGN		G Hazem El-Ass	ar, P.E.	ni). Nan Afri 2. an Chang Carrier, oan Fulfa Dagamerry, an at Agama riana (ball ant. 1900
Location: Orange Ave (SR	527) at Nela	Ave - Glen					Node: 497	7
Equipment: Eagle			CDI: 1/	22/20	CDO:		Date: 2/1	5/21
		В	ASIC TIN	IING				
Phase	1	2	3	4	5	6	7	8
Direction	SBL	NB		EB		SB		WB
Min Green (sec)	5	15		5		15		5
Vehicle Gap (sec)	3.0	3.0		3.0		3.0		3.0
Max Green 1 (sec)	15	50		15		50		15
Max Green 2 (sec)	15	50		15		50		15
Yellow (sec)	4.8	4.8		3.8		4.8		4.0
All-Red (sec)	2.0	2.0		2.4		2.0		2.0
Walk (sec)		7		7		7		7
Flash Don't Walk (sec)		17		20		9		20
Recall/Memory	NL	SF/LK		NL		SF/LK		NL
Delay (sec)				CD 10				DRT 10
	1>6							
Detector Switching	120			v		V		
Dual Entry		Y		Y		Y		Y
Overlap								
Flash		Y		R		Y		R
Speed (mph)	45	45		25		45		35
Vehicle Distance (ft)	85	87		102		90		100
Crossing Distance (ft)		59.0		70.0		30.0		67.0
Ped Clearence (sec)		17		20		9		19
		COOR	DINATIO	N PLANS				
Coordination Pattern	1/1/1	2/1/1	3/1/1			Day	Time	Pattern
Cycle	130	110	150	0	0	1	0:01	FREE
Split 1	18	18	27			1	10:00	2/1/1
Split 2	92	72	103			1	19:30	FREE
Split 3	0	0	0			2	0:01	FREE
Split 4	20 0	20 0	20 0			2	6:00 9:00	1/1/1 2/1/1
Split 5 Split 6	110	90	130			2	14:00	3/1/1
Split 7	0	0	0			2	18:30	2/1/1
Split 8	20	20	20			2	20:30	FREE
Offset	10	90	68			7	0:01	FREE
Lagging Phases	0/0/0/0	0/0/0/0	0/0/0/0			7	9:00	2/1/1
Source Day	Equate 1	Equate 2	Equate 3	Equate 4	Equate 5	7	19:45	FREE
1		A						
2 7	3	4	5	6				
1								
Notes:								
1. Offset referenced to start of mainstree	et green							
2. Use Plan Force-offs								
3. Use Max Inhibit during coordination								
							1	1

APPENDIX C

Existing Capacity Analysis Worksheet

nersection	
nt Delay, s/veh	0.9

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			4			4			\$	
Traffic Vol, veh/h	0	721	66	17	615	1	17	0	9	0	0	0
Future Vol, veh/h	0	721	66	17	615	1	17	0	9	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	0	1	2	0	1	0	6	0	0	0	0	0
Mvmt Flow	0	775	71	18	661	1	18	0	10	0	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	662	0	0	846	0	0	1509	1509	811	1514	1544	662
Stage 1	-	-	-	-	-	-	811	811	-	698	698	-
Stage 2	-	-	-	-	-	-	698	698	-	816	846	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.16	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.16	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.16	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.554	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	936	-	-	800	-	-	97	122	383	99	116	465
Stage 1	-	-	-	-	-	-	367	396	-	434	445	-
Stage 2	-	-	-	-	-	-	425	445	-	374	381	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	936	-	-	800	-	-	94	118	383	94	112	465
Mov Cap-2 Maneuver	-	-	-	-	-	-	94	118	-	94	112	-
Stage 1	-	-	-	-	-	-	367	396	-	434	429	-
Stage 2	-	-	-	-	-	-	410	429	-	365	381	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.3			41.2			0		
HCM LOS							E			А		
Minor Lane/Major Mymt		NBI n1	FBI	FBT	FBR	WBI	WBT	WBR	SBI n1			

NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	
127	936	-	-	800	-	-	-	
0.22	-	-	-	0.023	-	-	-	
41.2	0	-	-	9.6	0	-	0	
E	Α	-	-	А	А	-	Α	
0.8	0	-	-	0.1	-	-	-	
	0.22 41.2 E	127 936 0.22 - 41.2 0 E A	127 936 - 0.22 41.2 0 - E A -	127 936 - - 0.22 - - - - 41.2 0 - - - E A - - -	127 936 - - 800 0.22 - - - 0.023 41.2 0 - - 9.6 E A - - A	127 936 - - 800 - 0.22 - - - 0.023 - 41.2 0 - - 9.6 0 E A - - A A	127 936 - - 800 - - 0.22 - - - 0.023 - - 41.2 0 - - 9.6 0 - E A - - A A -	127 936 - - 800 - - - 0.22 - - - 0.023 - - - 41.2 0 - - 9.6 0 - 0 E A - - A A - A

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HCM 6th Signalized Intersection Summary 2: Hansel Ave & Hoffner Ave

a.

2: Hansel Ave & Hollne	IAVE										7	еак пои
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations		đ,			*	1		-a^+	1			
Traffic Volume (veh/h)	6	268	0	0	380	255	33	1342	534	0	0	C
Future Volume (veh/h)	6	268	0	0	380	255	33	1342	534	0	0	(
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00	-	1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1900	1870	0	0	1870	1885	1811	1826	1870			
Adj Flow Rate, veh/h	6	276	0	0	392	263	34	1384	551			
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97			
Percent Heavy Veh, %	0	2	0	0	2	1	6	5	2			
Cap, veh/h	30	370	Ŭ	Ũ	453	387	54	2302	1050			
Arrive On Green	0.24	0.24	0.00	0.00	0.24	0.24	0.66	0.66	0.66			
Sat Flow, veh/h	7	1529	0.00	0.00	1870	1598	81	3475	1585			
Grp Volume(v), veh/h	282	0	0	0	392	263	761	657	551			
Grp Sat Flow(s), veh/h/ln	1536	0	0	0	1870	1598	1822	1735	1585			
Q Serve(g_s), s	1.0	0.0	0.0	0.0	26.1	19.4	31.4	26.8	23.4			
	27.1	0.0	0.0	0.0	26.1	19.4	31.4	26.8	23.4			
Cycle Q Clear(g_c), s Prop In Lane	0.02	0.0	0.0	0.0	20.1	19.4	0.04	20.0	23.4			
		0			450			1110				
Lane Grp Cap(c), veh/h	400	0	0	0	453	387	1207	1149	1050			
V/C Ratio(X)	0.71	0.00	0.00	0.00	0.87	0.68	0.63	0.57	0.52			
Avail Cap(c_a), veh/h	646	0	0	0	706	603	1207	1149	1050			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(I)	0.97	0.00	0.00	0.00	1.00	1.00	1.00	1.00	1.00			
Uniform Delay (d), s/veh	44.1	0.0	0.0	0.0	47.2	44.7	12.7	11.9	11.3			
Incr Delay (d2), s/veh	2.2	0.0	0.0	0.0	7.0	2.1	2.5	2.1	1.9			
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(95%),veh/In	13.1	0.0	0.0	0.0	18.9	12.6	18.2	15.2	12.7			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	46.3	0.0	0.0	0.0	54.2	46.8	15.2	14.0	13.2			
LnGrp LOS	D	A	A	A	D	D	В	В	В			
Approach Vol, veh/h		282			655			1969				
Approach Delay, s/veh		46.3			51.2			14.2				
Approach LOS		D			D			В				
Timer - Assigned Phs		2		4				8				
Phs Duration (G+Y+Rc), s		92.6		37.4				37.4				
Change Period (Y+Rc), s		6.5		5.9				5.9				
Max Green Setting (Gmax), s		68.5		49.1				49.1				
Max Q Clear Time (g_c+l1), s		33.4		28.1				29.1				
Green Ext Time (p_c), s		16.0		3.3				1.6				
Intersection Summary												
HCM 6th Ctrl Delay			25.7									
HCM 6th LOS			С									

	1	*	t	1	1	ŧ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	5				3	**
Traffic Volume (veh/h)	407	0	0	0	269	1245
Future Volume (veh/h)	407	0	0	0	269	1245
Initial Q (Qb), veh	0	0			0	0
Ped-Bike Adj(A_pbT)	1.00	1.00			1.00	
Parking Bus, Adj	1.00	1.00			1.00	1.00
Work Zone On Approach	No					No
Adj Sat Flow, veh/h/ln	1870	0			1870	1841
Adj Flow Rate, veh/h	433	0			286	1324
Peak Hour Factor	0.94	0.94			0.94	0.94
Percent Heavy Veh, %	2	0			2	4
Cap, veh/h	0	0			1753	3333
Arrive On Green	0.00	0.00			0.95	0.95
Sat Flow, veh/h	0				1781	3589
Grp Volume(v), veh/h	0.0				286	1324
Grp Sat Flow(s),veh/h/ln					1781	1749
Q Serve(g_s), s					1.2	3.7
Cycle Q Clear(g_c), s					1.2	3.7
Prop In Lane					1.00	
Lane Grp Cap(c), veh/h					1753	3333
V/C Ratio(X)					0.16	0.40
Avail Cap(c_a), veh/h					1753	3333
HCM Platoon Ratio					1.00	1.00
Upstream Filter(I)					1.00	1.00
Uniform Delay (d), s/veh					0.2	0.2
Incr Delay (d2), s/veh					0.2	0.4
Initial Q Delay(d3),s/veh					0.0	0.0
%ile BackOfQ(95%),veh/ln					0.2	0.3
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh					0.4	0.6
LnGrp LOS					A	A
Approach Vol, veh/h						1610
Approach Delay, s/veh						0.5
Approach LOS						0.5 A
		0				~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
Timer - Assigned Phs		2				
Phs Duration (G+Y+Rc), s		130.0				
Change Period (Y+Rc), s		6.1				
Max Green Setting (Gmax), s		70.9				
Max Q Clear Time (g_c+l1), s		5.7				
Green Ext Time (p_c), s		16.2				
Intersection Summary						
HCM 6th Ctrl Delay			0.5			
HCM 6th LOS			A			

Intersection						
Int Delay, s/veh	6.8					
			NDT			007
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1 5	_		4 67
Traffic Vol, veh/h	140	8		0	0	
Future Vol, veh/h	140	8	15	0	0	67
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	64	64	64	64	64	64
Heavy Vehicles, %	0	0	7	0	0	1
Mymt Flow	219	13	23	0	0	105
NA - ' /NA'	Maria		Materia		Materia	
Major/Minor	Minor1		Major1	<u> </u>	Major2	-
Conflicting Flow All	128	23	0	0	23	0
Stage 1	23	-	-	-	-	-
Stage 2	105	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	871	1060	-	-	1605	-
Stage 1	1005	-	-	-	-	-
Stage 2	924	-	-	-	-	-
Platoon blocked, %			-			-
Mov Cap-1 Maneuver	871	1060	-	-	1605	-
Mov Cap-2 Maneuver	871	-	-	-	-	-
Stage 1	1005	_				
Stage 2	924	-	-	-	-	-
Slaye Z	924	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.6		0		0	
HCM LOS	В					
Miner Lene (Meier M)		NDT			CDI	007
Minor Lane/Major Mvmt		NBT		WBLn1	SBL	SBT
Capacity (veh/h)		-	-	879	1605	-
HCM Lane V/C Ratio		-	-	0.263	-	-
HCM Control Delay (s)		-	-	10.6	0	-
HCM Lane LOS		-	-	В	А	-
					•	

HCM 95th %tile Q(veh)

1.1

0

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4.			4.			4			4	
Traffic Vol, veh/h	1	0	2	141	61	8	3	6	2	16	13	176
Future Vol, veh/h	1	0	2	141	61	8	3	6	2	16	13	176
Peak Hour Factor	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66
Heavy Vehicles, %	0	0	0	1	0	0	33	17	0	0	8	0
Mvmt Flow	2	0	3	214	92	12	5	9	3	24	20	267
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	7.8			11.2			8.8			9.7		
HCM LOS	А			В			А			А		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	27%	33%	67%	8%
Vol Thru, %	55%	0%	29%	6%
Vol Right, %	18%	67%	4%	86%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	11	3	210	205
LT Vol	3	1	141	16
Through Vol	6	0	61	13
RT Vol	2	2	8	176
Lane Flow Rate	17	5	318	311
Geometry Grp	1	1	1	1
Degree of Util (X)	0.026	0.006	0.42	0.365
Departure Headway (Hd)	5.568	4.684	4.749	4.229
Convergence, Y/N	Yes	Yes	Yes	Yes
Сар	641	758	754	851
Service Time	3.618	2.748	2.795	2.256
HCM Lane V/C Ratio	0.027	0.007	0.422	0.365
HCM Control Delay	8.8	7.8	11.2	9.7
HCM Lane LOS	А	А	В	А
HCM 95th-tile Q	0.1	0	2.1	1.7

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Intersection												
Int Delay, s/veh	75.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1			ţ,			đ b				
Traffic Vol, veh/h	9	4 7	0	0	106	237	1	1442	9	0	0	0
Future Vol, veh/h	9	7	0	0	106	237	1	1442	9	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	1081	749504	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	11	0	0	0	1	0	0	7	0	0	0	0
Mvmt Flow	10	8	0	0	114	255	1	1551	10	0	0	0
Major/Minor	Minor2			Minor1			Major1					
Conflicting Flow All	835	1563	-	-	1558	781	0	0	0			
Stage 1	000	0	-	-	1558	-	-	-	-			
Stage 2	835	1563	-	-	0	-	-	-	-			
Critical Hdwy	7.72	6.5	-	-	6.52	6.9	4.1	-	-			
Critical Hdwy Stg 1	-	-	-	-	5.52	-	-	-	-			
Critical Hdwy Stg 2	6.72	5.5	-	-	-	-	-	-	-			
Follow-up Hdwy	3.61	4	-	-	4.01	3.3	2.2	-	-			
Pot Cap-1 Maneuver	246	113	0	0	~ 113	342	-	-	-			
Stage 1		-	0	0	174		-	-	-			
Stage 2	310	174	0	0	-	-	-	-	-			
Platoon blocked, %			-					-	-			
Mov Cap-1 Maneuver	-	113	-	-	~ 113	342	-	-	-			
Mov Cap-2 Maneuver	-	113	-	-	~ 113	-	-	-	-			
Stage 1	-	-	-	-	174	-	-	-	-			
Stage 2	27	174	-	-	-	-	-	-	-			
Approach	EB			WB			NB					
HCM Control Delay, s				\$ 398.5								
HCM LOS				φ 0000.0 F								
				,								
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1	WBI n1						
Capacity (veh/h)		-	-	-		210						
HCM Lane V/C Ratio		-	-	-	-	1.756						
HCM Control Delay (s)		-	-	-		\$ 398.5						
HCM Lane LOS		_	-	-	-	φ 000.0 F						
HCM 95th %tile Q(veh)		-	-	-	-	25.3						
. ,						20.0						
Notes												
~: Volume exceeds capacity	/ \$:De	lav excee	ds 300s	+: Co	mputatio	n Not Defi	ned *:	All maior	volume i	n platoon		

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Intersection								
Intersection Delay, s/veh Intersection LOS	9.7							
Intersection LOS	А							
Movement	FRI	EBD	NRI	NRT	SBT	SBD		

Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y			é.	1.		
Traffic Vol, veh/h	1	31	221	11	15	20	
Future Vol, veh/h	1	31	221	11	15	20	
Peak Hour Factor	0.64	0.64	0.64	0.64	0.64	0.64	
Heavy Vehicles, %	0	0	0	9	0	0	
Mvmt Flow	2	48	345	17	23	31	
Number of Lanes	1	0	0	1	1	0	
Approach	EB		NB		SB		
Opposing Approach			SB		NB		
Opposing Lanes	0		1		1		
Conflicting Approach Left	SB		EB				
Conflicting Lanes Left	1		1		0		
Conflicting Approach Right	NB				EB		
Conflicting Lanes Right	1		0		1		
HCM Control Delay	7.5		10.4		7.3		
HCM LOS	А		В		А		

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	95%	3%	0%
Vol Thru, %	5%	0%	43%
Vol Right, %	0%	97%	57%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	232	32	35
LT Vol	221	1	0
Through Vol	11	0	15
RT Vol	0	31	20
Lane Flow Rate	362	50	55
Geometry Grp	1	1	1
Degree of Util (X)	0.425	0.059	0.061
Departure Headway (Hd)	4.219	4.25	4.03
Convergence, Y/N	Yes	Yes	Yes
Сар	852	848	893
Service Time	2.26	2.252	2.036
HCM Lane V/C Ratio	0.425	0.059	0.062
HCM Control Delay	10.4	7.5	7.3
HCM Lane LOS	В	А	А
HCM 95th-tile Q	2.1	0.2	0.2

HCM 6th Signalized Intersection Summary 8: Hansel Ave & Fairlane Ave

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	le Ave										A.W. 1	
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBI
Lane Configurations		4			î,		5	4 12				
Traffic Volume (veh/h)	22	4 31	0	0	101	134	12	1066	6	0	0	
Future Volume (veh/h)	22	31	0	0	101	134	12	1066	6	0	0	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1900	1900	0	0	1900	1900	1900	1767	1900			
Adj Flow Rate, veh/h	28	40	0	0	129	172	15	1367	8			
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78			
Percent Heavy Veh, %	0	0	0	0	0	0	0	9	0			
Cap, veh/h	61	73	0	0	151	202	1270	2402	14			
Arrive On Green	0.20	0.20	0.00	0.00	0.20	0.20	0.70	0.70	0.70			
Sat Flow, veh/h	107	356	0	0	738	984	1810	3421	20			
Grp Volume(v), veh/h	68	0	0	0	0	301	15	670	705			
Grp Sat Flow(s),veh/h/ln	463	0	0	0	0	1723	1810	1678	1763			
Q Serve(g_s), s	2.2	0.0	0.0	0.0	0.0	21.9	0.3	25.8	25.8			
Cycle Q Clear(q c), s	24.1	0.0	0.0	0.0	0.0	21.9	0.3	25.8	25.8			
Prop In Lane	0.41		0.00	0.00		0.57	1.00		0.01			
Lane Grp Cap(c), veh/h	134	0	0	0	0	353	1270	1178	1238			
V/C Ratio(X)	0.51	0.00	0.00	0.00	0.00	0.85	0.01	0.57	0.57			
Avail Cap(c_a), veh/h	358	0	0	0	0	614	1270	1178	1238			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(I)	1.00	0.00	0.00	0.00	0.00	1.00	1.00	1.00	1.00			
Uniform Delay (d), s/veh	48.2	0.0	0.0	0.0	0.0	49.8	5.8	9.6	9.6			
Incr Delay (d2), s/veh	3.3	0.0	0.0	0.0	0.0	8.1	0.0	2.0	1.9			
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(95%),veh/ln	4.2	0.0	0.0	0.0	0.0	15.6	0.2	13.8	14.4			
Unsig. Movement Delay, s/veh							-					
LnGrp Delay(d),s/veh	51.5	0.0	0.0	0.0	0.0	57.9	5.8	11.6	11.5			
LnGrp LOS	D	A	A	A	A	E	A	В	В			
Approach Vol, veh/h		68			301			1390				
Approach Delay, s/veh		51.5			57.9			11.5				
Approach LOS		D			E			В				
Timer - Assigned Phs		2		4				8				
Phs Duration (G+Y+Rc), s		97.7		32.3				32.3				
Change Period (Y+Rc), s		6.4		* 5.7				* 5.7				
Max Green Setting (Gmax), s		71.6		* 47				* 46				
Max Q Clear Time (g_c+l1), s		27.8		26.1				23.9				
Green Ext Time (p_c), s		12.4		0.3				2.7				
Intersection Summary												
HCM 6th Ctrl Delay			21.0									
HCM 6th LOS			C									
Notes												

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

ersection	
Delay s/veh	80

Intersection												
Int Delay, s/veh	8.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		t.			et.						412	
Traffic Vol, veh/h	0	8	11	90	18	0	0	0	0	43	1454	8
Future Vol, veh/h	0	8	11	90	18	0	0	0	0	43	1454	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Free	Free	Free								
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	82	82	82	82	82	82	82	82	82	82	82	82
Heavy Vehicles, %	0	0	0	1	0	0	0	0	0	0	4	14
Mvmt Flow	0	10	13	110	22	0	0	0	0	52	1773	10

Conflicting Flow All - 1882 892 996 1887 - 0 0 Stage 1 - 1882 - 0 0 - </th <th>Major/Minor</th> <th>Minor2</th> <th></th> <th></th> <th>Minor1</th> <th></th> <th></th> <th>Major2</th> <th></th> <th></th>	Major/Minor	Minor2			Minor1			Major2		
Stage 2 - 0 - 996 1887 - - - Critical Hdwy - 6.5 6.9 7.52 6.5 - 4.1 - Critical Hdwy Stg 1 - 5.5 - - - - - - Critical Hdwy Stg 2 - - - 6.52 5.5 - - - Critical Hdwy Stg 2 - - - 6.52 5.5 - - - Critical Hdwy Stg 2 - - - 6.52 5.5 - - - - Critical Hdwy Stg 2 - - 6.52 5.5 - <td></td> <td>-</td> <td>1882</td> <td>892</td> <td>996</td> <td>1887</td> <td>-</td> <td>0</td> <td>0</td> <td></td>		-	1882	892	996	1887	-	0	0	
Critical Hdwy - 6.5 6.9 7.52 6.5 - 4.1 - Critical Hdwy Stg 1 - 5.5 - <t< td=""><td>Stage 1</td><td>-</td><td>1882</td><td>-</td><td>0</td><td>0</td><td>-</td><td>-</td><td>-</td><td></td></t<>	Stage 1	-	1882	-	0	0	-	-	-	
Critical Hdwy Stg 1 - 5.5 -	Stage 2	-	0	-	996	1887	-	-	-	-
Critical Hdwy Stg 2 - - 6.52 5.5 - Stage 1 0 121 - - - 0 -	Critical Hdwy	-	6.5	6.9	7.52	6.5	-	4.1	-	-
Follow-up Hdwy - 4 3.3 3.51 4 - 2.2 - - Pot Cap-1 Maneuver 0 72 289 200 71 0 - <t< td=""><td></td><td>-</td><td>5.5</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td></t<>		-	5.5	-	-	-	-	-	-	-
Pot Cap-1 Maneuver 0 72 289 200 71 0 - </td <td>Critical Hdwy Stg 2</td> <td>-</td> <td>-</td> <td>-</td> <td>6.52</td> <td>5.5</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td>	Critical Hdwy Stg 2	-	-	-	6.52	5.5	-	-	-	-
Stage 1 0 121 - - 0 -		-	4	3.3	3.51		-	2.2	-	-
Stage 2 0 - - 264 120 0 - <td< td=""><td>Pot Cap-1 Maneuver</td><td>0</td><td>72</td><td>289</td><td>200</td><td>71</td><td>0</td><td>-</td><td>-</td><td>-</td></td<>	Pot Cap-1 Maneuver	0	72	289	200	71	0	-	-	-
Platoon blocked, % -		-	121	-			-		-	-
Mov Cap-1 Maneuver - 72 289 171 71 - <td>Stage 2</td> <td>0</td> <td>-</td> <td>-</td> <td>264</td> <td>120</td> <td>0</td> <td>-</td> <td>-</td> <td>-</td>	Stage 2	0	-	-	264	120	0	-	-	-
Mov Cap-2 Maneuver - 72 - 171 71 -									-	-
Stage 1 - 121 -		-	72	289	171	71	-	-	-	-
Stage 2 - - 231 120 - <th< td=""><td>Mov Cap-2 Maneuver</td><td>-</td><td>72</td><td>-</td><td>171</td><td>71</td><td>-</td><td>-</td><td>-</td><td>-</td></th<>	Mov Cap-2 Maneuver	-	72	-	171	71	-	-	-	-
Approach EB WB SB HCM Control Delay, s 39.6 127.2		-	121	-	-		-	-	-	-
HCM Control Delay, s 39.6 127.2	Stage 2	-	-	-	231	120	-	-	-	-
HCM Control Delay, s 39.6 127.2										
	Approach	EB			WB			SB		
	HCM Control Delay, s	39.6			127.2					
HCM LOS E F	HCM LOS	E			F					

Minor Lane/Major Mvmt	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	127	138	-	-	-
HCM Lane V/C Ratio	0.182	0.954	-	-	-
HCM Control Delay (s)	39.6	127.2	-	-	-
HCM Lane LOS	E	F	-	-	-
HCM 95th %tile Q(veh)	0.6	6.6	-	-	-

Intersection	
Intersection Delay, s/veh	9.6
Intersection LOS	A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	87	23	2	4	59	160	7	102	7	20	1	9
Future Vol, veh/h	87	23	2	4	59	160	7	102	7	20	1	9
Peak Hour Factor	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69
Heavy Vehicles, %	0	0	0	0	0	0	0	1	14	0	0	0
Mvmt Flow	126	33	3	6	86	232	10	148	10	29	1	13
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	9.4			9.8			9.6			8.6		
HCMLOS	А			А			А			А		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	6%	78%	2%	67%
Vol Thru, %	88%	21%	26%	3%
Vol Right, %	6%	2%	72%	30%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	116	112	223	30
LT Vol	7	87	4	20
Through Vol	102	23	59	1
RT Vol	7	2	160	9
Lane Flow Rate	168	162	323	43
Geometry Grp	1	1	1	1
Degree of Util (X)	0.234	0.222	0.378	0.062
Departure Headway (Hd)	5.002	4.928	4.212	5.167
Convergence, Y/N	Yes	Yes	Yes	Yes
Сар	713	724	850	687
Service Time	3.066	2.985	2.257	3.246
HCM Lane V/C Ratio	0.236	0.224	0.38	0.063
HCM Control Delay	9.6	9.4	9.8	8.6
HCM Lane LOS	А	А	А	А
HCM 95th-tile Q	0.9	0.8	1.8	0.2

Intersection	
Int Delay, s/veh	

Int Delay, s/veh	1.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		á.			T.		1	1				
Traffic Vol, veh/h	5	24	0	0	14	43	15	1205	10	0	0	0
Future Vol, veh/h	5	24	0	0	14	43	15	1205	10	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	120	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	0	0	5	0	8	0	0	0	0
Mvmt Flow	5	25	0	0	15	45	16	1268	11	0	0	0

Major/Minor	Minor2		1	Minor1			Major1			
Conflicting Flow All	674	1311	-	-	1306	640	0	0	0	
Stage 1	0	0	-	-	1306	-	-	-	-	
Stage 2	674	1311	-	-	0	-	-	-	-	
Critical Hdwy	7.5	6.5	-	-	6.5	7	4.1	-	-	
Critical Hdwy Stg 1	-	-	-	-	5.5	-	-	-	-	
Critical Hdwy Stg 2	6.5	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	-	-	4	3.35	2.2	-	-	
Pot Cap-1 Maneuver	344	160	0	0	161	411	-	-	-	
Stage 1	-	-	0	0	232	-	-	-	-	
Stage 2	415	231	0	0	-	-	-	-	-	
Platoon blocked, %								-	-	
Mov Cap-1 Maneuver	285	160	-	-	161	411	-	-	-	
Mov Cap-2 Maneuver	285	160	-	-	161	-	-	-	-	
Stage 1	-	-	-	-	232	-	-	-	-	
Stage 2	346	231	-	-	-	-	-	-	-	
Approach	EB			WB			NB			
HCM Control Delay, s	30.2			20.1						
HCM LOS	D			С						
Minor Lane/Major Mymt		NBI	NBT	NBR	FBI n1	WBI n1				

winor Lane/wajor wwmt	INDL	IND I	INDR	EBLUI	VVBLNI
Capacity (veh/h)	-	-	-	173	298
HCM Lane V/C Ratio	-	-	-	0.176	0.201
HCM Control Delay (s)	-	-	-	30.2	20.1
HCM Lane LOS	-	-	-	D	С
HCM 95th %tile Q(veh)	-	-	-	0.6	0.7

Lanes, Volumes, Timings 12: Orange Ave & Lancaster Rd

	٠	7	1	Ť	ŧ	~		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø8	
Lane Configurations	ካካ	1	5	**	**	1		
Traffic Volume (vph)	177	197	265	1294	1169	225		
Future Volume (vph)	177	197	265	1294	1169	225		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Storage Length (ft)	225	0	300			350		
Storage Lanes	1	1	1			1		
Taper Length (ft)	25		25					
Satd. Flow (prot)	3433	1482	1752	3406	3312	1568		
Flt Permitted	0.950		0.132					
Satd. Flow (perm)	3433	1482	243	3406	3312	1568		
Right Turn on Red		Yes				Yes		
Satd. Flow (RTOR)		44				218		
_ink Speed (mph)	40			45	45			
Link Distance (ft)	1228			1363	902			
Travel Time (s)	20.9			20.7	13.7			
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91		
Heavy Vehicles (%)	2%	9%	3%	6%	9%	3%		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	195	216	291	1422	1285	247		
Turn Type	Prot	pm+ov	pm+pt	NA	NA	pm+ov		
Protected Phases	3	. 1	1	6	2	3	8	
Permitted Phases		3	6			2		
Fotal Split (s)	20.0	24.0	24.0	90.0	66.0	20.0	20.0	
Total Lost Time (s)	6.8	7.1	7.1	6.8	6.8	6.8		
Act Effct Green (s)	11.2	31.7	84.9	85.2	64.4	82.5		
Actuated g/C Ratio	0.10	0.29	0.77	0.77	0.59	0.75		
v/c Ratio	0.56	0.47	0.78	0.54	0.66	0.20		
Control Delay	53.1	27.9	27.8	5.9	18.5	1.3		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	53.1	27.9	27.8	5.9	18.5	1.3		
LOS	D	С	С	А	В	А		
Approach Delay	39.9			9.6	15.7			
Approach LOS	D			А	В			
Queue Length 50th (ft)	68	97	75	170	314	5		
Queue Length 95th (ft)	104	161	176	230	427	26		
nternal Link Dist (ft)	1148			1283	822			
Furn Bay Length (ft)	225		300			350		
Base Capacity (vph)	411	501	420	2637	1940	1254		
Starvation Cap Reductn	0	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0	0		
Reduced v/c Ratio	0.47	0.43	0.69	0.54	0.66	0.20		
ntersection Summary								
	her							
Cycle Length: 110								
Actuated Cycle Length: 110								
Offset: 35 (32%), Referenced to phas	e 2:SBT a	nd 6:NBTL.	Start of Gree	en				
Control Type: Actuated-Coordinated		= . =,						
Maximum v/c Ratio: 0.78								
ntersection Signal Delay: 15.6				Int	ersection L	OS: B		
ntersection Capacity Utilization 69.3%	%				J Level of S			
Analysis Period (min) 15								
Splits and Phases: 12: Orange Ave	& Lancas	ter Rd					· · · ·	
940 S							الال	

Splits and Pridses. 12. Orange Ave & Lancaster Rd 1 Ø2 (R) 24 s 66 s 20 s 1 Ø2 (R) 20 s 1 Ø2 (R) 20 s 20 s 20 s

Synchro 11 Report Page 1

HCM 6th Signalized Intersection Summary 13: Orange Ave & Nela Ave

a.

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations		4		3	ţ,		5	† 1 ₂		5	4 12	
Traffic Volume (veh/h)	0	0	1	29	1	131	4	1421	17	69	1282	
Future Volume (veh/h)	0	0	1	29	1	131	4	1421	17	69	1282	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00	-	1.00	1.00		1.00	1.00	-	1.00	1.00	-	1.0
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.0
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1841	1900	1885	1900	1811	1811	1900	1811	190
Adj Flow Rate, veh/h	0	0	1	32	1	142	4	1545	18	75	1393	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.9
Percent Heavy Veh, %	0.02	0.02	0.02	4	0.02	1	0.02	6	6	0.02	6	0.0
Cap, veh/h	0	0	166	198	1	165	317	2469	29	280	2810	
Arrive On Green	0.00	0.00	0.10	0.10	0.10	0.10	0.71	0.71	0.71	0.04	0.80	0.8
Sat Flow, veh/h	0.00	0.00	1610	1394	11	1601	393	3484	41	1810	3526	0.0
Grp Volume(v), veh/h	0	0	1010	32	0	143	4	762	801	75	680	71
	-		-		0					1810		
Grp Sat Flow(s),veh/h/ln	0	0	1610	1394	-	1612	393	1721	1804		1721	181
Q Serve(g_s), s	0.0	0.0	0.1	2.7	0.0	11.4	0.4	30.1	30.2	1.3	17.2	17.
Cycle Q Clear(g_c), s	0.0	0.0	0.1	2.8	0.0	11.4	6.2	30.1	30.2	1.3	17.2	17.
Prop In Lane	0.00	•	1.00	1.00	•	0.99	1.00	1000	0.02	1.00	1071	0.0
Lane Grp Cap(c), veh/h	0	0	166	198	0	166	317	1220	1279	280	1371	144
V/C Ratio(X)	0.00	0.00	0.01	0.16	0.00	0.86	0.01	0.63	0.63	0.27	0.50	0.5
Avail Cap(c_a), veh/h	0	0	171	205	0	174	317	1220	1279	371	1371	144
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.0
Upstream Filter(I)	0.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	0.73	0.73	0.7
Uniform Delay (d), s/veh	0.0	0.0	52.3	53.6	0.0	57.4	7.4	9.9	9.9	9.0	4.4	4.
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.4	0.0	32.3	0.1	2.4	2.3	0.4	0.9	0.
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.
%ile BackOfQ(95%),veh/ln	0.0	0.0	0.1	1.8	0.0	10.1	0.1	15.6	16.3	1.0	7.6	7.
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	0.0	0.0	52.4	54.0	0.0	89.7	7.5	12.3	12.2	9.4	5.4	5.
LnGrp LOS	А	А	D	D	А	F	А	В	В	А	А	
Approach Vol, veh/h		1			175			1567			1470	
Approach Delay, s/veh		52.4			83.1			12.3			5.5	
Approach LOS		D			F			В			А	
Timer - Assigned Phs	1	2		4		6		8				
Phs Duration (G+Y+Rc), s	11.5	99.0		19.6		110.4		19.6				
Change Period (Y+Rc), s	6.8	6.8		* 6.2		6.8		* 6.2				
Max Green Setting (Gmax), s	11.2	85.2		* 14		103.2		* 14				
Max Q Clear Time (g_c+I1), s	3.3	32.2		2.1		19.2		13.4				
Green Ext Time (p_c), s	0.1	15.5		0.0		12.7		0.1				
Intersection Summary												
HCM 6th Ctrl Delay			13.1									
HCM 6th LOS			В									
			-									

User approved pedestrian interval to be less than phase max green. * HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

itersection	
t Delay, s/veh	

Intersection												
Int Delay, s/veh	1.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			4			\$			\$	
Traffic Vol, veh/h	4	880	28	17	525	8	9	2	40	4	0	3
Future Vol, veh/h	4	880	28	17	525	8	9	2	40	4	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	1	4	6	3	0	13	0	0	0	0	0
Mvmt Flow	4	957	30	18	571	9	10	2	43	4	0	3

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	580	0	0	987	0	0	1593	1596	972	1615	1607	576
Stage 1	-	-	-	-	-	-	980	980	-	612	612	-
Stage 2	-	-	-	-	-	-	613	616	-	1003	995	-
Critical Hdwy	4.1	-	-	4.16	-	-	7.23	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.254	-	-	3.617	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1004	-	-	684	-	-	81	108	309	84	106	521
Stage 1	-	-	-	-	-	-	287	331	-	484	487	-
Stage 2	-	-	-	-	-	-	461	485	-	294	325	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1004	-	-	684	-	-	78	103	309	68	101	521
Mov Cap-2 Maneuver	-	-	-	-	-	-	78	103	-	68	101	-
Stage 1	-	-	-	-	-	-	284	328	-	480	468	-
Stage 2	-	-	-	-	-	-	440	466	-	249	322	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.3			31			40.8		
HCM LOS							D			E		
Minor Lane/Major Mymt		NBI n1	FBI	FRT	FBR	WRI	WRT	WBR	SBI n1			

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBI	WBR	SBLn1	
Capacity (veh/h)	193	1004	-	-	684	-	-	108	
HCM Lane V/C Ratio	0.287	0.004	-	-	0.027	-	-	0.07	
HCM Control Delay (s)	31	8.6	0	-	10.4	0	-	40.8	
HCM Lane LOS	D	А	Α	-	В	А	-	E	
HCM 95th %tile Q(veh)	1.1	0	-	-	0.1	-	-	0.2	

HCM 6th Signalized Intersection Summary 2: Hansel Ave & Hoffner Ave

			12	35	12105		1912		2029	1	312	,
	-	-	7	1		~	1	T	1	*	ŧ	*
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations		ą			*	1		41	1			
Traffic Volume (veh/h)	8	283	0	0	301	232	38	1333	613	0	0	
Future Volume (veh/h)	8	283	0	0	301	232	38	1333	613	0	0	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1900	1870	0	0	1856	1856	1856	1841	1885			
Adj Flow Rate, veh/h	8	292	0	0	310	239	39	1374	632			
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97			
Percent Heavy Veh, %	0	2	0	0	3	3	3	4	1			
Cap, veh/h	27	332	0	0	396	335	67	2457	1125			
Arrive On Green	0.21	0.21	0.00	0.00	0.21	0.21	0.70	0.70	0.70			
Sat Flow, veh/h	13	1557	0	0	1856	1572	94	3490	1598			
Grp Volume(v), veh/h	300	0	0	0	310	239	758	655	632			
Grp Sat Flow(s), veh/h/ln	1570	0	0	0	1856	1572	1836	1749	1598			
Q Serve(g_s), s	4.9	0.0	0.0	0.0	23.7	21.2	31.2	26.6	29.0			
Cycle Q Clear(g_c), s	28.5	0.0	0.0	0.0	23.7	21.2	31.2	26.6	29.0			
Prop In Lane	0.03	0.0	0.00	0.00	23.1	1.00	0.05	20.0	1.00			
Lane Grp Cap(c), veh/h	359	0	0.00	0.00	396	335	1293	1231	1125			
	0.83	0.00	0.00	0.00	0.78	0.71	0.59	0.53	0.56			
V/C Ratio(X)	506	0.00	0.00	0.00	546	462	1293	1231	1125			
Avail Cap(c_a), veh/h									-			
HCM Platoon Ratio	1.00	1.00 0.00	1.00 0.00	1.00 0.00	1.00 1.00	1.00	1.00	1.00 1.00	1.00 1.00			
Upstream Filter(I)	0.98					1.00	1.00					
Uniform Delay (d), s/veh	56.2	0.0	0.0	0.0	55.7	54.7	11.2	10.5	10.9			
Incr Delay (d2), s/veh	8.1	0.0	0.0	0.0	5.1	3.2	2.0	1.6	2.0			
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(95%),veh/ln	17.6	0.0	0.0	0.0	17.3	13.6	18.1	15.2	15.2			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	64.3	0.0	0.0	0.0	60.8	57.9	13.1	12.2	12.9			
LnGrp LOS	<u> </u>	A	A	A	E	E	В	В	В			
Approach Vol, veh/h		300			549			2045				
Approach Delay, s/veh		64.3			59.5			12.7				
Approach LOS		E			E			В				
Timer - Assigned Phs		2		4				8				
Phs Duration (G+Y+Rc), s		112.1		37.9				37.9				
Change Period (Y+Rc), s		6.5		5.9				5.9				
Max Green Setting (Gmax), s		93.5		44.1				44.1				
Max Q Clear Time (g_c+I1), s		33.2		25.7				30.5				
Green Ext Time (p_c), s		19.9		2.6				1.4				
Intersection Summary												
HCM 6th Ctrl Delay			27.0									
HCM 6th LOS			С									

	4	*	1	1	4	ŧ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	3				3	**
Traffic Volume (veh/h)	345	0	0	0	278	1381
Future Volume (veh/h)	345	0	0	0	278	1381
Initial Q (Qb), veh	0	0			0	0
Ped-Bike Adj(A_pbT)	1.00	1.00			1.00	
Parking Bus, Adj	1.00	1.00			1.00	1.00
Work Zone On Approach	No					No
Adj Sat Flow, veh/h/ln	1856	0			1900	1796
Adj Flow Rate, veh/h	356	0			287	1424
Peak Hour Factor	0.97	0.97			0.97	0.97
Percent Heavy Veh, %	3	0			0	7
Cap, veh/h	0	0			1784	3274
Arrive On Green	0.00	0.00			0.96	0.96
Sat Flow, veh/h	0				1810	3503
Grp Volume(v), veh/h	0.0				287	1424
Grp Sat Flow(s),veh/h/ln					1810	1706
Q Serve(g_s), s					1.1	4.4
Cycle Q Clear(g_c), s					1.1	4.4
Prop In Lane					1.00	
Lane Grp Cap(c), veh/h					1784	3274
V/C Ratio(X)					0.16	0.43
Avail Cap(c_a), veh/h					1784	3274
HCM Platoon Ratio					1.00	1.00
Upstream Filter(I)					1.00	1.00
Uniform Delay (d), s/veh					0.1	0.2
Incr Delay (d2), s/veh					0.2	0.4
Initial Q Delay(d3),s/veh					0.0	0.0
%ile BackOfQ(95%),veh/In					0.2	0.3
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh					0.3	0.6
LnGrp LOS					A	Α
Approach Vol, veh/h						1711
Approach Delay, s/veh						0.6
Approach LOS						А
Timer - Assigned Phs		2				
Phs Duration (G+Y+Rc), s		150.0				
Change Period (Y+Rc), s		6.1				
Max Green Setting (Gmax), s		97.9				
Max Q Clear Time (g_c+l1), s		6.4				
Green Ext Time (p_c), s		19.4				
Intersection Summary						
HCM 6th Ctrl Delay			0.6			
HCM 6th LOS			0.0 A			
			A			

1.1						
Intersection						
Int Delay, s/veh	5.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		t.			1
Traffic Vol, veh/h	101	4	1 36	3	0	4 32
Future Vol, veh/h	101	4	36	3	0	32
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	71	71	71	71	71	71
Heavy Vehicles, %	0	0	0	0	0	3
Mymt Flow	142	6	51	4	0	45
		•	•.	•	· ·	
	••• •					
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	98	53	0	0	55	0
Stage 1	53	-	-	-	-	-
Stage 2	45	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	906	1020	-	-	1563	-
Stage 1	975	-	-	-	-	-
Stage 2	983	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	906	1020	-	-	1563	-
Mov Cap-2 Maneuver	906	-	-	-	-	-
Stage 1	975	-	-	-	-	-
Stage 2	983	-	-	-	-	
Clugo 2	000					
Approach	WB		NB		SB	
HCM Control Delay, s	9.7		0		0	
HCM LOS	А					
Minor Lane/Major Mvmt		NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)		-		910	1563	
HCM Lane V/C Ratio		-	-	0.163	- 1303	-
HCM Control Delay (s)		-		9.7	- 0	-
HCM Lane LOS		-		9.7 A	A	-
HCM 05th 9/ tile O(uch)		-	-	A	A	-

A 0.6

-

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0

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HCM 95th %tile Q(veh)

Intersection			
Intersection Delay, s/veh	8		
Intersection LOS	А		

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			\$	
Traffic Vol, veh/h	9	8	4	70	46	6	7	26	1	1	16	118
Future Vol, veh/h	9	8	4	70	46	6	7	26	1	1	16	118
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles, %	0	0	0	1	2	0	0	4	0	0	0	1
Mvmt Flow	11	10	5	88	58	8	9	33	1	1	20	148
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	7.7			8.5			7.8			7.7		
HCM LOS	А			А			А			А		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	21%	43%	57%	1%
Vol Thru, %	76%	38%	38%	12%
Vol Right, %	3%	19%	5%	87%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	34	21	122	135
LT Vol	7	9	70	1
Through Vol	26	8	46	16
RT Vol	1	4	6	118
Lane Flow Rate	42	26	152	169
Geometry Grp	1	1	1	1
Degree of Util (X)	0.053	0.033	0.186	0.18
Departure Headway (Hd)	4.509	4.483	4.388	3.846
Convergence, Y/N	Yes	Yes	Yes	Yes
Сар	798	801	805	938
Service Time	2.517	2.497	2.484	1.849
HCM Lane V/C Ratio	0.053	0.032	0.189	0.18
HCM Control Delay	7.8	7.7	8.5	7.7
HCM Lane LOS	A	А	А	А
HCM 95th-tile Q	0.2	0.1	0.7	0.7

Intersection												
Int Delay, s/veh	37.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		£			t,			414				
Traffic Vol, veh/h	12	4	0	0	83	192	1	1526	4	0	0	0
Future Vol, veh/h	12	4	0	0	83	192	1	1526	4	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	10817	749504	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	8	25	0	0	1	2	0	4	0	0	0	0
Mvmt Flow	12	4	0	0	86	198	1	1573	4	0	0	0
Major/Minor	Minor2			Minor1			Major1					
Conflicting Flow All	832	1579	-	-	1577	789	0	0	0			
Stage 1	0	0	-	-	1577	-	-	-	-			
Stage 2	832	1579	-	-	0	-	-	-	-			
Critical Hdwy	7.66	7	-	-	6.52	6.94	4.1	-	-			
Critical Hdwy Stg 1	-	-	-	-	5.52	-	-	-	-			
Critical Hdwy Stg 2	6.66	6	-	-	-	-	-	-	-			
Follow-up Hdwy	3.58	4.25	-	-	4.01	3.32	2.2	-	-			
Pot Cap-1 Maneuver	252	87	0	0	110	333	-	-	-			
Stage 1	-	-	0	0	170	-	-	-	-			
Stage 2	317	134	0	0	-	-	-	-	-			
Platoon blocked, %								-	-			
Mov Cap-1 Maneuver	37	87	-	-	110	333	-	-	-			
Mov Cap-2 Maneuver	37	87	-	-	110	-	-	-	-			
Stage 1	-	-	-	-	170	-	-	-	-			
Stage 2	64	134	-	-	-	-	-	-	-			
Approach	EB			WB			NB					
HCM Control Delay, s	133.6			238.3								
HCM LOS	F			F								

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1
Capacity (veh/h)	-	-	-	43	207
HCM Lane V/C Ratio	-	-	-	0.384	1.37
HCM Control Delay (s)	-	-	-	133.6	238.3
HCM Lane LOS	-	-	-	F	F
HCM 95th %tile Q(veh)	-	-	-	1.3	16.1

Intersection						
Intersection Delay, s/veh	7.7					
Intersection LOS	А					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
		LDIX	NDL		001	ODIX
Lane Configurations	14			-A	T.	
Traffic Vol, veh/h	12	31	88	38	36	23
Future Vol, veh/h	12	31	88	38	36	23
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
	-	-		-	-	-

Heavy Vehicles, %	0	0	0	3	0	0	
Mvmt Flow	14	37	106	46	43	28	
Number of Lanes	1	0	0	1	1	0	
Approach	EB		NB		SB		
Opposing Approach			SB		NB		
Opposing Lanes	0		1		1		
Conflicting Approach Left	SB		EB				
Conflicting Lanes Left	1		1		0		
Conflicting Approach Right	NB				EB		
Conflicting Lanes Right	1		0		1		
HCM Control Delay	7.3		8.1		7.3		
HCM LOS	А		А		А		

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	70%	28%	0%
Vol Thru, %	30%	0%	61%
Vol Right, %	0%	72%	39%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	126	43	59
LT Vol	88	12	0
Through Vol	38	0	36
RT Vol	0	31	23
Lane Flow Rate	152	52	71
Geometry Grp	1	1	1
Degree of Util (X)	0.176	0.058	0.076
Departure Headway (Hd)	4.184	4.005	3.871
Convergence, Y/N	Yes	Yes	Yes
Сар	856	900	917
Service Time	2.219	2.005	1.931
HCM Lane V/C Ratio	0.178	0.058	0.077
HCM Control Delay	8.1	7.3	7.3
HCM Lane LOS	А	А	А
HCM 95th-tile Q	0.6	0.2	0.2

HCM 6th Signalized Intersection Summary 8: Hansel Ave & Fairlane Ave

8: Hansel Ave & Fairlan	e Ave										P.IVI. P	еак поц
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBI
Lane Configurations		4			1.		5	4 15				
Traffic Volume (veh/h)	10	22	0	0	1 37	64	16	1383	6	0	0	
Future Volume (veh/h)	10	22	0	0	37	64	16	1383	6	0	0	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1752	1900	0	0	1900	1900	1811	1841	1900			
Adj Flow Rate, veh/h	11	23	0	0	39	68	17	1471	6			
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94			
Percent Heavy Veh, %	10	0	0	0	0	0	6	4	0			
Cap, veh/h	42	70	0	0	50	88	1446	2995	12			
Arrive On Green	0.08	0.08	0.00	0.00	0.08	0.08	0.84	0.84	0.84			
Sat Flow, veh/h	129	863	0	0	621	1084	1725	3572	15			
Grp Volume(v), veh/h	34	0	0	0	0	107	17	720	757			
Grp Sat Flow(s),veh/h/ln	992	0	0	0	0	1705	1725	1749	1838			
Q Serve(g_s), s	0.1	0.0	0.0	0.0	0.0	9.2	0.2	17.0	17.0			
Cycle Q Clear(g_c), s	9.3	0.0	0.0	0.0	0.0	9.2	0.2	17.0	17.0			
Prop In Lane	0.32		0.00	0.00	0.0	0.64	1.00		0.01			
Lane Grp Cap(c), veh/h	112	0	0	0	0	138	1446	1466	1541			
V/C Ratio(X)	0.30	0.00	0.00	0.00	0.00	0.77	0.01	0.49	0.49			
Avail Cap(c_a), veh/h	526	0.00	0.00	0.00	0.00	560	1446	1466	1541			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(I)	1.00	0.00	0.00	0.00	0.00	1.00	1.00	1.00	1.00			
Uniform Delay (d), s/veh	64.6	0.0	0.0	0.0	0.0	67.6	2.0	3.3	3.3			
Incr Delay (d2), s/veh	1.7	0.0	0.0	0.0	0.0	12.3	0.0	1.2	1.1			
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(95%),veh/ln	2.3	0.0	0.0	0.0	0.0	8.0	0.0	8.2	8.5			
Unsig. Movement Delay, s/veh	2.0	0.0	0.0	0.0	0.0	0.0	0.1	0.2	0.0			
LnGrp Delay(d),s/veh	66.3	0.0	0.0	0.0	0.0	79.9	2.0	4.5	4.5			
LnGrp LOS	E	A	A	A	A	E	A	A	A			
Approach Vol, veh/h		34			107			1494				
Approach Delay, s/veh		66.3			79.9			4.5				
Approach LOS		00.5 E			73.5 E			4.5 A				
		_			L							
Timer - Assigned Phs Phs Duration (G+Y+Rc), s		2 132.1		4 17.9				<u>8</u> 17.9			_	
Change Period (Y+Rc), s		6.4		* 5.7				* 5.7				
		6.4 88.6		* 50				* 49				
Max Green Setting (Gmax), s Max Q Clear Time (g_c+I1), s		88.6 19.0		11.3				11.2				
		19.0		0.2				11.2				
Green Ext Time (p_c), s		15.1		0.2				1.0				
Intersection Summary			40.7									
HCM 6th Ctrl Delay			10.7									
HCM 6th LOS			В									
Notes												

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection												
Int Delay, s/veh	5.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Long Configurations											**	

Lane Configurations		1.			4						a th		
Traffic Vol, veh/h	0	6	23	52	26	0	0	0	0	27	1535	6	
Future Vol, veh/h	0	6	23	52	26	0	0	0	0	27	1535	6	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Free	Free	Free									
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85	
Heavy Vehicles, %	0	0	5	0	0	0	0	0	0	0	7	0	
Mvmt Flow	0	7	27	61	31	0	0	0	0	32	1806	7	

Major/Minor	Minor2			Minor1			Major2			
Conflicting Flow All	-	1874	907	971	1877	-	0	0	0	
Stage 1	-	1874	-	0	0	-	-	-	-	
Stage 2	-	0	-	971	1877	-	-	-	-	
Critical Hdwy	-	6.5	7	7.5	6.5	-	4.1	-	-	
Critical Hdwy Stg 1	-	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	6.5	5.5	-	-	-	-	
Follow-up Hdwy	-	4	3.35	3.5	4	-	2.2	-	-	
Pot Cap-1 Maneuver	0	73	273	210	72	0	-	-	-	
Stage 1	0	122	-	-	-	0	-	-	-	
Stage 2	0	-	-	275	122	0	-	-	-	
Platoon blocked, %								-	-	
Mov Cap-1 Maneuver	-	73	273	175	72	-	-	-	-	
Mov Cap-2 Maneuver	-	73	-	175	72	-	-	-	-	
Stage 1	-	122	-	-	-	-	-	-	-	
Stage 2	-	-	-	233	122	-	-	-	-	
Approach	EB			WB			SB			
HCM Control Delay, s	30.7			100.3						
HCM LOS	D			F						

Minor Lane/Major Mvmt	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	174	118	-	-	-
HCM Lane V/C Ratio	0.196	0.778	-	-	-
HCM Control Delay (s)	30.7	100.3	-	-	-
HCM Lane LOS	D	F	-	-	-
HCM 95th %tile Q(veh)	0.7	4.5	-	-	-

ntersection	
ntersection Delay, s/veh	7.8
ntersection LOS	А

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	40	47	6	5	44	67	5	49	18	36	8	12
Future Vol, veh/h	40	47	6	5	44	67	5	49	18	36	8	12
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	0	4	0	0	2	0	0	2	0	0	0	0
Mvmt Flow	44	52	7	6	49	74	6	54	20	40	9	13
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	8			7.7			7.8			7.9		
HCM LOS	А			А			А			А		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	7%	43%	4%	64%
Vol Thru, %	68%	51%	38%	14%
Vol Right, %	25%	6%	58%	21%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	72	93	116	56
LT Vol	5	40	5	36
Through Vol	49	47	44	8
RT Vol	18	6	67	12
Lane Flow Rate	80	103	129	62
Geometry Grp	1	1	1	1
Degree of Util (X)	0.096	0.126	0.143	0.078
Departure Headway (Hd)	4.337	4.401	3.999	4.492
Convergence, Y/N	Yes	Yes	Yes	Yes
Сар	828	817	900	800
Service Time	2.353	2.414	2.012	2.508
HCM Lane V/C Ratio	0.097	0.126	0.143	0.077
HCM Control Delay	7.8	8	7.7	7.9
HCM Lane LOS	А	А	А	А
HCM 95th-tile Q	0.3	0.4	0.5	0.3

rsection	
Delay s/veh	29

Intersection												
Int Delay, s/veh	2.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		é.			ţ,		1	1				
Traffic Vol, veh/h	14	42	0	0	16	46	30	1342	16	0	0	0
Future Vol, veh/h	14	42	0	0	16	46	30	1342	16	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	120	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	7	0	0	0	6	2	13	4	6	0	0	0
Mvmt Flow	15	44	0	0	17	48	32	1413	17	0	0	0

Major/Minor	Minor2		1	Vinor1			Major1		
Conflicting Flow All	779	1494	-	-	1486	715	0	0	0
Stage 1	0	0	-	-	1486	-	-	-	-
Stage 2	779	1494	-	-	0	-	-	-	-
Critical Hdwy	7.64	6.5	-	-	6.62	6.94	4.36	-	-
Critical Hdwy Stg 1	-	-	-	-	5.62	-	-	-	-
Critical Hdwy Stg 2	6.64	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.57	4	-	-	4.06	3.32	2.33	-	-
Pot Cap-1 Maneuver	277	124	0	0	119	373	-	-	-
Stage 1	-	-	0	0	180	-	-	-	-
Stage 2	344	188	0	0	-	-	-	-	-
Platoon blocked, %								-	-
Mov Cap-1 Maneuver	215	124	-	-	119	373	-	-	-
Mov Cap-2 Maneuver	215	124	-	-	119	-	-	-	-
Stage 1	-	-	-	-	180	-	-	-	-
Stage 2	271	188	-	-	-	-	-	-	-
Approach	EB			WB			NB		
HCM Control Delay, s	48.7			25.4					
HCM LOS	E			D					
Minor Lane/Major Mymt		NRI	NRT	NRR	FBI n1	WBI n1			

Minor Lane/Major Wivmt	NBL	NRI	NRK	EBTUI	WBLUI	
Capacity (veh/h)	-	-	-	139	241	
HCM Lane V/C Ratio	-	-	-	0.424	0.271	
HCM Control Delay (s)	-	-	-	48.7	25.4	
HCM Lane LOS	-	-	-	E	D	
HCM 95th %tile Q(veh)	-	-	-	1.9	1.1	

Lanes, Volumes, Timings 12: Orange Ave & Lancaster Rd

	٦	7	1	1	ŧ	1	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø8
Lane Configurations	ካካ	7		**	^		
Traffic Volume (vph)	ר ר 245	165	1 307	TT 1192	1325	244	
Future Volume (vph)	245 245	165	307	1192	1325	244	
Ideal Flow (vphpl)	1900	1900	1900	192	1900	1900	
	225		300	1900	1900	350	
Storage Length (ft)	225	0				350	
Storage Lanes		I	1			I	
Taper Length (ft)	25	1501	25	2474	2242	4500	
Satd. Flow (prot)	3502	1524	1719	3471	3343	1568	
Flt Permitted	0.950	4504	0.070	0.174	0040	4500	
Satd. Flow (perm)	3502	1524	127	3471	3343	1568	
Right Turn on Red		Yes				Yes	
Satd. Flow (RTOR)		17				71	
Link Speed (mph)	40			45	45		
Link Distance (ft)	1228			1363	902		
Travel Time (s)	20.9		_	20.7	13.7	_	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	
Heavy Vehicles (%)	0%	6%	5%	4%	8%	3%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	255	172	320	1242	1380	254	
Turn Type	Prot	pm+ov	pm+pt	NA	NA	pm+ov	
Protected Phases	3	1	1	6	2	3	8
Permitted Phases		3	6			2	
Total Split (s)	50.0	25.0	25.0	100.0	75.0	50.0	50.0
Total Lost Time (s)	6.8	7.1	7.1	6.8	6.8	6.8	
Act Effct Green (s)	16.3	62.3	119.8	120.1	73.8	96.9	
Actuated g/C Ratio	0.11	0.42	0.80	0.80	0.49	0.65	
v/c Ratio	0.67	0.27	0.62	0.45	0.84	0.24	
Control Delay	72.9	27.5	29.4	11.1	38.7	8.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	72.9	27.5	29.4	11.1	38.7	8.0	
LOS	E	C	C	В	D	A	
Approach Delay	54.6	-	-	14.9	33.9		
Approach LOS	D			B	C		
Queue Length 50th (ft)	125	102	204	287	574	63	
Queue Length 95th (ft)	169	154	320	504	723	104	
Internal Link Dist (ft)	1148	TVT	520	1283	822	TUT	
Turn Bay Length (ft)	225		300	1200	JLL	350	
Base Capacity (vph)	1008	642	517	2778	1644	1306	
Starvation Cap Reductn	000	042	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.25	0.27	0.62	0.45	0.84	0.19	
	0.20	0.27	0.02	0.45	0.04	0.19	
Intersection Summary							
Area Type:	Other						
Cycle Length: 150							
Actuated Cycle Length: 150							
Offset: 16 (11%), Referenced to p		nd 6:NBTL,	Start of Gree	en			
Control Type: Actuated-Coordina Maximum v/c Ratio: 0.84	ted						
Intersection Signal Delay: 28.1				Inte	ersection L	0S' C	
Intersection Capacity Utilization 7	7 9%				J Level of S		
Analysis Period (min) 15	1.070						
niaiysis reliou (11111) 13							
Splits and Phases: 12: Orange	Ave & Lancast	ter Rd					

Splits and Phases: 12: Orange Ave & Lancaster Rd

\$ Ø1	Ø2 (R)	Ø3	24 1
25 s	75 s	50 s	
Ø6 (R)			
100 s		50 s	

HCM 6th Signalized Intersection Summary 13: Orange Ave & Nela Ave

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations				1			3	4 15		3	† Ъ	
Traffic Volume (veh/h)	0	4	1	30	1	76	4	1431	63	117	1433	
Future Volume (veh/h)	0	0	1	30	0	76	4	1431	63	117	1433	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00	•	1.00	1.00	•	1.00	1.00		1.00	1.00	•	1.0
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.0
Work Zone On Approach	1.00	No	1.00	1.00	No	1.00	1.00	No	1.00	1.00	No	1.0
Adj Sat Flow, veh/h/ln	1900	1900	1900	1841	1900	1841	1530	1841	1870	1870	1781	190
Adj Flow Rate, veh/h	0	0	1000	31	0	78	4	1475	65	121	1477	100
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.9
Percent Heavy Veh, %	0.37	0.37	0.37	4	0.37	4	25	4	2	2	0.37	0.3
Cap, veh/h	0	0	99	133	0	99	274	2639	116	309	2954	
Arrive On Green	0.00	0.00	0.06	0.06	0.00	0.06	0.77	0.77	0.77	0.04	1.00	1.0
	0.00	0.00	1610	1394	0.00	1610	292	3412	150	1781	3468	1.0
Sat Flow, veh/h												
Grp Volume(v), veh/h	0	0	1	31	0	78	4	754	786	121	721	75
Grp Sat Flow(s),veh/h/ln	0	0	1610	1394	0	1610	292	1749	1814	1781	1692	178
Q Serve(g_s), s	0.0	0.0	0.1	3.2	0.0	7.2	0.5	25.8	26.0	2.0	0.0	0.
Cycle Q Clear(g_c), s	0.0	0.0	0.1	3.3	0.0	7.2	0.5	25.8	26.0	2.0	0.0	0.
Prop In Lane	0.00		1.00	1.00		1.00	1.00		0.08	1.00		0.0
Lane Grp Cap(c), veh/h	0	0	99	133	0	99	274	1352	1403	309	1442	151
V/C Ratio(X)	0.00	0.00	0.01	0.23	0.00	0.79	0.01	0.56	0.56	0.39	0.50	0.5
Avail Cap(c_a), veh/h	0	0	148	177	0	150	274	1352	1403	490	1442	151
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.33	1.33	1.3
Upstream Filter(I)	0.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	0.54	0.54	0.5
Uniform Delay (d), s/veh	0.0	0.0	66.1	67.6	0.0	69.4	3.9	6.8	6.8	6.6	0.0	0.
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.9	0.0	14.4	0.1	1.7	1.6	0.4	0.7	0.
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.
%ile BackOfQ(95%),veh/ln	0.0	0.0	0.1	2.1	0.0	6.0	0.1	13.1	13.7	1.5	0.5	0.
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	0.0	0.0	66.1	68.5	0.0	83.8	4.0	8.4	8.4	7.0	0.7	0.
LnGrp LOS	A	A	E	E	A	F	A	A	A	A	A	
Approach Vol, veh/h		1			109			1544			1600	
Approach Delay, s/veh		66.1			79.5			8.4			1.1	
Approach LOS		E			E			A			A	
Timer - Assigned Phs	1	2		4		6		8				
Phs Duration (G+Y+Rc), s	11.8	122.8		15.4		134.6		15.4				
Change Period (Y+Rc), s	6.8	6.8		* 6.2		6.8		* 6.2				
Max Green Setting (Gmax), s	20.2	96.2		* 14		123.2		* 14				
Max Q Clear Time (g_c+l1), s	4.0	28.0		2.1		2.0		9.2				
Green Ext Time (p_c), s	0.2	15.7		0.0		14.5		0.2				
Intersection Summary												
HCM 6th Ctrl Delay			7.2									
HCM 6th LOS			A									
Notes												

User approved pedestrian interval to be less than phase max green. * HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

APPENDIX D

ITE Trip Generation Worksheets

Charter School (K-12) (538)

Vehicle Trip Ends vs: Students

On a: Weekday,

AM Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies: 4

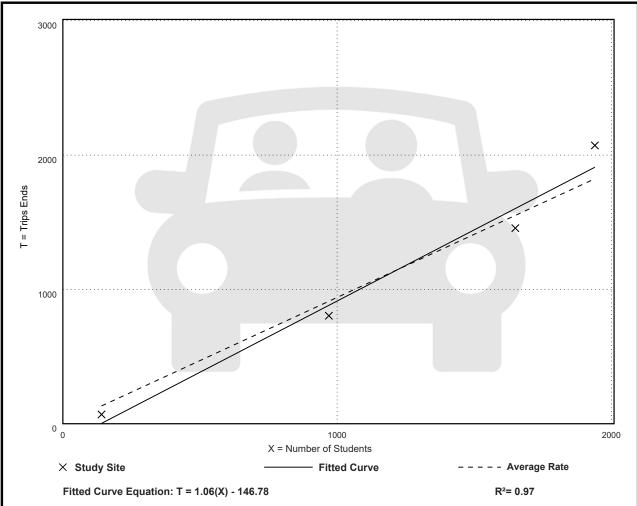
Avg. Num. of Students: 1175

Directional Distribution: 53% entering, 47% exiting

Vehicle Trip Generation per Student

Average Rate	Range of Rates	Standard Deviation
0.94	0.49 - 1.07	0.15

Data Plot and Equation





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Charter School (K-12) (538)

Vehicle Trip Ends vs: Students

On a: Weekday,

PM Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies: 4

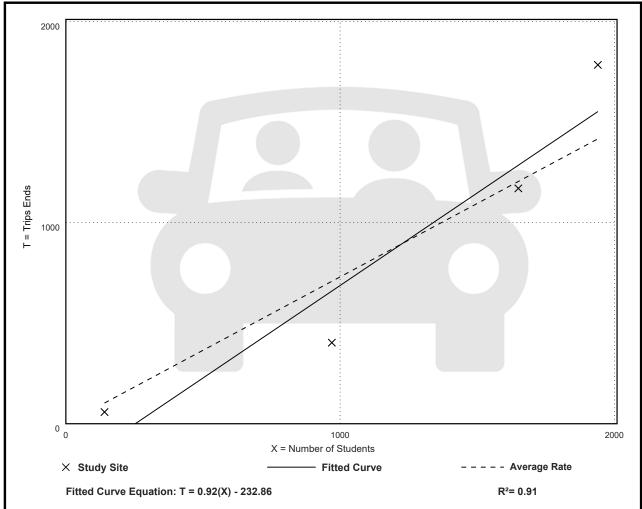
Avg. Num. of Students: 1175

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Student

Average Rate	Range of Rates	Standard Deviation
0.73	0.41 - 0.92	0.23

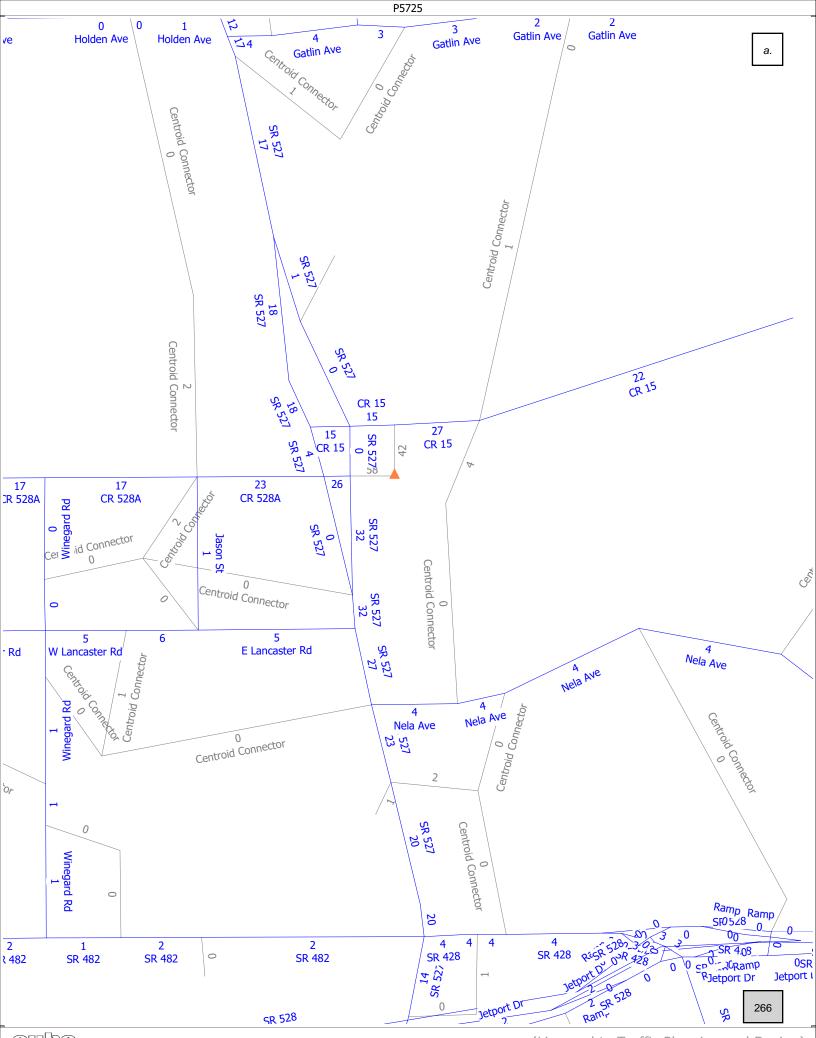
Data Plot and Equation



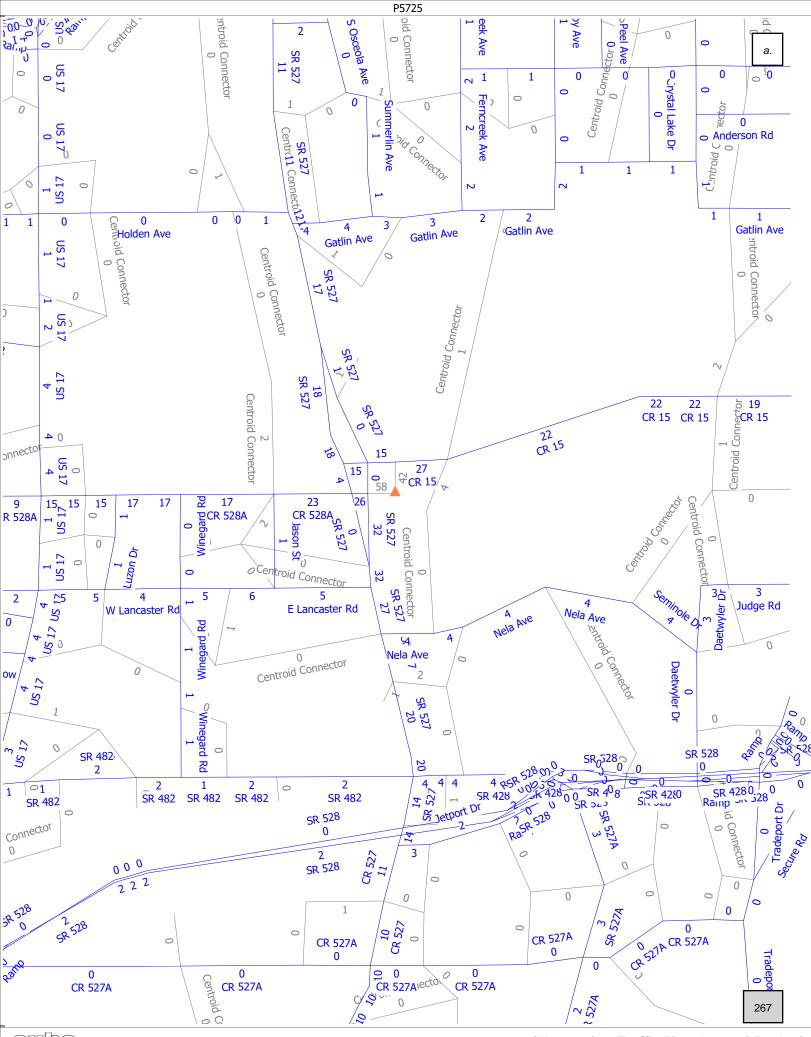


APPENDIX E

Model Distribution Output

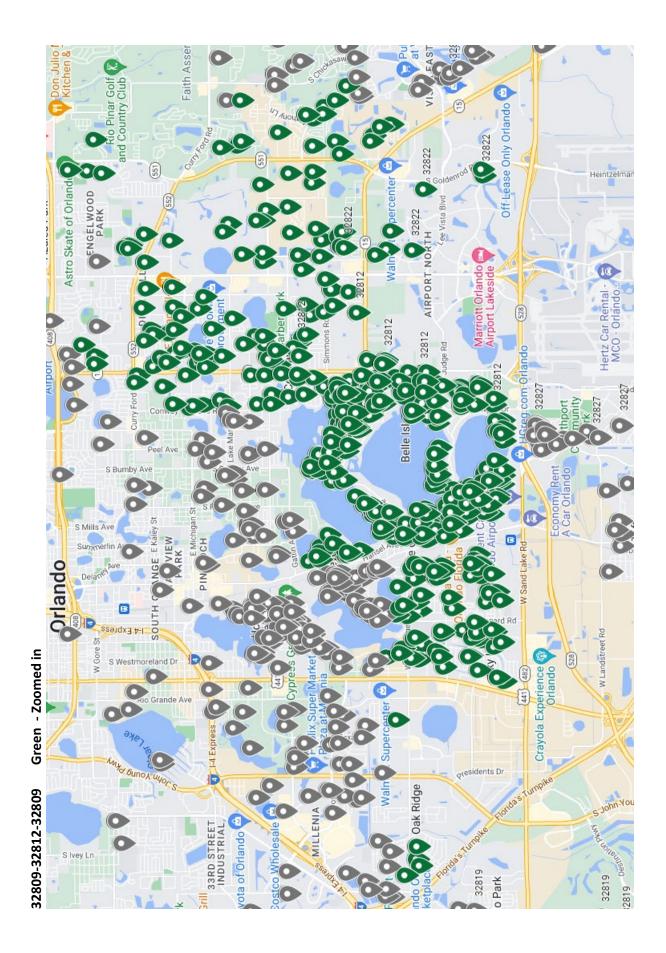


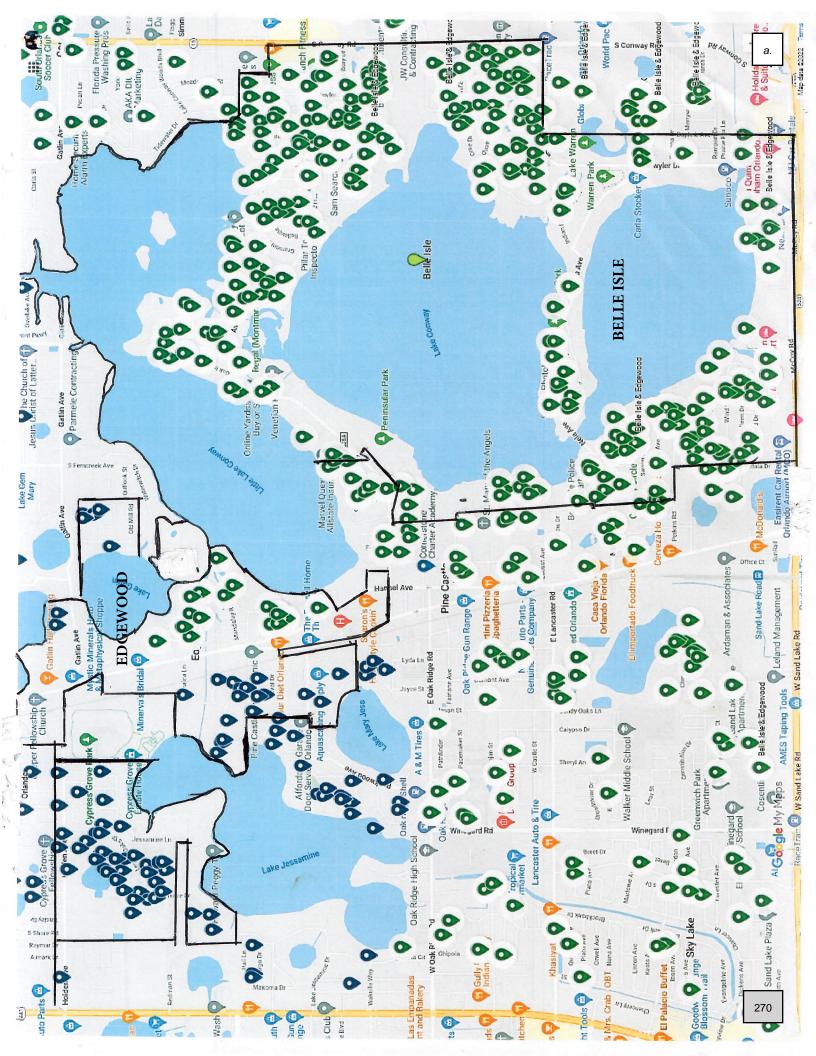
(Licensed to Traffic Planning and Design)



(Licensed to Traffic Planning and Design)

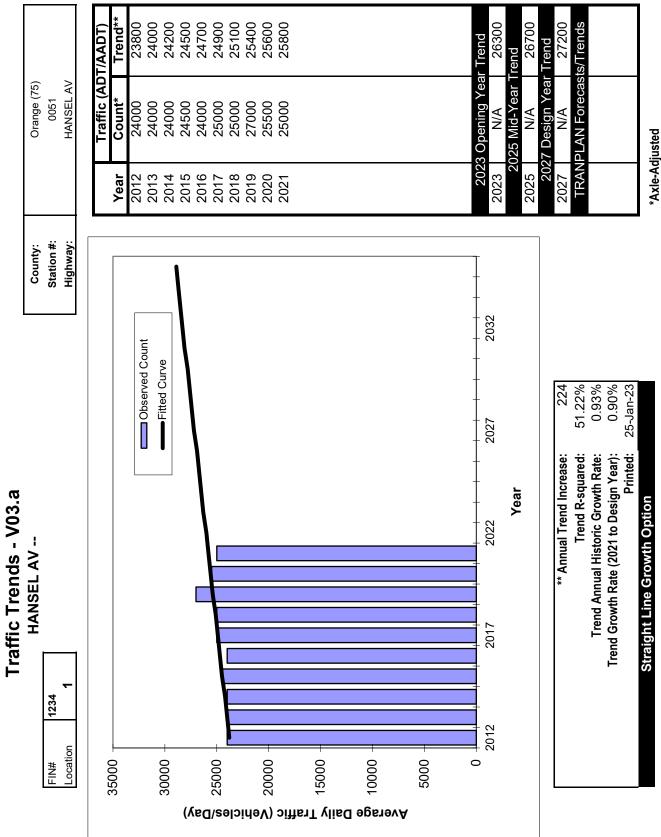
Cornerstone Charter Academy Heatmaps December 2022 a.





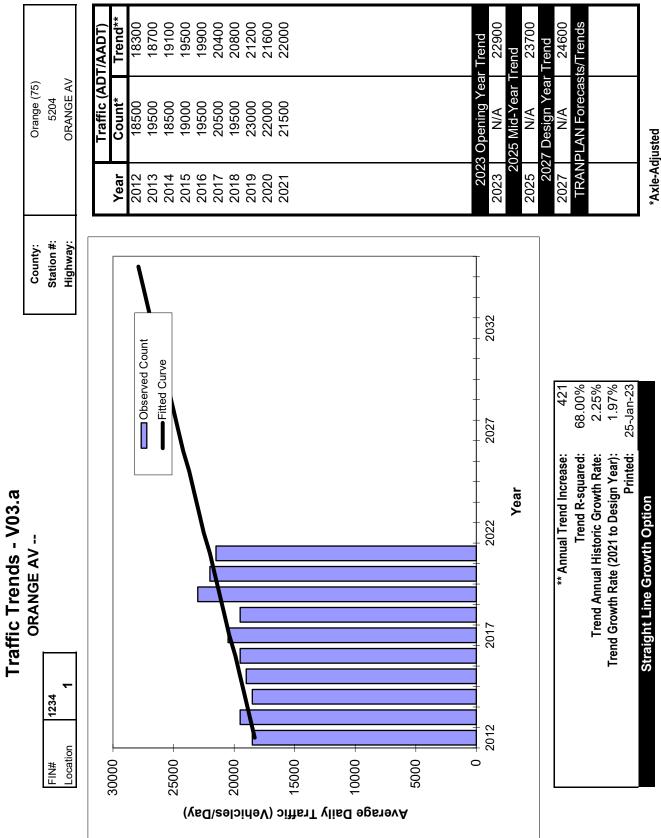
APPENDIX F

Trends Analysis Worksheets

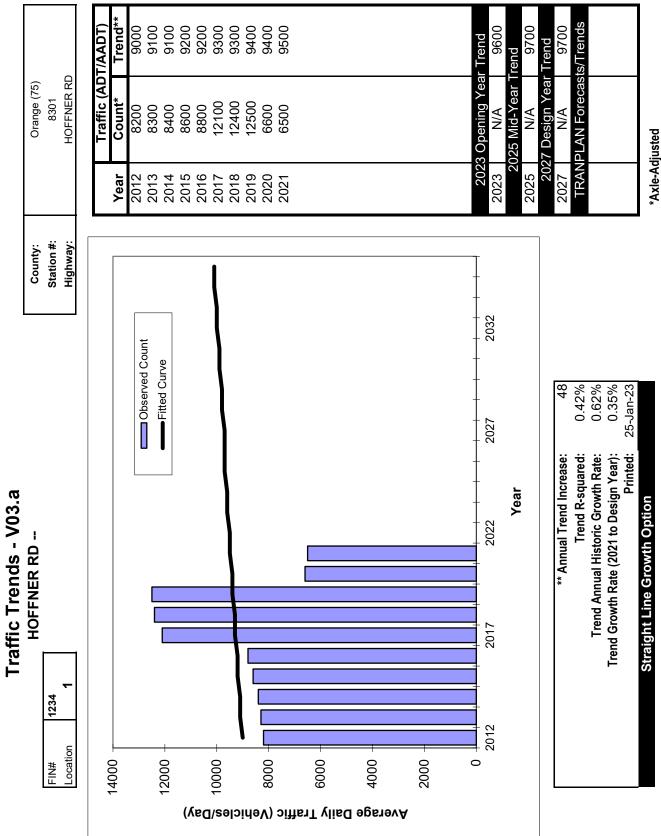


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APPENDIX G

Projected Capacity Analysis Worksheets

Intersection									
Int Delay, s/veh	3.6								
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR
Lane Configurations		\$			\$			4	
Traffic Vol, veh/h	0	862	87	156	621	1	17	0	9

Traffic Vol, veh/h	0	862	87	156	621	1	17	0	9	0	0	0	
Future Vol, veh/h	0	862	87	156	621	1	17	0	9	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93	
Heavy Vehicles, %	0	1	2	0	1	0	6	0	0	0	0	0	
Mvmt Flow	0	927	94	168	668	1	18	0	10	0	0	0	

SBL

SBT

\$

SBR

Major/Minor	Major1			Major2			Minor1			Minor2			
Conflicting Flow All	669	0	0	1021	0	0	1979	1979	974	1984	2026	669	
Stage 1	-	-	-	-	-	-	974	974	-	1005	1005	-	
Stage 2	-	-	-	-	-	-	1005	1005	-	979	1021	-	
Critical Hdwy	4.1	-	-	4.1	-	-	7.16	6.5	6.2	7.1	6.5	6.2	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.16	5.5	-	6.1	5.5	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.16	5.5	-	6.1	5.5	-	
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.554	4	3.3	3.5	4	3.3	
Pot Cap-1 Maneuver	931	-	-	688	-	-	45	62	308	46	58	461	
Stage 1	-	-	-	-	-	-	298	333	-	294	322	-	
Stage 2	-	-	-	-	-	-	286	322	-	304	316	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	931	-	-	688	-	-	31	38	308	31	35	461	
Mov Cap-2 Maneuver	-	-	-	-	-	-	31	38	-	31	35	-	
Stage 1	-	-	-	-	-	-	298	333	-	294	197	-	
Stage 2	-	-	-	-	-	-	175	197	-	294	316	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0			2.4			171.9			0			
HCM LOS							F			А			

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	45	931	-	-	688	-	-	-
HCM Lane V/C Ratio	0.621	-	-	-	0.244	-	-	-
HCM Control Delay (s)	171.9	0	-	-	11.9	0	-	0
HCM Lane LOS	F	А	-	-	В	А	-	А
HCM 95th %tile Q(veh)	2.3	0	-	-	1	-	-	-

HCM 6th Signalized Intersection Summary 2: Hansel Ave & Hoffner Ave

a.

Projected AM A.M. Peak Hour

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		aî.			*	1		412	1			
Traffic Volume (veh/h)	6	291	0	0	384	258	191	1393	673	0	0	0
Future Volume (veh/h)	6	291	0	0	384	258	191	1393	673	0	0	0
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1900	1870	0	0	1870	1885	1811	1826	1870			
Adj Flow Rate, veh/h	6	300	0	0	396	266	197	1436	694			
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97			
Percent Heavy Veh, %	0	2	0	0	2	1	6	5	2			
Cap, veh/h	30	374	0	0	457	390	268	2070	1047			
Arrive On Green	0.24	0.24	0.00	0.00	0.24	0.24	0.66	0.66	0.66			
Sat Flow, veh/h	6	1530	0	0	1870	1598	406	3134	1585			
Grp Volume(v), veh/h	306	0	0	0	396	266	876	757	694			
Grp Sat Flow(s),veh/h/ln	1536	0	0	0	1870	1598	1806	1735	1585			
Q Serve(g_s), s	1.1	0.0	0.0	0.0	26.4	19.6	41.6	34.2	34.4			
Cycle Q Clear(g_c), s	27.5	0.0	0.0	0.0	26.4	19.6	41.6	34.2	34.4			
Prop In Lane	0.02	•	0.00	0.00	4	1.00	0.22		1.00			
Lane Grp Cap(c), veh/h	404	0	0	0	457	390	1192	1145	1047			
V/C Ratio(X)	0.76	0.00	0.00	0.00	0.87	0.68	0.73	0.66	0.66			_
Avail Cap(c_a), veh/h	646	0	0	0	706	603	1192	1145	1047			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(I)	0.96	0.00	0.00 0.0	0.00	1.00	1.00	1.00	1.00	1.00			
Uniform Delay (d), s/veh	44.5	0.0		0.0	47.1	44.5	14.6	13.3	13.3			
Incr Delay (d2), s/veh Initial Q Delay(d3),s/veh	2.8 0.0	0.0 0.0	0.0 0.0	0.0 0.0	7.1 0.0	2.1 0.0	4.0 0.0	3.0 0.0	3.3 0.0			
%ile BackOfQ(95%),veh/ln	14.1	0.0	0.0	0.0	19.1	12.7	23.2	18.9	17.6			
Unsig. Movement Delay, s/veh	14.1	0.0	0.0	0.0	19.1	12.1	23.2	10.9	0.11			
LnGrp Delay(d),s/veh	47.4	0.0	0.0	0.0	54.2	46.6	18.6	16.3	16.7			
LnGrp LOS	47.4 D	0.0 A	0.0 A	0.0 A	54.2 D	40.0 D	10.0 B	10.3 B	10.7 B			
Approach Vol, veh/h	D	306	A	~	662	D	В	2327	В			
Approach Delay, s/veh		47.4			51.2			17.3				
Approach LOS		47.4 D			51.2 D			нВ				
		_			U							
Timer - Assigned Phs		2		4				8				
Phs Duration (G+Y+Rc), s		92.3		37.7				37.7				
Change Period (Y+Rc), s		6.5		5.9				5.9				
Max Green Setting (Gmax), s		68.5		49.1				49.1				
Max Q Clear Time (g_c+l1), s		43.6		28.4 3.4				29.5 1.7				
Green Ext Time (p_c), s		16.6		3.4				1.7				
Intersection Summary												
HCM 6th Ctrl Delay			26.9									
HCM 6th LOS			С									

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	1	*	t	-	1	Ţ
	· •	325	23.05	1	20533	्र
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	5				7	**
Traffic Volume (veh/h)	569	0	0	0	292	1277
Future Volume (veh/h)	569	0	0	0	292	1277
Initial Q (Qb), veh	0	0			0	0
Ped-Bike Adj(A_pbT)	1.00	1.00			1.00	
Parking Bus, Adj	1.00	1.00			1.00	1.00
Work Zone On Approach	No					No
Adj Sat Flow, veh/h/ln	1870	0			1870	1841
Adj Flow Rate, veh/h	605	0			311	1359
Peak Hour Factor	0.94	0.94			0.94	0.94
Percent Heavy Veh, %	2	0			2	4
Cap, veh/h	0	0			1753	3333
Arrive On Green	0.00	0.00			0.95	0.95
Sat Flow, veh/h	0.00	0.00			1781	3589
Grp Volume(v), veh/h	0.0				311	1359
Grp Sat Flow(s), veh/h/ln	0.0				1781	1749
Q Serve(g_s), s					1.3	3.9
Cycle Q Clear(g_c), s					1.3	3.9
Prop In Lane					1.00	5.9
Lane Grp Cap(c), veh/h					1753	3333
V/C Ratio(X)					0.18	0.41
					1753	3333
Avail Cap(c_a), veh/h					1.00	1.00
HCM Platoon Ratio						
Upstream Filter(I)					1.00	1.00
Uniform Delay (d), s/veh					0.2	0.2
Incr Delay (d2), s/veh					0.2	0.4
Initial Q Delay(d3),s/veh					0.0	0.0
%ile BackOfQ(95%),veh/ln					0.2	0.3
Unsig. Movement Delay, s/veh					0.4	0.0
LnGrp Delay(d),s/veh					0.4	0.6
LnGrp LOS					A	А
Approach Vol, veh/h						1670
Approach Delay, s/veh						0.6
Approach LOS						А
Timer - Assigned Phs		2				
Phs Duration (G+Y+Rc), s		130.0				
Change Period (Y+Rc), s		6.1				
Max Green Setting (Gmax), s		70.9				
Max Q Clear Time (g_c+I1), s		5.9				
Green Ext Time (p_c), s		17.2				
. ,						
Intersection Summary			0.0			
HCM 6th Ctrl Delay			0.6			
HCM 6th LOS			A			

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
		VVDR		NDK	ODL	
Lane Configurations	¥	8	1 5	0	0	2 27
Traffic Vol, veh/h				0		
Future Vol, veh/h	0	8	15	0	0	227
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	64	64	64	64	64	64
Heavy Vehicles, %	0	0	7	0	0	1
Mvmt Flow	0	13	23	0	0	355
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	378	23	0	0	23	0
Stage 1	23	-	-	-	-	-
Stage 2	355	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	628	1060	-	-	1605	-
Stage 1	1005	-	-		-	-
Stage 2	714	-	-	-	-	-
Platoon blocked, %	7.14		-	-		-
Mov Cap-1 Maneuver	628	1060			1605	_
Mov Cap-1 Maneuver Mov Cap-2 Maneuver	628	-	-	-	- 1005	-
				-		-
Stage 1	1005	-	-	-	-	-
Stage 2	714	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	8.4		0		0	
HCM LOS	A		·		•	
Minor Lane/Major Mvmt		NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)		-	-	1060	1605	-
HCM Lane V/C Ratio		-	-	0.012	-	-
HCM Control Delay (s)		-	-	8.4	0	-
HCM Lane LOS			-	A	Ă	
			-	•		-

0

0

HCM 95th %tile Q(veh)

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4.			4	
Traffic Vol, veh/h	112	0	0	0	0	8	0	0	0	202	0	0
Future Vol, veh/h	112	0	0	0	0	8	0	0	0	202	0	0
Peak Hour Factor	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66
Heavy Vehicles, %	0	0	0	1	0	0	33	17	0	0	8	0
Mvmt Flow	170	0	0	0	0	12	0	0	0	306	0	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB				WB			NB		SB		
Opposing Approach	WB				EB			SB		NB		
Opposing Lanes	1				1			1		1		
Conflicting Approach Left	SB				NB			EB		WB		
Conflicting Lanes Left	1				1			1		1		
Conflicting Approach Right	NB				SB			WB		EB		
Conflicting Lanes Right	1				1			1		1		
HCM Control Delay	9.3				7.3			0		10.4		
HCM LOS	А				А			-		В		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	0%	100%	0%	100%
Vol Thru, %	100%	0%	0%	0%
Vol Right, %	0%	0%	100%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	0	112	8	202
LT Vol	0	112	0	202
Through Vol	0	0	0	0
RT Vol	0	0	8	0
Lane Flow Rate	0	170	12	306
Geometry Grp	1	1	1	1
Degree of Util (X)	0	0.229	0.014	0.387
Departure Headway (Hd)	4.995	4.854	4.251	4.552
Convergence, Y/N	Yes	Yes	Yes	Yes
Сар	0	740	840	793
Service Time	3.03	2.879	2.286	2.573
HCM Lane V/C Ratio	0	0.23	0.014	0.386
HCM Control Delay	8	9.3	7.3	10.4
HCM Lane LOS	N	А	А	В
HCM 95th-tile Q	0	0.9	0	1.8

Intersection												
Int Delay, s/veh	139.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4 7			1 74			11.				
Traffic Vol, veh/h	9	7	0	0	174	32	62	1842	9	0	0	0
Future Vol, veh/h	9	7	0	0	174	32	62	1842	9	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	-	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	11	0	0	0	1	0	0	7	0	0	0	0
Mvmt Flow	10	8	0	0	187	34	67	1981	10	0	0	0
Major/Minor	Minor2			Minor1			Major1					
Conflicting Flow All	1218	2125		-	2120	996	0	0	0			
Stage 1	0	0		-	2120	- 330	-	-	0			
Stage 2	1218	2125	-	-	0	-	-	-	-			
Critical Hdwy	7.72	6.5	-	-	6.52	6.9	4.1	-	-			
Critical Hdwy Stg 1	-	0.5	-		5.52	0.9	4.1	-	-			
Critical Hdwy Stg 2	6.72	5.5	-	-	5.52	-	-	-	-			
Follow-up Hdwy	3.61	4	-		4.01	3.3	2.2	-	-			
Pot Cap-1 Maneuver	127	51	0	0	~ 50	247	2.2	-	-			
Stage 1	-	-	0	0	~ 91	- 241	-	-	-			
Stage 2	178	91	0	0	- 91	-	-	-	-			
Platoon blocked, %	170	91	0	0	-	-	-	-	-			
Mov Cap-1 Maneuver	_	51	-	_	~ 50	247	_	-	-			
Mov Cap-1 Maneuver	-	51	-	-	~ 50	241	-	_	_			
Stage 1	-	-	-	-	~ 91	-	-	-	-			
Stage 2	-	- 91	-	-	- 91	-	_	-	-			
Slaye 2	-	31	-	-	-	-	-	-	-			
Approach	EB			WB			NB					
				\$ 1447			IND					
HCM Control Delay, s HCM LOS	-			\$ 1447 F								
	-			г								
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1	WBLn1						
Capacity (veh/h)		INDL -		- 100	LDLIII	57						
HCM Lane V/C Ratio		-			-	3.886						
HCM Control Delay (s)		-	-	-	-	\$ 1447						
HCM Lane LOS		-	-		-	ֆ 1447 F						
HCM 95th %tile Q(veh)		-	-	-	-	г 24						
		-	-	-	-	24						
Notes												
~: Volume exceeds capacity	/ \$: De	lay excee	ds 300s	+: Co	nputation	n Not Def	ined *:	All major	volume i	n platoon		

HCM 6th Signalized Intersection Summary 6: Hansel Ave & Waltham Ave

Projected AM A.M. Peak Hour

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4 7			î.			1842				
Traffic Volume (veh/h)	9	7	0	0	174	32	62	1842	9	0	0	0
Future Volume (veh/h)	9	7	0	0	174	32	62	1842	9	0	0	0
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1737	1900	0	0	1885	1900	1900	1796	1900			
Adj Flow Rate, veh/h	10	8	0	0	187	34	67	1981	10			
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93			
Percent Heavy Veh, %	11	0	0	0	1	0	0	7	0			
Cap, veh/h	43	35	0	0	304	55	73	2272	12			
Arrive On Green	0.04	0.04	0.00	0.00	0.20	0.20	0.22	0.22	0.22			
Sat Flow, veh/h	1027	822	0	0	1552	282	111	3454	18			
Grp Volume(v), veh/h	18	0	0	0	0	221	1077	0	981			
Grp Sat Flow(s),veh/h/ln	1849	0	0	0	0	1834	1791	0	1793			
Q Serve(g_s), s	1.2	0.0	0.0	0.0	0.0	14.3	76.4	0.0	67.9			
Cycle Q Clear(g_c), s	1.2	0.0	0.0	0.0	0.0	14.3	76.4	0.0	67.9			
Prop In Lane	0.56		0.00	0.00		0.15	0.06		0.01			
Lane Grp Cap(c), veh/h	78	0	0	0	0	360	1178	0	1179			
V/C Ratio(X)	0.23	0.00	0.00	0.00	0.00	0.61	0.91	0.00	0.83			
Avail Cap(c_a), veh/h	78	0	0	0	0	360	1178	0	1179			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	0.33	0.33	0.33			
Upstream Filter(I)	1.00	0.00	0.00	0.00	0.00	1.00	1.00	0.00	1.00			
Uniform Delay (d), s/veh	60.2	0.0	0.0	0.0	0.0	47.8	47.3	0.0	44.0			
Incr Delay (d2), s/veh	6.7	0.0	0.0	0.0	0.0	7.6	12.4	0.0	6.9			
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(95%),veh/ln	1.3	0.0	0.0	0.0	0.0	11.8	51.0	0.0	44.4			
Unsig. Movement Delay, s/veh	67.0	0.0	0.0	0.0	0.0	55.4	59.7	0.0	50.9			
LnGrp Delay(d),s/veh									50.9 D			
LnGrp LOS	E	A 18	A	A	A 221	E	E	A	U			
Approach Vol, veh/h		67.0						2058				
Approach Delay, s/veh		67.0 E			55.4 E			55.5 E				
Approach LOS		_			E			_				
Timer - Assigned Phs		2		4				8				
Phs Duration (G+Y+Rc), s		90.0		10.0				30.0				
Change Period (Y+Rc), s		4.5		4.5				4.5				
Max Green Setting (Gmax), s		85.5		5.5				25.5				
Max Q Clear Time (g_c+I1), s		78.4		3.2				16.3				_
Green Ext Time (p_c), s		6.2		0.0				0.8				
Intersection Summary												
HCM 6th Ctrl Delay			55.6									
HCM 6th LOS			E									

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Intersection Delay, s/veh 0 Intersection LOS -	Intersection			
Intersection LOS -	Intersection Delay, s/veh	0		
	Intersection LOS	-		

Movement EBL EBR NBL NBT SBT SBR Lane Configurations Y 1
Traffic Vol, veh/h 0
Future Vol, veh/h 0 0 0 0 0 Peak Hour Factor 0.64 0.64 0.64 0.64 0.64 0.64 Heavy Vehicles, % 0 0 0 9 0 0 Mvmt Flow 0 0 0 0 0 0 0
Peak Hour Factor 0.64
Heavy Vehicles, % 0 0 0 9 0 0 Mvmt Flow 0
Mvmt Flow 0 0 0 0 0 0
Number of Lanes 1 0 0 1 1 0
Approach EB NB SB
Opposing Approach SB NB
Opposing Lanes 0 1 1
Conflicting Approach Left SB EB
Conflicting Lanes Left 1 0
Conflicting Approach Right NB EB
Conflicting Lanes Right 1 0 1
HCM Control Delay 0 0 0
HCM LOS

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	0%	0%	0%
Vol Thru, %	100%	100%	100%
Vol Right, %	0%	0%	0%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	0	0	0
LT Vol	0	0	0
Through Vol	0	0	0
RT Vol	0	0	0
Lane Flow Rate	0	0	0
Geometry Grp	1	1	1
Degree of Util (X)	0	0	0
Departure Headway (Hd)	4.053	3.9	3.9
Convergence, Y/N	Yes	Yes	Yes
Сар	0	0	0
Service Time	2.053	1.9	1.9
HCM Lane V/C Ratio	0	0	0
HCM Control Delay	7.1	6.9	6.9
HCM Lane LOS	N	Ν	Ν
HCM 95th-tile Q	0	0	0

HCM 6th Signalized Intersection Summary 8: Hansel Ave & Fairlane Ave

Projected AM A.M. Peak Hour

	▲	→	1	1	+	*	1	1	1	4	ţ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1 0			1 ,		1	4 13				
Traffic Volume (veh/h)	22	0	0	0	0	447	12	1077	0	0	0	0
Future Volume (veh/h)	22	0	0	0	0	447	12	1077	0	0	0	0
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1900	1900	0	0	1900	1900	1900	1767	1900			
Adj Flow Rate, veh/h	28	0	0	0	0	573	15	1381	0			
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78			
Percent Heavy Veh, %	0	0	0	0	0	0	0	9	0			
Cap, veh/h	59	0	0	0	0	577	1134	2104	0			
Arrive On Green	0.36	0.00	0.00	0.00	0.00	0.36	0.63	0.63	0.00			
Sat Flow, veh/h	10	0	0	0	0	1610	1810	3445	0			
Grp Volume(v), veh/h	28	0	0	0	0	573	15	1381	0			
Grp Sat Flow(s), veh/h/ln	10	0	0	0	0	1610	1810	1678	0			
Q Serve(g_s), s	0.5	0.0	0.0	0.0	0.0	46.1	0.4	33.9	0.0			
Cycle Q Clear(g_c), s	46.6	0.0	0.0	0.0	0.0	46.1	0.4	33.9	0.0			
Prop In Lane	1.00	0.0	0.00	0.00	0.0	1.00	1.00	00.0	0.00			
Lane Grp Cap(c), veh/h	59	0	0.00	0.00	0	577	1134	2104	0.00			
V/C Ratio(X)	0.48	0.00	0.00	0.00	0.00	0.99	0.01	0.66	0.00			
Avail Cap(c_a), veh/h	59	0.00	0.00	0.00	0.00	577	1134	2104	0.00			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(I)	1.00	0.00	0.00	0.00	0.00	1.00	1.00	1.00	0.00			
Uniform Delay (d), s/veh	65.0	0.00	0.00	0.0	0.0	41.5	9.1	15.4	0.0			
Incr Delay (d2), s/veh	6.6	0.0	0.0	0.0	0.0	35.5	0.0	1.6	0.0			
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(95%),veh/ln	1.9	0.0	0.0	0.0	0.0	32.0	0.0	18.1	0.0			
Unsig. Movement Delay, s/veh	1.9	0.0	0.0	0.0	0.0	32.0	0.5	10.1	0.0			
LnGrp Delay(d),s/veh	71.5	0.0	0.0	0.0	0.0	77.1	9.1	17.0	0.0			
LnGrp LOS	71.5 E		0.0 A			E	9.1 A	17.0 B	0.0 A			
	E	A 28	A	A	A	<u> </u>	A		A			
Approach Vol, veh/h					573			1396				
Approach Delay, s/veh		71.5			77.1			16.9				
Approach LOS		E			E			В				
Timer - Assigned Phs		2		4				8				
Phs Duration (G+Y+Rc), s		88.2		52.3				52.3				
Change Period (Y+Rc), s		6.4		* 5.7				* 5.7				
Max Green Setting (Gmax), s		71.6		* 47				* 46				
Max Q Clear Time (g_c+I1), s		35.9		48.6				48.1				
Green Ext Time (p_c), s		13.1		0.0				0.0				
Intersection Summary												
HCM 6th Ctrl Delay			34.9									
HCM 6th LOS			С									
Notes												

Notes
* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection												
Int Delay, s/veh	38.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		Δ.									416	
Traffic Vol, veh/h	0	1 8	91	91	4 18	0	0	0	0	43	1647	85
Future Vol, veh/h	0	8	91	91	18	0	0	0	0	43	1647	85
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free
RT Channelized	-	- 0.00	None	-	-	None	-	-	None	-	-	None
Storage Length	-	_	None	-	_	None	-	-	-	_	-	None
Veh in Median Storage, #		0	-	-	0	-	-	-	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	
Peak Hour Factor	82	82	82	82	82	82	82	82	82	82	82	82
Heavy Vehicles, %	02	02	02	1	02	02	02	02	02	02	4	14
Mvmt Flow	0	10	111	111	22	0	0	0	0	52	2009	104
IVIVITIL FIUW	0	10	111	111	22	U	0	0	0	52	2009	104
Major/Minor	Minor2			Minor1						Major2		
Conflicting Flow All	-	2165	1057	1114	2217	-				0	0	0
Stage 1	-	2165	-	0	0	-				-	-	-
Stage 2	-	0	-	1114	2217	-				-	-	-
Critical Hdwy	-	6.5	6.9	7.52	6.5	-				4.1	-	-
Critical Hdwy Stg 1	-	5.5	-	-	-	-				-	-	-
Critical Hdwy Stg 2	-	-	-	6.52	5.5	-				-	-	-
Follow-up Hdwy	-	4	3.3	3.51	4	-				2.2	-	-
Pot Cap-1 Maneuver	0	48	225	164	44	0				-	-	-
Stage 1	0	87	-	-	-	0				-	-	
Stage 2	0	-	-	224	82	0				-	-	-
Platoon blocked, %											-	-
Mov Cap-1 Maneuver	-	48	225	~ 70	44	-				-	-	-
Mov Cap-2 Maneuver	-	48	-	~ 70	44	-				-	-	-
Stage 1	-	87	-	-	-	-				-	-	-
Stage 2	-	-	-	~ 101	82	-				-	-	-
Approach	EB			WB						SB		
HCM Control Delay, s	63.5			\$ 637.2						00		
HCM LOS	03.5 F			φ 037.2 F								
	Г			I ⁻								
Minor Lane/Major Mvmt		FBI n1	WBLn1	SBL	SBT	SBR						
Capacity (veh/h)		173	64	JDL	001	ODIC						
HCM Lane V/C Ratio		0.698	2.077	-	-	-						
HCM Control Delay (s)			\$ 637.2	-	-	-						
HCM Lane LOS		63.5 F	ຈັບວ7.2 F	-	-	-						
		4.2	F 12.6	-	-	-						
HCM 95th %tile Q(veh)		4.2	12.0	-	-	-						
Notes												

-: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Synchro 11 Report Page 3 a.

Intersection		
Intersection Delay, s/veh Intersection LOS	9.8	
Intersection LOS	А	

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4.			4	
Traffic Vol, veh/h	0	274	2	4	60	0	7	0	22	0	0	0
Future Vol, veh/h	0	274	2	4	60	0	7	0	22	0	0	0
Peak Hour Factor	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69
Heavy Vehicles, %	0	0	0	0	0	0	0	1	14	0	0	0
Mvmt Flow	0	397	3	6	87	0	10	0	32	0	0	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach		EB		WB			NB				SB	
Opposing Approach		WB		EB			SB				NB	
Opposing Lanes		1		1			1				1	
Conflicting Approach Left		SB		NB			EB				WB	
Conflicting Lanes Left		1		1			1				1	
Conflicting Approach Right		NB		SB			WB				EB	
Conflicting Lanes Right		1		1			1				1	
HCM Control Delay		10.4		8			7.8				0	
HCM LOS		В		А			А				-	

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	24%	0%	6%	0%
Vol Thru, %	0%	99%	94%	100%
Vol Right, %	76%	1%	0%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	29	276	64	0
LT Vol	7	0	4	0
Through Vol	0	274	60	0
RT Vol	22	2	0	0
Lane Flow Rate	42	400	93	0
Geometry Grp	1	1	1	1
Degree of Util (X)	0.053	0.449	0.113	0
Departure Headway (Hd)	4.548	4.038	4.397	5.019
Convergence, Y/N	Yes	Yes	Yes	Yes
Сар	791	884	819	0
Service Time	2.552	2.097	2.401	3.026
HCM Lane V/C Ratio	0.053	0.452	0.114	0
HCM Control Delay	7.8	10.4	8	8
HCM Lane LOS	А	В	А	Ν
HCM 95th-tile Q	0.2	2.4	0.4	0

Intersection												
Int Delay, s/veh	18.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1 40			1 4		ň	1				
Traffic Vol, veh/h	5	140	0	0	14	43	15	1217	145	0	0	0
Future Vol, veh/h	5	140	0	0	14	43	15	1217	145	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	120	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	-	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	0	0	5	0	8	0	0	0	0
Mvmt Flow	5	147	0	0	15	45	16	1281	153	0	0	0
Major/Minor	Minor2			Minor1			Major1					
Conflicting Flow All	680	1466	-	-	1390	717	0	0	0			
Stage 1	0	0	-	-	1390	-	-	-	-			
Stage 2	680	1466	-	-	0		-		-			
Critical Hdwy	7.5	6.5	-	-	6.5	7	4.1	-	_			
Critical Hdwy Stg 1	-	-	-	-	5.5	-	-	-	-			
Critical Hdwy Stg 2	6.5	5.5	-	-	-	-	-	-	-			
Follow-up Hdwy	3.5	4		-	4	3.35	2.2	-	-			
Pot Cap-1 Maneuver	341	~ 129	0	0	144	365	-	-	-			
Stage 1	-	-	0	0	211	-			-			
Stage 2	412	194	0	0	-	-	-	-	-			
Platoon blocked, %								-	-			
Mov Cap-1 Maneuver	275	~ 129	-	-	144	365	-	-	-			
Mov Cap-2 Maneuver	275	~ 129	-	-	144	-	-	-	-			
Stage 1	-	-	-	-	211	-	-	-	-			
Stage 2	336	194	-	-	-	-	-	-	-			
Ŭ												
Approach	EB			WB			NB					
HCM Control Delay, s	195.3			22.5								
HCM LOS	F			C								
				5								
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1	WBLn1						
Capacity (veh/h)		THE -	-		131	265						
HCM Lane V/C Ratio		-	-		1.165	0.226						
HCM Control Delay (s)		-	-	-	195.3	22.5						
HCM Lane LOS		-	-		195.5 F	22.J C						
HCM 95th %tile Q(veh)		-	-	-	9	0.8						
. ,					3	0.0						
Notes	^ -											
~: Volume exceeds capacity	y \$:De	lay excee	ds 300s	+: Co	mputation	n Not Defi	ned *:	All major	volume i	n platoon		

Lanes, Volumes, Timings 12: Orange Ave & Lancaster Rd

a.

and Configurations No. A		٨	1	1	1	ţ	4	
Tarlie Volme (vph) turbe Volme (vph) turbe Volme (vph) 1900 1900 1900 1900 1900 1900 1900 300 300 300 300 300 300 300 30	Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø8
Taffic Values (vb) time Volume (vph) time Volume (vph) time Volume (vph) 1900 1900 1900 1900 1900 1900 1900 1900 1900 1901 091 091 1900 1901 091 091 1900 1901 091 091 1900 1901 091 091 1900 1901 091 091 1900 1900 1901 091 091 1900 1900 190 091 1900 1900 191 1900 1900 1901 091 1900 1900 1900 1901 1900 1900 1900 1900 190 1900 1900 1900 190 190 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 190 1900 1900 1900 1900 1900 190 1900 1900 1900 1900 1900 190 1900 1900 1900 1900 1900 1900 1900 1900	Lane Configurations	ሻሻ	1	ň	**	**	1	
deal Flow (vphp) in 1900 1900 1900 1900 1900 1900 1900 Storage Langh (r) 25 25 345 Flow (ront) 343 1482 1752 3406 3312 1566 1 Fernited 0 990 0.010 343 1482 1752 3406 3312 1566 1 Fernited 0 990 0.010 345 150 190 345 150 191 345 150 190 345 150 190 34	Traffic Volume (vph)	199			1402	1273		
Strage Leigh (f) 225 0 300 350 Tape Leigh (f) 25 25 Tape Leigh (f) 25 25 Tape Leigh (f) 25 25 Tape Leigh (f) 25 25 Tape Leigh (f) 343 142 15 Tape Leigh (f) 343 142 15 Tape Leigh (f) 343 142 15 Tape Leigh (f) 343 142 168 3406 3312 1568 Tape Leigh (f) 343 142 168 3406 3312 1568 Tape Leigh (f) 343 142 168 3406 3312 1568 Tape Leigh (f) 40 45 45 Tape Leigh (f) 20 91 091 091 091 091 Tape Leigh (f) 20 95 1541 1399 270 Tape Time (f) 20 92 19 295 1541 1399 270 Tape Time (f) 20 40 00 60 20 00 200 Tape Time (f) 20 92 19 295 1541 1399 270 Tame Time (f) 20 40 00 60 20 00 200 Tape Time (f) 20 40 00 60 20 00 200 Tape Time (f) 60 68 71 71 71 68 68 68 68 28 Tame Time (f) 60 06 0 240 00 00 60 20 00 200 Tape Time (f) 68 71 71 71 68 68 88 58 4 Tape Time (f) 60 06 0 240 00 00 00 00 00 Tape Time (f) 68 71 71 71 68 68 88 58 4 Tape Time (f) 61 71 73 58 44 64 7 26 8 81.1 State Time (f) 61 71 73 58 44 64 7 26 8 81.1 State Time (f) 71 73 58 44 64 7 26 8 81.1 State Time (f) 71 73 58 44 64 7 26 8 81.1 State Time (f) 71 73 58 44 64 7 26 8 81.1 State Time (f) 71 73 58 7 844 64 7 26 8 81.1 State Time (f) 71 73 58 7 844 64 7 26 8 81.1 State Time (f) 71 73 58 7 844 64 7 20 Tape Time Time (f) 74 78 100 110 205 386 16 Tape Leigh (f) 74 128 422 40 Tape Leigh (f) 74 128 42 424 422 40 Tape Leigh (f) 74 128 42 42	Future Volume (vph)							
Stange Lamis 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	ldeal Flow (vphpl)				1900	1900		
Tape Length (f) 25 25 The Permited 0,900 0,101 152 The Permited 0,900 0,101 152 Star Lew (pm) 343 1482 168 3406 3312 1568 158 Star Lew (pm) 343 1482 168 3406 3312 1568 158 Star Lew (PM) 33 168 168 3406 3312 1568 158 Star Lew (PM) 33 168 168 3406 3312 1568 158 Star Lew (PM) 40 45 45 158 Star Lew (PM) 140 145 45 158 Star Lew (PM) 140 140 145 45 158 Star Lew (PM) 140 140 145 145 145 148 Star Lew (PM) 140 140 141 150 149 149 149 149 149 149 149 149 149 149	Storage Length (ft)							
Sake FEW (perch) 3433 1482 1752 3406 3312 1568 Sake FEW (perch) 3433 1482 168 3406 3312 1568 Sake FEW (PTOR) 333 183 183 183 Ink Destame (ft) 1228 1363 902 Treel Time (ft) 1228 1363 902 Sake FEW (Profix) 33 183 183 Samed Lame (ft) 1228 1363 902 Sake FEW (Profix) 249 95 954 95 Samed Lame Taffic (5) and Group FEW (ph) 219 219 1541 1399 270 Urin Type Profit prin+ov prin+ov prin+ov prin+ov prin+ov prin+ov Samed Lame Taffic (5) a 6 2 3 6 2 200 Ciral Losi Time (b) 6.8 7.1 7.1 6.8 6.8 6.8 6.8 6.8 6.8 6.8 6.8 6.8 6.8 6.8 6.8 6.8 6.8 6.8 6.8 6.8 6.8 <t< td=""><td></td><td>-</td><td>1</td><td>-</td><td></td><td></td><td>1</td><td></td></t<>		-	1	-			1	
if herminder 0.950 0.101 skl. Flow (prom) 343 1482 156 3406 3312 1568 skl. Flow (RTOR) 33 45			4.400		0.400	0040	4500	
Sake Fow (spem) 343 1482 166 3406 312 1568 The Version of the Version of			1482		3406	3312	1568	
Sigh T um on Red Yes Yes and Speed (mph) 40 45 45 and Speed (mph) 40 45 45 and Speed (mph) 40 45 45 and Speed (mph) 40 40 1091 091 Travel Time (s) 209 207 1137 Seek Hour Eator 0.091 0.91 0.91 0.91 teamy Vehicles (%) 2% 9% 3% 6% 9% 3% Smart Lane Travel From (mph) 219 219 215 1541 1399 270 Um Type Prov (mph) 219 219 215 1541 1399 270 Travel Time (s) 68 7.1 1 6 2 3 8 Protected Phases 3 1 1 6 2 3 8 Protected Phases 3 6 2 Team Source (mph) 177 33.5 84.4 947 65.6 911 Standard Grade 0.011 0.030 0.050 0.74 0.74 tot Elef Clearn (s) 117 33.5 84.4 947 65.6 911 Standard 20 Rade 0.011 0.030 077 0.77 0.74 tot Elef Clearn (s) 117 33.5 84.4 947 65.6 911 Standard 20 Rade 0.011 0.030 077 0.77 0.74 tot Elef Clearn (s) 117 33.5 84.4 947 65.8 911 Standard 20 Rade 0.011 0.030 0.00 0.0 0.0 0.0 Standard 20 Rade 0.011 0.030 0.077 0.74 tot Elef Clearn (s) 117 33.5 84.4 947 65.8 911 Standard 20 Rade 0.011 0.030 0.0 0.0 0.0 0.0 Standard 20 Rade 0.011 0.030 0.0 0.0 0.0 0.0 Standard 20 Rade 0.011 0.030 0.0 0.0 0.0 0.0 Standard 20 Rade 0.011 0.025 338 16 Jaceue Length Sth (t) 116 170 4214 224 422 Standard 20 Rade 0.030 0.0 0.0 0.0 0.0 Standard 20 Rade 0.030 0.0 0.0 0.0 0.0 Standard 20 Rade 0.030 0.0 0.0 0.0 0.0 Standard 20 Rade 0.030 0.0 0.0 0.0 Standard 20 Rade 0.030 0.0 0.0 0.0 Standard 20 Rade 0.030 0.0 0.0 0.0 Standard 20 Reductin 0 0 0 0 0 0 0 Standard 20 Reductin 0 0 0 0 0 0 0 Standard 20 Reductin 0 0 0 0 0 0 0 Standard 20 Reductin 0 0 0 0 0 0 0 Standard 20 Reductin 0 0 0 0 0 0 Standard 20 Reduc			1400		2406	2240	1500	
Sale Flow (RTOR) 33 183 In Speed (mp) 40 45 45 ink Detance (t) 1228 1363 902 Presel Fune (t) 0.91 0.91 0.91 0.91 Pack Hour Factor 0.91 0.91 0.91 0.91 0.91 Pack Hour Factor 0.91 0.91 0.91 0.91 0.91 Core of Pow (vh) 219 219 255 1541 1399 270 Funct Factor 0.91 0.91 0.91 0.91 0.91 0.91 Orable Of (vh) 219 219 256 1541 1399 270 Functed Phases 3 6 2 8 20 20 Orable Disk (vh) 6.8 7.1 7.1 6.8 6.8 6.8 1.1 Vehated Of Pases 3.9 6.6 21.6 2.1 2.1 2.1 Orable Disk (vh) 0.0 0.0 0.0 0.0 0.0 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.		3433		100	3400	3312		
Link Speed (mpt) 40 46 46 46 Link Distance (mpt) 228 1036 902 Travel Time (s) 209 207 13.7 Hear Vehicles (%) 2% 9% 3% 6% 9% 3% Hear Vehicles (%) 2% 9% 3% 6% 9% 3% Hear Vehicles (%) 2% 9% 3% 6% 9% 3% Hear Vehicles (%) 2% 9% Hear Vehicles (%) 2% Hear								
and, Defance (n) 128 1633 902 Pack Hour (skotor 0.91 0.91 0.91 0.91 0.91 Pack Hour (skotor 0.91 0.91 0.91 0.91 0.91 Pack Hour (skotor 0.91 0.91 0.91 0.91 0.91 Stared Lane Triffic (%) 2% 9% 3% 6% 9% 3% Stared Lane Triffic (%) 2% 9% 1541 1389 270 fum Type Prot pm·opt NA NA pm/opt 8 remitted Phases 3 1 6 2 3 8 remitted Phases 3 1 71 6 8.8 6.8 otal Stiff (3) 20.0 24.0 94.0 90.0 66.0 20.0 20.0 otal Stiff (4) 0.11 0.30 0.77 0.57 0.74 0.22 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 <td< td=""><td></td><td>40</td><td>55</td><td></td><td>45</td><td>45</td><td>105</td><td></td></td<>		40	55		45	45	105	
Travel Time (a) 20.9 20.7 13.7 Seak Hour Earlor 0.91 0.91 0.91 0.91 0.91 0.91 teawy Vehicles (%) 2% 9% 3% 6% 9% 3% strend Lane Triffer (%)								
Pack Havr Safor 0, 0, 1								
deary Vehicles (%) 2% 9% 3% 6% 9% 3% ame Group Flow (vph) 219 219 225 1541 1399 270 Prote prime v prime v prime v prime v prime v prime v Protected Phases 3 6 2 3 8 Parmited Phases 3 6 2 2 8 Protected Phases 3 6 2 2 8 Call Split (s) 2.00 24.0 90.0 66.0 20.0 20.0 Call List Time (s) 6.8 7.1 7.1 6.8 6.8 6.8 Call List Time (s) 6.8 7.1 7.1 6.8 6.8 6.8 Call List Time (s) 6.8 7.1 7.1 6.5 6.1 1 Call List Time (s) 6.8 7.1 7.1 7.7 0.57 0.74 0.22 Cantro Delay 5.9 2.6 3.9.9 6.6 21.6 2.1 1 Davese Length S0h (th) 7.6 10.0 <			0.91	0.91			0.91	
Shared Lane Traffic (%) and Group Flow (vph) 219 219 219 219 141 1399 270 Turn Type (vph) 219 219 219 14 1399 270 Turn Type (vph) 219 219 240 90.0 66.0 200 20.0 Total Las Time (s) 6.8 7.1 7.1 6.8 6.8 6.8 At Effic Green (s) 11.7 33.5 84.4 84.7 62.6 81.1 Valuated gC Reio (0.111 0.30 0.77 0.77 0.77 0.74 //c Ratio 0.60 0.46 0.83 0.59 0.74 0.22 Darto Delay 0.0 0.0 0.0 0.0 0.0 0.0 Darto Delay 0.59 286 399 6.6 21.6 2.1 Darto Delay 0.0 0.0 0.0 0.0 0.0 0.0 Darto Delay 0.59 286 399 6.6 21.6 2.1 Darto Delay 0.0 0.0 0.0 0.0 0.0 Darto Delay 0.0 0.0 0.0 0.0 Darto Darto Da								
ane Group Flow (vph) 219 219 295 1541 1399 270 Value Type Prote prevo preve to NA NA prevo Protected Phases 3 1 1 6 2 3 6 7 1 7 1 6 2 3 6 7 1 7 1 6 8 8 6 8 7 1 7 1 6 8 6 8 6 8 7 1 7 1 6 8 6 8 6 8 7 1 7 1 6 8 6 8 6 8 7 1 7 1 6 8 6 8 6 8 7 1 7 1 7 1 6 8 6 8 6 8 7 1 7 1 7 1 6 8 6 8 6 8 7 1 7 1 7 1 6 8 6 8 6 8 7 1 7 1 7 1 6 8 6 8 6 8 7 1 7 1 7 1 6 8 6 8 6 8 7 1 7 1 7 1 6 8 6 8 6 8 7 1 7 1 7 1 6 8 7 8 7 0 7 0 7 0 7 0		2 /0	070	070	070	570	0.0	
Um Type Toreladel Phases Permited Pha		219	219	295	1541	1399	270	
Prodecide Phases 3 1 1 6 2 3 8 Total Spit (s) 20.0 24.0 24.0 90.0 66.0 20.0 20.0 Total Spit (s) 20.0 24.0 24.0 90.0 66.0 20.0 20.0 Total List Time (s) 6.8 7.1 7.1 6.8 6.8 6.8 Vict Ent Green (s) 1.1.7 33.5 84.4 84.7 62.6 81.1 Vict Ent Green (s) 0.11 0.30 0.77 0.77 0.74 0.72 Vict Ent Green (s) 1.1.7 33.8 84.8 84.7 62.6 81.1 Vict Ent Green (s) 1.1.7 0.6 0.8 52.1 0.74 0.22 Control Delay 53.9 28.6 39.9 66.6 21.6 2.1 Date Ength S50 (ft) 115 170 #24.1 264 492 40 Dateue Leigh S51 (ft) 1148 1283 2822 1350 350<	Turn Type							
Permitted Phases Permi	Protected Phases							8
Total List Time (e) 6 88 7.1 7.1 6.8 6.8 6.8 6.8 Vet Eff Green (s) 11.7 335 844 847 62.8 81.1 Vet Eff Green (s) 11.7 335 844 847 62.8 81.1 Vet Eff Green (s) 0.60 0.46 0.33 0.59 0.74 0.22 Data Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Data Delay 0.0 0.0 0 0 0 0.0 Data Delay 0.0 0.0 0 0 0.0 0.0 Data Delay 0.0 0 0 0 0 0.0 0.0 Data Delay 0.0 0 0 0 0 0 0 0 0 0 0.0 Data Delay 0.0 0 0 0 0 0 0 0.0 Data Delay 0.0 0.0 0 0 0 0 0 0.0 Data Delay 0.0 0.0 0 0 0 0 0 0.0 Data Delay 0.0 0.0 0 0 0 0 0 0 0.0 Data Delay 0.0 0.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Permitted Phases							
Total List Time (e) 6 88 7.1 7.1 6.8 6.8 6.8 6.8 Vet Eff Green (s) 11.7 335 844 847 62.8 81.1 Vet Eff Green (s) 11.7 335 844 847 62.8 81.1 Vet Eff Green (s) 0.60 0.46 0.33 0.59 0.74 0.22 Data Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Data Delay 0.0 0.0 0 0 0 0.0 Data Delay 0.0 0.0 0 0 0.0 0.0 Data Delay 0.0 0 0 0 0 0.0 0.0 Data Delay 0.0 0 0 0 0 0 0 0 0 0 0.0 Data Delay 0.0 0 0 0 0 0 0 0.0 Data Delay 0.0 0.0 0 0 0 0 0 0.0 Data Delay 0.0 0.0 0 0 0 0 0 0.0 Data Delay 0.0 0.0 0 0 0 0 0 0 0.0 Data Delay 0.0 0.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Total Split (s)	20.0			90.0	66.0		20.0
Val: Eff. Green (s) 11.7 33.5 84.4 84.7 62.6 81.1 Actuated g/C Ratio 0.11 0.30 0.77 0.57 0.74 0.22 Chartio 0.60 0.46 0.83 0.59 0.74 0.22 Control Delay 53.9 28.6 39.9 6.6 21.6 2.1 Sueue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Of Delay 53.9 28.6 39.9 6.6 21.6 2.1 Sueue Length Softh (ft) 76 10.0 110 205 396 16 Sueue Length Softh (ft) 115 170 #241 264 492 40 Itemal Link Dit(ft) 1148 1283 822 100 350 Sueue Length Softh (ft) 125 300 350 350 Sueue CapeReduch 0 0 0 0 0 Sueue Length Softh (ft) 125 30.0 350 350 Sueue Length Softh (ft) 0.7 0.59 0.74 0.22	Total Lost Time (s)							
<i>i</i> /c Ratio 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0,	Act Effct Green (s)		33.5				81.1	
Control Delay 53.9 28.6 39.9 6.6 21.6 2.1 Jaceue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Colal Delay 53.9 28.6 39.9 6.6 21.6 2.1 JoS D C D A C A Approach LOS D C D A C A Approach LOS D B B Jaceue Length 50th (th) 76 100 110 205 39.6 16 Jaceue Length 50th (th) 76 100 110 205 39.6 16 Jaceue Length 50th (th) 115 170 #241 264 492 40 Internal Link Dist (th) 1148 1283 822 Imm Bay Length (th) 225 300 350 Jaae Capacity (trph) 411 501 385 2622 1884 1223 Jacrvation Cap Reductin 0 0 0 0 0 0 0 Storage Cap Reductin 0 0 0 0 0 0 0 Reduced vic Ratio 0.53 0.44 0.77 0.59 0.74 0.22 Intersection Summary Available Capacity Unitation 0 Theresection Summary Available Capacity Unitation 10 Diffeet 36 (32%), Referenced to phase 2:SBT and 6:NBTL, Start of Green Control Type: Actuated -Coordinated Valuated Cycle Length: 110 Diffeet 36 (32%), Referenced to phase 2:SBT and 6:NBTL, Start of Green Control Type: Actuated -Coordinated Valuated Cycle Length: 110 Diffeet 35 (32%), Referenced to phase 2:SBT and 6:NBTL, Start of Green Control Type: Actuated -Coordinated Valuated Cycle Length: 110 Diffeet 35 (32%), Referenced to phase 2:SBT and 6:NBTL, Start of Green Control Type: Actuated -Coordinated Valuated Cycle Length: 110 Diffeet 35 (32%), Referenced to phase 2:SBT and 6:NBTL, Start of Green Control Type: Actuated -Coordinated Valuated Cycle Length: 110 Diffeet 35 (32%), Referenced to phase 2:SBT and 6:NBTL, Start of Green Control Type: Actuated -Coordinated Valuation with Ratio 0.83 Intersection LOS: B Intersection Signal Delay 18.0 Value Bay New 8 Lancaster Rd Value Bay New 8 Lancaster Rd Val	Actuated g/C Ratio	0.11	0.30	0.77	0.77	0.57	0.74	
Date Delay 0.0 0.0 0.0 0.0 0.0 folal Delay 53.9 28.6 39.9 6.6 21.6 2.1 OS D C D A C A Approach Delay 41.3 11.9 18.5	v/c Ratio	0.60	0.46	0.83	0.59	0.74	0.22	
Total Delay 53.9 28.6 39.9 6.6 21.6 2.1 LOS D C D A C A Approach Delay 41.3 11.9 18.5 A A Approach LOS D B B B B Daueue Length 50th (ft) 115 170 #241 264 492 40 Date Length 50th (ft) 115 170 #241 264 492 40 Date Length (ft) 225 300 350 350 350 Jase Capacity (vph) 411 501 385 2622 1884 1223 Staration Cap Reductn 0 0 0 0 0 350 Staration Cap Reductn 0 0 0 0 0 0 Note Cap Reductn 0 0 0 0 0 0 0 Valued Cycle Langth: 110 Cher Staratof Cycle Langth: 110 Staratof Cycle Lang	Control Delay	53.9	28.6	39.9	6.6	21.6	2.1	
OS D C D A C A Approach Delay 41.3 11.9 18.5 Approach Delay Analysis Approach Delsy 41.3 11.9 18.5 Approach Delay Analysis Deueu Length S0th (ft) 76 100 110 205 396 16 Dueue Length S0th (ft) 115 170 #241 284 492 40 Turn Bay Length (ft) 125 300 350 388 282 350 Sase Capacity (vph) 411 501 385 2622 1884 1223 Starvation Cap Reductn 0 0 0 0 0 0 0 Starvation Cap Reductn 0 0 0 0 0 0 0 Starvation Cap Reductn 0 0 0 0 0 0 0 0 Starvation Signal Delay: 110 0 0 0 0 0 0 0 0 0 17 17 18 12 12 12 12 12	Queue Delay					0.0		
Approach Delay 41.3 11.9 18.5 Approach LOS D B B Daueue Length 50th (ft) 115 170 #241 264 492 40 Dueue Length 50th (ft) 115 170 #241 264 492 40 Itm Bay Length (ft) 125 300 350 350 Jase Capacity (vph) 411 501 385 2622 1884 1223 Starvation Cap Reducth 0 0 0 0 0 0 0 Starvation Cap Reducth 0 0 0 0 0 0 0 Neal Type: Other Other <td>Total Delay</td> <td>53.9</td> <td></td> <td>39.9</td> <td>6.6</td> <td>21.6</td> <td></td> <td></td>	Total Delay	53.9		39.9	6.6	21.6		
Approach LOS D B B Dueue Length 50th (ft) 76 100 110 205 396 16 Dueue Length 50th (ft) 115 170 1283 822 1283 822 Furm Bay Length (ft) 225 300 350 386 382 Starvation Cap Reductn 0 0 0 0 0 Starvation Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0.53 0.44 0.77 0.59 0.74 <td< td=""><td>LOS</td><td></td><td>С</td><td>D</td><td></td><td></td><td>А</td><td></td></td<>	LOS		С	D			А	
During Length 50th (ft) 76 100 110 205 396 16 Dureu Length 95th (ft) 115 170 #241 264 492 40 Itemal Link Dist (ft) 1148 1283 822 1 <	Approach Delay							
Dueue Length 95th (ft) 115 170 #241 264 492 40 ntermal Link Dist (ft) 1148 1283 822 Image Length 95th (ft) 25 300 350 Base Capacity (vph) 411 501 385 2622 1884 1223 Starvation Cap Reductn 0 0 0 0 0 0 Splitback Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Area Type: Other Other Voice Length: 110 Note Catalactore Note Catalactore Actuated Oycle Length: 110 Actuated Cycle Length: 130 Intersection LOS: B Intersection Capacity Utilization 73.0% ICU Level of Service C Analysis Period (min) 15 Storage Capacity, queue may be longer. Queue shown is maximum after two cycles. Queue shown is maximum after two cycles. Queue shown is maximum after two cycles.								
ntemal Link Dist (ft) 1148 1283 822 Turm Bay Length (ft) 225 300 350 Jase Capacity (vph) 411 501 385 2622 1884 1223 Starvation Cap Reductn 0 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 0 Reduced vic Ratio 0.53 0.44 0.77 0.59 0.74 0.22 Intersection Summary Area Type: Other Cycle Length: 110 Cycle Length: 110 Offset: 35 (32%), Referenced to phase 2:SBT and 6:NBTL, Start of Green Control Type: Actuated Coordinated Waximum vic Ratio: 0.83 Intersection LOS: B Intersection Capacity Utilization 73.0% ICU Level of Service C Analysis Period (min) 15 ≠ 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Spills and Phases: 12: Orange Ave & Lancaster Rd ≠ 01								
Turn Bay Length (ft) 225 300 350 Base Capacity (vph) 411 501 385 2622 1884 1223 Starvation Cap Reductn 0 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0 Reduced vic Ratio 0.53 0.44 0.77 0.59 0.74 0.22 Intersection Summary Area Type: Other			170	#241			40	
Base Capacity (vph) 411 501 385 2622 1884 1223 Starvation Cap Reductn 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Reduced v/c Ratio 0.53 0.44 0.77 0.59 0.74 0.22 Intersection Summary					1283	822		
Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0.53 0.44 0.77 0.59 0.74 0.22 Intersection Summary			504		0000	100.1		
Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 Reduced vic Ratio 0.53 0.44 0.77 0.59 0.74 0.22 Intersection Summary Area Type: Other								
Storage Cap Reductn 0 0 0 0 0 0 Reduced v/c Ratio 0.53 0.44 0.77 0.59 0.74 0.22 Intersection Summary Area Type: Other Cycle Length: 110 Other Actuated Cycle Length: 110 Other Offset: 35 (32%), Referenced to phase 2:SBT and 6:NBTL, Start of Green Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.83 Intersection LOS: B Intersection Signal Delay: 18.0 Intersection LOS: B Intersection LOS: B Intersection LOS: B Intersection LOS: B Intersection Signal Delay: 18.0 Intersection LOS: B Intersection Signal Delay: 18.0 Intersection LOS: B Intersection LOS: B Intersection Signal Delay: 18.0 Queue shown is maximum after two cycles. Splits and Phases: 12: Orange Ave & Lancaster Rd Image: Poil Poil Poil Poil Poil Poil Poil Poil								
Reduced v/c Ratio 0.53 0.44 0.77 0.59 0.74 0.22 Intersection Summary Area Type: Other								
Area Type: Other Cycle Length: 110 Actuated Cycle Length: 110 Offset: 35 (32%), Referenced to phase 2:SBT and 6:NBTL, Start of Green Control Type: Actuated-Coordinated Waximum v/c Ratio: 0.83 Intersection LOS: B Intersection Signal Delay: 18.0 Intersection LOS: B Intersection Capacity Utilization 73.0% ICU Level of Service C Analysis Period (min) 15 ICU Level of Service C 4 951 percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Splits and Phases: 12: Orange Ave & Lancaster Rd Ø 01 Ø 02 (R) Ø 03 24 s 66 s 20 s								
Area Type: Other Cycle Length: 110 Actuated Cycle Length: 110 Offset: 35 (32%), Referenced to phase 2:SBT and 6:NBTL, Start of Green Control Type: Actuated-Coordinated Waximum v/c Ratio: 0.83 Intersection LOS: B Intersection Capacity Utilization 73.0% ICU Level of Service C Analysis Period (min) 15 # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Splits and Phases: 12: Orange Ave & Lancaster Rd		0.55	0.44	0.77	0.59	0.74	0.22	
Cycle Length: 110 Actuated Cycle Length: 110 Offset: 35 (32%), Referenced to phase 2:SBT and 6:NBTL, Start of Green Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.83 Intersection Capacity Utilization 73.0% Intersection Capacity Utilization 73.0% ICU Level of Service C Analysis Period (min) 15 # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Splits and Phases: 12: Orange Ave & Lancaster Rd Ø1 Ø2 (R) 24 s Ø6 s Ø0 s Ø0 s Ø0 s Ø0 s	Intersection Summary							
Actuated Cycle Length: 110 Offset: 35 (32%), Referenced to phase 2:SBT and 6:NBTL, Start of Green Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.83 Intersection LOS: B Intersection Capacity Utilization 73.0% ICU Level of Service C Analysis Period (min) 15 # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Splits and Phases: 12: Orange Ave & Lancaster Rd Image: Application of the process of th	21	ther						
Offset: 35 (32%), Referenced to phase 2:SBT and 6:NBTL, Start of Green Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.83 Intersection Signal Delay: 18.0 Intersection LOS: B Intersection Capacity Utilization 73.0% ICU Level of Service C Analysis Period (min) 15 # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Splits and Phases: 12: Orange Ave & Lancaster Rd Image: Actuated Coordinate of the service of the	Cycle Length: 110							
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.83 Intersection Signal Delay: 18.0 Intersection LOS: B Intersection Capacity Utilization 73.0% ICU Level of Service C Analysis Period (min) 15 # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Splits and Phases: 12: Orange Ave & Lancaster Rd \$ 01 02 (R) 24 s 66 s 20 s \$ 05 0 s 20 s								
Maximum v/c Ratio: 0.83 Intersection Signal Delay: 18.0 Intersection Capacity Utilization 73.0% ICU Level of Service C Analysis Period (min) 15 # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Splits and Phases: 12: Orange Ave & Lancaster Rd 20 s 20 s 20 s			nd 6:NBTL,	Start of Gree	en			
ntersection Signal Delay: 18.0 Intersection LOS: B ntersection Capacity Utilization 73.0% ICU Level of Service C Analysis Period (min) 15 # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Splits and Phases: 12: Orange Ave & Lancaster Rd Splits and Phases: 12: Orange Ave & Lancaster Rd 20 s 120 s 120 s 120 s								
ntersection Capacity Utilization 73.0% ICU Level of Service C Analysis Period (min) 15								
Analysis Period (min) 15 # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Splits and Phases: 12: Orange Ave & Lancaster Rd # Ø2 (R) 24 s 66 s 20 s # Ø8 90 s 20 s		0/						
 [#] 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Splits and Phases: 12: Orange Ave & Lancaster Rd ✓ Ø1 Ø2 (R) ✓ Ø3 20 s Ø6 (R) Ø0 s 		70			ici		Service C	
Queue shown is maximum after two cycles. Splits and Phases: 12: Orange Ave & Lancaster Rd \checkmark Ø1 \checkmark Ø2 (R) 24 s 66 s \checkmark Ø6 (R) \checkmark Ø8 90 s 20 s		oopooitu a		longer				
 ▲ Ø1 Ø2 (R) 24 s 66 s 20 s ▲ Ø8 90 s 20 s 			eue may De	e ionger.				
24 s 66 s 20 s	Splits and Phases: 12: Orange Av	e & Lancas	ter Rd					
24 s 66 s 20 s	•	4	0.02					14
ÿ6 (R) 90 s 20 s	▶ Ø1	🕴 🕈 Ø2	2 (R)					🏷 Ø3
90 s 20 s	24 s	66 s	a (1897) Sa					20 s
90 s 20 s								1.1
90 s 20 s	Ø6 (R)	•						₹₽ Ø8
	90 s							20 s

TPD, Inc. 02/07/2023 Synchro 11 Report Page 1

HCM 6th Signalized Intersection Summary 13: Orange Ave & Nela Ave

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
ane Configurations		1		3			5	† 1 ₂		5	† 12	
Traffic Volume (veh/h)	0	4	1	29	1	148	4	1514	17	85	1372	
Future Volume (veh/h)	0	0	1	29	1	148	4	1514	17	85	1372	
nitial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00	•	1.00	1.00		1.00	1.00		1.00	1.00	· ·	1.(
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.0
Vork Zone On Approach		No			No			No			No	
dj Sat Flow, veh/h/ln	1900	1900	1900	1841	1900	1885	1900	1811	1811	1900	1811	190
Adj Flow Rate, veh/h	0	0	1	32	1	161	4	1646	18	92	1491	100
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.9
Percent Heavy Veh, %	0.02	0.02	0.02	4	0.02	1	0.02	6	6	0.02	6	0.0
Cap, veh/h	0	0	173	205	1	172	316	2633	29	284	2978	
Arrive On Green	0.00	0.00	0.11	0.11	0.11	0.11	0.76	0.76	0.76	0.04	0.84	0.8
Sat Flow, veh/h	0.00	0.00	1610	1394	10	1602	358	3487	38	1810	3526	0.0
Grp Volume(v), veh/h	0	0	1010	32	0	162	4	811	853	92	727	76
Grp Sat Flow(s), veh/h/ln	0	0	1610	1394	0	1612	358	1721	1804	1810	1721	18
	0.0	0.0	0.1	2.7	0.0	13.0	0.4	28.4	28.5		14.8	14
Serve(g_s), s	0.0		0.1	2.7			0.4 4.1		28.5	1.3 1.3	14.0	
Cycle Q Clear(g_c), s	0.0	0.0	1.00	2.8 1.00	0.0	13.0 0.99	4.1	28.4	28.5	1.3	14.8	14 0.0
Prop In Lane	0.00	0	1.00		0			1000	1363	284	4450	
ane Grp Cap(c), veh/h		0		205	0	174	316	1299			1453	152
//C Ratio(X)	0.00	0.00	0.01	0.16	0.00	0.93	0.01	0.62	0.63	0.32	0.50	0.5
vail Cap(c_a), veh/h	0	0	173	205	0	174	316	1299	1363	373	1453	152
ICM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.0
Ipstream Filter(I)	0.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	0.64	0.64	0.0
Jniform Delay (d), s/veh	0.0	0.0	51.8	53.0	0.0	57.5	4.9	7.4	7.4	7.9	2.7	2
ncr Delay (d2), s/veh	0.0	0.0	0.0	0.4	0.0	49.2	0.1	2.3	2.2	0.4	0.8	0
nitial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0
6ile BackOfQ(95%),veh/ln Jnsig. Movement Delay, s/veh	0.0	0.0	0.1	1.8	0.0	12.1	0.1	13.9	14.4	1.3	5.1	5
.nGrp Delay(d),s/veh	0.0	0.0	51.8	53.4	0.0	106.8	5.0	9.6	9.6	8.3	3.5	3
nGrp LOS	A	A	D	D	A	F	A	A	A	A	A	J
pproach Vol, veh/h		1			194			1668		7.	1585	
pproach Delay, s/veh		51.8			98.0			9.6			3.8	
pproach LOS		D			50.0 F			A			A	
imer - Assigned Phs	1	2		4		6		8				
hs Duration (G+Y+Rc), s	11.6	105.2		20.2		116.8		20.2				
Change Period (Y+Rc), s	6.8	6.8		* 6.2		6.8		* 6.2				
lax Green Setting (Gmax), s	11.2	85.2		* 14		103.2		* 14				
fax Q Clear Time (g_c+l1), s	3.3	30.5		2.1		16.8		15.0				
Green Ext Time (p_c), s	0.1	17.8		0.0		14.7		0.0				
ntersection Summary												
ICM 6th Ctrl Delay			11.9									
ICM 6th LOS			В									

User approved pedestrian interval to be less than phase max green. * HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

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Intersection												
Int Delay, s/veh	3.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4.			4.			4.	
Traffic Vol, veh/h	4	1000	44	128	530	8	9	2	40	4	0	3
Future Vol, veh/h	4	1000	44	128	530	8	9	2	40	4	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	1	4	6	3	0	13	0	0	0	0	0
Mvmt Flow	4	1087	48	139	576	9	10	2	43	4	0	3
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	585	0	0	1135	0	0	1979	1982	1111	2001	2002	581
Stage 1	-	-	-	-	-	-	1119	1119	-	859	859	-
Stage 2	-	-	-	-	-	-	860	863	-	1142	1143	-
Critical Hdwy	4.1	-	-	4.16	-	-	7.23	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.254	-	-	3.617	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1000	-	-	601	-	-	43	62	257	45	60	517
Stage 1	-	-	-	-	-	-	239	285	-	354	376	-
Stage 2	-	-	-	-	-	-	336	374	-	246	277	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1000	-	-	601	-	-	31	40	257	26	39	517
Mov Cap-2 Maneuver	-	-	-	-	-	-	31	40	-	26	39	-
Stage 1	-	-	-	-	-	-	236	282	-	350	247	-
Stage 2	-	-	-	-	-	-	219	246	-	201	274	-
										0.5		
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			2.5			74.8			103.2		
HCM LOS							F			F		
			EDI	CDT					001 - 4			
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			

Capacity (veh/h) 103 1000 - - 601 - - 44 HCM Lane V/C Ratio 0.538 0.004 - - 0.231 - - 0.173 HCM Control Delay (s) 74.8 8.6 0 - 12.8 0 - 103.2 HCM Lane LOS F A A - B A - F HCM 95th %tile Q(veh) 2.5 0 - 0.9 - - 0.6	Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	
HCM Control Delay (s) 74.8 8.6 0 - 12.8 0 - 103.2 HCM Lane LOS F A A B A - F	Capacity (veh/h)	103	1000	-	-	601	-	-	44	
HCM Lane LOS F A A - B A - F	HCM Lane V/C Ratio	0.538	0.004	-	-	0.231	-	-	0.173	
	HCM Control Delay (s)	74.8	8.6	0	-	12.8	0	-	103.2	
HCM 95th %tile Q(veh) 2.5 0 0.9 0.6	HCM Lane LOS	F	А	А	-	В	А	-	F	
	HCM 95th %tile Q(veh)	2.5	0	-	-	0.9	-	-	0.6	

HCM 6th Signalized Intersection Summary 2: Hansel Ave & Hoffner Ave

a.

Projected PM P.M. Peak Hour

			•	Ŧ		-	7		1		+	*
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		3 02			•	1		≜ î♠	1			
Traffic Volume (veh/h)	8		0	0	304	234	166	1378	730	0	0	0
Future Volume (veh/h)	8	302	0	0	304	234	166	1378	730	0	0	0
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1900	1870	0	0	1856	1856	1856	1841	1885			
Adj Flow Rate, veh/h	8	311	0	0	313	241	171	1421	753			
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97			
Percent Heavy Veh, %	0	2	0	0	3	3	3	4	1			
Cap, veh/h	27	351	0	0	408	346	255	2236	1114			
Arrive On Green	0.22	0.22	0.00	0.00	0.22	0.22	0.70	0.70	0.70			
Sat Flow, veh/h	13	1598	0	0	1856	1572	365	3206	1598			
Grp Volume(v), veh/h	319	0	0	0	313	241	854	738	753			
Grp Sat Flow(s),veh/h/ln	1610	0	0	0	1856	1572	1822	1749	1598			
Q Serve(g_s), s	5.7	0.0	0.0	0.0	23.7	21.2	40.0	33.1	40.5			
Cycle Q Clear(g_c), s	29.5	0.0	0.0	0.0	23.7	21.2	40.0	33.1	40.5			
Prop In Lane	0.03		0.00	0.00		1.00	0.20		1.00			
Lane Grp Cap(c), veh/h	379	0	0	0	408	346	1271	1220	1114			
V/C Ratio(X)	0.84	0.00	0.00	0.00	0.77	0.70	0.67	0.61	0.68			
Avail Cap(c_a), veh/h	513	0	0	0	546	462	1271	1220	1114			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(I)	0.98	0.00	0.00	0.00	1.00	1.00	1.00	1.00	1.00			
Uniform Delay (d), s/veh	55.9	0.0	0.0	0.0	54.9	53.9	12.9	11.9	13.0			
Incr Delay (d2), s/veh	9.0	0.0	0.0	0.0	4.6	2.9	2.8	2.2	3.3			
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(95%),veh/ln	18.6	0.0	0.0	0.0	17.3	13.6	22.4	18.4	20.4			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	64.9	0.0	0.0	0.0	59.5	56.8	15.8	14.1	16.3			
LnGrp LOS	E	А	А	А	Е	E	В	В	В			
Approach Vol. veh/h		319			554			2345				
Approach Delay, s/veh		64.9			58.4			15.4				
Approach LOS		E			E			В				
Timer - Assigned Phs		2		4				8				
Phs Duration (G+Y+Rc), s		111.1		38.9				38.9				
Change Period (Y+Rc), s		6.5		5.9				5.9				
Max Green Setting (Gmax), s		93.5		44.1				44.1				
Max Q Clear Time (g c+l1), s		42.5		25.7				31.5				
Green Ext Time (p_c), s		24.8		2.6				1.5				
Intersection Summary												
HCM 6th Ctrl Delay			27.7									
HCM 6th LOS			С									

	4	•	Ť	1	4	ţ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	5				5	**
Traffic Volume (veh/h)	478	0	0	0	297	1411
Future Volume (veh/h)	478	0	0	0	297	1411
Initial Q (Qb), veh	0	0			0	0
Ped-Bike Adj(A_pbT)	1.00	1.00			1.00	
Parking Bus, Adj	1.00	1.00			1.00	1.00
Work Zone On Approach	No					No
Adj Sat Flow, veh/h/ln	1856	0			1900	1796
Adj Flow Rate, veh/h	493	0			306	1455
Peak Hour Factor	0.97	0.97			0.97	0.97
Percent Heavy Veh, %	3	0			0	7
Cap, veh/h	0	0			1784	3274
Arrive On Green	0.00	0.00			0.96	0.96
Sat Flow, veh/h	0.00	0.00			1810	3503
Grp Volume(v), veh/h	0.0				306	1455
Grp Sat Flow(s), veh/h/ln	0.0				1810	1455
Q Serve(g_s), s					1.2	4.5
Cycle Q Clear(g_c), s					1.2	4.5
Prop In Lane					1.2	4.5
					1784	3274
Lane Grp Cap(c), veh/h						0.44
V/C Ratio(X)					0.17	
Avail Cap(c_a), veh/h					1784	3274
HCM Platoon Ratio					1.00	1.00
Upstream Filter(I)					1.00	1.00
Uniform Delay (d), s/veh					0.1	0.2
Incr Delay (d2), s/veh					0.2	0.4
Initial Q Delay(d3),s/veh					0.0	0.0
%ile BackOfQ(95%),veh/In					0.2	0.4
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh					0.4	0.7
LnGrp LOS					A	А
Approach Vol, veh/h						1761
Approach Delay, s/veh						0.6
Approach LOS						А
Timer - Assigned Phs		2				
Phs Duration (G+Y+Rc), s		150.0				
Change Period (Y+Rc), s		6.1				
Max Green Setting (Gmax), s		97.9				
Max Q Clear Time (g_c+I1), s		6.5				
Green Ext Time (p c), s		20.4				
		20.4				
Intersection Summary			0.0			
HCM 6th Ctrl Delay			0.6			
HCM 6th LOS			А			

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	11011			UDE	4
Traffic Vol, veh/h	0	4	1 36	3	0	188
Future Vol, veh/h	0	4	36	3	0	188
Conflicting Peds, #/hr	0	4	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-	None	-	None
	- 0	none -	-	none -		none -
Storage Length	0	-	- 0	-	-	-
Veh in Median Storage, #	-		-			-
Grade, %	0	-	0	-	-	0
Peak Hour Factor	71	71	71	71	71	71
Heavy Vehicles, %	0	0	0	0	0	3
Mvmt Flow	0	6	51	4	0	265
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	318	53	0	0	55	0
Stage 1	53	- 55	-	-	- 55	-
	53 265	-				-
Stage 2	265 6.4		-	-	-	
Critical Hdwy		6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	679	1020	-	-	1563	-
Stage 1	975	-	-	-	-	-
Stage 2	784	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	679	1020	-	-	1563	-
Mov Cap-2 Maneuver	679	-	-	-	-	-
Stage 1	975	-	-	-	-	-
Stage 2	784	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	8.5		0		0	
HCM LOS	А					
Minor Long/Major Mumt		NBT	NDD	\//DL ~1	SBL	SBT
Minor Lane/Major Mvmt				WBLn1	-	
Capacity (veh/h)		-	-	1020	1563	-
HCM Lane V/C Ratio		-	-	0.006	-	-
HCM Control Delay (s)		-	-	8.5	0	-
HCM Lane LOS		-	-	A	Α	-
HCM 95th %tile Q(veh)		-	-	0	0	-

а.

Intersection

Synchro 11 Report Page 1

Intersection Delay, s/veh	8.6											
Intersection LOS	А											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations		4			4.			4			4	
Traffic Vol, veh/h	89	8	0	0	0	6	0	0	0	157	0	
Future Vol, veh/h	89	8	0	0	0	6	0	0	0	157	0	
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.8
Heavy Vehicles, %	0	0	0	1	2	0	0	4	0	0	0	
Mvmt Flow	111	10	0	0	0	8	0	0	0	196	0	
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	
Approach	EB				WB			NB		SB		
Opposing Approach	WB				EB			SB		NB		
Opposing Lanes	1				1			1		1		
Conflicting Approach Left	SB				NB			EB		WB		
Conflicting Lanes Left	1				1			1		1		
Conflicting Approach Right	NB				SB			WB		EB		
Conflicting Lanes Right	1				1			1		1		
HCM Control Delay	8.4				7			0		8.8		
HCMLOS	Δ				Δ			-		Δ		

Connicting Editors Hight					•	•	
HCM Control Delay	8.4			7	0	8.8	
HCM LOS	А			А	-	А	
Lane	NBLn	1 EBLn1	WBLn1	SBLn1			
Vol Left, %	0%	5 92%	0%	100%			
Vol Thru, %	100%	8%	0%	0%			
Vol Right, %	0%	6 0%	100%	0%			
Sign Control	Sto	o Stop	Stop	Stop			
Traffic Vol by Lane) 97	6	157			
LT Vol) 89	0	157			
Through Vol) 8	0	0			
RT Vol) 0	6	0			
Lane Flow Rate) 121	8	196			
Geometry Grp		1 1	1	1			
Degree of Util (X)		0.153	0.008	0.236			
Departure Headway (Hd)	4.48	3 4.557	3.935	4.324			
Convergence, Y/N	Ye	s Yes	Yes	Yes			
Сар) 792	914	818			
Service Time	2.49	6 2.557	1.939	2.414			
HCM Lane V/C Ratio		0.153	0.009	0.24			
HCM Control Delay	7.	5 8.4	7	8.8			
HCM Lane LOS	1	I A	А	А			
HCM 95th-tile Q) 0.5	0	0.9			

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Intersection													
Int Delay, s/veh	54.2												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		ដ្ 4			1.			41					
Traffic Vol, veh/h	12	4	0	0	125	23	55	1876	4	0	0	0	
Future Vol, veh/h	12	4	0	0	125	23	55	1876	4	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	1081	749504	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97	
Heavy Vehicles, %	8	25	0	0	1	2	0	4	0	0	0	0	
Mvmt Flow	12	4	0	0	129	24	57	1934	4	0	0	0	
Major/Minor	Minor2			Minor1			Major1						
Conflicting Flow All	1146	2052	-	-	2050	969	0	0	0				
Stage 1	0	0	-	-	2050	-	-	-	-				
Stage 2	1146	2052	-	-	0	-	-	-	-				
Critical Hdwy	7.66	7	-	-	6.52	6.94	4.1	-	-				
Critical Hdwy Stg 1	-	-	-	-	5.52	-	-	-	-				
Critical Hdwy Stg 2	6.66	6	-	-	-	-	-	-	-				
Follow-up Hdwy	3.58	4.25	-	-	4.01	3.32	2.2	-	-				
Pot Cap-1 Maneuver	147	42	0	0	~ 56	253	-	-	-				
Stage 1	-	-	0	0	~ 98	-	-	-	-				
Stage 2	202	74	0	0	-	-	-	-	-				
Platoon blocked, %								-	-				
Mov Cap-1 Maneuver	-	42	-	-	~ 56	253	-	-	-				
Mov Cap-2 Maneuver	-	42	-	-	~ 56	-	-	-	-				
Stage 1	-	-	-	-	~ 98	-	-	-	-				
Stage 2	-	74	-	-	-	-	-	-	-				
Approach	EB			WB			NB						
HCM Control Delay, s			(\$ 769.3									
HCM LOS	-			F									
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1	WBLn1							
Capacity (veh/h)		-	-	-	-	64							
HCM Lane V/C Ratio		-	-	-	-	2.384							
HCM Control Delay (s)		-	-	-	-	\$ 769.3							
HCM Lane LOS		-	-	-	-	F							
HCM 95th %tile Q(veh)		-	-	-	-	14.9							
Notes													
~: Volume exceeds capacity	(\$: Do	lay excee	de 300e	+. Co	moutation	n Not Defi	ned *·	All major	volume i	n nlatoon			
. Volume exceeds capacity	φ. Dei	ay excee	43 0005	+. 00	inputatiol	I NOT DEIII	ilicu .	Airmajoi	Volume	in pidtooli			

HCM 6th Signalized Intersection Summary 6: Hansel Ave & Waltham Ave

6: Hansel Ave & Waltha	am Ave										P.M. P	eak Hour
	٠	+	*	4	Ļ	•	1	t	1	1	ţ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			ţ,			412				
Traffic Volume (veh/h)	12	4	0	0	125	23	55	1876	4	0	0	0
Future Volume (veh/h)	12	4	0	0	125	23	55	1876	4	0	0	0
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00	-	1.00	1.00	-	1.00	1.00	-	1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1781	1530	0	0	1885	1870	1900	1841	1900			
Adj Flow Rate, veh/h	12	4	0	0	129	24	57	1934	4			
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97			
Percent Heavy Veh, %	8	25	0	0	1	2	0	4	0			
Cap, veh/h	43	14	0	0	337	63	65	2306	5			
Arrive On Green	0.04	0.04	0.00	0.00	0.22	0.22	0.65	0.65	0.65			
Sat Flow, veh/h	1106	369	0	0	1546	288	100	3567	8			
Grp Volume(v), veh/h	16	0	0	0	0	153	1045	0	950			
Grp Sat Flow(s), veh/h/ln	1474	0	0	0	0	1833	1836	0	1839			
Q Serve(g_s), s	1.5	0.0	0.0	0.0	0.0	10.0	65.3	0.0	52.9			
Cycle Q Clear(g_c), s	1.5	0.0	0.0	0.0	0.0	10.0	65.3	0.0	52.9			
Prop In Lane	0.75	0.0	0.00	0.00	0.0	0.16	0.05	0.0	0.00			
Lane Grp Cap(c), veh/h	58	0	0.00	0.00	0	399	1187	0	1189			
V/C Ratio(X)	0.28	0.00	0.00	0.00	0.00	0.38	0.88	0.00	0.80			
Avail Cap(c_a), veh/h	58	0.00	0.00	0.00	0.00	399	1187	0.00	1189			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(I)	1.00	0.00	0.00	0.00	0.00	1.00	1.00	0.00	1.00			
Uniform Delay (d), s/veh	65.3	0.0	0.0	0.0	0.0	46.7	20.3	0.0	18.1			
Incr Delay (d2), s/veh	11.5	0.0	0.0	0.0	0.0	2.8	9.5	0.0	5.7			
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(95%),veh/ln	1.3	0.0	0.0	0.0	0.0	8.6	37.1	0.0	30.1			
Unsig. Movement Delay, s/veh	1.0	0.0	0.0	0.0	0.0	0.0	07.1	0.0	00.1			
LnGrp Delay(d),s/veh	76.8	0.0	0.0	0.0	0.0	49.5	29.8	0.0	23.8			
LnGrp LOS	E	A	A	A	A	D	C	A	C			
Approach Vol, veh/h		16			153			1995				
Approach Delay, s/veh		76.8			49.5			26.9				
Approach LOS		E			D			C				
					5							_
Timer - Assigned Phs		2		4				8				
Phs Duration (G+Y+Rc), s		95.0		10.0				35.0				
Change Period (Y+Rc), s		4.5		4.5				4.5				
Max Green Setting (Gmax), s		90.5		5.5				30.5				
Max Q Clear Time (g_c+I1), s		67.3		3.5				12.0				
Green Ext Time (p_c), s		16.3		0.0				0.8				
Intersection Summary												
HCM 6th Ctrl Delay			28.9									
HCM 6th LOS			С									

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Intersection				
Intersection Delay, s/veh	0			
Intersection LOS	-			

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			et.	t.	
Traffic Vol, veh/h	0	0	0	0	0	0
Future Vol, veh/h	0	0	0	0	0	0
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Heavy Vehicles, %	0	0	0	3	0	0
Mvmt Flow	0	0	0	0	0	0
Number of Lanes	1	0	0	1	1	0
Approach	EB			NB	SB	
Opposing Approach				SB	NB	
Opposing Lanes	0			1	1	
Conflicting Approach Left	SB			EB		
Conflicting Lanes Left	1			1	0	
Conflicting Approach Right	NB				EB	
Conflicting Lanes Right	1			0	1	
HCM Control Delay	0			0	0	
HCM LOS	-			-	-	

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	0%	0%	0%
Vol Thru, %	100%	100%	100%
Vol Right, %	0%	0%	0%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	0	0	0
LT Vol	0	0	0
Through Vol	0	0	0
RT Vol	0	0	0
Lane Flow Rate	0	0	0
Geometry Grp	1	1	1
Degree of Util (X)	0	0	0
Departure Headway (Hd)	3.951	3.9	3.9
Convergence, Y/N	Yes	Yes	Yes
Сар	0	0	0
Service Time	1.951	1.9	1.9
HCM Lane V/C Ratio	0	0	0
HCM Control Delay	7	6.9	6.9
HCM Lane LOS	N	Ν	Ν
HCM 95th-tile Q	0	0	0

HCM 6th Signalized Intersection Summary 8: Hansel Ave & Fairlane Ave

Projected PM P.M. Peak Hour

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			1		3	4 12				
Traffic Volume (veh/h)	10	0	0	0	0	389	16	1397	0	0	0	0
Future Volume (veh/h)	10	0	0	0	0	389	16	1397	0	0	0	0
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1752	1900	0	0	1900	1900	1811	1841	1900			
Adj Flow Rate, veh/h	11	0	0	0	0	414	17	1486	0			
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94			
Percent Heavy Veh, %	10	0	0	0	0	0	6	4	0			
Cap, veh/h	79	0	0	0	0	451	1102	2235	0			
Arrive On Green	0.28	0.00	0.00	0.00	0.00	0.28	0.64	0.64	0.00			
Sat Flow, veh/h	110	0	0	0	0	1610	1725	3589	0			
Grp Volume(v), veh/h	11	0	0	0	0	414	17	1486	0			
Grp Sat Flow(s),veh/h/ln	110	0	ů 0	0	0	1610	1725	1749	Õ			
Q Serve(\underline{q}_s), s	1.6	0.0	0.0	0.0	0.0	37.4	0.5	40.0	0.0			
Cycle Q Clear(g_c), s	39.0	0.0	0.0	0.0	0.0	37.4	0.5	40.0	0.0			
Prop In Lane	1.00	0.0	0.00	0.00	0.0	1.00	1.00	40.0	0.00			
Lane Grp Cap(c), veh/h	79	0	0.00	0.00	0	451	1102	2235	0.00			
V/C Ratio(X)	0.14	0.00	0.00	0.00	0.00	0.92	0.02	0.66	0.00			
Avail Cap(c_a), veh/h	129	0.00	0.00	0.00	0.00	529	1102	2235	0.00			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(I)	1.00	0.00	0.00	0.00	0.00	1.00	1.00	1.00	0.00			
Uniform Delay (d), s/veh	71.2	0.0	0.00	0.0	0.0	52.3	9.9	17.0	0.0			
Incr Delay (d2), s/veh	0.9	0.0	0.0	0.0	0.0	20.1	0.0	17.0	0.0			
Initial Q Delay(d3),s/veh	0.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(95%),veh/ln	0.0	0.0	0.0	0.0	0.0	24.6	0.0	22.1	0.0			
Unsig. Movement Delay, s/veh	0.0	0.0	0.0	0.0	0.0	24.0	0.4	22.1	0.0			
LnGrp Delay(d),s/veh	72.1	0.0	0.0	0.0	0.0	72.3	9.9	18.6	0.0			
LnGrp LOS	72.1 E	0.0 A	0.0 A	0.0 A	0.0 A	72.3 E	9.9 A	10.0 B	0.0 A			
	<u> </u>	11	A	A	414	E	A	1503	A			
Approach Vol, veh/h												
Approach Delay, s/veh		72.1			72.3			18.5				
Approach LOS		E			E			В				
Timer - Assigned Phs		2		4				8				
Phs Duration (G+Y+Rc), s		102.2		47.8				47.8				
Change Period (Y+Rc), s		6.4		* 5.7				* 5.7				
Max Green Setting (Gmax), s		88.6		* 50				* 49				
Max Q Clear Time (g_c+l1), s		42.0		41.0				39.4				
Green Ext Time (p_c), s		15.9		0.0				2.7				
Intersection Summary												
HCM 6th Ctrl Delay			30.4									
HCM 6th LOS			С									
Notes												

Notes
* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection												
Int Delay, s/veh	14.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1 6			a						at to	
Traffic Vol, veh/h	0		86	53	26	0	0	0	0	27	1677	69
Future Vol, veh/h	0	6	86	53	26	0	0	0	0	27	1677	69
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	0	0	5	0	0	0	0	0	0	0	7	0
Mvmt Flow	0	7	101	62	31	0	0	0	0	32	1973	81
Major/Minor	Minor			Minort						Majara		
Major/Minor	Minor2	0070		Minor1	0140					Major2		0
Conflicting Flow All	-	2078	1027	1054	2118	-				0	0	0
Stage 1	-	2078	-	0	0	-				-	-	-
Stage 2	-	0	-	1054	2118	-				-	-	-
Critical Hdwy	-	6.5	7	7.5	6.5	-				4.1	-	-
Critical Hdwy Stg 1	-	5.5	-	-	-	-				-	-	-
Critical Hdwy Stg 2	-	-	-	6.5	5.5	-				-	-	-
Follow-up Hdwy	-	4	3.35	3.5	4	-				2.2	-	-
Pot Cap-1 Maneuver	0	54	227	183	51	0				-	-	-
Stage 1	0	96	-	-	-	0				-	-	-
Stage 2	0	-	-	245	92	0				-	-	-
Platoon blocked, %											-	-
Mov Cap-1 Maneuver	-	54	227	91	51	-				-	-	-
Mov Cap-2 Maneuver	-	54	-	91	51	-				-	-	-
Stage 1	-	96	-	-	-	-				-	-	-
Stage 2	-	-	-	126	92	-				-	-	-
Approach	EB			WB						SB		
HCM Control Delay, s	47.3			\$ 303								
HCM LOS	E			F								
	L											
					0.0.7							
Minor Lane/Major Mvmt		EBLn1		SBL	SBT	SBR						
Capacity (veh/h)		188	72	-	-	-						
HCM Lane V/C Ratio		0.576	1.291	-	-	-						
HCM Control Delay (s)		47.3	\$ 303	-	-	-						
HCM Lane LOS		E	F	-	-	-						
HCM 95th %tile Q(veh)		3.1	7.4	-	-	-						
Notes												
	(¢. Do		ode 300a	t: Cor	noutation	Not Defin	vod *	All major	volume i	n nlatoon		
~: Volume exceeds capacity	y	ay exce	eds 300s	+. 001	nputation	Not Defin	ieu	: All major	volume l	n piatoon		

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			\$			\$	
Traffic Vol, veh/h	0	258	6	5	44	0	5	0	30	0	0	0
Future Vol, veh/h	0	258	6	5	44	0	5	0	30	0	0	0
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	0	4	0	0	2	0	0	2	0	0	0	0
Mvmt Flow	0	287	7	6	49	0	6	0	33	0	0	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach		EB		WB			NB				SB	
Opposing Approach		WB		EB			SB				NB	
Opposing Lanes		1		1			1				1	
Conflicting Approach Left		SB		NB			EB				WB	
Conflicting Lanes Left		1		1			1				1	
Conflicting Approach Right		NB		SB			WB				EB	
Conflicting Lanes Right		1		1			1				1	
HCM Control Delay		9.1		7.6			7.4				0	
HCMLOS		А		А			А				-	

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	14%	0%	10%	0%
Vol Thru, %	0%	98%	90%	100%
Vol Right, %	86%	2%	0%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	35	264	49	0
LT Vol	5	0	5	0
Through Vol	0	258	44	0
RT Vol	30	6	0	0
Lane Flow Rate	39	293	54	0
Geometry Grp	1	1	1	1
Degree of Util (X)	0.045	0.331	0.064	0
Departure Headway (Hd)	4.168	4.064	4.211	4.704
Convergence, Y/N	Yes	Yes	Yes	Yes
Сар	865	883	840	0
Service Time	2.168	2.098	2.291	2.706
HCM Lane V/C Ratio	0.045	0.332	0.064	0
HCM Control Delay	7.4	9.1	7.6	7.7
HCM Lane LOS	А	А	А	N
HCM 95th-tile Q	0.1	1.5	0.2	0

Intersection												
Int Delay, s/veh	49.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		el.			1.		7	14				
Traffic Vol, veh/h	14	140	0	0	166	46	30	1355	129	0	0	0
Future Vol, veh/h	14	140	0	0	166	46	30	1355	129	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	120	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	7	0	0	0	6	2	13	4	6	0	0	0
Mvmt Flow	15	147	0	0	175	48	32	1426	136	0	0	0
Major/Minor	Minor2			Minor1			Major1					
Conflicting Flow All	865	1626	-	-	1558	781	0	0	0			
Stage 1	0	0	-	-	1558	-	-	-	-			
Stage 2	865	1626	-	-	0	-	-	-	-			
Critical Hdwy	7.64	6.5	-	-	6.62	6.94	4.36	-	-			
Critical Hdwy Stg 1	-	-	-	-	5.62	-	-	-	-			
Critical Hdwy Stg 2	6.64	5.5	-	-	-	-	-	-	-			
Follow-up Hdwy	3.57	4	-	-	4.06	3.32	2.33	-	-			
Pot Cap-1 Maneuver	240	~ 103	0	0	~ 107	338	-	-	-			
Stage 1	-	-	0	0	~ 165	-	-	-	-			
Stage 2	305	162	0	0	-	-	-	-	-			
Platoon blocked, %		100			(05	000		-	-			
Mov Cap-1 Maneuver	-	~ 103	-	-	~ 107	338	-	-	-			
Mov Cap-2 Maneuver	-	~ 103	-	-	~ 107	-	-	-	-			
Stage 1	-	-	-	-	~ 165	-	-	-	-			
Stage 2	-	162	-	-	-	-	-	-	-			
Annroach	ED.						ND					
Approach	EB			WB \$437			NB					
HCM Control Delay, s												
HCM LOS	-			F								
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1	WBLn1						
Capacity (veh/h)		NDL		NDR	LDLIII	126						
HCM Lane V/C Ratio		-	-	-	-	1.771						
		-	-	-	-	\$ 437						
HCM Control Delay (s) HCM Lane LOS		-	-	-	-	ə 437 F						
HCM 25th %tile Q(veh)		-	-	-	-	۲ 17.1						
. ,		-	-	-	-	17.1						
Notes						_				_		_
~· Volume exceeds canacit	v \$.Do	lav ovcoc	de 300e	+ Co	moutation	Not Defi	ned *·	All major	volume i	n nlatoon		

-: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Lanes, Volumes, Timings 12: Orange Ave & Lancaster Rd

100 s TPD, Inc.

02/07/2023

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	٨	1	1	1	ţ	4	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø8
Lane Configurations	ሻሻ	1	7	**	**	1	
Traffic Volume (vph)	263	167	310	1280	1414	262	
Future Volume (vph)	263	167	310	1280	1414	262	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	225	0	300			350	
Storage Lanes	1	1	1			1	
Taper Length (ft)	25	150.1	25	0.474	00.40	4500	
Satd. Flow (prot)	3502	1524	1719	3471	3343	1568	
Flt Permitted	0.950	4504	0.050	0.474	0040	4500	
Satd. Flow (perm)	3502	1524	90	3471	3343	1568	
Right Turn on Red		Yes 13				Yes 56	
Satd. Flow (RTOR) Link Speed (mph)	40	13		45	45	00	
Link Opeed (nph)	1228			1363	902		
Travel Time (s)	20.9			20.7	13.7		
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	
Heavy Vehicles (%)	0.50	6%	5%	4%	8%	3%	
Shared Lane Traffic (%)	0,0	070	070	τ /υ	070	070	
Lane Group Flow (vph)	274	174	323	1333	1473	273	
Turn Type	Prot	pm+ov	pm+pt	NA	NA	pm+ov	
Protected Phases	3	1	1	6	2	3	8
Permitted Phases		3	6			2	
Total Split (s)	50.0	25.0	25.0	100.0	75.0	50.0	50.0
Total Lost Time (s)	6.8	7.1	7.1	6.8	6.8	6.8	
Act Effct Green (s)	17.3	63.7	118.8	119.1	72.4	96.5	
Actuated g/C Ratio	0.12	0.42	0.79	0.79	0.48	0.64	
v/c Ratio	0.68	0.27	0.64	0.48	0.91	0.27	
Control Delay	72.0	27.2	34.0	13.4	45.5	9.3	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	72.0	27.2	34.0	13.4	45.5	9.3	
LOS	E	С	С	B	D	А	
Approach Delay	54.6 D			17.4 В	39.9 D		
Approach LOS Queue Length 50th (ft)	135	105	234	342	656	78	
Queue Length 95th (ft)	177	153	350	638	#889	124	
Internal Link Dist (ft)	1148	100	000	1283	822	127	
Turn Bay Length (ft)	225		300	1200	022	350	
Base Capacity (vph)	1008	654	501	2755	1612	1289	
Starvation Cap Reductn	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.27	0.27	0.64	0.48	0.91	0.21	
Intersection Summary							
Area Type:	Other						
Cycle Length: 150	Other						
Actuated Cycle Length: 150							
Offset: 16 (11%), Referenced to	phase 2:SRT ar	nd 6:NBTI	Start of Gree	en			
Control Type: Actuated-Coordina		ia olitere,		511			
Maximum v/c Ratio: 0.91							
Intersection Signal Delay: 31.9				Inte	ersection L	OS: C	
Intersection Capacity Utilization 8	31.0%				J Level of S		
Analysis Period (min) 15							
# 95th percentile volume excee	eds capacity, qu	eue may be	longer.				
Queue shown is maximum aft		·	-				
Splits and Phases: 12: Orange	Ave & Lancasi	er Rd					
1 I I I I I I I I I I I I I I I I I I I	2224						\$ ~
3 Ø1	Ø2 (R)						🏅 Ø3
25 s 75	S						50 s
1 Ø6 (R)							AL28

Synchro 11 Report Page 1

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HCM 6th Signalized Intersection Summary 13: Orange Ave & Nela Ave

a.

	٠	-	7	1	-	*	1	1	1	1	Ŧ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
ane Configurations		1		3	1.		5	4 15		5	4 1	
Traffic Volume (veh/h)	0	4	1	30	1	90	4	1508	64	131	1510	
Future Volume (veh/h)	0	0	1	30	0	90	4	1508	64	131	1510	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00	-	1.00	1.00		1.00	1.00		1.00	1.00	-	1.0
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.0
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1841	1900	1841	1530	1841	1870	1870	1781	190
Adj Flow Rate, veh/h	0	0	1	31	0	93	4	1555	66	135	1557	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.9
Percent Heavy Veh, %	0.01	0.01	0.01	4	0.01	4	25	4	2	2	8	0.0
Cap, veh/h	0	0	114	146	0	114	255	2612	110	283	2922	
Arrive On Green	0.00	0.00	0.07	0.07	0.00	0.07	0.76	0.76	0.76	0.04	1.00	1.0
Sat Flow, veh/h	0.00	0.00	1610	1394	0.00	1610	270	3419	145	1781	3469	1.0
Grp Volume(v), veh/h	0	0	1010	31	0	93	4	793	828	135	760	79
	0	0	1610	1394	0	93 1610	270	1749	020 1815	1781	1692	178
Grp Sat Flow(s),veh/h/ln	0.0	0.0	0.1							2.4		0.
Q Serve(g_s), s				3.2	0.0	8.5	0.5	29.4	29.7		0.0	
Cycle Q Clear(g_c), s	0.0	0.0	0.1	3.3	0.0	8.5	0.5	29.4	29.7	2.4	0.0	0.
Prop In Lane	0.00	0	1.00	1.00	0	1.00	1.00	4000	0.08	1.00	4400	0.0
Lane Grp Cap(c), veh/h	0	0	114	146	0	114	255	1336	1386	283	1426	150
V/C Ratio(X)	0.00	0.00	0.01	0.21	0.00	0.82	0.02	0.59	0.60	0.48	0.53	0.5
Avail Cap(c_a), veh/h	0	0	148	177	0	150	255	1336	1386	464	1426	150
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.33	1.33	1.3
Upstream Filter(I)	0.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	0.42	0.42	0.4
Uniform Delay (d), s/veh	0.0	0.0	64.8	66.3	0.0	68.7	4.2	7.6	7.7	8.7	0.0	0.
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.7	0.0	22.1	0.1	1.9	1.9	0.5	0.6	0.
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.
%ile BackOfQ(95%),veh/ln	0.0	0.0	0.1	2.1	0.0	7.6	0.1	15.0	15.6	2.3	0.4	0.
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	0.0	0.0	64.8	67.0	0.0	90.8	4.4	9.6	9.6	9.2	0.6	0.
LnGrp LOS	A	A	E	E	A	F	A	A	A	A	A	
Approach Vol, veh/h		1			124			1625			1694	
Approach Delay, s/veh		64.8			84.9			9.6			1.3	
Approach LOS		E			F			А			А	
Timer - Assigned Phs	1	2		4		6		8				
Phs Duration (G+Y+Rc), s	11.8	121.4		16.8		133.2		16.8				
Change Period (Y+Rc), s	6.8	6.8		* 6.2		6.8		* 6.2				
Max Green Setting (Gmax), s	20.2	96.2		* 14		123.2		* 14				
Max Q Clear Time (g_c+l1), s	4.4	31.7		2.1		2.0		10.5				
Green Ext Time (p_c), s	0.3	17.5		0.0		16.4		0.1				
Intersection Summary												
HCM 6th Ctrl Delay			8.2									
HCM 6th LOS			A									
Notes												

User approved pedestrian interval to be less than phase max green. * HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

APPENDIX H

Queueing Data Collection

Prepared by National Data & Surveying Services

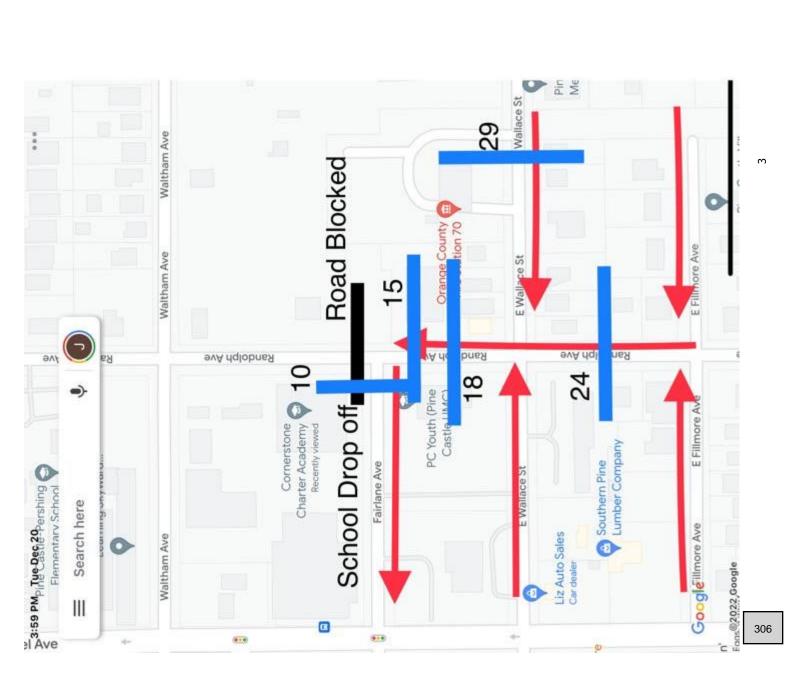
Max Queue Study

Location: Cornerstone Charter Academy , 5903 Randolph Ave City: Belle Isle, FL

12/6/2022	Tuesday
Date:	Day:

i	Drop-off Max Queue Length (# of Cars)	e Length (# of Cars)
Time	High School	Lower School K-5
7:00 AM	2	2
7:05 AM	З	1
7:10 AM	9	3
7:15 AM	4	7
7:20 AM	16	6
7:25 AM	17	10
7:30 AM	17	4
7:35 AM	26	0
7:40 AM	24	5
7:45 AM	17	3
7:50 AM	2	7
7:55 AM	1	8
8:00 AM	0	0
8:05 AM	0	0
8:10 AM	0	4
8:15 AM	0	2
8:20 AM	0	0
8:25 AM	0	0
Totals	135	65

Time	Pick-up Max Queue Length (# of Cars) High School K-	: Length (# of Cars) Lower School K-5
2:00 PM	0	7
2:05 PM	0	10
2:10 PM	8	14
2:15 PM	8	17
2:20 PM	8	19
2:25 PM	10	19
2:30 PM	10	15
2:35 PM	10	16
2:40 PM	11	5
2:45 PM	18	15
2:50 PM	24	19
2:55 PM	29	16
3:00 PM	34	1
3:05 PM	15	3
3:10 PM	5	5
3:15 PM	7	3
3:20 PM	9	5
3:25 PM	0	0
Totals	203	189



High School PM 2-3:30

2pm-2:25 No Line 2:25 10 Cars 2:30 10 Cars 2:35 10 Cars 2:40 11 Cars 2:45 18 Cars 2:45 18 Cars 2:50 24 Cars 2:55 29 Cars 3:00 34 Cars 3:05 15 Cars 3:05 15 Cars 3:05 15 Cars

Hoffr Earthscape Orlai Landscaping, Irri Marinell Dr Marinell Dr Wilks Ave Waltham Ave Marinell Dr **Orlando Seawall Services** 528A Waltham Ave Wilks Ave 0 Foy St Waltham Ave Wilks Ave 15 Ð Rand **Bandolph Ave** evA rdolobnes evA ridiobnes Xtreme Sight Fishing -Super Awesome Cool Pottery Hoffner Ave 😨 Cornerstone Charter Academy Recently viewed OCPS Teaching and Learning Skyward.. Fairlane Ave Pine Castle-Pershing Elementary School Search here 3:42 PM Tue Dec 20 Waltham Av ©2022 Google Google III 307 œ Hancol Ave 4

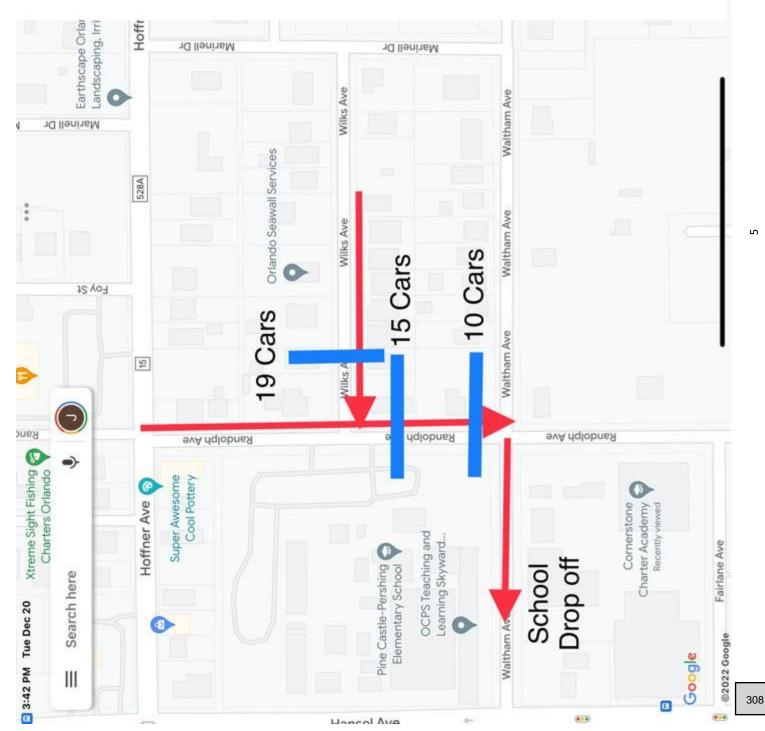
Lower K-5 AM 7-830

7:00-7:20 = No line

7:25 = 10 cars Blue Line 7:30-8:30 = No line

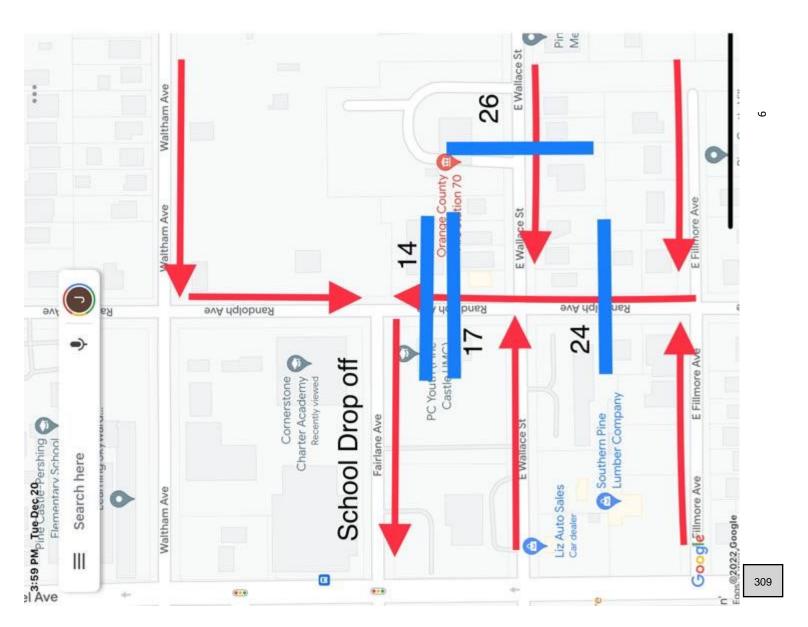
Lower K-5 PM 2-3:30

2:00 No Line 2:05 10 Cars 2:15 17 Cars 2:15 17 Cars 2:20 19 Cars 2:25 19 Cars 2:30 15 Cars 2:35 16 Cars 2:45 15 Cars 2:45 15 Cars 2:45 15 Cars 2:55 16 Cars





7:25 15 No Line 7:20 16 Cars 7:25 17 Cars 7:30 17 Cars 7:35 26 Cars 7:40 24 Cars 7:45 17 Cars 7:45 17 Cars 7:45 17 Cars



a.

APPENDIX I

SimTraffic Microsimulation Output

Intersection: 1: Randolph Ave & Hoffner Ave

Movement	EB	WB	NB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	271	708	212
Average Queue (ft)	45	460	91
95th Queue (ft)	267	831	255
Link Distance (ft)	502	698	358
Upstream Blk Time (%)	4	13	4
Queuing Penalty (veh)	35	100	2
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 2: Hansel Ave & Hoffner Ave

Movement	EB	WB	WB	NB	NB	NB
Directions Served	LT	Т	R	LT	Т	R
Maximum Queue (ft)	532	492	250	577	585	345
Average Queue (ft)	269	246	144	408	426	326
95th Queue (ft)	461	491	297	540	573	408
Link Distance (ft)	642	502		653	653	
Upstream Blk Time (%)	1	1		0	2	
Queuing Penalty (veh)	5	8		1	22	
Storage Bay Dist (ft)			225			320
Storage Blk Time (%)		12	1		17	10
Queuing Penalty (veh)		33	3		121	75

Intersection: 3: Hoffner Ave & Orange Ave

Movement	WB	SB	SB	SB
Directions Served	L	L	Т	Т
Maximum Queue (ft)	648	364	470	490
Average Queue (ft)	382	128	275	263
95th Queue (ft)	676	250	427	422
Link Distance (ft)	642		554	554
Upstream Blk Time (%)	1		0	0
Queuing Penalty (veh)	5		0	0
Storage Bay Dist (ft)		460		
Storage Blk Time (%)		0	0	
Queuing Penalty (veh)		1	1	

Intersection: 4: Randolph Ave & Wilks Ave

Movement	WB	NB	SB
Directions Served	LR	TR	LT
Maximum Queue (ft)	53	30	228
Average Queue (ft)	14	8	58
95th Queue (ft)	53	74	258
Link Distance (ft)	482	249	358
Upstream Blk Time (%)		2	7
Queuing Penalty (veh)		4	17
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 5: Randolph Ave & Waltham Ave

Movement	EB	WB	SB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	87	33	199
Average Queue (ft)	41	9	94
95th Queue (ft)	107	32	247
Link Distance (ft)	337	236	249
Upstream Blk Time (%)	1		17
Queuing Penalty (veh)	1		43
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 6: Hansel Ave & Waltham Ave

Movement	EB	WB	NB	NB
Directions Served	LT	TR	LT	TR
Maximum Queue (ft)	76	118	399	384
Average Queue (ft)	24	93	366	369
95th Queue (ft)	62	131	404	393
Link Distance (ft)		115	362	362
Upstream Blk Time (%)		6	6	9
Queuing Penalty (veh)		13	63	86
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 8: Hansel Ave & Fairlane Ave

Movement	EB	WB	NB	NB	NB
Directions Served	LT	TR	L	Т	TR
Maximum Queue (ft)	106	307	174	284	291
Average Queue (ft)	32	298	28	249	249
95th Queue (ft)	91	348	129	281	281
Link Distance (ft)	339	292		241	241
Upstream Blk Time (%)		48		34	35
Queuing Penalty (veh)		0		229	236
Storage Bay Dist (ft)			150		
Storage Blk Time (%)				56	
Queuing Penalty (veh)				8	

Intersection: 9: Fairlane Ave & Orange Ave

Movement	EB	WB	SB	SB
Directions Served	TR	LT	LT	TR
Maximum Queue (ft)	248	236	81	74
Average Queue (ft)	114	182	8	6
95th Queue (ft)	275	241	70	59
Link Distance (ft)	413	339		
Upstream Blk Time (%)	3			
Queuing Penalty (veh)	0			
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 10: Randolph Ave & Wallace St

Movement	EB	WB	NB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	300	146	91
Average Queue (ft)	114	49	33
95th Queue (ft)	338	143	73
Link Distance (ft)	471	478	1060
Upstream Blk Time (%)	5		
Queuing Penalty (veh)	14		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 11: Wallace St & Hansel Ave

Movement	EB	WB	NB	NB	NB
Directions Served	LT	TR	L	Т	TR
Maximum Queue (ft)	242	450	144	627	636
Average Queue (ft)	105	201	12	429	435
95th Queue (ft)	215	467	77	772	793
Link Distance (ft)	254	471			
Upstream Blk Time (%)	4	10			
Queuing Penalty (veh)	6	10			
Storage Bay Dist (ft)			120		
Storage Blk Time (%)				47	
Queuing Penalty (veh)				8	

Intersection: 12: Orange Ave & Lancaster Rd

Movement	EB	EB	EB	NB	NB	NB	SB	SB	SB	
Directions Served	L	L	R	L	Т	Т	Т	Т	R	
Maximum Queue (ft)	145	186	186	291	300	294	450	430	216	
Average Queue (ft)	57	102	81	157	134	132	349	259	56	
95th Queue (ft)	136	170	159	257	247	247	491	406	166	
Link Distance (ft)		1166	1166		1285	1285				
Upstream Blk Time (%)										
Queuing Penalty (veh)										
Storage Bay Dist (ft)	225			300					350	
Storage Blk Time (%)		0		1	0			1	0	
Queuing Penalty (veh)		0		6	0			3	0	

Intersection: 13: Orange Ave & Nela Ave

Movement	EB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	LTR	L	TR	L	Т	TR	L	Т	TR
Maximum Queue (ft)	11	112	169	17	304	308	149	230	240
Average Queue (ft)	0	29	67	1	148	121	51	71	83
95th Queue (ft)	5	75	132	10	277	256	109	185	198
Link Distance (ft)	446		1119		1312	1312		1285	1285
Upstream Blk Time (%)									
Queuing Penalty (veh)									
Storage Bay Dist (ft)		145		200			185		
Storage Blk Time (%)			1		3			0	
Queuing Penalty (veh)			0		0			0	

Intersection: 19: Entrance #1

Movement	NB	SB
Directions Served	L	R
Maximum Queue (ft)	62	71
Average Queue (ft)	34	51
95th Queue (ft)	48	70
Link Distance (ft)	19	4
Upstream Blk Time (%)	76	70
Queuing Penalty (veh)	158	98
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 20:

Movement	EB	B21	B22	B23	B24	B25	B26
Directions Served	L	Т	Т	Т	Т	Т	Т
Maximum Queue (ft)	169	78	230	93	219	71	319
Average Queue (ft)	135	48	159	51	125	29	151
95th Queue (ft)	185	87	300	108	278	74	391
Link Distance (ft)		9	158	23	150	2	251
Upstream Blk Time (%)	83	42	70	66	56	5	45
Queuing Penalty (veh)	0	87	145	136	116	11	92
Storage Bay Dist (ft)							
Storage Blk Time (%)							
Queuing Penalty (veh)							

Intersection: 28: Wallace St & Entrance #2

Movement	EB	WB
Directions Served	LT	TR
Maximum Queue (ft)	410	80
Average Queue (ft)	129	34
95th Queue (ft)	409	62
Link Distance (ft)	478	328
Upstream Blk Time (%)	11	
Queuing Penalty (veh)	46	
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 29: Entrance #1 & Waltham Ave

Movement	EB	WB	B15
Directions Served	TR	LT	Т
Maximum Queue (ft)	230	258	23
Average Queue (ft)	136	123	5
95th Queue (ft)	291	325	57
Link Distance (ft)	236	456	681
Upstream Blk Time (%)	31	5	
Queuing Penalty (veh)	38	4	
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 31: E.S. Exit & Waltham Ave

Movement	NB
Directions Served	LR
Maximum Queue (ft)	78
Average Queue (ft)	34
95th Queue (ft)	52
Link Distance (ft)	13
Upstream Blk Time (%)	16
Queuing Penalty (veh)	54
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 34: Orange Ave

Movement	
Directions Served	
Maximum Queue (ft)	
Average Queue (ft)	
95th Queue (ft)	
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 38:

Movement	WB	SB	SB
Directions Served	L	LT	Т
Maximum Queue (ft)	78	116	114
Average Queue (ft)	20	14	11
95th Queue (ft)	55	107	99
Link Distance (ft)	254	261	261
Upstream Blk Time (%)		1	0
Queuing Penalty (veh)		14	3
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 40: Orange Ave & Waltham Ave

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 53: E.S. Exit

Movement	NB	B30	B14	B16	B51	B47	B17
Directions Served	Т	Т	Т	Т	Т	Т	Т
Maximum Queue (ft)	166	410	272	97	93	138	360
Average Queue (ft)	135	374	229	61	57	96	298
95th Queue (ft)	150	422	310	90	87	143	456
Link Distance (ft)	82	326	188	13	10	52	280
Upstream Blk Time (%)	100	95	93	46	33	91	87
Queuing Penalty (veh)	345	329	320	160	113	313	300
Storage Bay Dist (ft)							
Storage Blk Time (%)							
Queuing Penalty (veh)							

Intersection: 55: Hoffner Ave

Movement	EB	WB
Directions Served	TR	LT
Maximum Queue (ft)	153	308
Average Queue (ft)	13	135
95th Queue (ft)	174	324
Link Distance (ft)	698	282
Upstream Blk Time (%)		17
Queuing Penalty (veh)		0
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 4039

Intersection: 1: Randolph Ave & Hoffner Ave

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	248	676	227	43
Average Queue (ft)	20	320	84	10
95th Queue (ft)	136	700	226	37
Link Distance (ft)	498	701	341	302
Upstream Blk Time (%)	0	3	2	
Queuing Penalty (veh)	1	21	1	
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 2: Hansel Ave & Hoffner Ave

Movement	EB	WB	WB	NB	NB	NB
Directions Served	LT	Т	R	LT	Т	R
Maximum Queue (ft)	447	497	250	670	664	345
Average Queue (ft)	217	302	166	378	390	269
95th Queue (ft)	378	550	310	693	730	442
Link Distance (ft)	647	498		652	652	
Upstream Blk Time (%)	0	6		1	1	
Queuing Penalty (veh)	0	36		6	7	
Storage Bay Dist (ft)			225			320
Storage Blk Time (%)		20	2		8	6
Queuing Penalty (veh)		48	6		62	44

Intersection: 3: Hoffner Ave & Orange Ave

Movement	WB	SB	SB	SB
Directions Served	L	L	Т	Т
Maximum Queue (ft)	659	258	444	462
Average Queue (ft)	529	105	258	250
95th Queue (ft)	737	205	396	390
Link Distance (ft)	647		553	553
Upstream Blk Time (%)	14		0	0
Queuing Penalty (veh)	67		0	0
Storage Bay Dist (ft)		460		
Storage Blk Time (%)			0	
Queuing Penalty (veh)			1	

Intersection: 4: Randolph Ave & Wilks Ave

Movement	WB	NB	SB
Directions Served	LR	TR	LT
Maximum Queue (ft)	33	29	76
Average Queue (ft)	5	2	12
95th Queue (ft)	25	20	94
Link Distance (ft)	481	263	341
Upstream Blk Time (%)			0
Queuing Penalty (veh)			0
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 5: Randolph Ave & Waltham Ave

Movement	EB	WB	SB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	83	30	126
Average Queue (ft)	35	6	63
95th Queue (ft)	69	26	180
Link Distance (ft)	350	221	263
Upstream Blk Time (%)			5
Queuing Penalty (veh)			7
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 6: Hansel Ave & Waltham Ave

Movement	EB	WB	NB	NB
Directions Served	LT	TR	LT	TR
Maximum Queue (ft)	69	87	384	391
Average Queue (ft)	16	53	349	364
95th Queue (ft)	52	86	422	412
Link Distance (ft)		115	363	363
Upstream Blk Time (%)		0	9	13
Queuing Penalty (veh)		0	88	129
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 8: Hansel Ave & Fairlane Ave

Movement	EB	WB	NB	NB	NB
Directions Served	LT	TR	L	Т	TR
Maximum Queue (ft)	56	322	148	260	259
Average Queue (ft)	13	289	16	232	233
95th Queue (ft)	42	380	93	282	285
Link Distance (ft)	334	307		230	230
Upstream Blk Time (%)		40		26	26
Queuing Penalty (veh)		0		191	194
Storage Bay Dist (ft)			150		
Storage Blk Time (%)			0	40	
Queuing Penalty (veh)			0	7	

Intersection: 9: Fairlane Ave & Orange Ave

Movement	EB	WB	SB
Directions Served	TR	LT	TR
Maximum Queue (ft)	179	197	15
Average Queue (ft)	69	119	1
95th Queue (ft)	140	219	9
Link Distance (ft)	407	334	
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 10: Randolph Ave & Wallace St

Movement	EB	WB	NB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	232	104	64
Average Queue (ft)	66	34	25
95th Queue (ft)	192	92	54
Link Distance (ft)	477	482	1060
Upstream Blk Time (%)	1		
Queuing Penalty (veh)	3		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 11: Wallace St & Hansel Ave

Movement	EB	WB	NB	NB	NB
Directions Served	LT	TR	L	Т	TR
Maximum Queue (ft)	152	439	144	626	631
Average Queue (ft)	134	323	14	298	314
95th Queue (ft)	151	442	86	667	689
Link Distance (ft)		477			
Upstream Blk Time (%)		8			
Queuing Penalty (veh)		4			
Storage Bay Dist (ft)			120		
Storage Blk Time (%)			0	25	
Queuing Penalty (veh)			0	8	

Intersection: 12: Orange Ave & Lancaster Rd

Movement	EB	EB	EB	NB	NB	NB	SB	SB	SB
Directions Served	L	L	R	L	Т	Т	Т	Т	R
Maximum Queue (ft)	184	221	206	324	450	455	457	452	375
Average Queue (ft)	101	137	75	217	213	216	412	375	149
95th Queue (ft)	174	196	165	332	370	368	501	513	407
Link Distance (ft)		1166	1166		1285	1285			
Upstream Blk Time (%)									
Queuing Penalty (veh)									
Storage Bay Dist (ft)	225			300					350
Storage Blk Time (%)	0	0		3	1			15	0
Queuing Penalty (veh)	0	0		19	3			41	0

Intersection: 13: Orange Ave & Nela Ave

Movement	EB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	LTR	L	TR	L	Т	TR	L	Т	TR
Maximum Queue (ft)	11	85	114	49	319	290	179	192	193
Average Queue (ft)	1	27	40	3	127	98	87	22	29
95th Queue (ft)	6	66	84	27	274	240	159	109	110
Link Distance (ft)	446		1119		1312	1312		1285	1285
Upstream Blk Time (%)									
Queuing Penalty (veh)									
Storage Bay Dist (ft)		145		200			185		
Storage Blk Time (%)			0		3		1	0	
Queuing Penalty (veh)			0		0		7	0	

Intersection: 14: ES Exit & Waltham Ave

Movement	NB
Directions Served	LR
Maximum Queue (ft)	55
Average Queue (ft)	32
95th Queue (ft)	42
Link Distance (ft)	15
Upstream Blk Time (%)	8
Queuing Penalty (veh)	19
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 17: Hoffner Ave

Movement	EB	WB
Directions Served	TR	LT
Maximum Queue (ft)	2	417
Average Queue (ft)	0	128
95th Queue (ft)	2	367
Link Distance (ft)	701	550
Upstream Blk Time (%)		4
Queuing Penalty (veh)		0
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 19: Entance #1

Movement	NB	SB
Directions Served	L	R
Maximum Queue (ft)	67	70
Average Queue (ft)	34	48
95th Queue (ft)	52	69
Link Distance (ft)	9	5
Upstream Blk Time (%)	84	71
Queuing Penalty (veh)	214	71
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 20:

Movement	EB	B21	B22	B23	B24	B25	B26
Directions Served	L	Т	Т	Т	Т	Т	Т
Maximum Queue (ft)	148	86	200	83	205	61	266
Average Queue (ft)	117	50	126	42	106	23	97
95th Queue (ft)	160	95	267	103	276	69	317
Link Distance (ft)		17	145	23	170	10	257
Upstream Blk Time (%)	84	16	59	54	43	22	26
Queuing Penalty (veh)	0	24	88	81	64	33	38
Storage Bay Dist (ft)							
Storage Blk Time (%)							
Queuing Penalty (veh)							

Intersection: 28: Wallace St & Entrance #2

Movement	EB	WB
Directions Served	LT	TR
Maximum Queue (ft)	336	87
Average Queue (ft)	79	31
95th Queue (ft)	290	66
Link Distance (ft)	482	328
Upstream Blk Time (%)	6	
Queuing Penalty (veh)	18	
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 29: Entance #1 & Waltham Ave

Movement	EB	WB
Directions Served	TR	LT
Maximum Queue (ft)	208	173
Average Queue (ft)	105	77
95th Queue (ft)	237	186
Link Distance (ft)	221	438
Upstream Blk Time (%)	18	
Queuing Penalty (veh)	20	
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 34: Orange Ave

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 38:

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Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 40: Orange Ave & Waltham Ave

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euing Penalty (veh)	

Intersection: 53: ES Exit & ES Drop off

Movement	NB	B30	B31	B16	B51	B54	B55
Directions Served	Т	Т	Т	Т	Т	Т	Т
Maximum Queue (ft)	169	413	240	90	95	110	314
Average Queue (ft)	139	385	210	64	68	81	269
95th Queue (ft)	153	406	268	91	97	118	395
Link Distance (ft)	86	334	165	13	17	34	240
Upstream Blk Time (%)	96	97	95	67	69	92	89
Queuing Penalty (veh)	408	411	401	283	291	389	378
Storage Bay Dist (ft)							
Storage Blk Time (%)							
Queuing Penalty (veh)							

Network Summary

Network wide Queuing Penalty: 4231

Intersection: 1: Randolph Ave & Hoffner Ave

Movement	EB	WB	NB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	22	501	119
Average Queue (ft)	1	192	43
95th Queue (ft)	10	427	108
Link Distance (ft)	504	696	357
Upstream Blk Time (%)		0	
Queuing Penalty (veh)		1	
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 2: Hansel Ave & Hoffner Ave

••						
Movement	EB	WB	WB	NB	NB	NB
Directions Served	LT	Т	R	LT	Т	R
Maximum Queue (ft)	414	515	250	596	627	345
Average Queue (ft)	238	296	180	379	375	272
95th Queue (ft)	368	525	313	579	615	425
Link Distance (ft)	664	504		659	659	
Upstream Blk Time (%)		3		0	0	
Queuing Penalty (veh)		18		2	3	
Storage Bay Dist (ft)			225			320
Storage Blk Time (%)		19	1		10	3
Queuing Penalty (veh)		49	3		68	24

Intersection: 3: Orange Ave & Hoffner Ave

Movement	WB	SB	SB	SB
Directions Served	L	L	Т	Т
Maximum Queue (ft)	675	265	441	452
Average Queue (ft)	467	123	263	250
95th Queue (ft)	751	231	406	395
Link Distance (ft)	664		558	558
Upstream Blk Time (%)	1		0	0
Queuing Penalty (veh)	8		0	0
Storage Bay Dist (ft)		460		
Storage Blk Time (%)			0	
Queuing Penalty (veh)			0	

Intersection: 4: Randolph Ave & Wilks Ave

Movement	WB
Directions Served	LR
Maximum Queue (ft)	34
Average Queue (ft)	8
95th Queue (ft)	31
Link Distance (ft)	544
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Randolph Ave & Waltham Ave

Movement	EB	WB	SB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	58	31	52
Average Queue (ft)	35	7	32
95th Queue (ft)	55	27	45
Link Distance (ft)	509	222	257
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 6: Hansel Ave & Waltham Ave

Movement	EB	WB	NB	NB
Directions Served	LT	TR	LT	TR
Maximum Queue (ft)	114	276	136	149
Average Queue (ft)	37	263	15	17
95th Queue (ft)	104	283	99	106
Link Distance (ft)			357	357
Upstream Blk Time (%)			0	0
Queuing Penalty (veh)			0	0
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 7: Orange Ave

Movement	WB	SB
Directions Served	L	L
Maximum Queue (ft)	70	6
Average Queue (ft)	25	0
95th Queue (ft)	64	4
Link Distance (ft)	260	
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		290
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 8: Hansel Ave & Fairlane Ave

Movement	EB	WB	B14	B16	B17	B18	NB	NB	NB
Directions Served	L	TR	Т	Т	Т	Т	L	Т	Т
Maximum Queue (ft)	118	583	196	109	258	98	102	256	259
Average Queue (ft)	32	461	88	38	89	29	9	216	196
95th Queue (ft)	108	676	222	107	278	102	57	285	280
Link Distance (ft)	341	482	111	6	197	87		239	239
Upstream Blk Time (%)	0	41	34	6	20	5		5	3
Queuing Penalty (veh)	0	190	158	29	92	23		34	20
Storage Bay Dist (ft)							150		
Storage Blk Time (%)								17	
Queuing Penalty (veh)								2	

Intersection: 9: Orange Ave & Fairlane Ave

Movement	EB	WB	SB	SB
Directions Served	TR	LT	LT	TR
Maximum Queue (ft)	127	189	16	10
Average Queue (ft)	50	90	1	0
95th Queue (ft)	99	173	14	6
Link Distance (ft)	595	341		
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 10: Randolph Ave & Wallace St

Movement	EB	WB	NB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	55	58	66
Average Queue (ft)	36	30	23
95th Queue (ft)	52	50	55
Link Distance (ft)	476		1059
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 11: Hansel Ave & Wallace St

Movement	EB	WB	NB	NB	NB
Directions Served	LT	TR	L	T	TR
Maximum Queue (ft)	222	98	32	264	244
Average Queue (ft)	115	37	1	53	42
95th Queue (ft)	207	75	21	171	155
Link Distance (ft)	260	476			
Upstream Blk Time (%)	0				
Queuing Penalty (veh)	0				
Storage Bay Dist (ft)			120		
Storage Blk Time (%)				2	
Queuing Penalty (veh)				0	

Intersection: 12: Orange Ave & Lancaster Rd

Movement	EB	EB	EB	NB	NB	NB	SB	SB	SB	
Directions Served	L	L	R	L	Т	Т	Т	Т	R	
Maximum Queue (ft)	143	167	166	285	249	273	394	403	347	
Average Queue (ft)	50	97	65	145	114	116	238	251	63	
95th Queue (ft)	124	155	135	242	211	225	349	361	179	
Link Distance (ft)		1220	1220		1281	1281	901	901		
Upstream Blk Time (%)										
Queuing Penalty (veh)										
Storage Bay Dist (ft)	245			350					350	
Storage Blk Time (%)				0				1	0	
Queuing Penalty (veh)				0				2	0	

Intersection: 13: Orange Ave & Nela Ave

Movement	EB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	LTR	L	TR	L	Т	TR	L	Т	TR
Maximum Queue (ft)	10	81	153	40	301	274	114	200	239
Average Queue (ft)	1	26	61	2	122	85	44	59	76
95th Queue (ft)	6	65	119	25	254	209	92	161	188
Link Distance (ft)	477		1265		937	937		1281	1281
Upstream Blk Time (%)									
Queuing Penalty (veh)									
Storage Bay Dist (ft)		145		200			185		
Storage Blk Time (%)			0		2		0	0	
Queuing Penalty (veh)			0		0		0	0	

Intersection: 19:

Movement	NB	SB
Directions Served	L	R
Maximum Queue (ft)	97	89
Average Queue (ft)	39	49
95th Queue (ft)	66	74
Link Distance (ft)	1	5
Upstream Blk Time (%)	40	11
Queuing Penalty (veh)	113	21
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 20:

Movement	EB	B21	B22
Directions Served	L	Т	Т
Maximum Queue (ft)	126	39	23
Average Queue (ft)	88	3	1
95th Queue (ft)	123	23	19
Link Distance (ft)		18	145
Upstream Blk Time (%)	21	1	0
Queuing Penalty (veh)	0	2	0
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 28: Wallace St

Movement	EB	WB
Directions Served	LT	TR
Maximum Queue (ft)	84	31
Average Queue (ft)	38	11
95th Queue (ft)	60	35
Link Distance (ft)	468	346
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 29: Waltham Ave

Movement	EB	WB
Directions Served	TR	LT
Maximum Queue (ft)	74	74
Average Queue (ft)	35	37
95th Queue (ft)	58	61
Link Distance (ft)	222	450
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 30: Orange Ave/Hansel Ave

Movement	SE	SE
Directions Served	R	R
Maximum Queue (ft)	19	54
Average Queue (ft)	1	3
95th Queue (ft)	14	23
Link Distance (ft)	1109	1109
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 37: Hoffner Ave

Movement	EB	WB
Directions Served	TR	LT
Maximum Queue (ft)	2	256
Average Queue (ft)	0	67
95th Queue (ft)	2	178
Link Distance (ft)	696	526
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 38: Orange Ave & Waltham Ave

Movement	
Directions Served	
Maximum Queue (ft)	
Average Queue (ft)	
95th Queue (ft)	
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Network Summary

Network wide Queuing Penalty: 862

Intersection: 4: Randolph Ave & Hoffner Ave

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	359	722	316	48
Average Queue (ft)	28	407	142	10
95th Queue (ft)	188	850	338	35
Link Distance (ft)	494	710	351	355
Upstream Blk Time (%)	1	10	13	
Queuing Penalty (veh)	5	65	8	
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 5: Hansel Ave & Wallace St

Movement	EB	WB	NB	NB	NB
MOVEMENT	LD	VVD	ND	ND	ND
Directions Served	LT	TR	L	Т	TR
Maximum Queue (ft)	268	387	51	514	559
Average Queue (ft)	186	303	2	154	180
95th Queue (ft)	267	382	30	368	414
Link Distance (ft)	262	497			
Upstream Blk Time (%)	5				
Queuing Penalty (veh)	4				
Storage Bay Dist (ft)			120		
Storage Blk Time (%)				10	
Queuing Penalty (veh)				3	

Intersection: 6: Randolph Ave & Wallace St

Movement	EB	WB	NB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	88	44	42
Average Queue (ft)	46	27	22
95th Queue (ft)	71	46	46
Link Distance (ft)	497	453	1059
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 7: Hoffner Ave

Movement	WB
Directions Served	LT
Maximum Queue (ft)	426
Average Queue (ft)	181
95th Queue (ft)	442
Link Distance (ft)	410
Upstream Blk Time (%)	14
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 9: Hansel Ave & Waltham Ave

Movement	EB	WB	NB	NB
Directions Served	LT	TR		TR
			L I	
Maximum Queue (ft)	139	279	318	324
Average Queue (ft)	57	252	75	85
95th Queue (ft)	150	319	266	279
Link Distance (ft)			356	356
Upstream Blk Time (%)			0	1
Queuing Penalty (veh)			4	5
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 10: Randolph Ave & Waltham Ave

Movement	EB	WB	SB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	126	39	57
Average Queue (ft)	45	9	32
95th Queue (ft)	121	34	48
Link Distance (ft)	509	222	257
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 12: Orange Ave & Waltham Ave

Movement	WB
Directions Served	L
Maximum Queue (ft)	46
Average Queue (ft)	22
95th Queue (ft)	40
Link Distance (ft)	438
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 13: Hansel Ave & Fairlane Ave

Movement	EB	WB	B14	B16	B17	B18	NB	NB	NB
Directions Served	L	TR	Т	Т	Т	Т	L	Т	Т
Maximum Queue (ft)	70	593	162	199	158	93	174	270	276
Average Queue (ft)	14	496	78	80	46	21	16	244	244
95th Queue (ft)	48	688	193	222	158	85	86	289	296
Link Distance (ft)	328	499	90	109	104	86		246	246
Upstream Blk Time (%)		39	32	24	15	4		15	16
Queuing Penalty (veh)		167	136	103	62	19		111	121
Storage Bay Dist (ft)							150		
Storage Blk Time (%)							0	30	
Queuing Penalty (veh)							0	5	

Intersection: 15: Orange Ave & Fairlane Ave

Movement	EB	WB	SB	SB
Directions Served	TR	LT	LT	TR
Maximum Queue (ft)	155	188	5	12
Average Queue (ft)	72	115	0	0
95th Queue (ft)	136	212	5	6
Link Distance (ft)	595	328	382	382
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Movement	NB	SB
Directions Served	L	R
Maximum Queue (ft)	91	86
Average Queue (ft)	39	48
95th Queue (ft)	66	74
Link Distance (ft)	10	5
Upstream Blk Time (%)	37	10
Queuing Penalty (veh)	93	17
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 20:

Movement	EB	B21	B22
Directions Served	L	Т	Т
Maximum Queue (ft)	128	36	23
Average Queue (ft)	84	4	1
95th Queue (ft)	123	26	21
Link Distance (ft)		17	145
Upstream Blk Time (%)	15	1	0
Queuing Penalty (veh)	0	2	0
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 28: Wallace St

Movement	EB	WB
Directions Served	LT	TR
Maximum Queue (ft)	70	60
Average Queue (ft)	39	29
95th Queue (ft)	59	50
Link Distance (ft)	453	347
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 29: Waltham Ave

Movement	EB	WB
Directions Served	TR	LT
Maximum Queue (ft)	80	75
Average Queue (ft)	35	35
95th Queue (ft)	62	61
Link Distance (ft)	222	445
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 30: Hansel Ave & Hoffner Ave

Movement	EB	WB	WB	NB	NB	NB
Directions Served	LT	Т	R	LT	Т	R
Maximum Queue (ft)	456	508	250	679	679	345
Average Queue (ft)	231	331	171	499	516	307
95th Queue (ft)	422	605	322	781	812	436
Link Distance (ft)	637	494		659	659	
Upstream Blk Time (%)	1	11		3	3	
Queuing Penalty (veh)	2	66		25	31	
Storage Bay Dist (ft)			225			320
Storage Blk Time (%)		30	3		17	10
Queuing Penalty (veh)		72	8		127	70

Intersection: 31: Orange Ave & Hoffner Ave

Movement	WB	SB	SB	SB
Directions Served			 T	 T
Maximum Queue (ft)	650	322	483	467
Average Queue (ft)	540	106	266	250
95th Queue (ft)	759	223	414	405
Link Distance (ft)	637		558	558
Upstream Blk Time (%)	20		0	0
Queuing Penalty (veh)	99		0	0
Storage Bay Dist (ft)		460		
Storage Blk Time (%)			0	
Queuing Penalty (veh)			1	

Intersection: 37: Orange Ave & Hansel Ave

<i>I</i> ovement	
Directions Served	
<i>I</i> aximum Queue (ft)	
Average Queue (ft)	
95th Queue (ft)	
ink Distance (ft)	
Jpstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 38: Orange Ave & Lancaster Rd

Movement	EB	EB	EB	NB	NB	NB	SB	SB	SB	
Directions Served	L	L	R	L	Т	Т	Т	Т	R	
Maximum Queue (ft)	196	213	158	324	448	430	410	404	373	
Average Queue (ft)	100	137	74	223	226	228	365	340	187	
95th Queue (ft)	187	202	149	344	391	381	434	455	443	
Link Distance (ft)			1223		1281	1281				
Upstream Blk Time (%)										
Queuing Penalty (veh)										
Storage Bay Dist (ft)	225	225		300					350	
Storage Blk Time (%)	0	0	0	5	1			15	0	
Queuing Penalty (veh)	0	0	0	31	4			41	3	

Intersection: 39: Orange Ave & Nela Ave

Movement	EB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	LTR	L	TR	L	Т	TR	L	Т	TR
Maximum Queue (ft)	11	97	128	24	373	333	195	212	190
Average Queue (ft)	1	30	41	2	135	99	87	25	34
95th Queue (ft)	7	72	93	12	294	238	165	117	116
Link Distance (ft)	477		1265		937	937		1281	1281
Upstream Blk Time (%)									
Queuing Penalty (veh)									
Storage Bay Dist (ft)		145		200			185		
Storage Blk Time (%)			0		3		2	0	
Queuing Penalty (veh)			0		0		13	0	

Intersection: 44: Randolph Ave & Wilks Ave

Movement	WB	NB
Directions Served	LR	TR
Maximum Queue (ft)	38	130
Average Queue (ft)	6	24
95th Queue (ft)	28	137
Link Distance (ft)	544	257
Upstream Blk Time (%)		5
Queuing Penalty (veh)		6
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 48: Orange Ave

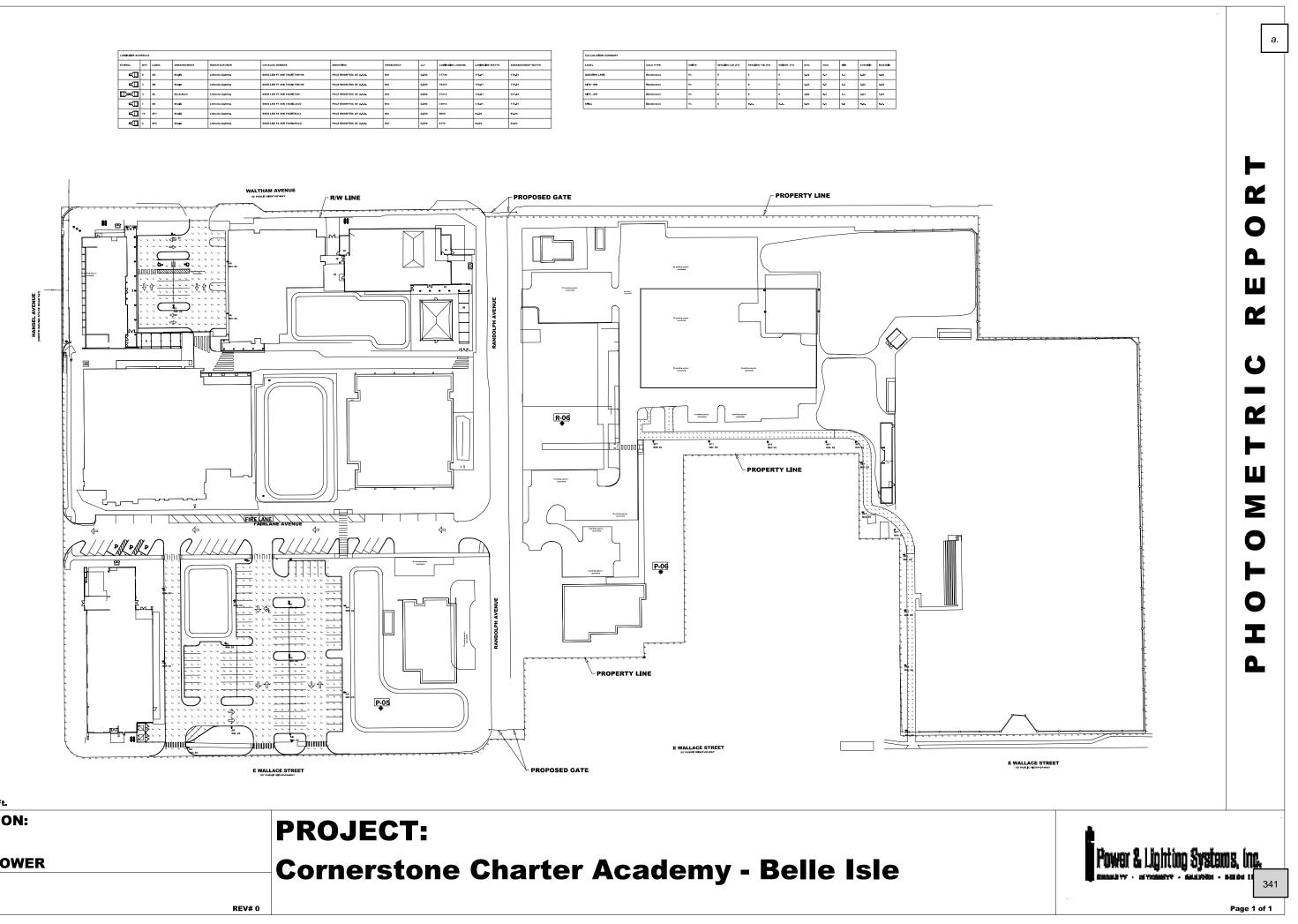
Movement	SB	SB
Directions Served	L	Т
Maximum Queue (ft)	56	20
Average Queue (ft)	4	1
95th Queue (ft)	38	27
Link Distance (ft)		248
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	220	
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 1531

LUN MARE SCH	nbder consul										
SYMBOL	QTY	LABEL	ARRANGEMENT	MANUFACTURER	CATALOG NUMBER	MOUNTING	EMERGENCY		LUMINAIRE LUMENS	LUMINAIRE WATTS	ARRANGEMENT WATTS
Å	8	84	Single	Lithonia Lighting	DSX0 LED P7 40K 70CRI T4M HS	POLE MOUNTED: 20' A.F. 6.	NA	0.800	17739	170-01	170,81
Å	3	88	Single	Lithonia Lighting	DSX0 LED P7 40K 70CR TSM HS	POLE MOUNTED: 20' A.F.G.	NIA	0.800	18419	170-01	170,81
ß	3	sc	Rock-Rock	Lithonia Lighting	DSX0 LED P7 40K TOCH TSM	POLE MOUNTED: 20' A.F.G.	NIA	0.800	21214	170,01	341_62
Å	,	80	Siegle	Lithonia Lighting	DSX0 LED P7 40K 70CH LCCO	POLE MOUNTED: 20' A.F.G.	NA	0.800	14914	178-81	170,81
Å	10	8F1	Single	Lithonia Lighting	DSX0 LED P4 40K 70CM BLC3	POLE MOUNTED: 20' A.F. 6.	NA	0.800	8094	93,04	93,04
Ð	2	8F2	Singer	Lithonia Lighting	DSX0 LED P4 40K TOCH RCCO	POLE MOUNTED: 20' A.P.G.	NA	0.800	8170	93,04	93,04

	CALCULATION SUMMARY										
	LABEL	CALC TYPE	UNETS	SPACING L-R (FT)	SPACING T-B (FT)	невант (РТ)	AVG	NAX	MEN	AVGIN	NAXH
	QUEUING LANE	Illuminance	F4	5	6	0	2,42	њ	1.1	2_20	4.82
I	SITE - NW	Illuninanse	F4	*	•	0	4.23	•	1.4	3.02	4.84
	stra . sw	Illuminanse	Fe	*	•	0	4.00	84	14	3,64	7,84
	selu	Illuminanse	F4	*	H.A.	N.A.	6 <u>.</u> 02	0.5	••	N.A.	N.A.



Scale: 1 inch= 48 Ft.

DESCRIPTION:

SITE

NORMAL POWER

DATE:

1/30/2023

City of Belle Isle

1600 Nela Avenue, Belle Isle, Florida 32809 * Tel 407-851-7730 * Fax 407-240-2222

APPLICATION FOR CHANGE IN ZONING CLASSIFICATION

** Per LDC, Chap. 42, Art. III, Sec. 42-61, a \$165.00 filing fee must be attached with EACH application ** **A COMPLETE SURVEY MUST ACCOMPANY ALL REZONING APPLICATIONS**

Case #							
Owner name Cornerstone Charter Academy, Inc.							
Ownerphone 407 895-0324 (Jean Abi-Aoun)							
Owner address 5903 Randolph Ave, Orlando, FL 32809							
Afformer/Consultant Name Jean Abi-Aoun, PE Florida Engr. Group, Inc.							
Afformer/Consultant Phone 407 895-0324 JAbiaoun@feg-inc.us							
Surveyor (Informal Subdivision)							
Zoning of property Square footage of property							
C-1 21,798 SF							
C-1 to PD							
Previous Use was:							
ire area where the school will be							
of use.							
Owner Signature m.G.Brooks Million Smith							
\$165.00 Check #/Cash							
P&Z Board Approved or Denied Application? (circle one)							

Sec. 42-65. - Zoning changes.

The council may from time to time amend or supplement the regulations and districts fixed by any code adopted pursuant to this article.

(1) Changes to the Land Development Code.

- a. Proposed changes may be suggested by the council, by the board, or by the mayor.
- b. All proposed changes to the Land Development Code shall be in form of ordinances, and shall follow the notice requirement set forth by the Florida Statutes on adopting ordinances.
- c. The board shall review all proposed changes to the Land Development Code, and prepare a recommendation to the council on the proposed changes.
- d. The council shall hold two readings on all ordinances. The council shall adopt changes to the Land Development Code only after holding at least one advertised public hearing in accordance with Florida Statutes.

(2) Changes to the official zoning map (rezoning of property).

- a. Proposed changes to the official zoning map, hereinafter referred to as <u>rezonings</u>, may be suggested by the council, by the board, by the mayor or by the owner, or agent for the owner, of the property subject to the changes proposed. In the latter case, the owner or agent for the owner, hereinafter referred as the petitioner, shall be required to assume the cost of public notice and other costs incidental to hearings in accordance with section 42-61.
- b. The petitioner shall make application for a rezoning by submitting a completed application, a legal description of the property (complete survey), and a statement of purpose explaining the reason for changing the zoning district classification.
- c. The board and the council shall make such investigation as it may determine and shall hold a public hearing or hearings, with due public notice and in accordance with Florida Statutes, on all rezoning requests.
- d. The city shall notify the applicable water supplier upon submittal of any rezoning request that would increase water and wastewater demand in order to effectively coordinate water supply planning and ensure capacity and availability.

а.



City of Belle Isle

1600 Nela Avenue, Belle Isle, Florida 32809 * Tel 407-851-7730 * Fax 407-240-2222

APPLICATION FOR CHANGE IN ZONING CLASSIFICATION

** Per LDC, Chap. 42, Art. III, Sec. 42-61, a \$165.00 filing fee must be attached with EACH application ** **A COMPLETE SURVEY MUST ACCOMPANY ALL REZONING APPLICATIONS**

Date: January 30, 2023	Case #						
Address or location of subdivision	Ownername Pine Castle Methodist Church, Inc.						
942 Fairlane Avenue	0 1						
24-23-29-3400-00-093	407-400-4121						
	Owner address 731 Fairlane Ave, Orlando, FL 32809						
Applicants Name CORNERSTONE CHARLER ACADEMY INC.	Afformer Consultant Name Jean ADI-AOUN, PE						
Applicants Address	Florida Engr. Group, Inc.						
	·						
906 WIAL THAM AVENUE	JAbiaoun@feg-inc.us						
Applicants City, State, Zip ORLANDO Fr 32809	Surveyor (Informal Subdivision)						
Applicants Phone 407 885-0324 JEAN ABI-ADWAY	Zoning of property Square footage of property C-1 50,658 SF						
Applicants City, State, Zip ORLANDO, FZ 32809 Applicants Phone 407 885-0324 JEAN ABI-ADUM Applicants Email JABJADUME FEG INC. US	C-1 50,058 5F						
Request is hereby made for a change in Zoning Classification from	C-1 to PD						
Current Use is: Church Ministry	Previous Use was:						
Reason for request and proposed use (required); Cornerstone Charter School - Rezone enti:	re area where the school will be						
in operation to PD zoning for consistency	v of use.						
If Owner owns any adjacent parcels, please list Parcel ID #s:							
Applicant Signature	Owner Signature Mark Hesseling, Trustee Mark Hand						
	the second secon						
	cp-p						
FOR OFFICE USE ONLY: Application Recd On Application Recd By	· · · · · · · · · · · · · · · · · · ·						
FOR OFFICE USE ONLY: Application Recd By Application Recd On Application Recd By P&Z Case # P&Z Hearing Date	\$165.00 Check #/Cash						

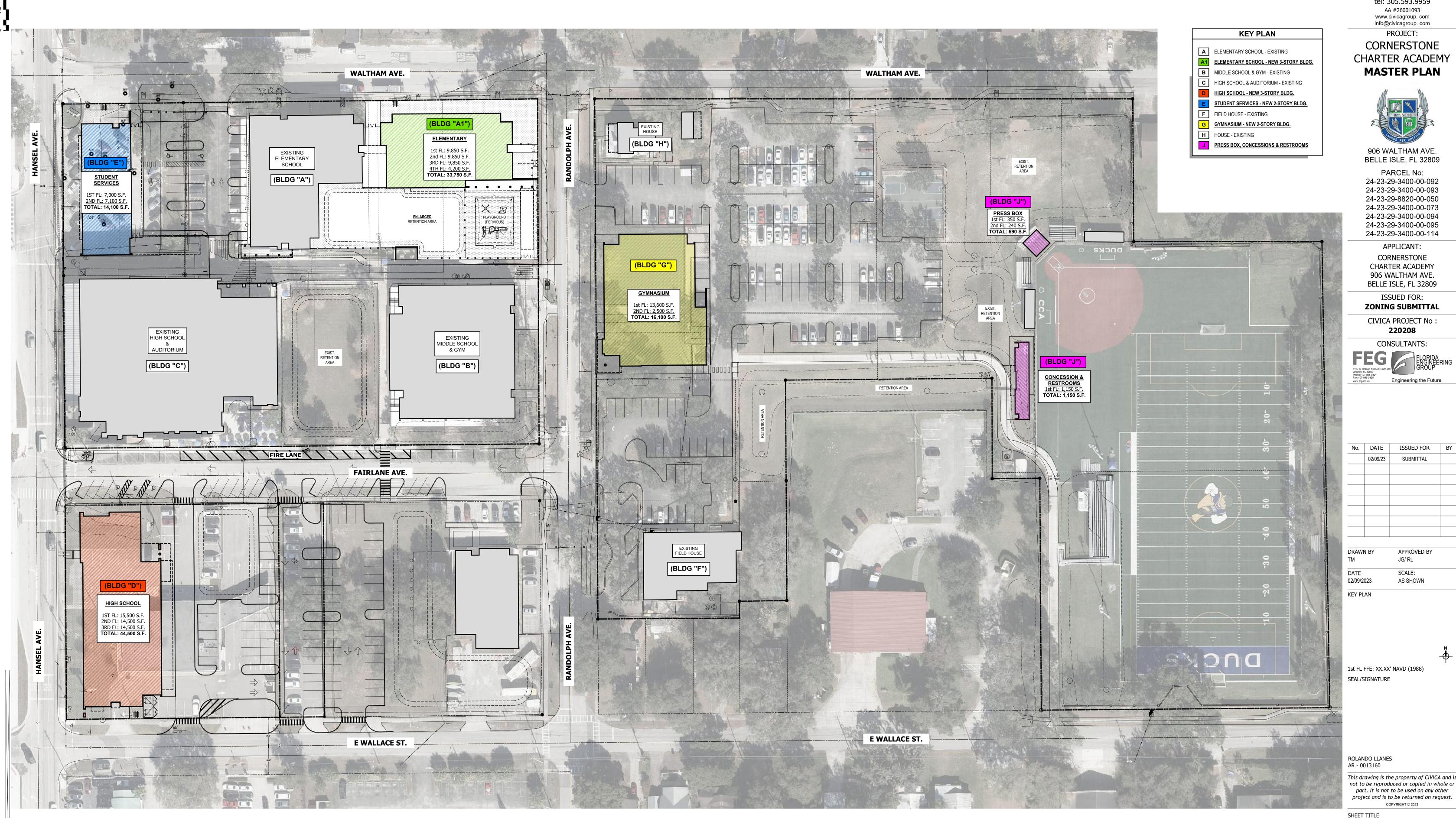
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- b. The petitioner shall make application for a rezoning by submitting a completed application, a legal description of the property (complete survey), and a statement of purpose explaining the reason for changing the zoning district classification.
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- d. The city shall notify the applicable water supplier upon submittal of any rezoning request that would increase water and wastewater demand in order to effectively coordinate water supply planning and ensure capacity and availability.





1st FL FFE: XX.XX' NAVD (1988)

CA

VI

8323 NW 12th St. Suite 106 Doral, FL 33126 tel: 305.593.9959 AA #26001093 www.civicagroup. com info@civicagroup. com PROJECT:

PARCEL No:

APPLICANT:

CORNERSTONE

CHARTER ACADEMY

906 WALTHAM AVE.

ISSUED FOR:

220208

CONSULTANTS:

02/09/23

Engineering the Future

SUBMITTAL

APPROVED BY JG/ RL

SCALE: AS SHOWN

ROLANDO LLANES AR - 0013160

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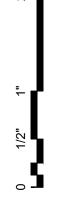
SHEET TITLE

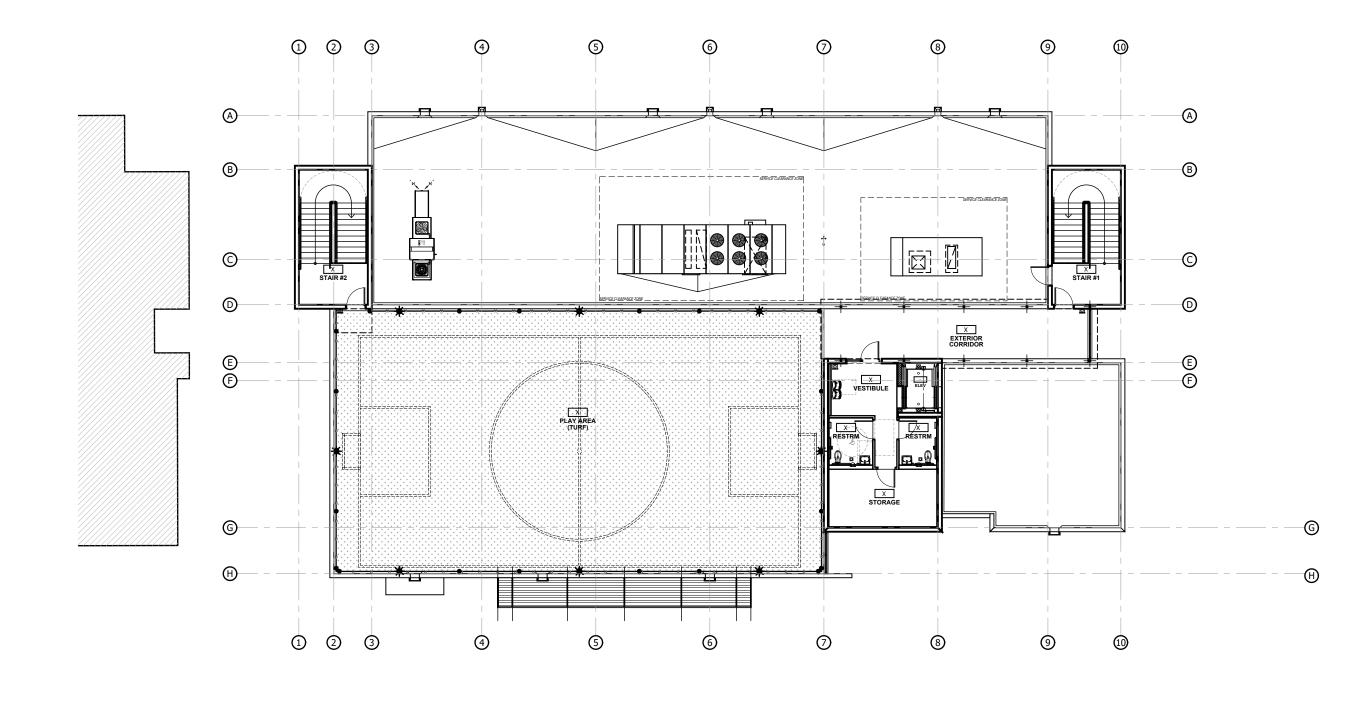


SHEET NUMBER

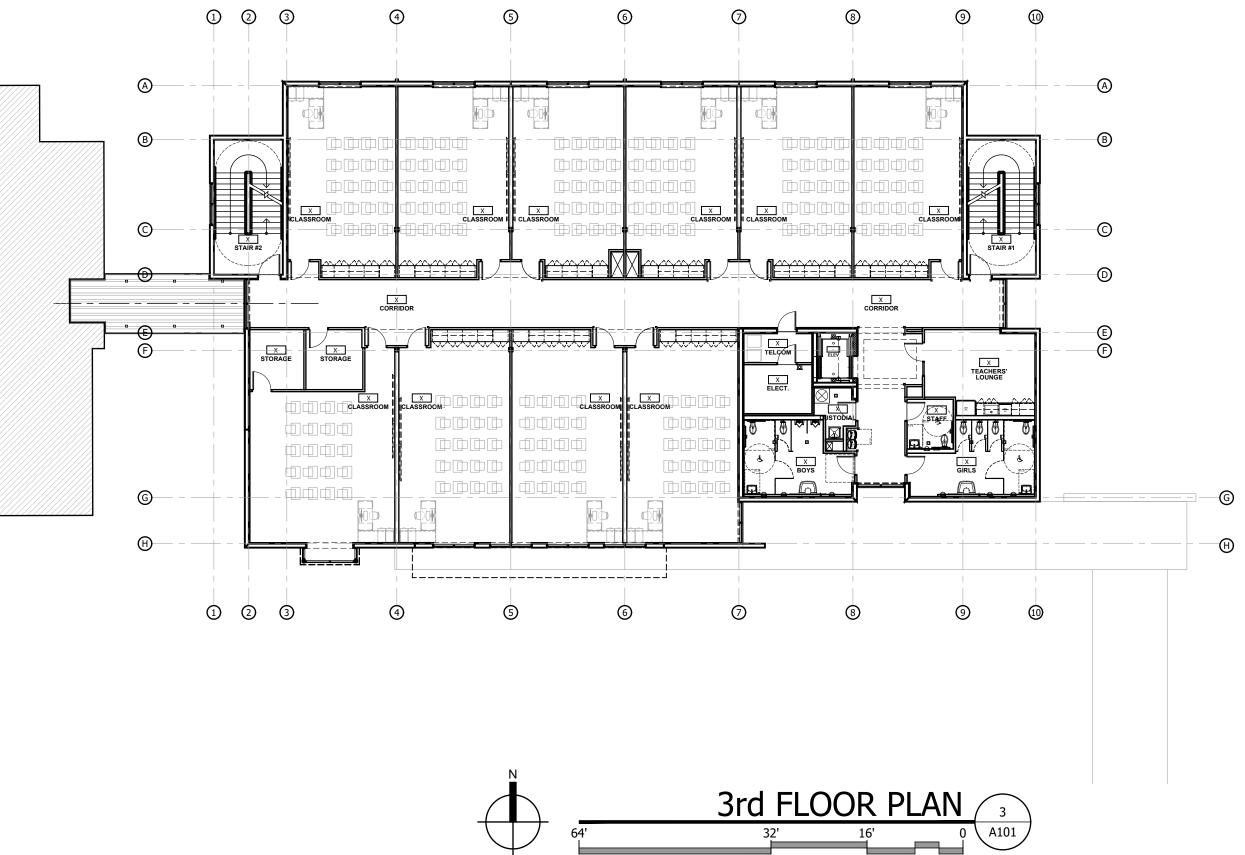


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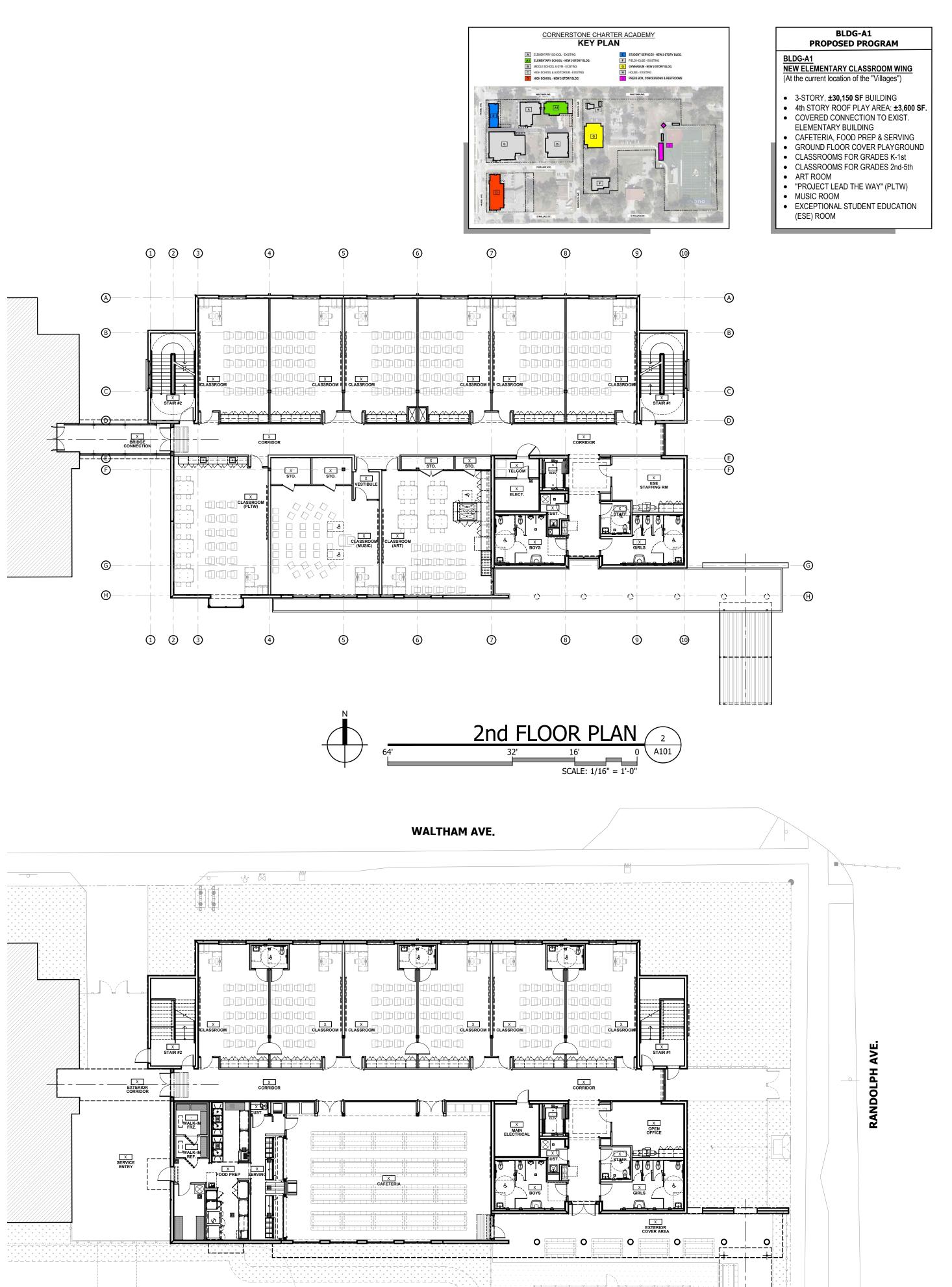




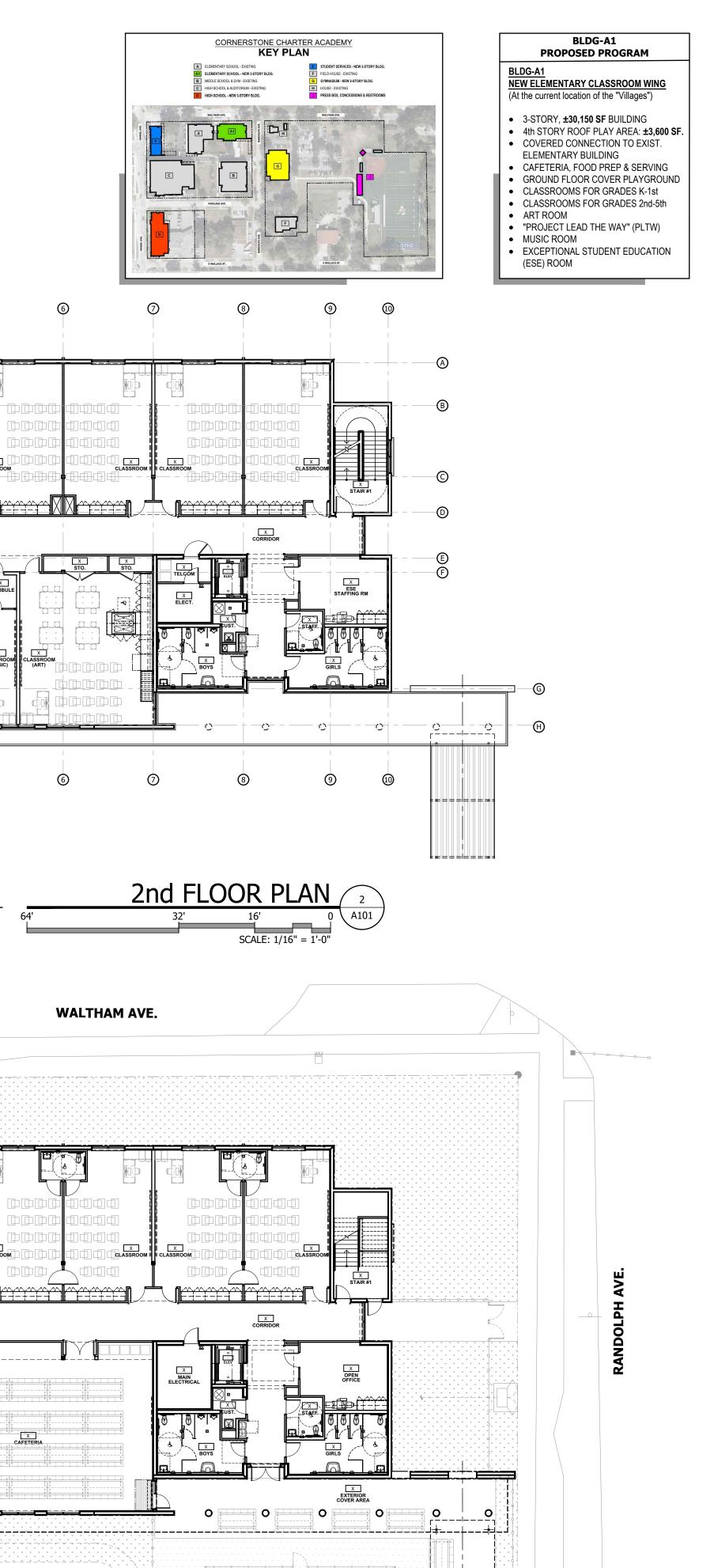






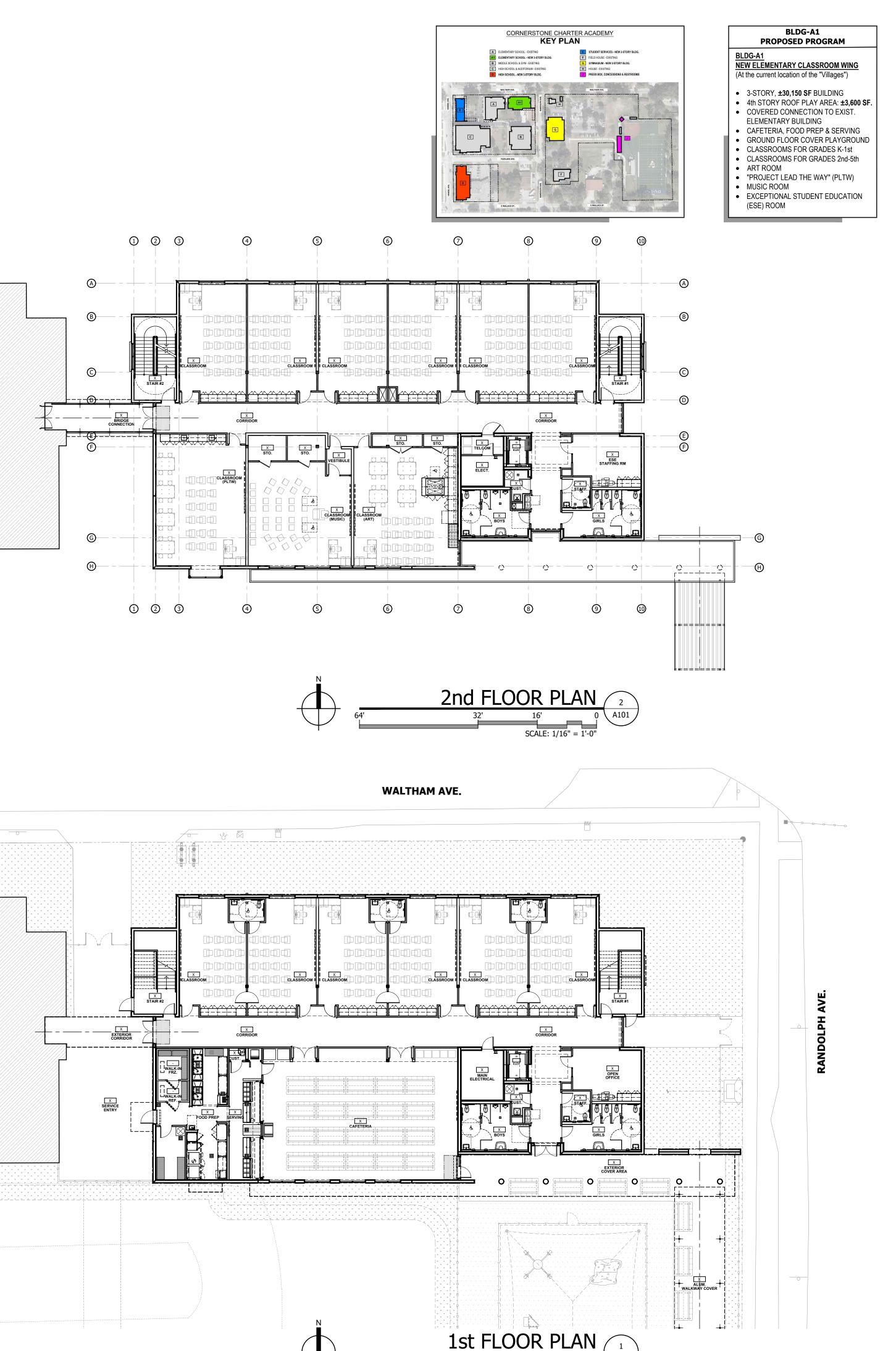


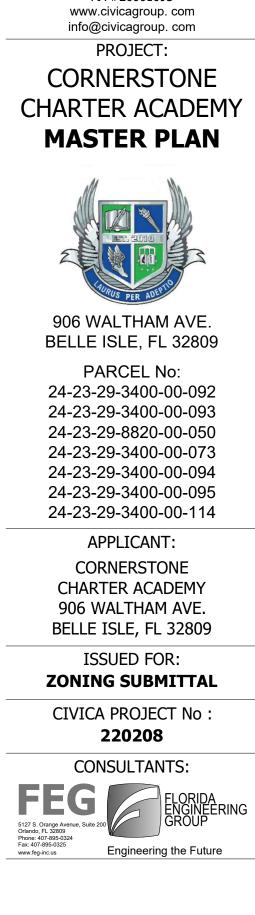
A101 SCALE: 1/16" = 1'-0"



A101

SCALE: 1/16" = 1'-0"





C A

CTUR

8323 NW 12th St. Suite 106

Doral, FL 33126

tel: 305.593.9959

AA #26001093

A R C

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ТМ		JG/ RL	
DATE		SCALE:	
02/09/2023		AS SHOWN	
KEY PL	AN		

1st FL FFE: XX.XX' NAVD (1988) SEAL/SIGNATURE

ROLANDO LLANES AR - 0013160

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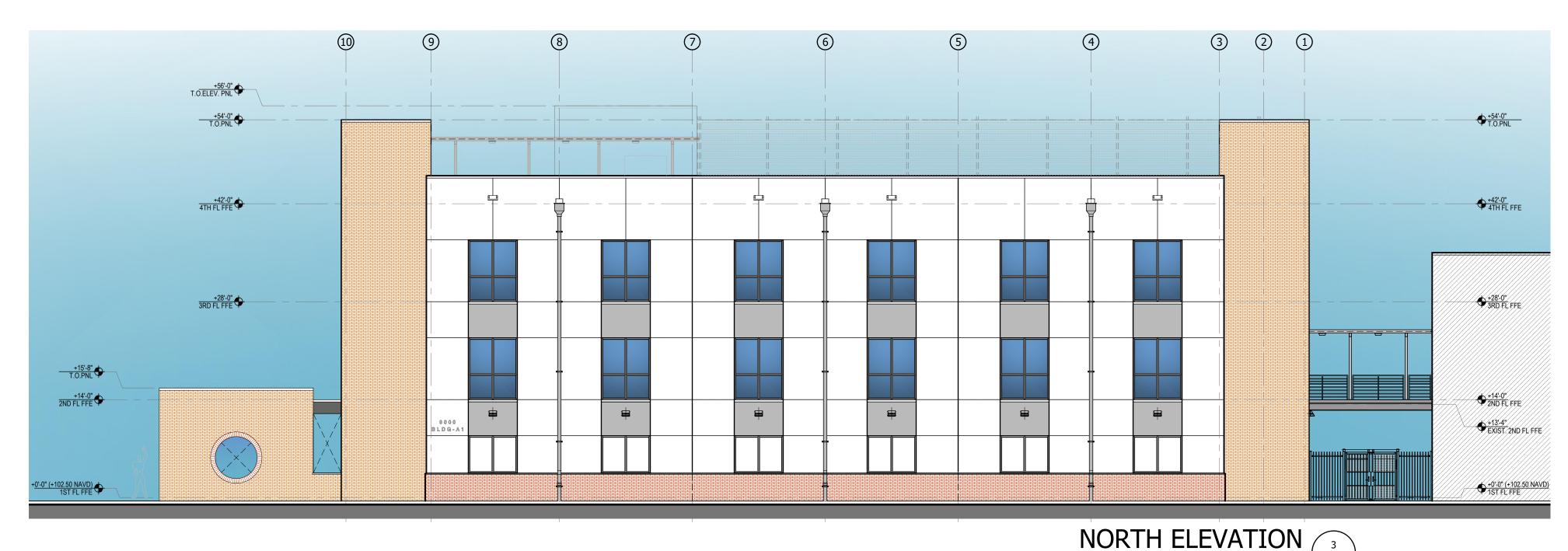
BLDG-A1

SHEET TITLE

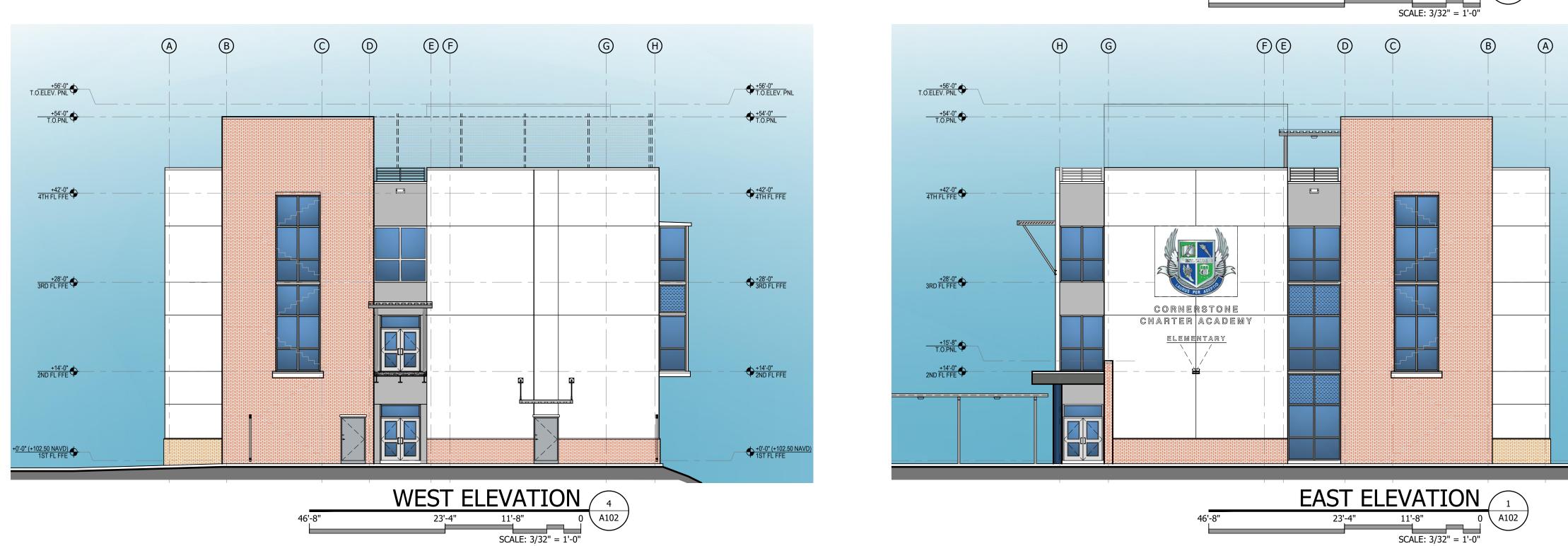
FLOOR PLANS

SHEET NUMBER









SOUTH ELEVATION (2) 23'-4" A102 11'-8"

SCALE: 3/32" = 1'-0"

46'-8"



+0'-0" (+102.50 NAV 1ST FL FFE

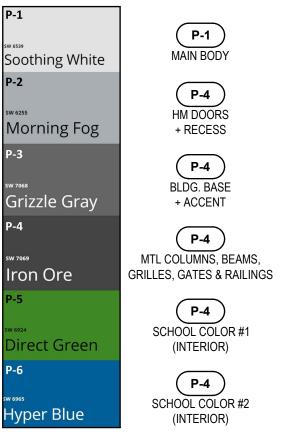


PROPOSED MATERIALS & COLOR SELECTION

COLOR SELECTION BASED ON SHERWIN WILLIAMS COLOR PALETTE:

- HM DOORS/ FRAMES (INTERIOR & EXTERIOR) P-2 2. METAL STAIRS/ GUARDRAIL/ RAILING & EXPOSED STEEL MEMBERS -SW7069 "IRON ORE" (MATCH EXIST.)
- WALKWAY COVER, CANOPIES & SUNSHADES ANODIZED ALUM. STOREFRONTS & WINDOWS - ANODIZED ALUM.
- 5. GLASS SOLAR-E PLUS GREY (SPANDREL GLASS TO MATCH)

EXTERIOR BUILDING COLORS:







M-2 ALUM. STOREFRONTS, DOORS & WINDOWS: ANODIZED ALUMINUM W/ SOLAR-E PLUS GREY GLASS





M-3 ROOF COLOR/ MATERIAL: LOW SLOPE (WHITE) T.P.O. GATES & DECORATIVE PICKET FENCE



M-5 HORIZONTAL SUNSHADE DRAGINE , A CLEAR ANODIZED ALUMINUM (SILVER)



WALKWAY COVER & MATERIAI M-6 WALKWAT COVER A MULTICE AND LEASE CLEAR ANODIZED ALUMINUM (SILVER)









8323 NW 12th St. Suite 106 Doral, FL 33126 tel: 305.593.9959 AA #26001093 www.civicagroup. com info@civicagroup. com PROJECT: CORNERSTONE CHARTER ACADEMY **MASTER PLAN** 906 WALTHAM AVE. BELLE ISLE, FL 32809 PARCEL No: 24-23-29-3400-00-092 24-23-29-3400-00-093 24-23-29-8820-00-050 24-23-29-3400-00-073 24-23-29-3400-00-094 24-23-29-3400-00-095

24-23-29-3400-00-114 APPLICANT: CORNERSTONE CHARTER ACADEMY 906 WALTHAM AVE. BELLE ISLE, FL 32809

ISSUED FOR: ZONING SUBMITTAL

CIVICA PROJECT No : 220208



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ГМ		JG/ RL	
DATE		SCALE:	
02/09/2023		AS SHOWN	

KEY PLAN

1st FL FFE: XX.XX' NAVD (1988) SEAL/SIGNATURE

ROLANDO LLANES AR - 0013160

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SHEET TITLE



SHEET NUMBER

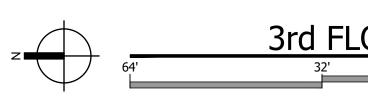




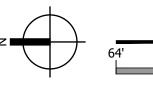












1st FLOOR PLAN

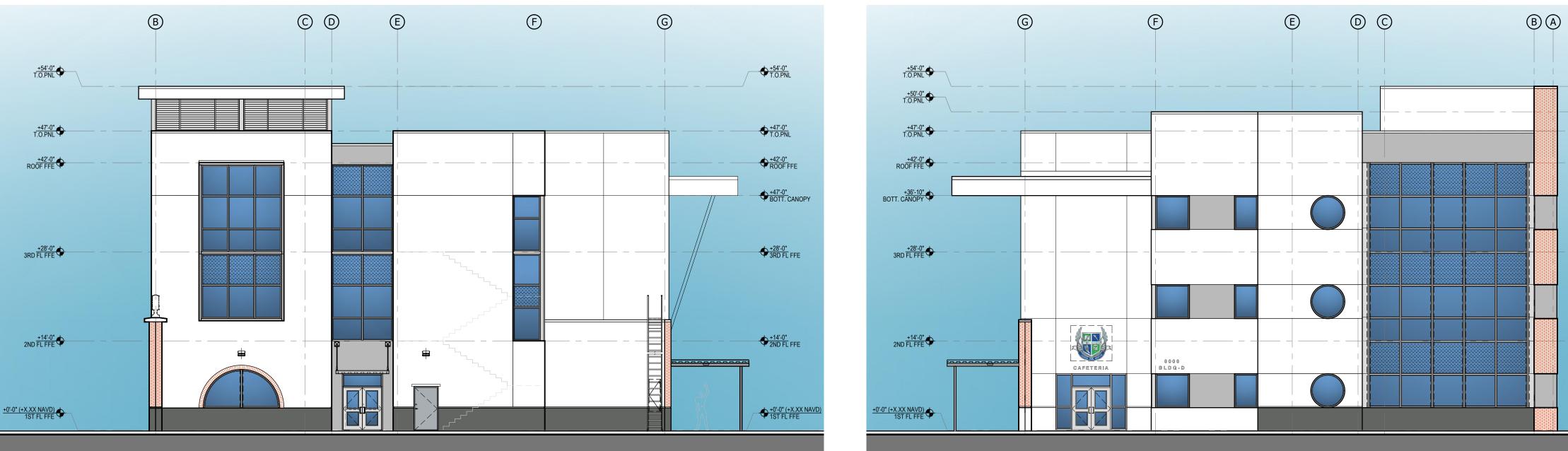
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↓ A103

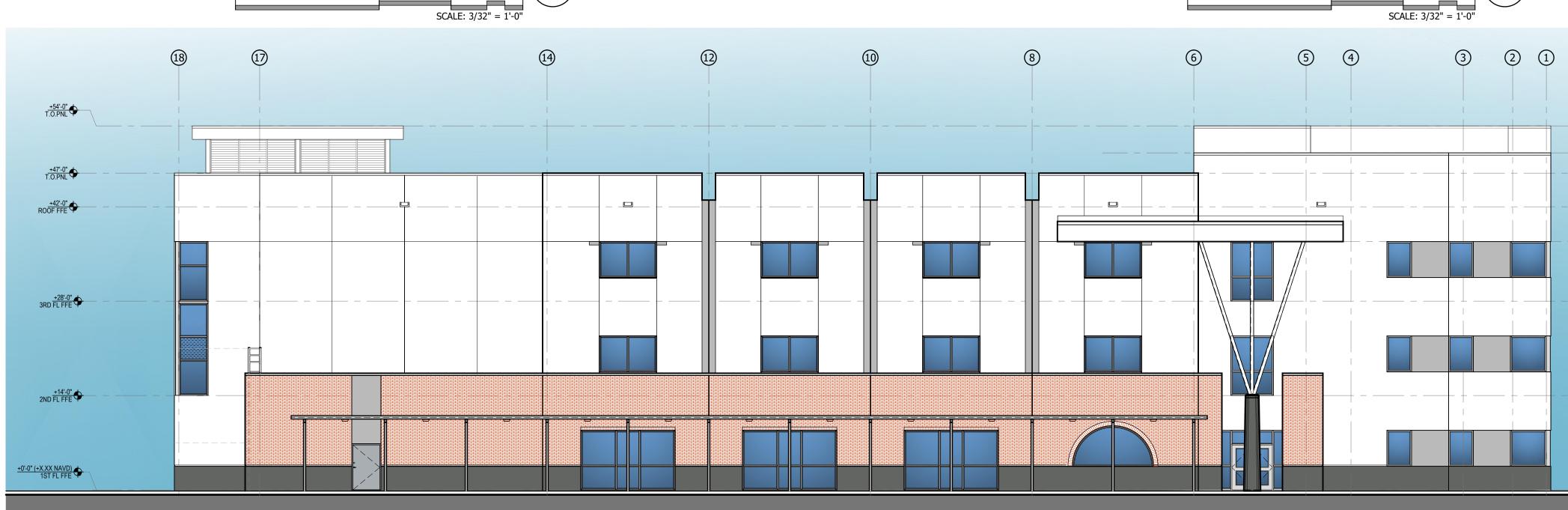
SHEET NUMBER



347



WEST ELEVATION 4 23'-4" 11'-8" 0 SCALE: 3/32" = 1'-0" 4





 SOUTH ELEVATION
 2

 23'-4"
 11'-8"
 0
 2

 SCALE: 3/32" = 1'-0"
 2
 4104

SCALE: 3/32" = 1'-0"

46'-8"

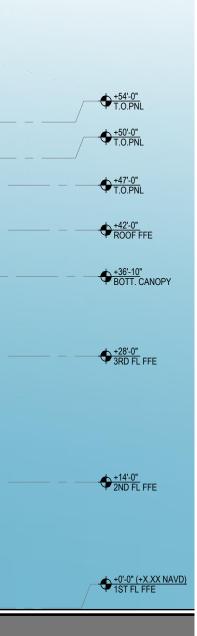
NORTH ELEVATION 3

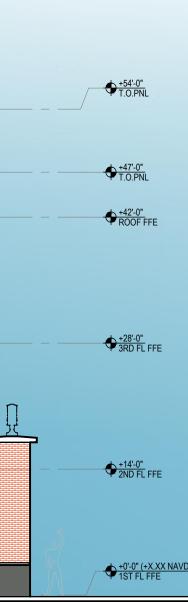
11'-8"

23'-4"

A104







PROPOSED MATERIALS & COLOR SELECTION

COLOR SELECTION BASED ON SHERWIN WILLIAMS COLOR PALETTE:

- HM DOORS/ FRAMES (INTERIOR & EXTERIOR) P-2
 METAL STAIRS/ GUARDRAIL/ RAILING & EXPOSED STEEL MEMBERS -SW7069 "IRON ORE" (MATCH EXIST.)
- SW7069 "IRON ORE" (MATCH EXIST.) 3. WALKWAY COVER, CANOPIES & SUNSHADES - ANODIZED ALUM. 4. STOREFRONTS & WINDOWS - ANODIZED ALUM.
- 5. GLASS SOLAR-E PLUS GREY (SPANDREL GLASS TO MATCH)

EXTERIOR BUILDING COLORS:







M-2 ALUM. STOREFRONTS, DOORS & WINDOWS: ANODIZED ALUMINUM W/ SOLAR-E PLUS GREY GLASS





M-3 ROOF COLOR/ MATERIAL: LOW SLOPE (WHITE) T.P.O. GATES & DECORATIVE PICKET FENCE



M-5 HORIZONTAL SUNSHADE BRACKET & MA CLEAR ANODIZED ALUMINUM (SILVER)



M-6 WALKWAY COVER & MATERIAL: CLEAR ANODIZED ALUMINUM (SILVER)



CLEAR ANODIZED ALUMINUM (SILVER)





M-8 TILT-UP CONCRETE CONSTRUCTION, STEEL JOIST, STEEL BEAMS, STEEL COLUMNS & CONCRETE OVER CORRUGATED DECK



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ТМ		JG/ RL	
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KEY PLAN

1st FL FFE: XX.XX' NAVD (1988) SEAL/SIGNATURE

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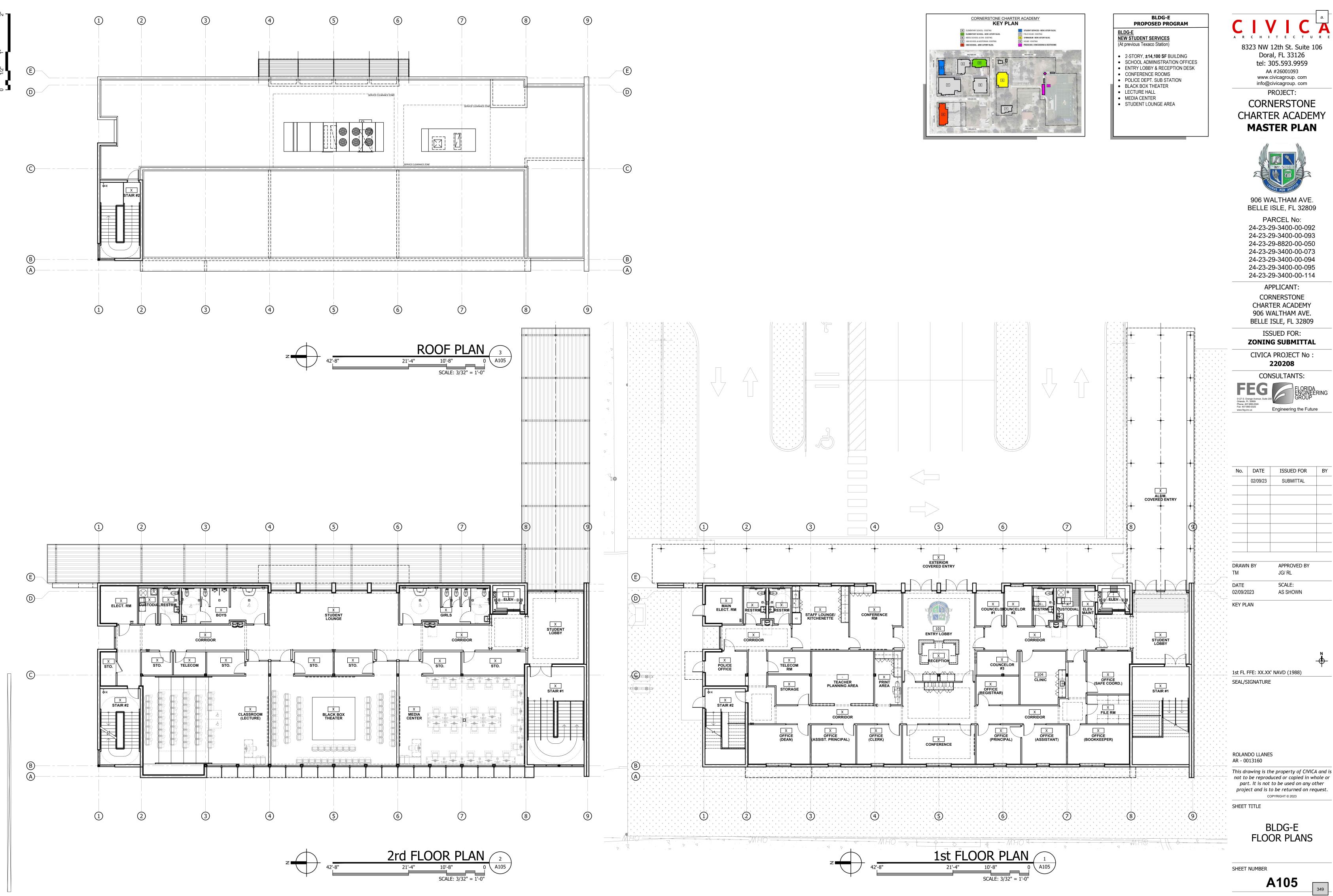
SHEET TITLE

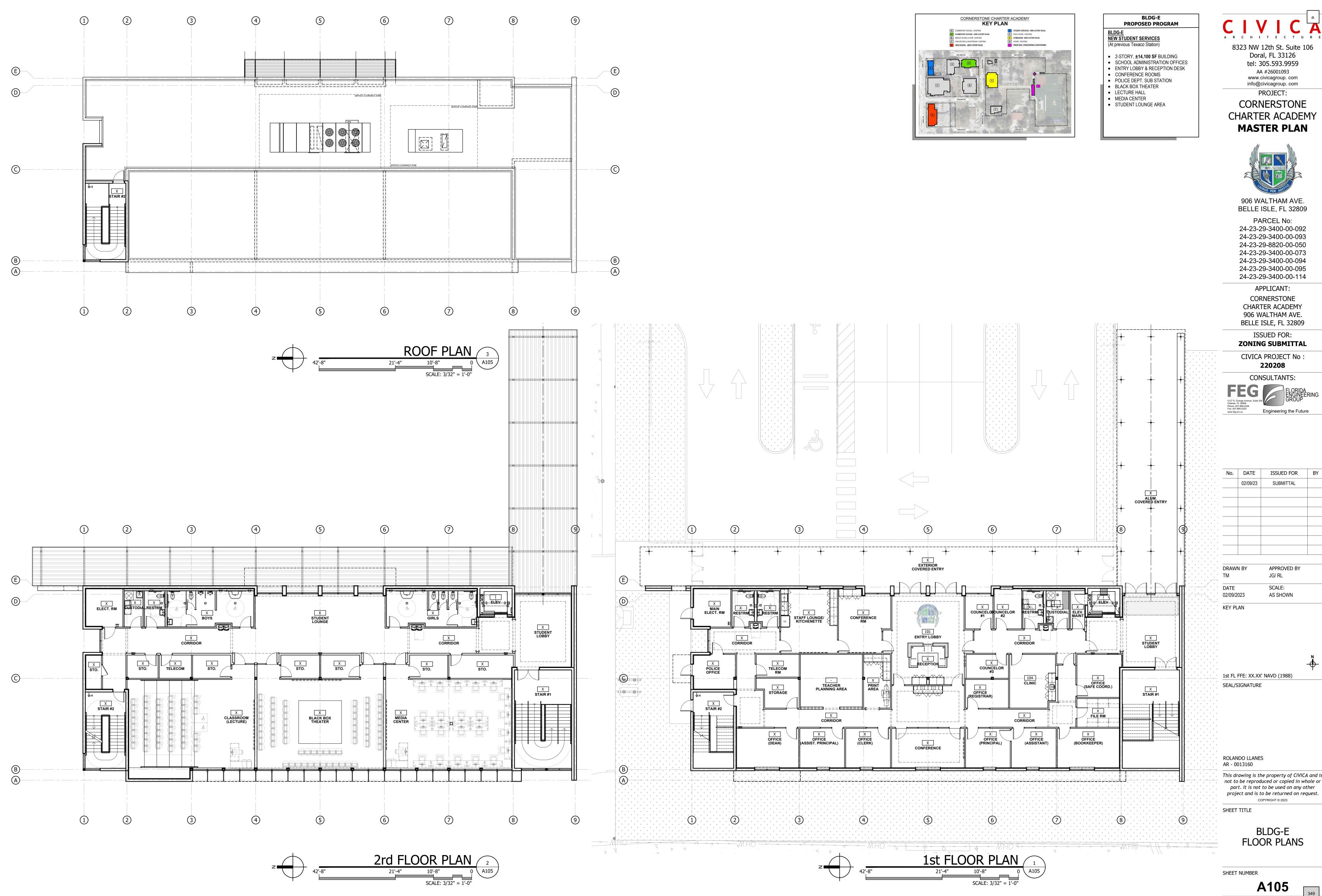


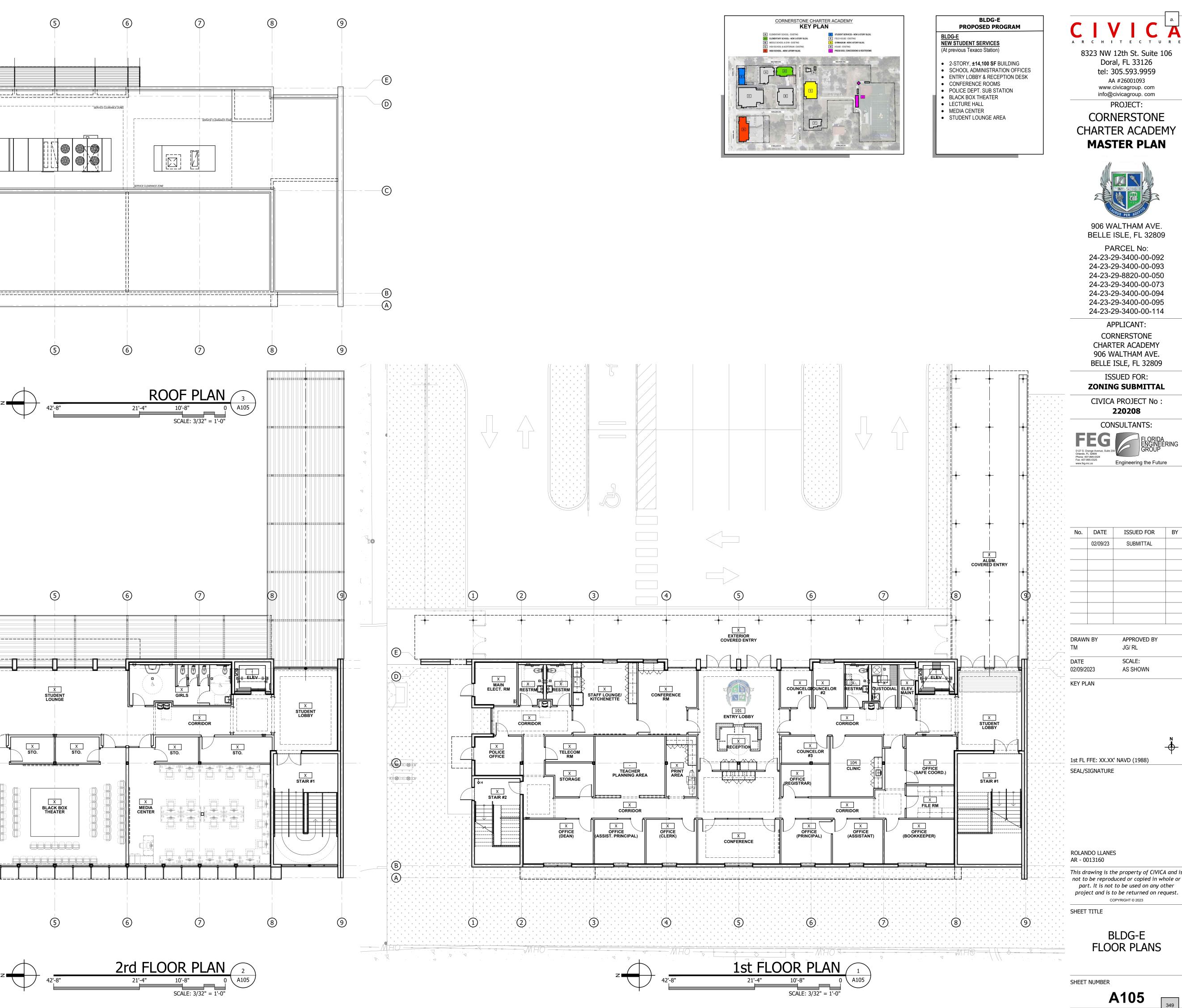
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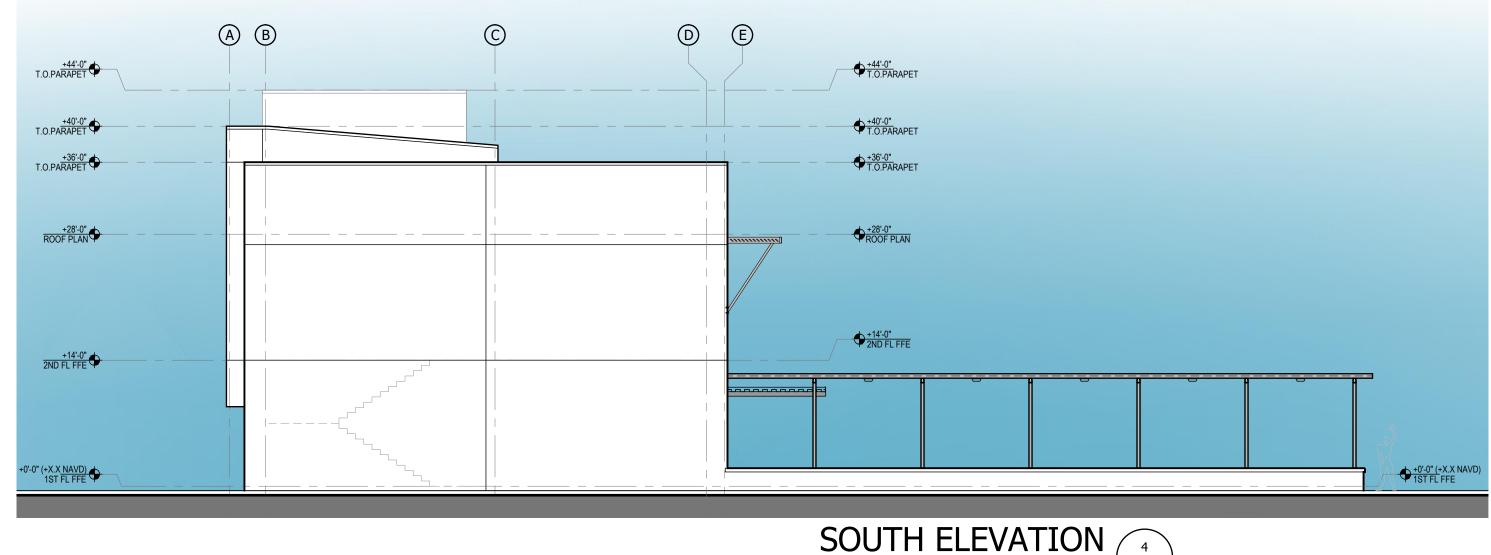


348









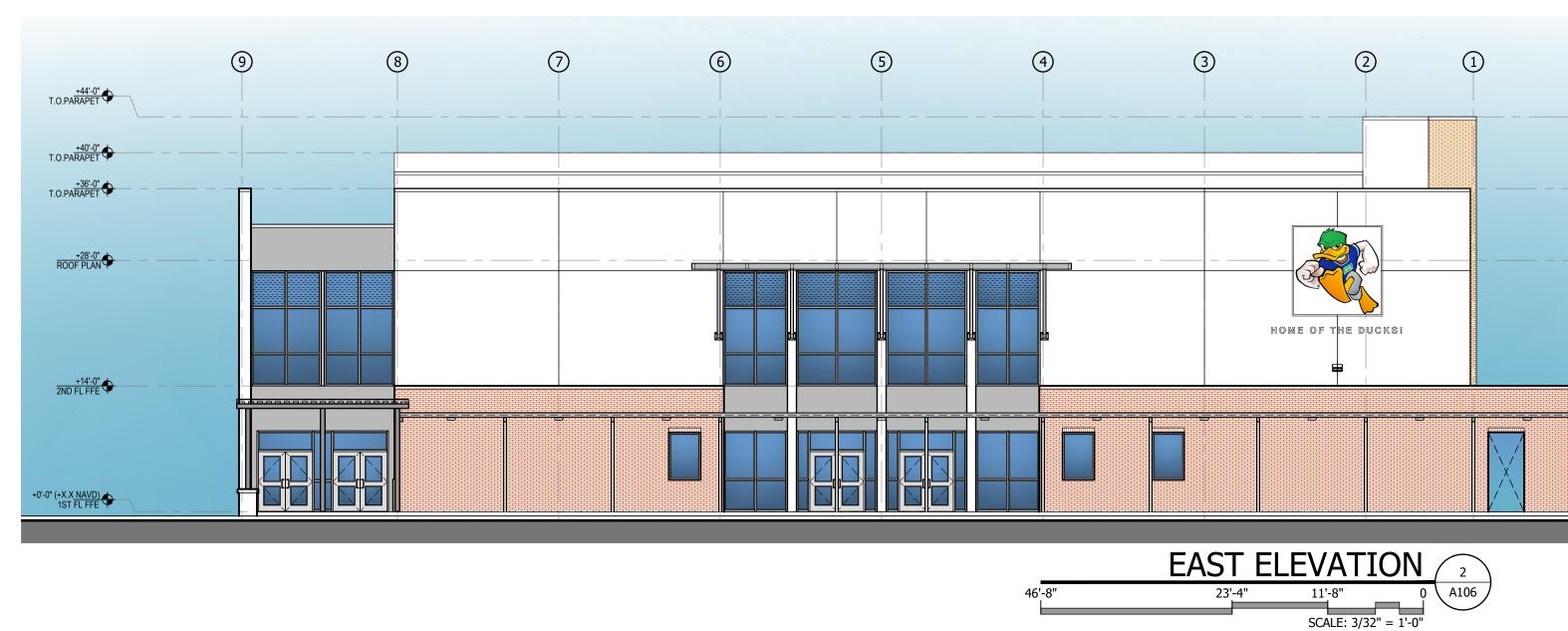
46'-8'

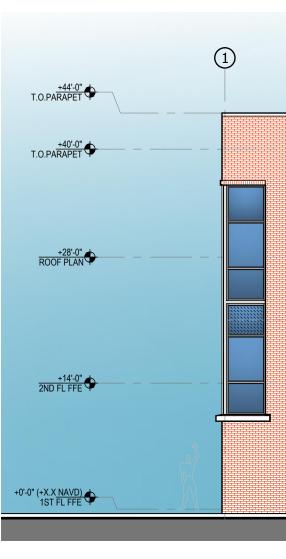
23'-4"

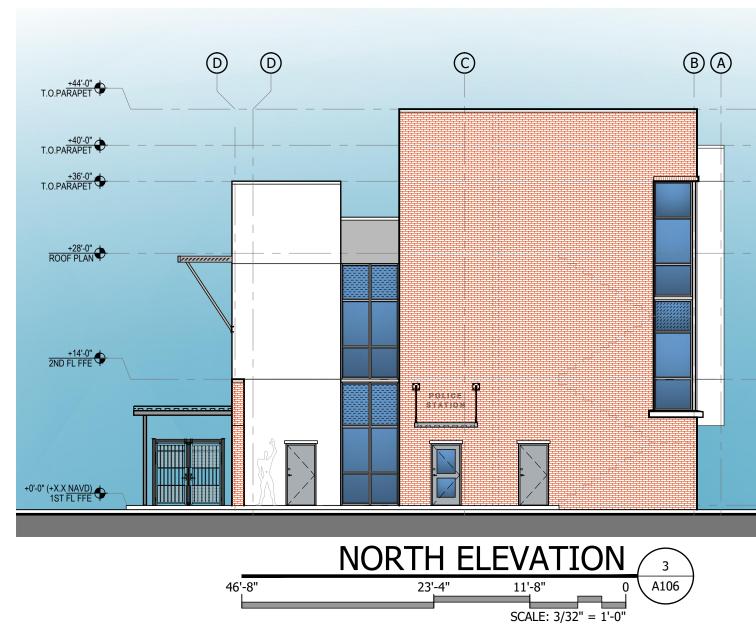
SCALE: 3/32" = 1'-0"

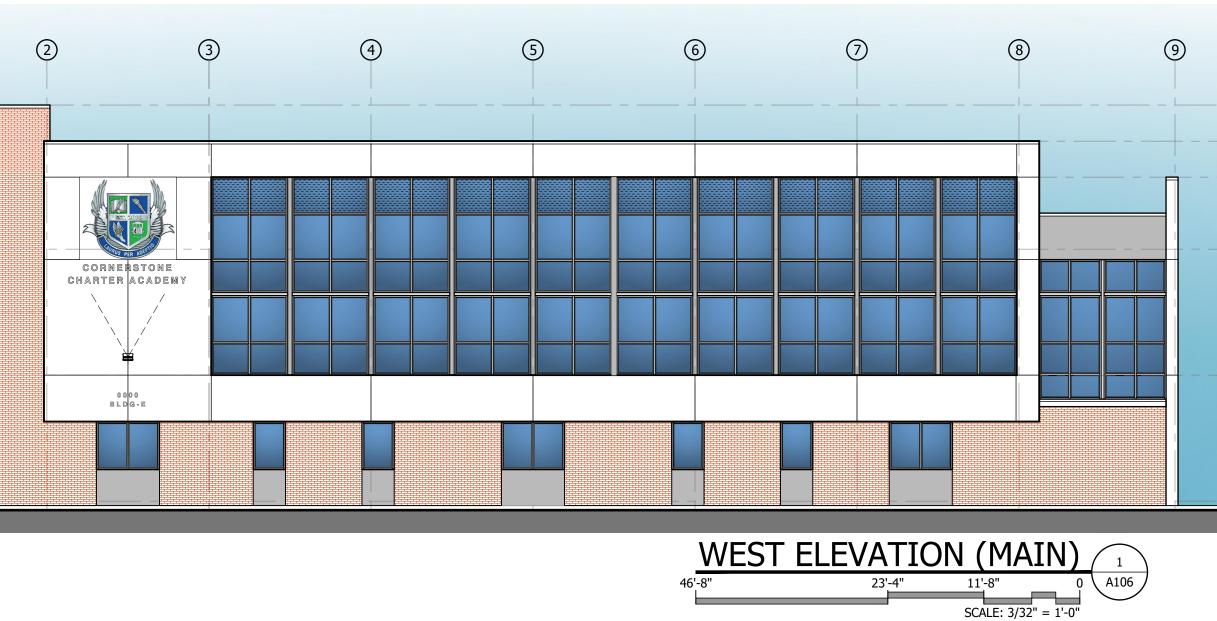
11'-8"

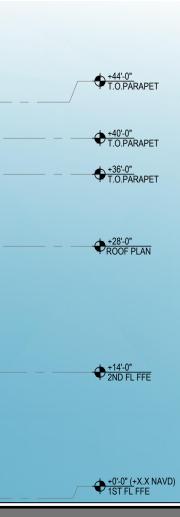
A106









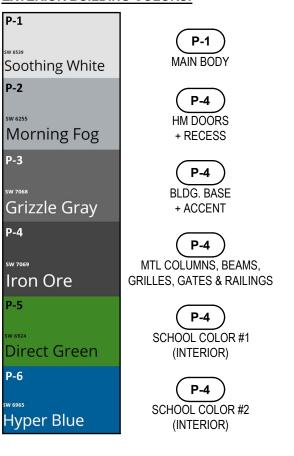




COLOR SELECTION BASED ON SHERWIN WILLIAMS COLOR PALETTE:

- HM DOORS/ FRAMES (INTERIOR & EXTERIOR) P-2 2. METAL STAIRS/ GUARDRAIL/ RAILING & EXPOSED STEEL MEMBERS -SW7069 "IRON ORE" (MATCH EXIST.)
- WALKWAY COVER, CANOPIES & SUNSHADES ANODIZED ALUM. STOREFRONTS & WINDOWS - ANODIZED ALUM.
- 5. GLASS SOLAR-E PLUS GREY (SPANDREL GLASS TO MATCH)

EXTERIOR BUILDING COLORS:







M-2 ALUM. STOREFRONTS, DOORS & WINDOWS: ANODIZED ALUMINUM W/ SOLAR-E PLUS GREY GLASS





M-3 ROOF COLOR/ MATERIAL: LOW SLOPE (WHITE) T.P.O. GATES & DECORATIVE PICKET FENCE



M-5 HORIZONTAL SUNSHADE BRACKET & MAT CLEAR ANODIZED ALUMINUM (SILVER)



WALKWAY COVER & MATERIAL M-6 WALKWAY OUVER & WALKWAL. CLEAR ANODIZED ALUMINUM (SILVER)



CLEAR ANODIZED ALUMINUM (SILVER)





TILT-UP CONCRETE CONSTRUCTION, STEEL JOIST, STEEL BEAMS, M-8 TILT-UP CONCRETE CONSTRUCTION, STEEL JOIST, STELL DE STEEL COLUMNS & CONCRETE OVER CORRUGATED DECK





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ТМ		JG/ RL	
DATE		SCALE:	
02/09/2023		AS SHOWN	

KEY PLAN

1st FL FFE: XX.XX' NAVD (1988) SEAL/SIGNATURE

ROLANDO LLANES AR - 0013160

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SHEET TITLE



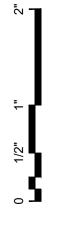
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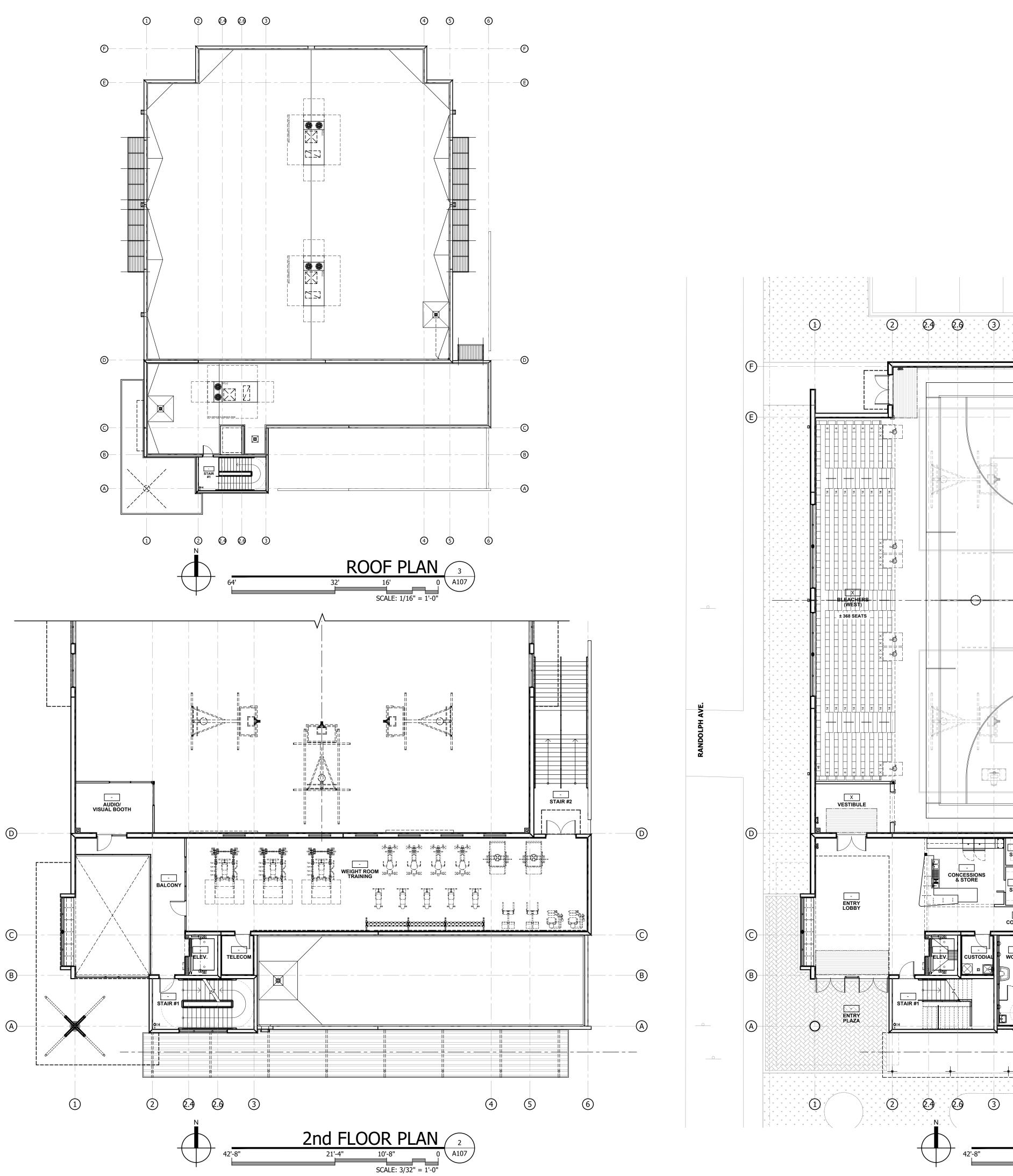


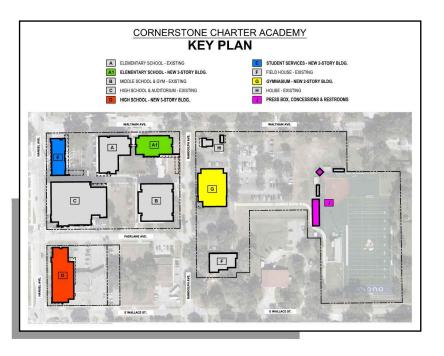














BLDG-G **NEW GYMNASIUM / MULTI-PURPOSE** (At the current location of "Turf Play Areas")

A R C

CTURE

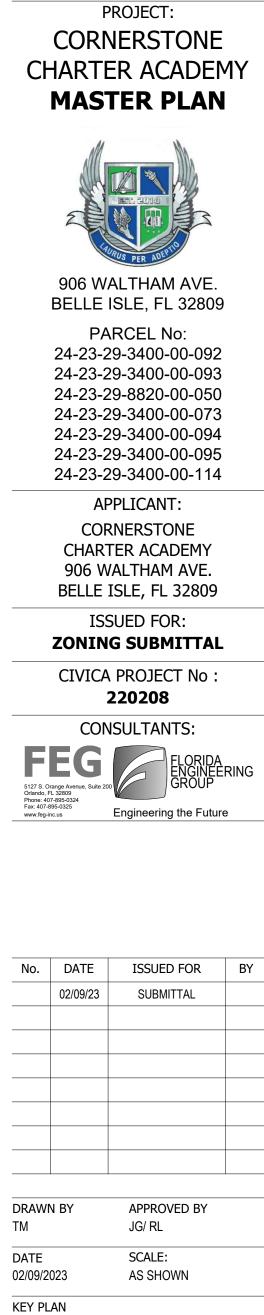
8323 NW 12th St. Suite 106

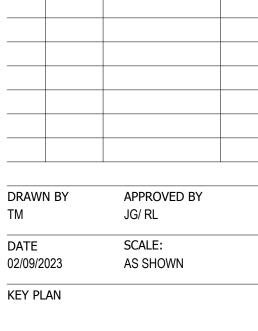
Doral, FL 33126

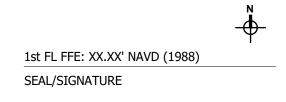
tel: 305.593.9959

AA #26001093 www.civicagroup. com info@civicagroup. com

- 2-STORY, ±16,100 SF BUILDING • GYMNASIUM / MULTI-PURPOSE SPACE
- WITH RETRACTABLE BLEACHERS REGULATION BASKETBALL /
- VOLLEYBALL COURT • A/V SYSTEM CONTROL ROOM
- CONCESSION STAND • GIRLS & BOYS LOCKER ROOMS
- WEIGHT TRAINING CLASSROOM • TRAINER W/ HYDROTHERAPY
- WHIRLPOOL TUBS
- COACH OFFICE SPORTS EQPMT. STORAGE



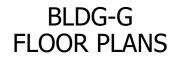




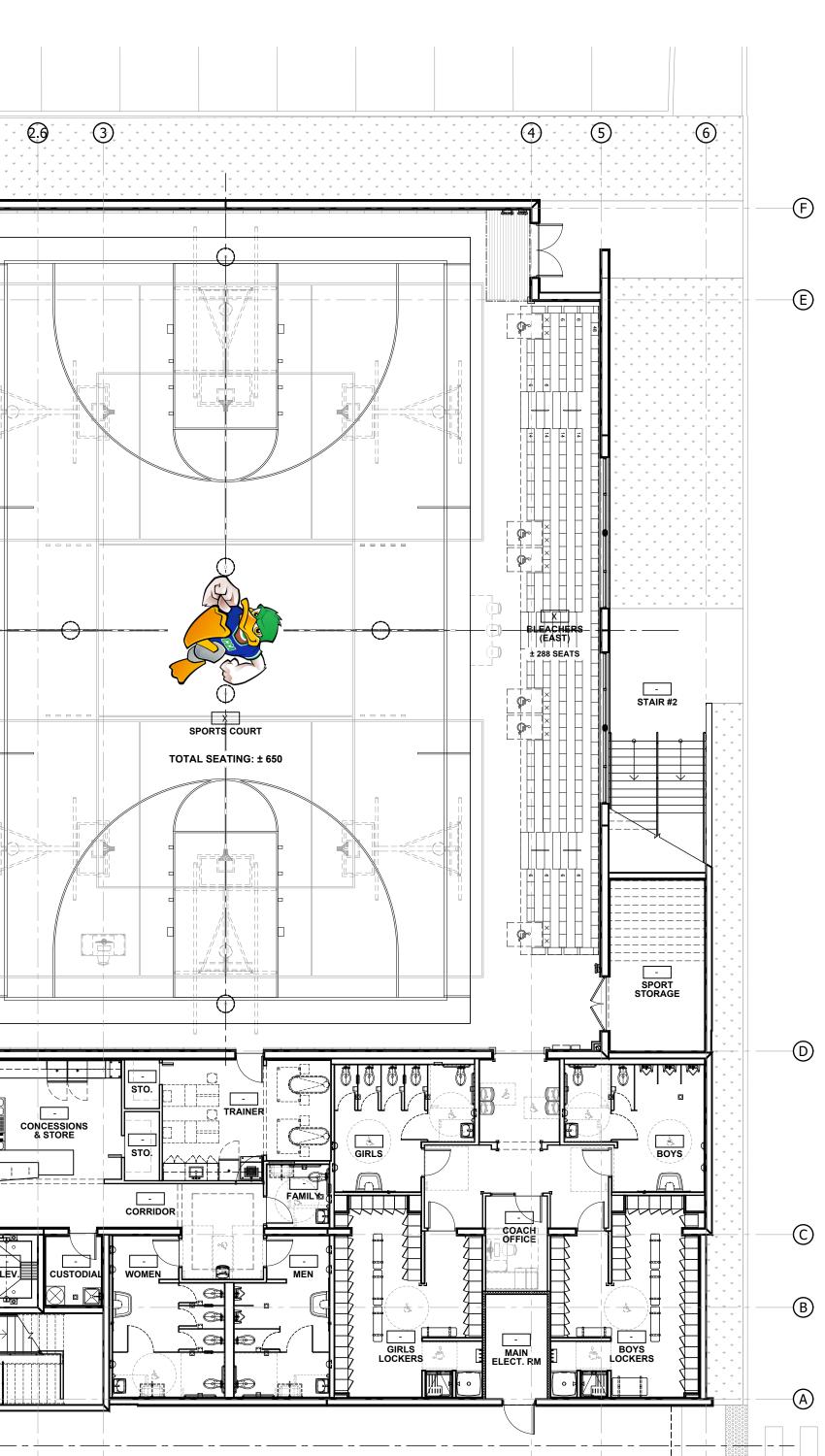
ROLANDO LLANES AR - 0013160

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SHEET NUMBER

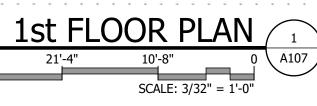


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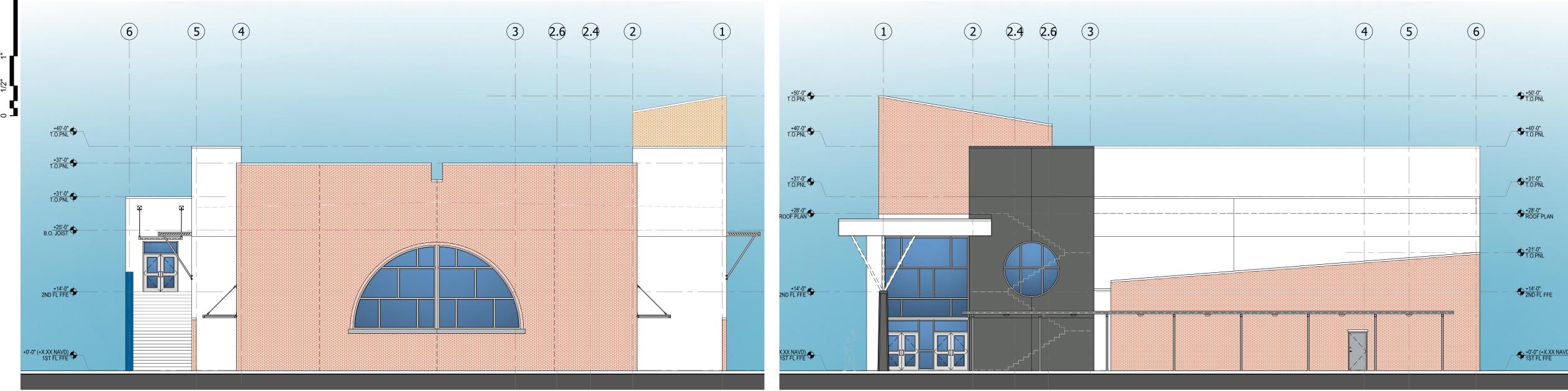
4

ِ (<u>5</u>) -

6

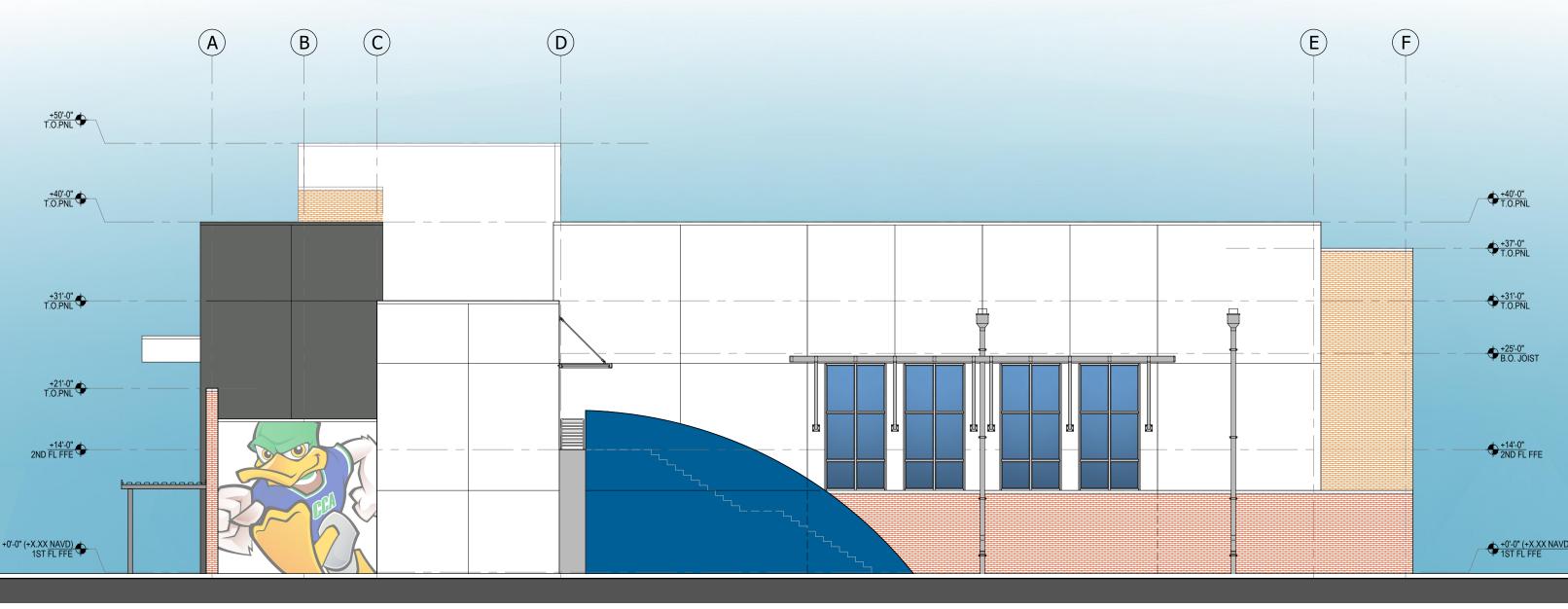


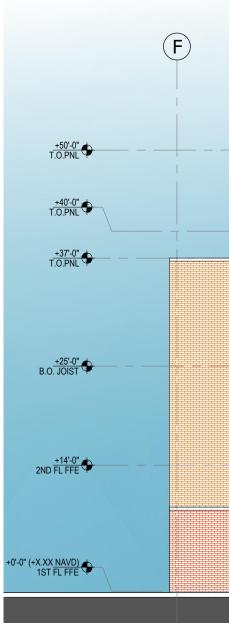
A107



NORTH ELEVATION 11'-8" 0 \ A108 23'-4" SCALE: 3/32" = 1'-0"

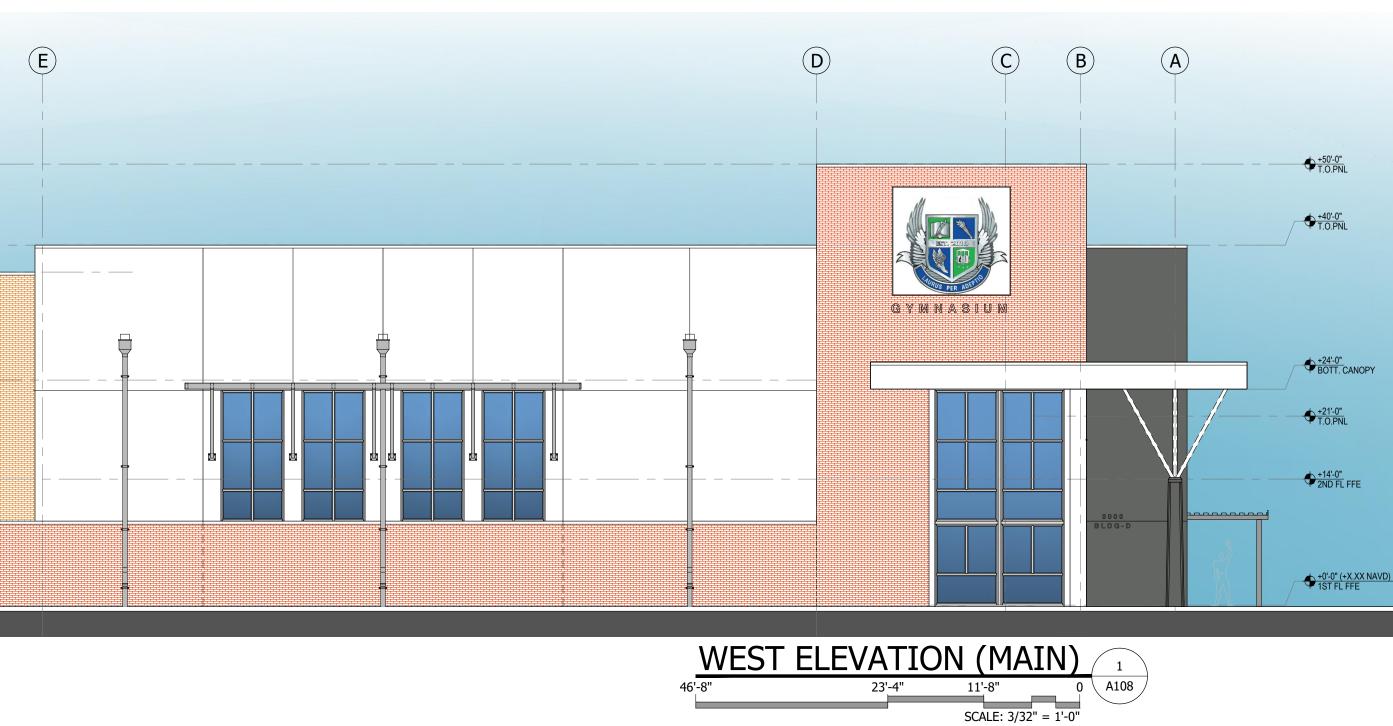
46'-8"











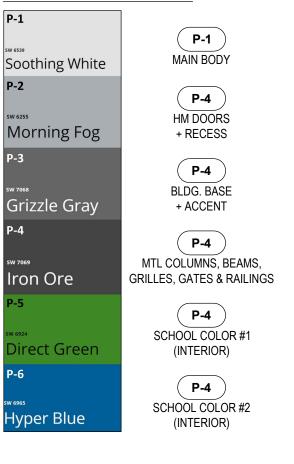
46'-8"

PROPOSED MATERIALS & COLOR SELECTION

COLOR SELECTION BASED ON SHERWIN WILLIAMS COLOR PALETTE:

- HM DOORS/ FRAMES (INTERIOR & EXTERIOR) P-2 2. METAL STAIRS/ GUARDRAIL/ RAILING & EXPOSED STEEL MEMBERS -
- SW7069 "IRON ORE" (MATCH EXIST.) WALKWAY COVER, CANOPIES & SUNSHADES ANODIZED ALUM. STOREFRONTS & WINDOWS - ANODIZED ALUM.
- 5. GLASS SOLAR-E PLUS GREY (SPANDREL GLASS TO MATCH)

EXTERIOR BUILDING COLORS:







M-2 ALUM. STOREFRONTS, DOORS & WINDOWS: ANODIZED ALUMINUM W/ SOLAR-E PLUS GREY GLASS









(M-5) CLEAR ANODIZED ALUMINUM (SILVER)



WALKWAY COVER & MATERIAL (M-6) CLEAR ANODIZED ALUMINUM (SILVER)

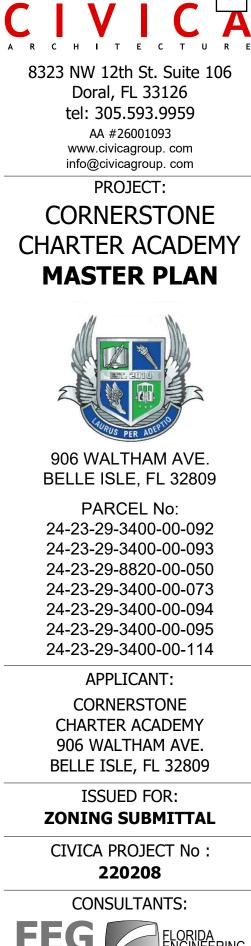


CLEAR ANODIZED ALUMINUM (SILVER)





M-8 TILT-UP CONCRETE CONSTRUCTION, STEEL JOIST, STEEL BEAMS, STEEL COLUMNS & CONCRETE OVER CORRUGATED DECK





2/09/23		
	SUBMITTAL	
Y	APPROVED BY	
	JG/ RL	
	SCALE:	
3	AS SHOWN	
		JG/ RL SCALE:

KEY PLAN

1st FL FFE: XX.XX' NAVD (1988)

SEAL/SIGNATURE

ROLANDO LLANES AR - 0013160

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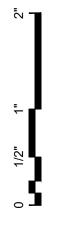
SHEET TITLE



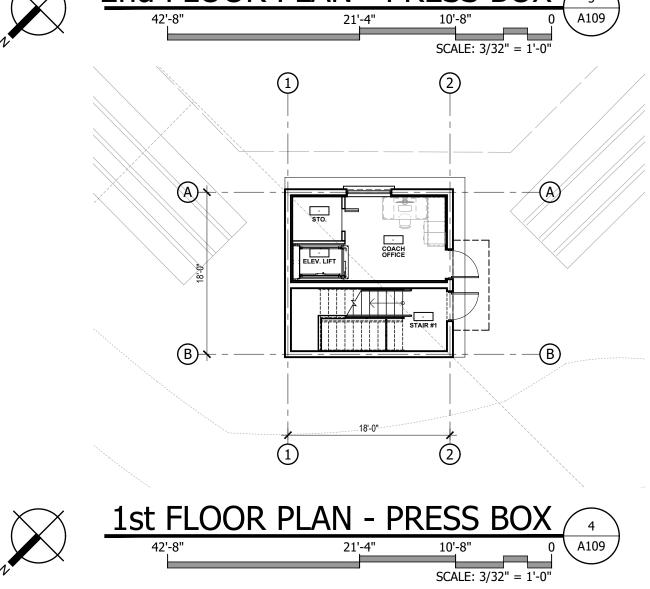
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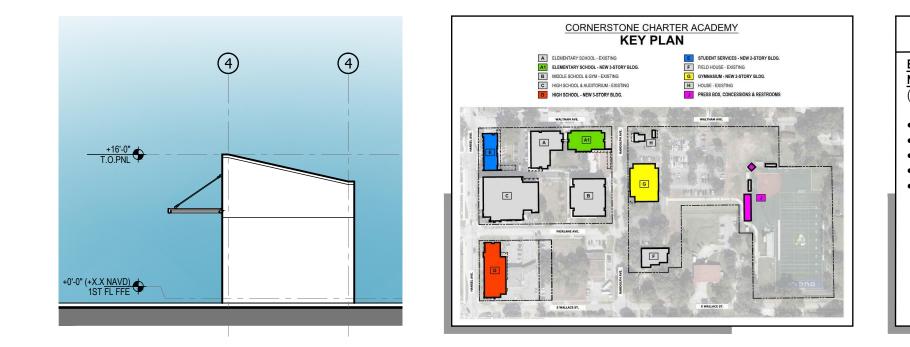


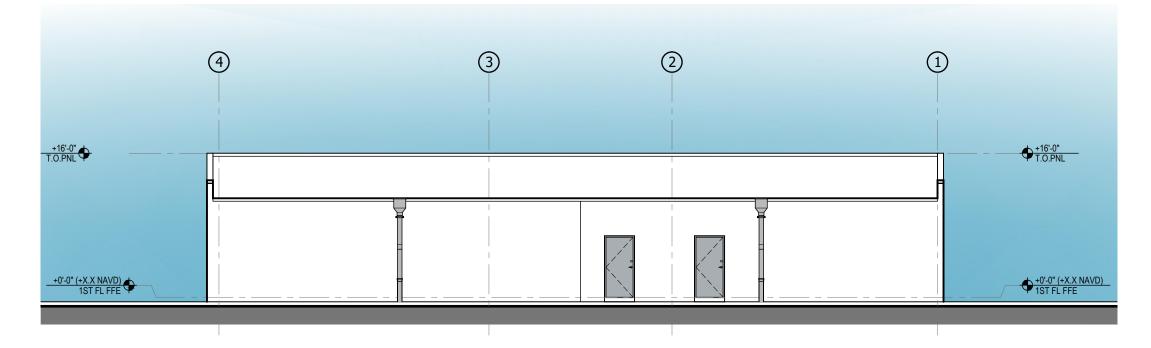
352

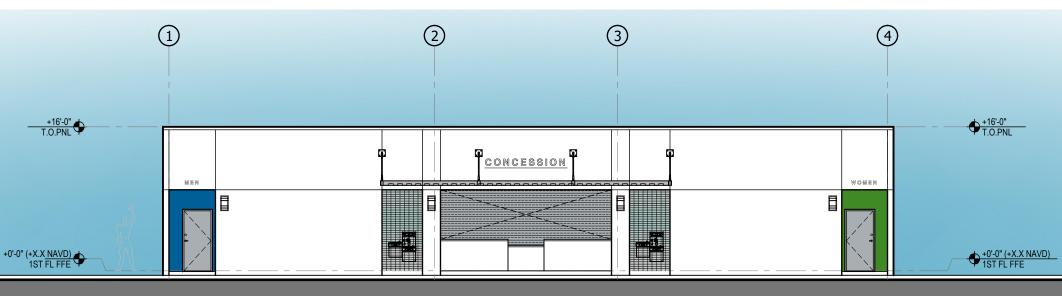


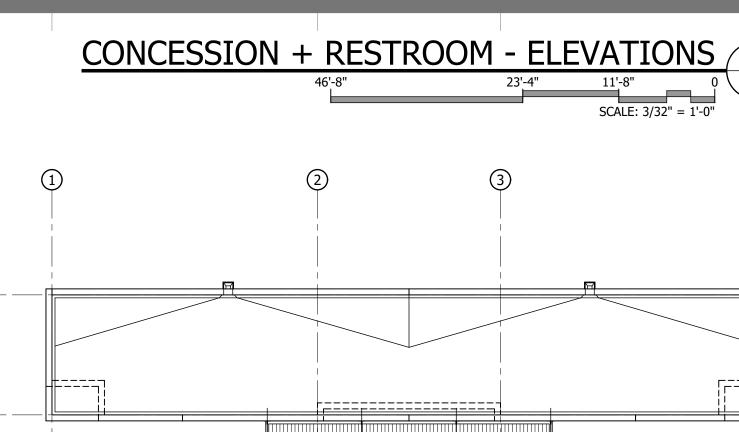
(1)(1)<u>+26'-0"</u> T.O.PNL +12'-10" 2ND FL FFE +0'-0" (+X.X NAVD) 1ST FL FFE PRESS BOX - ELEVATIONS 23'-4" 11'-8" A +╞═══╋═ ₿₩ 18'-0" (2) (1)2nd FLOOR PLAN - PRESS BOX 10'-8" 21'-4" (1)2

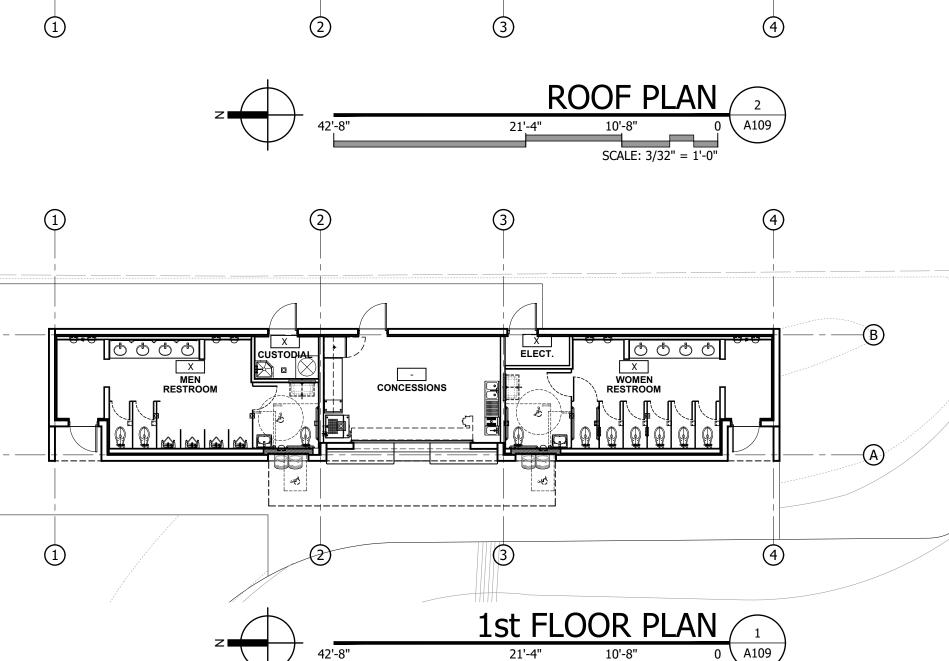










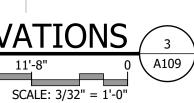


SCALE: 3/32" = 1'-0"

+26'-0" T.O.PNL

+12'-10" 2ND FL FFE

+0'-0" (+X.X NAVD) 1ST FL FFE



-

B

A -

B

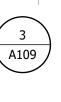
A -

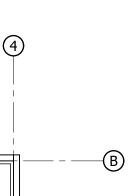
-B

BLDG-J PROPOSED PROGRAM

BLDG-J NEW FIELD CONCESSIONS & RESTROOMS (Near current location for "Sports Fields")

• 2-STORY, ±1,740 SF BUILDING CONCESSIONS RESTROOMS COACH OFFICE • BASEBALL/ SOFTBALL PRESS BOX







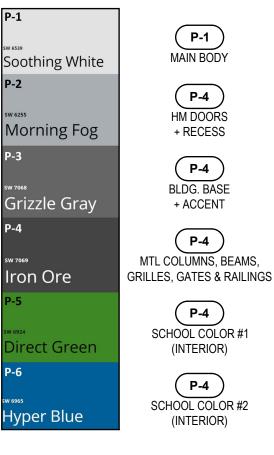
-A

PROPOSED MATERIALS & COLOR SELECTION

COLOR SELECTION BASED ON SHERWIN WILLIAMS COLOR PALETTE:

- HM DOORS/ FRAMES (INTERIOR & EXTERIOR) P-2 METAL STAIRS/ GUARDRAIL/ RAILING & EXPOSED STEEL MEMBERS -SW7069 "IRON ORE" (MATCH EXIST.)
- WALKWAY COVER, CANOPIES & SUNSHADES ANODIZED ALUM. STOREFRONTS & WINDOWS - ANODIZED ALUM.
- 5. GLASS SOLAR-E PLUS GREY (SPANDREL GLASS TO MATCH)

EXTERIOR BUILDING COLORS:







M-2 ALUM. STOREFRONTS, DOORS & WINDOWS: ANODIZED ALUMINUM W/ SOLAR-E PLUS GREY GLASS





M-3 ROOF COLOR/ MATERIAL: LOW SLOPE (WHITE) T.P.O. GATES & DECORATIVE PICKET FENCE



M-5 HORIZONTAL SUNSHADE DRAUNE I SUM CLEAR ANODIZED ALUMINUM (SILVER)



WALKWAY COVER & MATERIAL M-6 WALKWAT COVER & WALK BALL

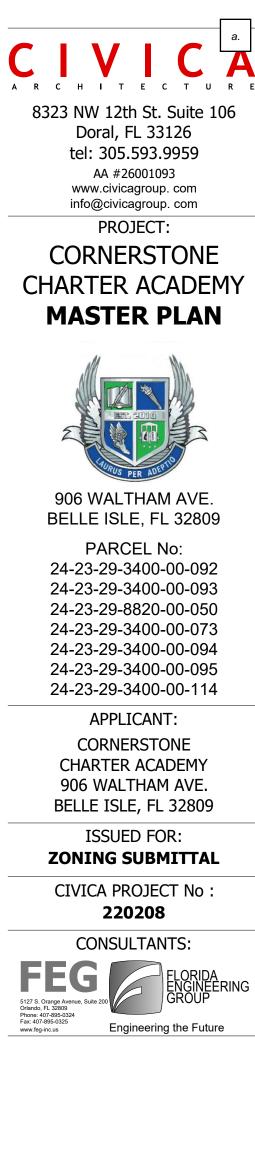


CLEAR ANODIZED ALUMINUM (SILVER)





TILT-UP CONCRETE CONSTRUCTION, STEEL JOIST, STEEL BEAMS, M-8 TILT-UP CONCRETE CONSTRUCTION, STELL SOLOT, STELL SOL



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DRAWN BY		APPROVED BY	
ТМ		JG/ RL	
DATE		SCALE:	
02/09/2023		AS SHOWN	

KEY PLAN

1st FL FFE: XX.XX' NAVD (1988) SEAL/SIGNATURE

ROLANDO LLANES AR - 0013160

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SHEET TITLE



SHEET NUMBER

